

Slope Stability Evaluation and Equipment Setback Distances for Burial Ground Excavations

Prepared for the U.S. Department of Energy
Assistant Secretary for Environmental Management

Contractor for the U.S. Department of Energy
under Contract DE-AC06-08RL14788



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Slope Stability Evaluation and Equipment Setback Distances for Burial Ground Excavations

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CH2M HILL Plateau Remediation Company

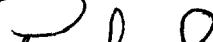
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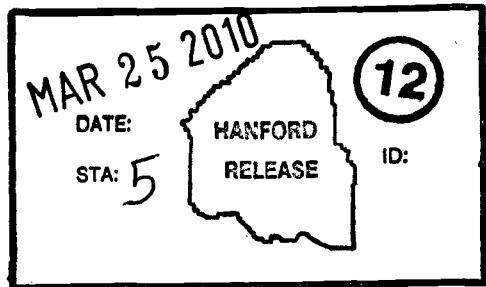
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Slope Stability Evaluation and Equipment Setback Distances
for
Burial Ground Excavations

1.0 Introduction

After 1970 Transuranic (TRU) and suspect TRU waste was buried in the ground with the intention that at some later date the waste would be retrieved and processed into a configuration for long term storage. To retrieve this waste the soil must be removed (excavated). Sloping the bank of the excavation is the method used to keep the excavation from collapsing and to provide protection for workers retrieving the waste. The purpose of this paper is to document the minimum distance (setback) that equipment must stay from the edge of the excavation to maintain a stable slope. .

This evaluation examines the equipment setback distance by dividing the equipment into two categories, (1) equipment used for excavation and (2) equipment used for retrieval. The section on excavation equipment will also discuss techniques used for excavation including the process of benching. Calculations 122633-C-004, "Slope Stability Analysis" (Attachment A), and 300013-C-001, "Crane Stability Analysis" (Attachment B), have been prepared to support this evaluation. As shown in the calculations the soil has the following properties:

- Unit weight 110 pounds per cubic foot
- Friction Angle (natural angle of repose) 38° or 1.28 horizontal to 1 vertical

Setback distances are measured from the top edge of the slope to the wheels/tracks of the vehicles and heavy equipment being utilized. The computer program utilized in the calculation uses the center of the wheel or track load for the analysis and this difference is accounted for in this evaluation.

2.0 Excavating and Excavation Equipment:

2.1 Removal of Overburden

Generally the waste trenches have four (4) to eight (8) feet of soil over the top of the waste containers. This soil layer (overburden) is provided to protect the environment from a release from the waste containers, protect the waste containers from the weather, and allow equipment traffic over the top of the trench. The overburden distributes the weight of the equipment to many containers which allows equipment traffic to pass over the trench safely. When excavation begins the confining effect from the overburden is removed and waste containers must be protected from the possibility of sliding or toppling into waste arrays. The protection is provided by keeping equipment back from the edge of the trench.

One of the load cases in the calculation determined the excavation equipment setback distance. This load case considered the weight of the heaviest piece of excavation equipment, the Komatsu® excavator, used for digging. Therefore, the results are applicable for all types of lesser weight equipment. In the calculation, the lateral soil pressure on the covered waste array was determined considering the excavator at different setback distances from the edge of the

array. The calculation also determined the strength of a waste drum accounting for potential deterioration. When lateral pressure is compared to drum strength a setback distance of 3 feet-6 inches (minimum) from the edge of the waste drum array yielded a factor of safety of 2.9 which is adequate. Figure 1 shows the configuration.

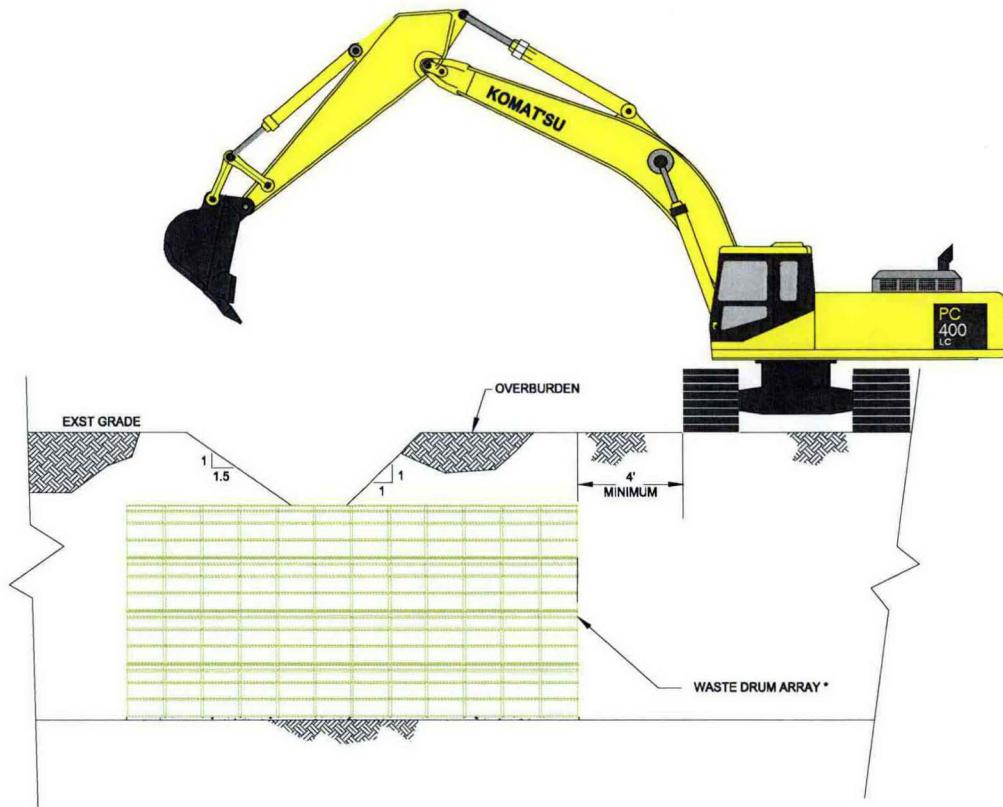


Figure 1
Removal of the Overburden

2.2 Excavation for Retrieval:

Once the overburden is removed and the waste array or containers are exposed, excavation begins to uncover the waste containers for retrieval. Since some of the waste containers are stacked on other containers excavation and retrieval may occur simultaneously. The depth of excavation can vary from 4 to 20 feet.

When excavating, the preferred method for slope stabilization is sloping with a side slope no steeper than a 1.5 horizontal to 1 vertical (1.5h to 1v). Other methods of slope stabilization (i.e. shoring, shielding, etc.) could be used if necessary and will be evaluated on a case by case basis. The 1.5h to 1v slope was selected because this is also the slope required for safe manned entry into the trench in accordance with OSHA criteria. When the side slope of a trench is shallower

than or equal to 1.5h to 1v, manned entry is allowed in the trench at the same time that an excavator is working from the top of the trench.

Excavation to the deeper depths (12 feet to 20 feet) could require a steepened slope (as steep as 1h to 1v) for the excavator to reach the material to be removed. The calculation evaluated slope stability at different excavated depths and different slope angles. The results demonstrate that the equipment centerline should be behind the sloped line for the soil angle of repose. The natural angle of repose for the LLBG was determined by laboratory analysis as 38° or 1.28h to 1v.

2.3 Setback Distances

To simplify development of excavation procedures for the Waste Retrieval Project, setback distances are limited to three conditions:

1. For the excavator when the tracks are perpendicular to the trench the setback distance is 0 feet for all slope angles (Maximum angle 1h : 1v).
2. For the excavator when the tracks are parallel to the trench the following is allowed:
 - While excavating with a slope angle 1.5h : 1v or shallower the set back distance is 0 feet from the edge of the track.
 - While excavating with a slope angle steeper than 1.5h : 1v (maximum angle is 1h :1v) the set back distance is 4 feet.
 - When reducing the slope form 1h : 1v (maximum) to a slope of 1.5h :1v, the setback distance to the edge of the track shall start at 4 feet. During this transition the setback distance maybe reduce to 0 feet (edge of the track).
3. For all other equipment (i.e. water trucks, cranes, golf carts, etc.), the setback distance is 4 feet.

2.4 Methodology Used to Determine Setback Distances

The setback distances were selected based on the attached calculations. The calculations demonstrate that the setback distance is a function of the angle of the side slope and the natural angle of repose (38°). Figure 2 was developed to graphically show the relationship between the different angles evaluated. The slope that controls the slope stability is the natural angle of repose or 1.28h : 1v (38°). The setback distances are controlled by a slope of 1h : 1v (45°). The following sections explain the rational used to develop the setback distances.

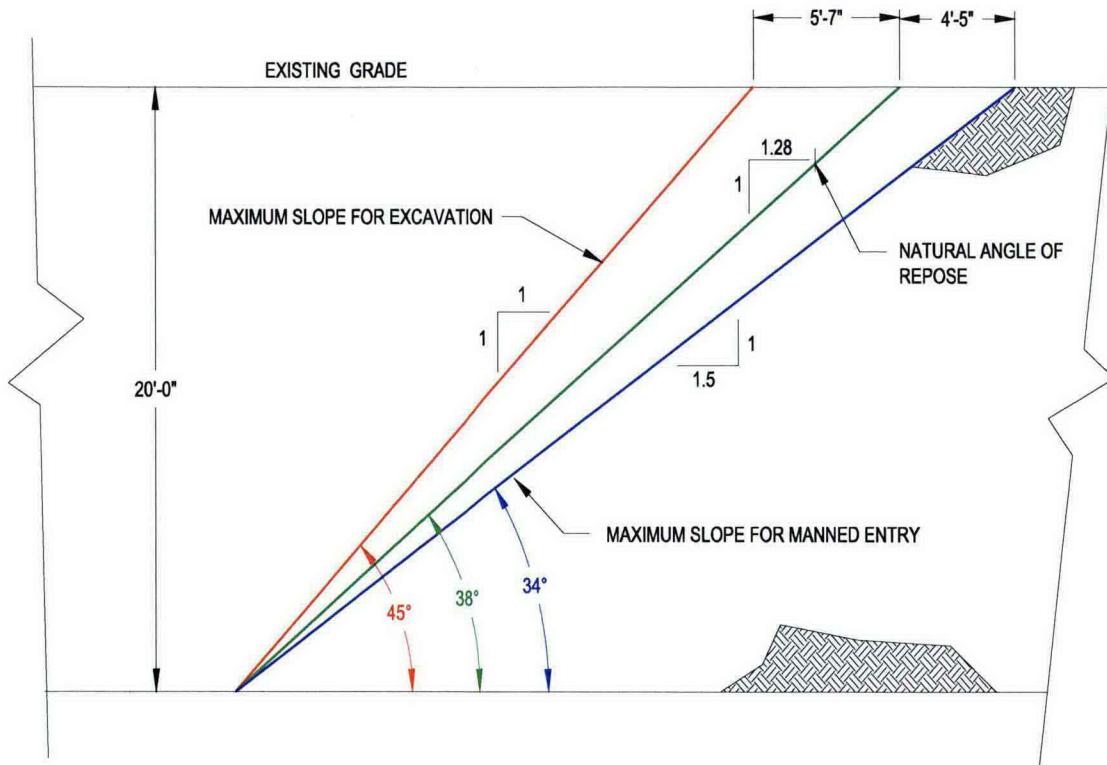


Figure 2
Comparison of Evaluated Slopes

2.4.1 Side Slope 1.5 horizontal to 1 vertical

The angle for a side slope 1.5h to 1v (34°) is less than the natural angle of repose (Figure 2) and is very stable. The calculation shows that the only effect any of the equipment has on a slope of this angle is some minor compaction of the top of the slope. When the tracks of the excavator are perpendicular they could extend over the top of the slope by two or three feet and the slope would still be stable.

2.4.2 Natural Angle of Repose (38°)

The natural angle of repose for this material is 38° or 1.28h to 1 v. The calculation shows that this angle controls the stability of the slope for this soil. Therefore the weight of the equipment should always stay behind this line. This angle will be used to calculate the setback distances.

2.4.3 Side Slope 1 horizontal to 1 vertical (45°)

The slope of 1h to 1v is the maximum slope needed for the excavator to reach depths to 20 feet. This slope only remains as long as necessary to achieve the trench depth required. Once the depth is obtained the side slopes are reduced to the 1.5h to 1v for personnel entry.

2.4.3.1 Setback distance for the excavator when the tracks are perpendicular to the excavation:

Figure 3 shows an example of this configuration. The calculation concludes that the center of the track should always be behind the slope for the natural angle of repose. In this configuration the center of the track is the center line of the excavator. Measuring to the center of the excavator can lead to confusion so to simplify, the setback distance will be measured from the top of the limiting slope (1h:1v) to the point where the track is closest to the excavation (see Figure 3). The setback is then determined as distance from the top edge of the 1h to 1v slope and top of the 1.28:1 slope (5 foot - 7 inches) minus $\frac{1}{2}$ of the length of the track (17 foot-7 inches \div 2 = 8 feet - 9 $\frac{1}{2}$ inches) which is a negative distance. A negative distance will not be allowed so the setback distance will be 0 (the tracks can come right up to the trench edge on the limiting 1h : 1v slope).

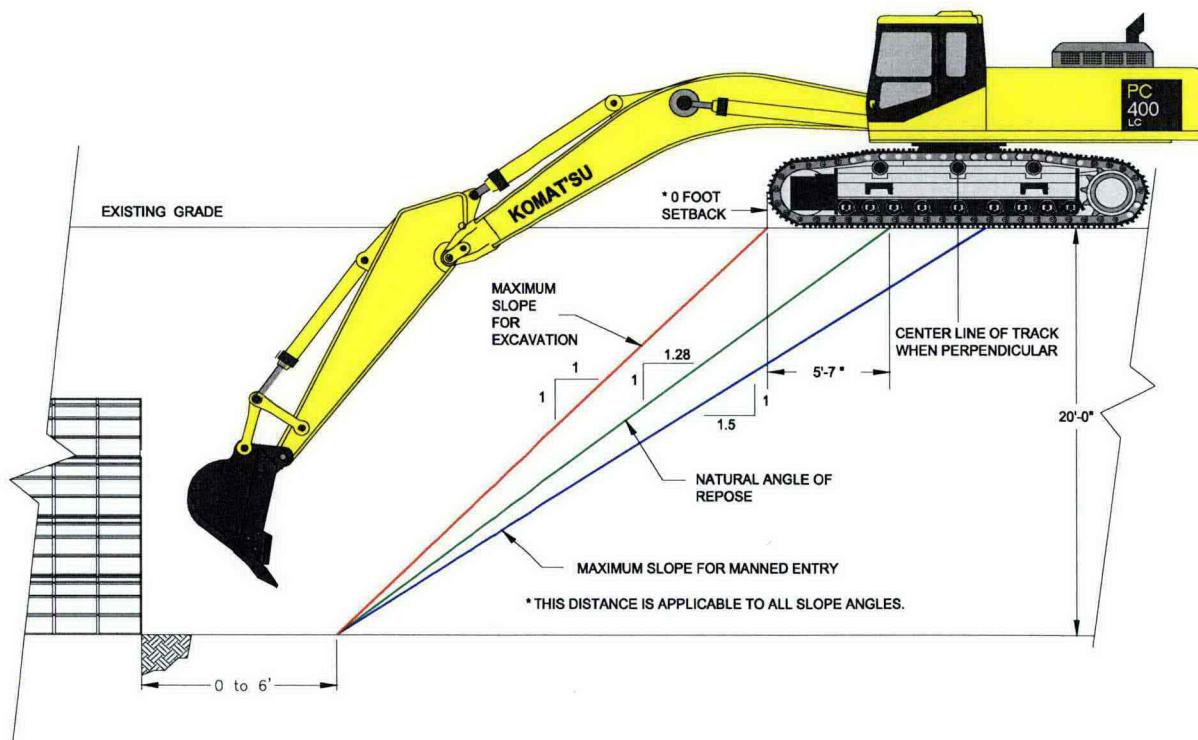


Figure 3
Excavation with the Excavator Tracks Perpendicular

2.4.3.2 Setback distance for the excavator when tracks are parallel to the excavation:

Figure 4 shows an example of this configuration. The calculation concludes that the center of the track should always be behind the slope for the natural angle of repose. Measuring to the center of the track can lead to confusion so to simplify, the setback distance will be measured from the top of the limiting slope (1h to 1v) to the edge of the track closest to the excavation. The setback distance is then the distance between the top of the 1h to 1v slope and top of the 1.28h to 1v slope (5 foot -7 inches) minus $\frac{1}{2}$ of the track width (3 foot \div 2 = 1 foot-6 inches) which equals 4 foot-1 inches (to simplify use 4 feet). For slopes with an angle shallower than the 1.28h : 1v the set back distance for the parallel condition can be 0 feet.

Once the trench has been excavated to the desired depth the angle of the side slope is transitioned from the 1h : 1v, needed for excavation, to a slope of 1.5h : 1v which is required for manned entry. As can be seen in Figure 4 the setback distance, for transition, can start at 4 feet from the top of slope to the edge of the closest track and be reduced to 0 feet (to the edge of the track) as the slope angle is reduced. This is acceptable since the center line of the track will always be at or behind the slope of the natural angle of repose (1.28h to 1v).

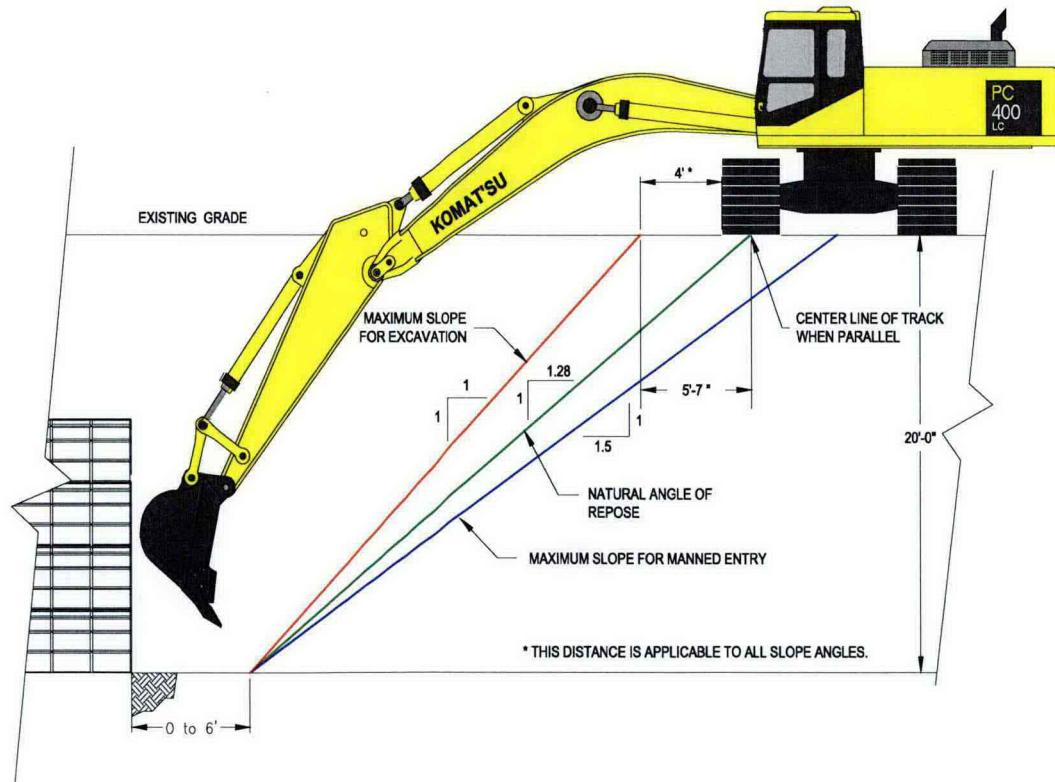


Figure 4
Excavating with the Excavator Tracks Parallel

2.4.4 Other Equipment Used for Excavation

There are several other pieces of equipment that are utilized during excavation (i.e. dump trucks, water trucks, front end loaders, etc.) The calculation concludes that these pieces of equipment should also stay behind the slope for the natural angle of repose. Due to the configuration of this type of equipment the setback distance is the same as whether the piece of equipment operates parallel or perpendicular to the trench. The setback is the distance between the top of the limiting 1:1 slope and top of the 1.28:1 slope (5 foot -7 inches) minus $\frac{1}{2}$ of the width or a set for dual tires similar to the rear tires on the water truck. (3 foot $\div 2 = 1$ foot-6 inches) which is approximately 4 feet (See figure 5).

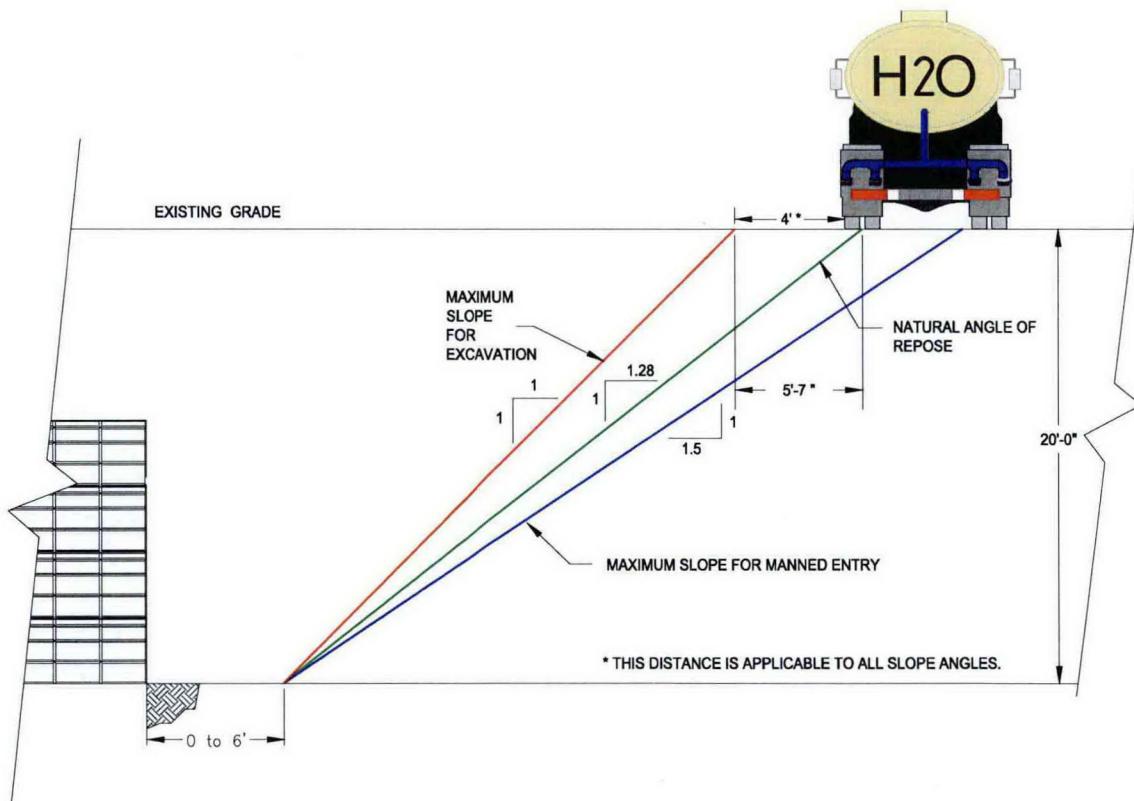


Figure 5
Setback Distance for All Other Type of Equipment

2.4.5 Benchling

Benching is a technique used to maintain slope stability and allow an excavator to reach areas that are too deep for the size of the excavator from the top of the trench.

2.4.5.1 Benching for slope stability

This type of benching is common for slopes that are perpendicular to the center line of the trench. This technique is covered in the excavation standard, 29 CFR 1926.650 Subpart P. From this section the definition of benching is “a method of protecting employees from cave-ins by excavating the sides of an excavation to form one or a series of horizontal levels or steps, usually with a vertical or near vertical surfaces between levels”. Figure 6, Section A-A shows how perpendicular benching is done. Note in the Section View that the slope ultimately is 1.5h to 1v.

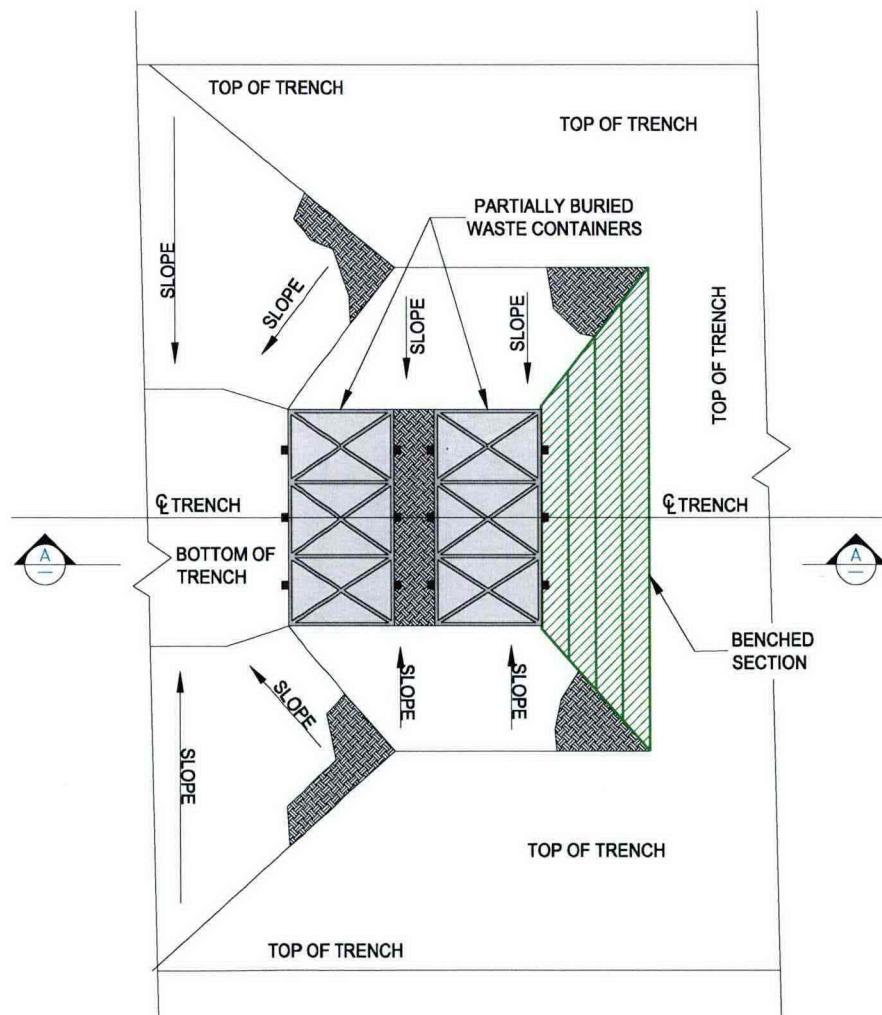


Figure 6 Perpendicular Benching

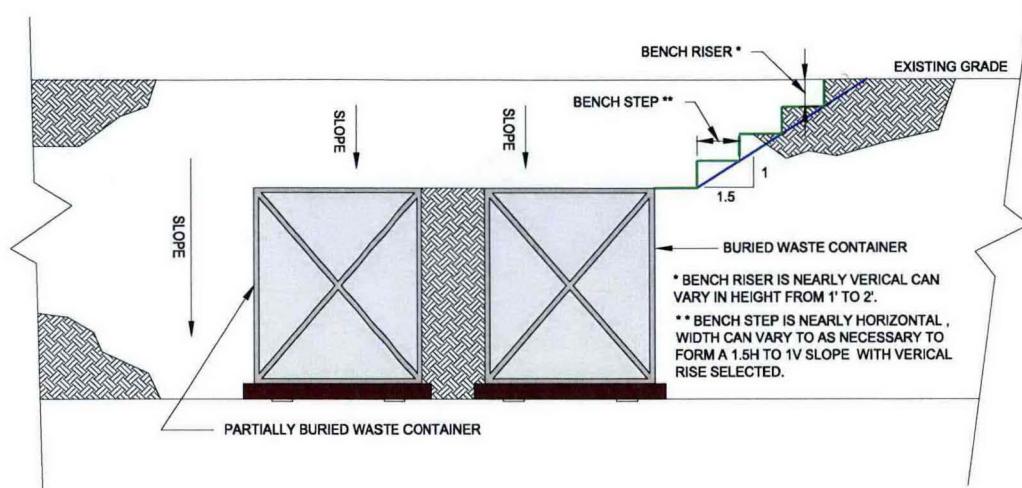


Figure 6 Perpendicular Benching
continued

2.4.5.2 Benching to aid in hard to reach areas

This type of benching is when excavating at a steep slope (e.g. 1h to 1v) is not feasible. Generally this condition is present when manned entry is needed either to retrieve upper level waste containers or perform hand excavation. In this situation the benching lowers the level at which the excavator is placed allowing it to be closer to the work. Figures 7 and 8 have been prepared to illustrate benching. Note the excavator is shown operating from a level base. This is for reference only, the slope that the excavator operates from is limited by the capability of the excavator to operate on a sloped surface and not the stability of the side slope. The slope stability and setback distances are the same as the excavation when benching is not used. Note the slope at the back of the bench is a function of the depth of the bench.

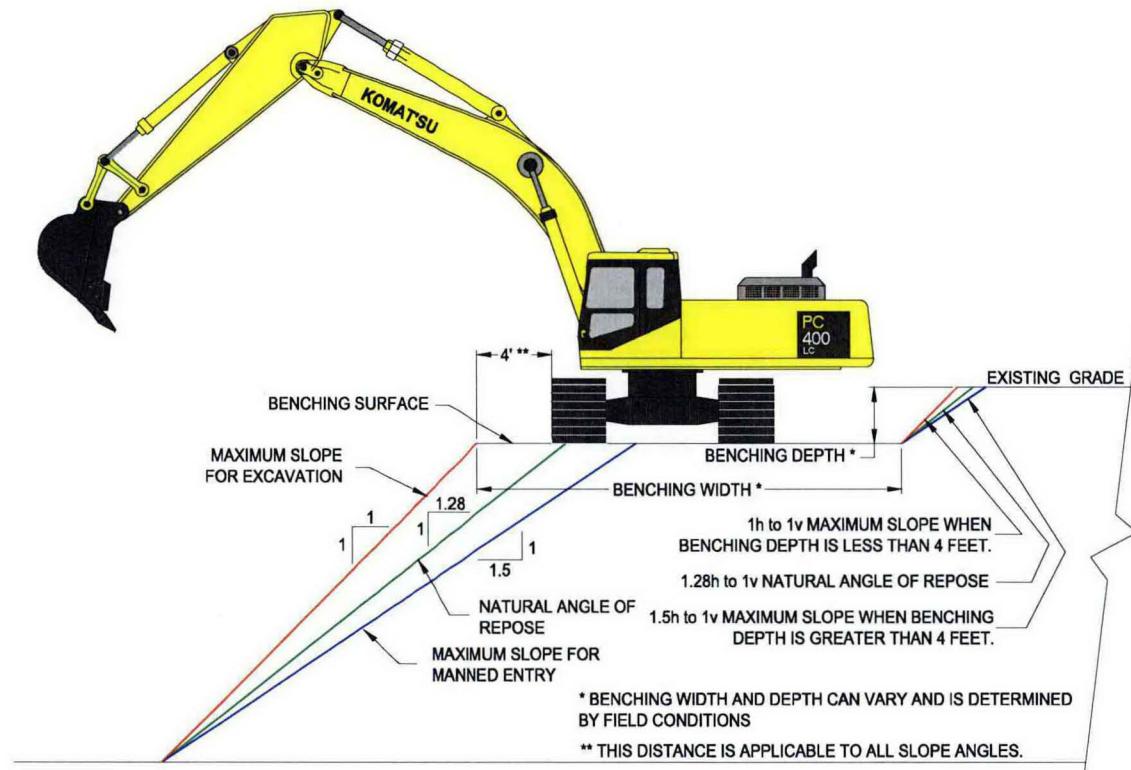


Figure 7
Benching with Excavator Tracks Parallel

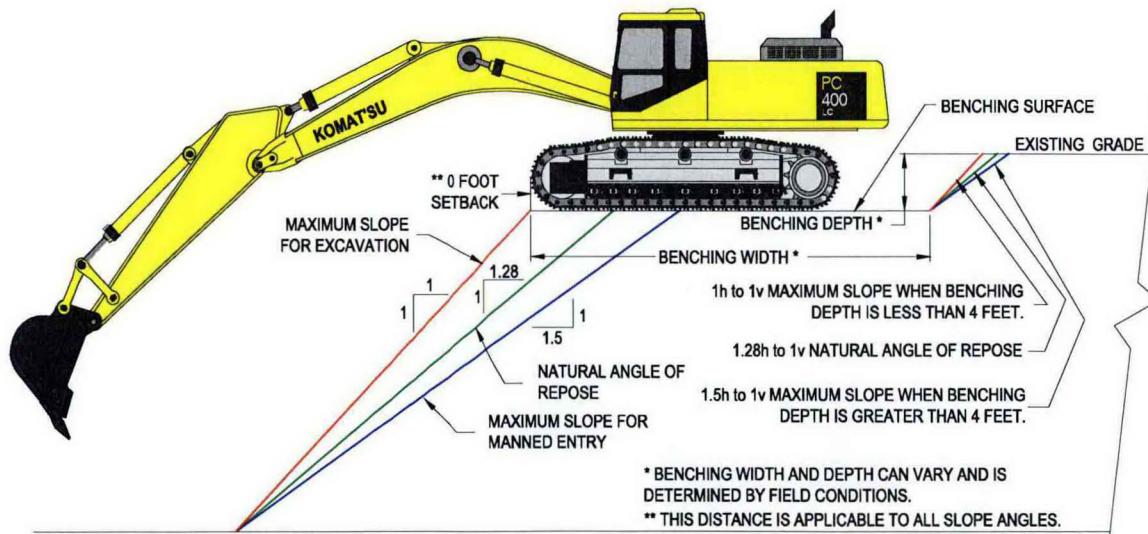


Figure 8
Benching with Excavator Tracks Perpendicular

2.4.5.3 Excavator stability associated with trench excavation

The specifications for the Komatsu excavators operated in the burial grounds indicate that the excavators can climb or descend a 35° slope (This is just slightly steeper than a $1.5h:1v$ slope). They recommend performing excavation operations on a slope of 30° maximum. For conservatism it is recommended that excavators operate from an approximately level grade (when on a bench or otherwise). For ease of identification in the field, a slope of $\pm 5^\circ$, which corresponds to “visually level”, would be adequate.

3.0 Equipment Used for Retrieval

Equipment used to retrieve waste containers vary from heavy cranes (tracked or rubber tired) to light weight golf carts. The calculations (Appendix A and B) demonstrate that from a slope stability aspect the $1.5h:1v$ slope, required for manned entry, would be stable if any of the equipment (i.e. water truck, front end loaders, track and rubber tire cranes, ect.) were to be operated directly at the top edge of the slope. The only affect on the soil would be some minor localized compaction at the top of the slope. However, from an industrial safety perspective (e.g. a worker exiting a vehicle and falling down the slope) the establishment of a setback distance is prudent. An example setback distance of 4 feet is shown in Figure 9; this example is not based on soil stability (for which no setback is required with the $1.5H:1V$ slope) but is based only on worker safety considerations.

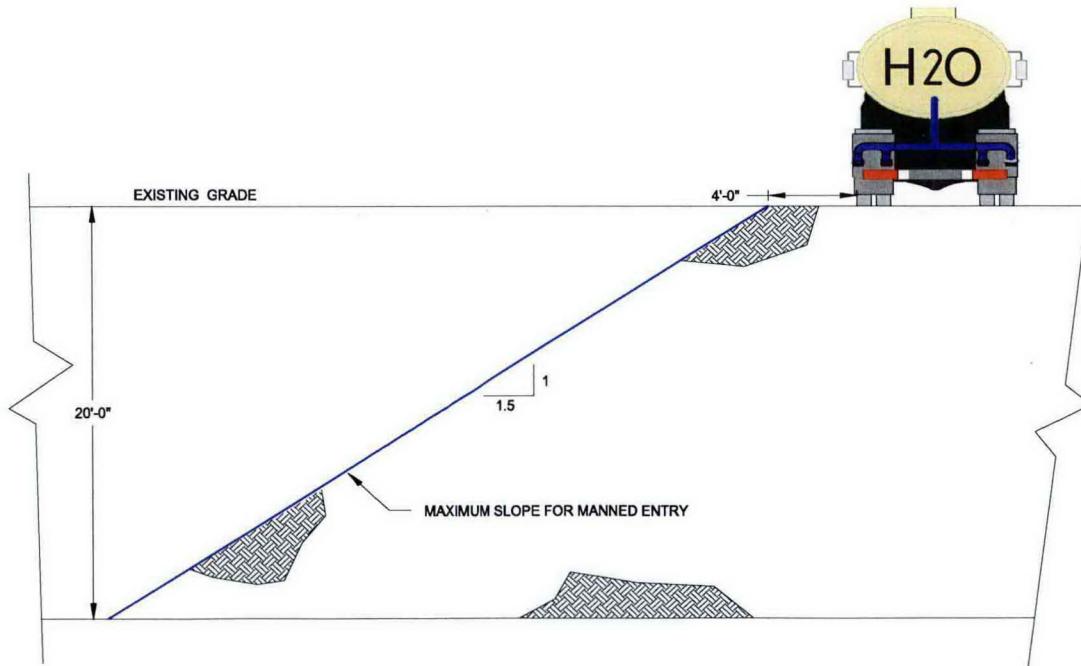


Figure 9
Equipment Setback Distance for Equipment used for Retrieval

Attachment A
Calculation 122633-C-004

Calculation Cover Sheet and Revision Summary

Calculation/Technical Basis Sheet

Section 1: Identification				
1. Project Number 122633	2. Modification Description Title/Subject Slope Stability Analysis	3. Page ii of ii		
4. Calculation Number 122633-C-004		5. Rev. No. 0		
Section 2: Scope, Objective, Method, Assumptions, Technical Basis				
<p>6. Scope and Objective This calculation was performed to support safe excavation operations connected with drum retrieval in trenches at the Central Waste Facility in the 200 West Area. There were six configurations presented with three different loading situations to evaluate. The loadings are given in the assumption section below. The configurations are shown in the figures.</p>				
<p>7. Method Hand calculation using MathCad 14.0 MO11 and Slope stability analysis using the interactive computer program STB2006. STB2006 was developed at Delft University and Verified by FGG. This program uses Bishops simplified method to determine the minimum factor of safety. The approved for use form is included in the appendix.</p>				
<p>8. Assumptions and Technical Basis</p> <ol style="list-style-type: none"> 1. The soil weight is assumed to be about 110pcf. This is a reasonable for the area of interest. The soils to be excavated contain gravel and are heavier than the sand from Ref. 1. 2. The angle of internal friction for the soil is 38o. (Ref. 1) 3. The dry soil is cohesionless but has an apparent cohesion of between 11psf and 14psf when freshly excavated allowing the newly excavated slope to stand at 1H:1V . 4. The excavator tracks are 31.5 inches wide, spaced 7'-7" c-c and exert a ground loading of 3,300 lbs/ft of track length. 5. Assume the bulk Poisson ratio for the soil, $\mu := 0.33$ (Ref. 5, Table 2-7). 6. Calculation was to determine equipment distances for 1:1 factors of safety. (1.1:1 minimum recommended) 				
Section 3: Design Inputs/References				
9. Ref. No.	10. Inputs/References (with Revision and/or Date or Source)			
1	Site Investigation Report WHC-SD-W025-SE-001			
2	Roark's Formulas for Stress and Strain, 7th Edition, McGraw Hill			
3	Advanced Soil Mechanics, 2nd Edition, Das, Taylor and Francis, New York, NY			
4	Foundation Analysis and Design, 5th Edition, Bowles, McGraw Hill			
Section 4: Conclusions				
<ol style="list-style-type: none"> 1. (Load Case 1) The tracked excavator can operate with the edge of the closest track within 3.5' of the face of the drum stack. 2. For the 4' and 7' deep excavation at 1:1 slope the excavator should be at least 3' from the top of the slope. 3. For the 10' and 13' deep excavation at 1:1 the excavator should be at least 4' from the top of the slope. 4. For the 16' deep excavation at 1:1 slope the excavator should be at least 5' from the top of the slope. 5. The 38 deg. angle of repose is a 1.28H:1V slope. The excavator should always be behind that slope line. 6. For the 16' deep excavation sloped at 1.5H:1V, the excavator should be at least 2' from the top of the slope. 7. The H-20 tandem axle truck should not operate closer than 4' from the top of the 1.5:1 slope. 				

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ENGINEERING
ANALYSIS

Calc. No.:122633-C-004

Revision: 0

Page No.: 1 of 9

Client: Fluor Hanford
 Subject: Slope Stability Analysis for
 Excavation Work At CWC Trench
 Location: CWC 200W

T.O./Job No.: 65400811.122633

Originated by: D.S. Messinger, P.E. D.S. Messinger Date: 11-23-07Checked by: David S. McShane, P.E. D.S. McShane Date: 11-23-07**OBJECTIVE:**

This calculation was performed to support safe excavation operations connected with drum retrieval in trenches at the Central Waste Facility in the 200 West Area. There were six configurations presented with three different loading situations to evaluate. The loadings are given in the assumption section below. The configurations are shown in the figures.

REFERENCE:

1. Site Investigation Report WHC-SD-W025-SE-001
2. Roark's Formulas for Stress and Strain, 7th Edition, McGraw Hill
3. Advanced Soil Mechanics, 2nd Edition, Das, Taylor and Francis, New York, NY
4. Foundation Analysis and Design, 5th Edition, Bowles, McGraw Hill

METHOD:

Hand calculation using MathCad 14.0 MO11 and Slope stability analysis using the interactive computer program STB2006. STB2006 was developed at Delft University and Verified by FGG. This program uses Bishops simplified method to determine the minimum factor of safety. The approved for use form is included in the appendix.

ASSUMPTIONS AND LIMITATIONS:

1. The soil weight is assumed to be about 110 pcf. This is a reasonable for the area of interest. The soils to be excavated contain gravel and are heavier than the sand from Ref. 1.
2. The angle of internal friction for the soil is 38°. (Ref. 1)
3. The dry soil is cohesionless but has an apparent cohesion of between 11psf and 14psf when freshly excavated allowing the newly excavated slope to stand at 1H:1V .
4. The excavator tracks are 31.5 inches wide, spaced 7'-7" c-c and exert a ground loading of 3,300 lbs/ft of track length.
5. Assume the bulk Poisson ratio for the soil, $\mu := 0.33$ (Ref. 5, Table 2-7).
6. Calculation was to determine equipment distances for 1:1 factors of safety. (1.1:1 minimum recommended)

CONCLUSION:

1. (Load Case 1) The tracked excavator can operate with the edge of the closest track within 3.5' of the face of the drum stack.
2. For the 4' and 7' deep excavation at 1:1 slope the excavator should be at least 3' from the top of the slope.
3. For the 10' and 13' deep excavation at 1:1 the excavator should be at least 4' from the top of the slope.
4. For the 16' deep excavation at 1:1 slope the excavator should be at least 5' from the top of the slope.
5. The 38 deg. angle of repose is a 1.28H:1V slope. The excavator should always be behind that slope line.
6. For the 16' deep excavation sloped at 1.5H:1V, the excavator should be at least 2' from the top of the slope.
7. The H-20 tandem axle truck should not operate closer than 4' from the top of the 1.5:1 slope.

CALCULATIONS:**Definitions of Terms:**

$$\text{Track half-width, } b := \frac{31.5}{2} \text{ in} \quad \text{Excavator ground pressure, } q := \frac{3300 \frac{\text{lbf}}{\text{ft}}}{2-b} = 8.73 \cdot \text{psi}$$

Soil unit weight, $\gamma := 110 \text{pcf}$ Internal friction angle of soil, $\phi := 38 \text{deg}$

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Calc. No.:122633-C-004

Revision: 0

Page No.: 2 of 9

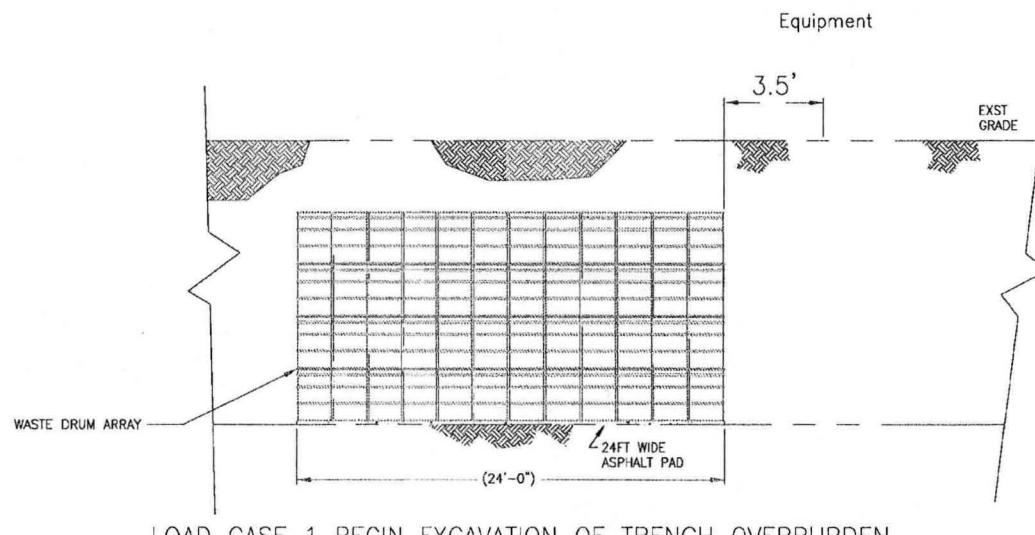
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$$\text{Active earth pressure coefficient, } K_A := \tan\left(45\text{deg} - \frac{\phi}{2}\right)^2 = 0.238$$

$$\text{At Rest earth pressure coefficient, } K_0 := 1 - \sin(\phi) = 0.384$$



Load case 1 is to determine the safe lateral distance from the drum stack for excavation equipment.

The Boussinesq equation for a point load can be written as $\sigma_r := \frac{P}{2\pi} \left(\frac{3r^2 z}{R^5} - \frac{1-2\mu}{R(R+z)} \right)$. For a strip loading

condition, the plane strain Poisson ratio is used rather than the bulk Poisson ratio. The expression for the plane strain Poisson ratio is $\mu' := \frac{\mu}{1-\mu} = 0.493$ $1-2\mu' = 0.015$ or nearly zero and can be ignored in the following functions. (Ref. 4, Eq. 11-20a.)

Expressions for the vertical and horizontal soil pressure, are shown below as functions.

$$\sigma_z(x, z) := 2 \frac{q}{\pi} \int_{-b}^b \frac{z^3}{[(x-s)^2 + z^2]^2} ds \quad \sigma_z(5\text{ft}, 4\text{ft}) = 86.032 \cdot \text{psf} \quad (\text{Ref. 3, Eq. 3.22})$$

$$\sigma_x(x, z) := 2 \frac{q}{\pi} \int_{-b}^b \frac{(x-s)^2 \cdot z}{[(x-s)^2 + z^2]^2} ds \quad \sigma_x(5\text{ft}, 4\text{ft}) = 123.029 \cdot \text{psf} \quad (\text{Ref. 3, Eq. 3.23})$$

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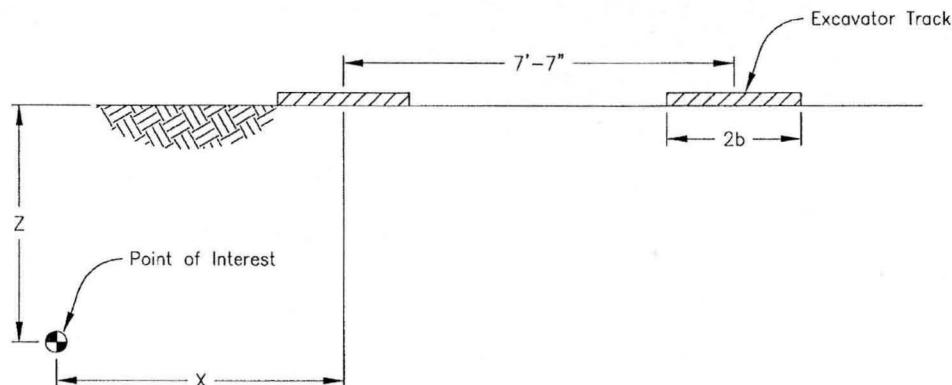
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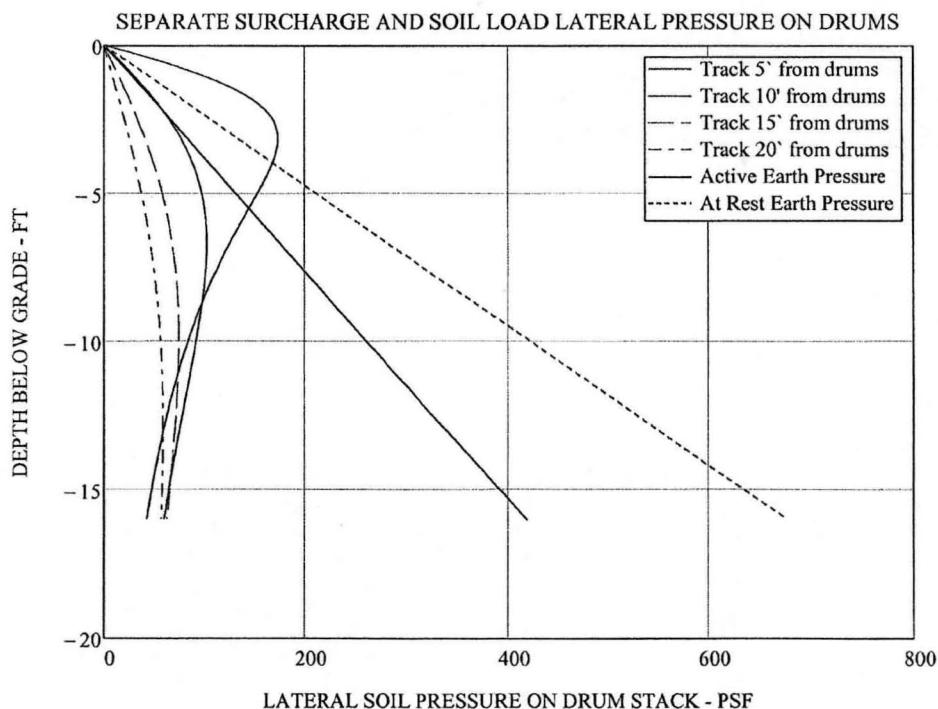
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T.O./Job No.: 65400811.122633

Originated by: D.S. Messinger, P.E. D.S. Messinger Date: 11-23-07Checked by: David S. McShane, P.E. David McShane Date: 11-23-07Excavator Track Configuration

Plot the lateral soil pressure for depth ranging from 0-16 feet and track offsets from 5 - 20 feet.

 $z := 0\text{ft}, .1\text{ft}..16\text{ft}$ $x := 5\text{ft}, 10\text{ft}..20\text{ft}$ 

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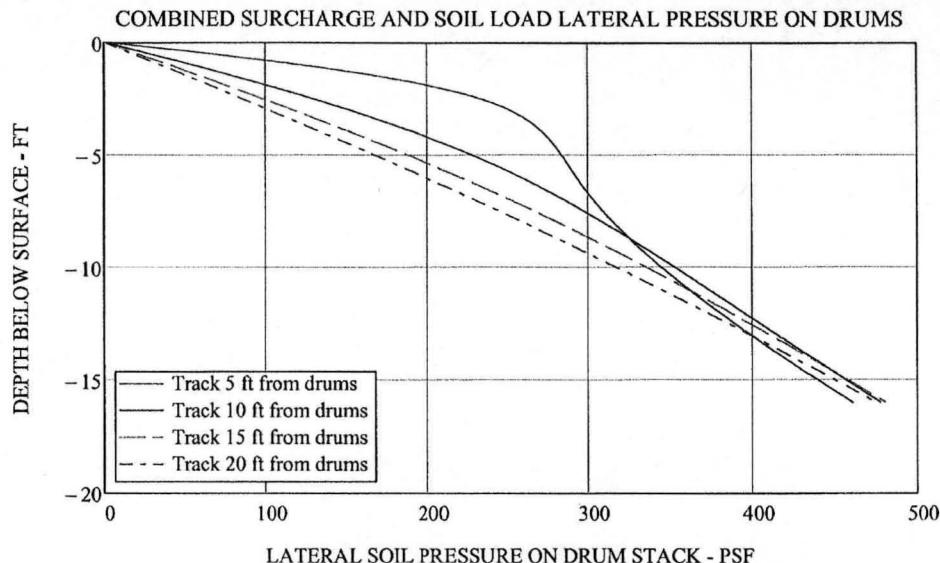
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No information could be found on drum collapse from external pressure so a lower limit pressure was determined by assuming the stiffening rings on the drum provided no increase in support against external pressure.

Assume 18 gage carbon steel drum:

Drum shell thickness, $t := 0.0478\text{in}$ Drum diameter, $D := 23.5\text{in}$ Drum height, $L_d := 34.5\text{in}$

Young's modulus for the steel, $E := 29000\text{ksi}$ Poisson's ratio for the steel, $\nu := 0.3$

$$\text{Drum radius, } r := \frac{D}{2} = 11.75\text{-in} \quad 4.9 \cdot r \sqrt{\frac{r}{t}} = 75.224\text{ ft} \quad \text{Use short tube formula}$$

$$q' := 0.807 \cdot \frac{E \cdot t^2}{L_d \cdot r} \cdot \sqrt{\left(\frac{1}{1-\nu^2}\right) \cdot \frac{t^2}{r^2}} = 1.3 \times 10^3 \cdot \text{psf} \quad \text{Roark Table 15.2 - Case 19}$$

$$60 < \left(\frac{L_d}{r}\right)^2 \cdot \left(\frac{r}{t}\right)^2 < 2.5 \cdot \left(\frac{r}{t}\right)^2 = 1 \quad q'' := \frac{0.92E}{\left(\frac{L_d}{r}\right) \cdot \left(\frac{r}{t}\right)^{2.5}} = 1.381 \times 10^3 \cdot \text{psf} \quad \text{Roark Table 15.2 - Case 20}$$

Fluor Hanford, Inc.

ENGINEERING
ANALYSIS

Calc. No.:122633-C-004

Revision: 0

Page No.: 5 of 9

Client: Fluor Hanford

Subject: Slope Stability Analysis for
Excavation Work At CWC Trench
Location: CWC 200W

T.O./Job No.: 65400811.122633

Originated by: D.S. Messinger, P.E. D. Messinger Date: 11-23-07Checked by: David S. McShane, P.E. DS McShane Date: 11-23-07Factors of Safety for Drum Collapse using Active Earth Pressure (Recommended)

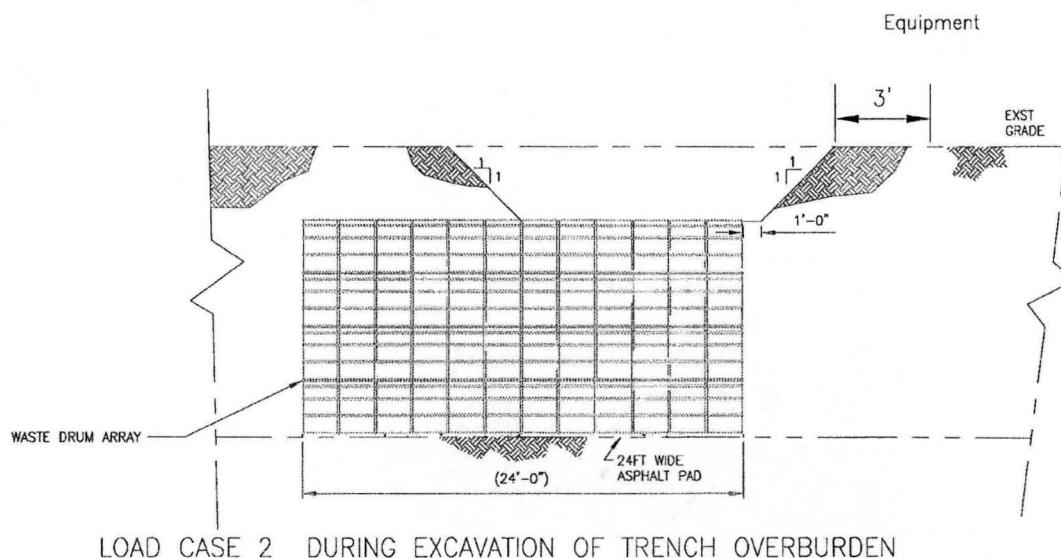
$$FS_{q'} := \frac{q'}{(\sigma_x(15\text{ft}, 16\text{ft}) + \sigma_x(22.6\text{ft}, 16\text{ft}) + \gamma \cdot 16\text{ft} \cdot K_A)} = 2.706$$

$$FS_{q''} := \frac{q''}{(\sigma_x(15\text{ft}, 16\text{ft}) + \sigma_x(22.6\text{ft}, 16\text{ft}) + \gamma \cdot 16\text{ft} \cdot K_A)} = 2.875$$

Factors of Safety for Drum Collapse using At Rest Earth Pressure

$$FS_{q'} := \frac{q'}{(\sigma_x(15\text{ft}, 16\text{ft}) + \sigma_x(22.6\text{ft}, 16\text{ft}) + \gamma \cdot 16\text{ft} \cdot K_0)} = 1.761$$

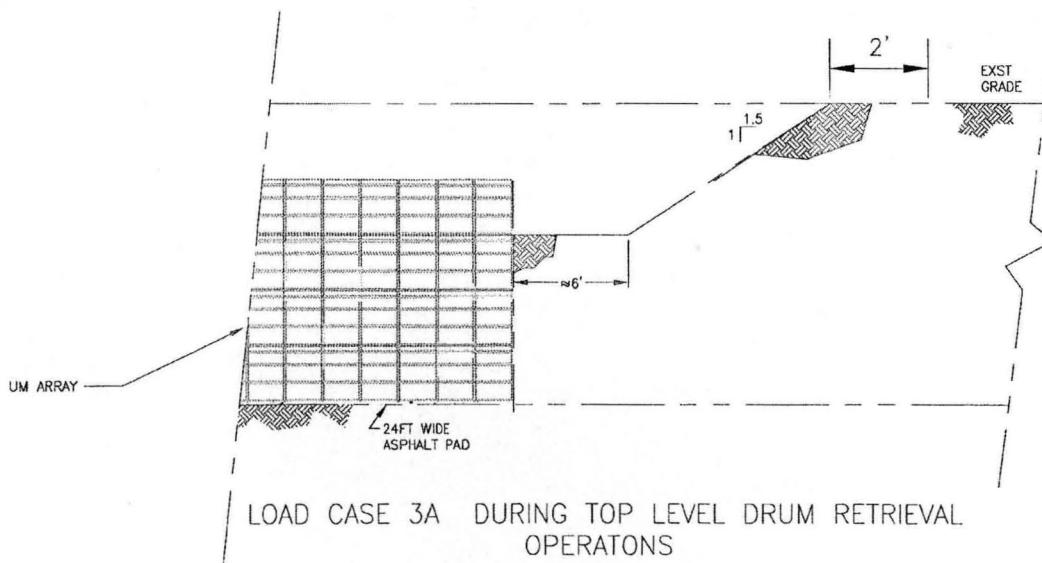
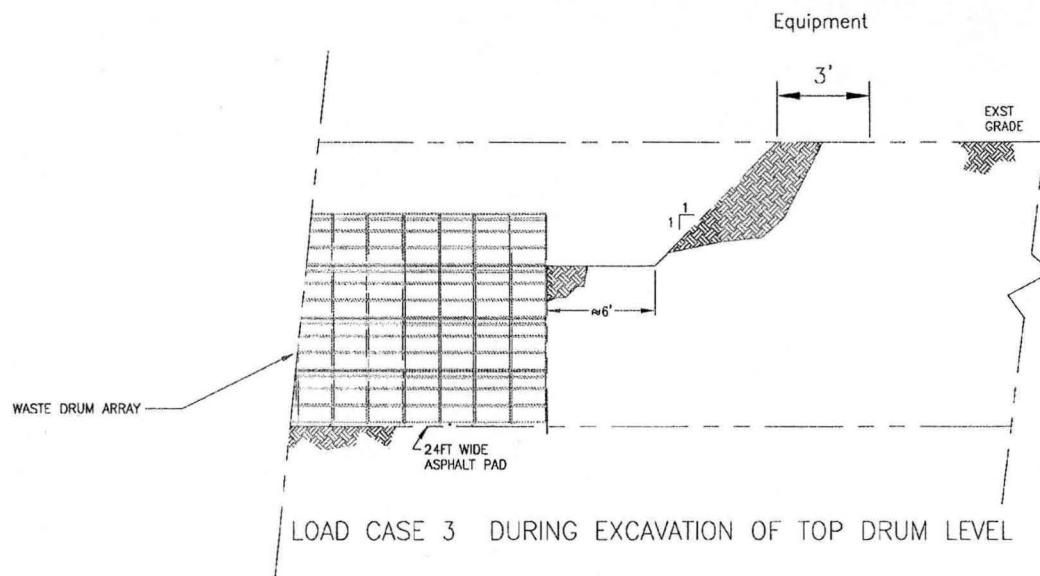
$$FS_{q''} := \frac{q''}{(\sigma_x(15\text{ft}, 16\text{ft}) + \sigma_x(22.6\text{ft}, 16\text{ft}) + \gamma \cdot 16\text{ft} \cdot K_0)} = 1.871$$

Following are the slope stability analysis by STB2006.

Fluor Hanford, Inc.

ENGINEERING
ANALYSISCalc. No.:122633-C-004
Revision: 0
Page No.: 6 of 9Client: Fluor Hanford
Subject: Slope Stability Analysis for
Excavation Work At CWC Trench
Location: CWC 200W

T.O./Job No.: 65400811.122633

Originated by: D.S. Messinger, P.E. D.S. Messinger Date: 11-23-07
Checked by: David S. McShane, P.E. D.S. McShane Date: 11-23-07

Fluor Hanford, Inc.

ENGINEERING
ANALYSIS

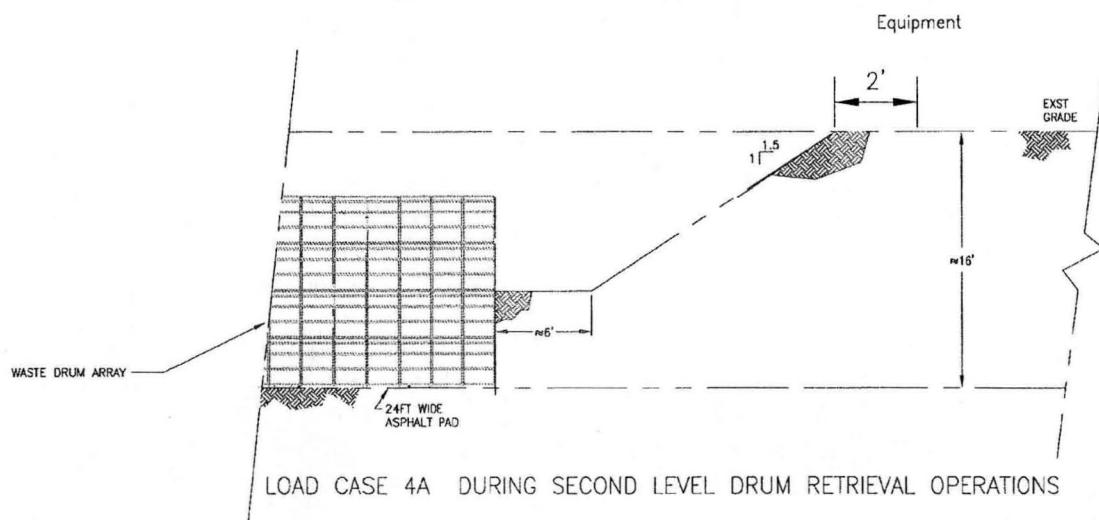
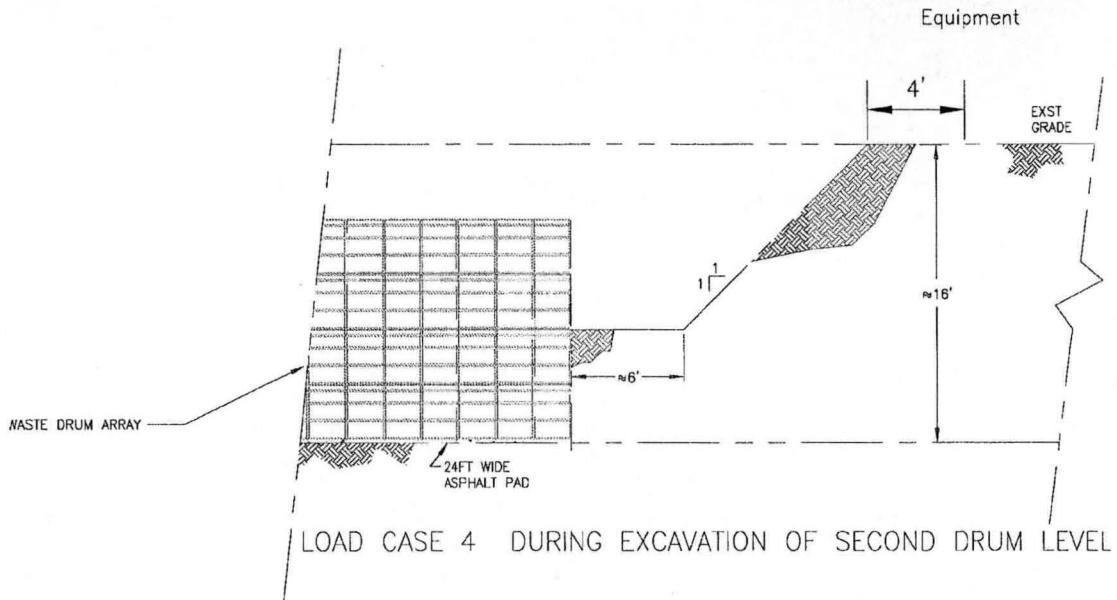
Calc. No.:122633-C-004

Revision: 0

Page No.: 7 of 9

Client: Fluor Hanford
 Subject: Slope Stability Analysis for
 Excavation Work At CWC Trench
 Location: CWC 200W

T.O./Job No.: 65400811.122633

Originated by: D.S. Messinger, P.E. *D.S. Messinger* Date: 11-23-07Checked by: David S. McShane, P.E. *DS McShane* Date: 11-23-07

Fluor Hanford, Inc.

ENGINEERING
ANALYSIS

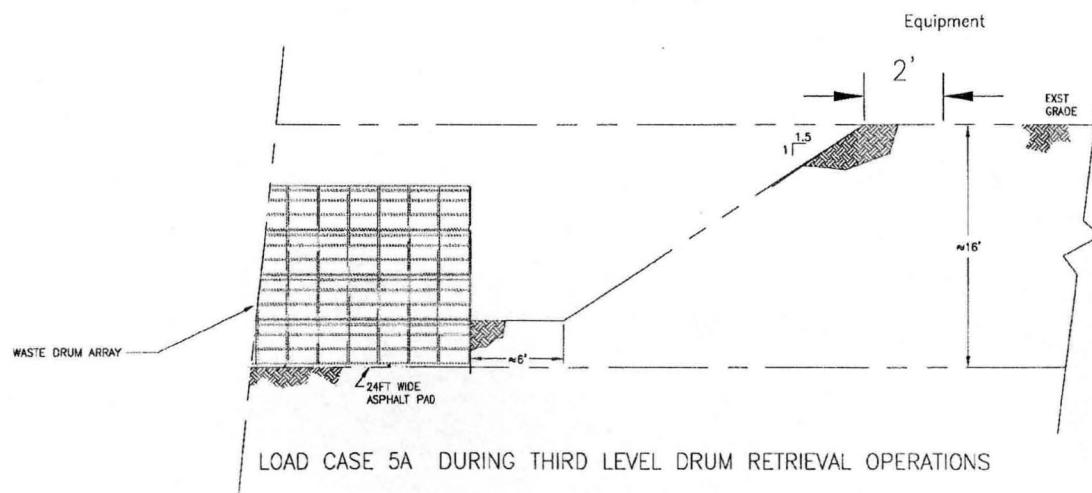
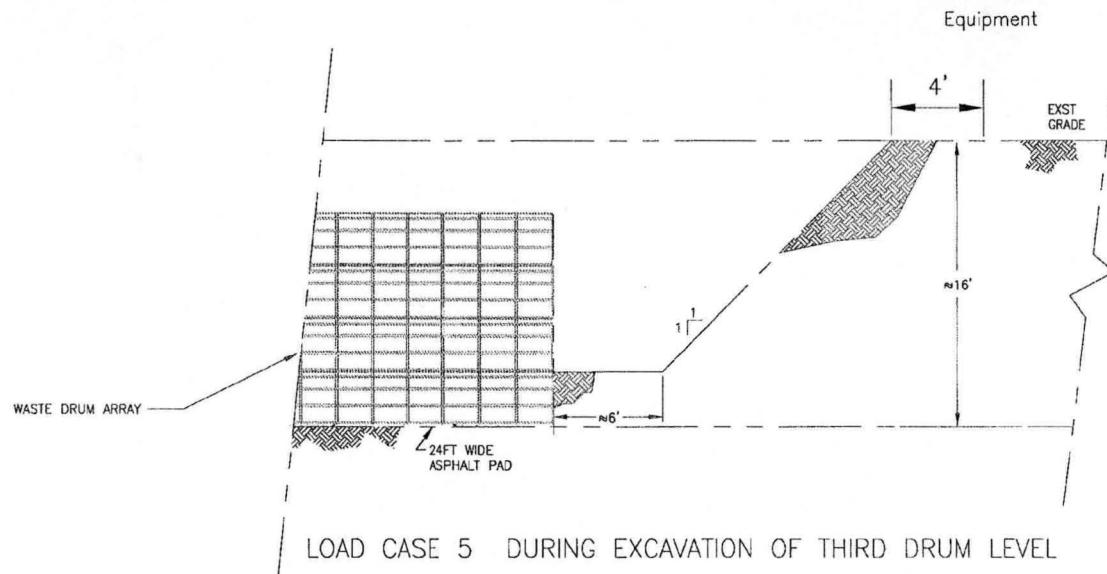
Calc. No.:122633-C-004

Revision: 0

Page No.: 8 of 9

Client: Fluor Hanford
 Subject: Slope Stability Analysis for
 Excavation Work At CWC Trench
 Location: CWC 200W

T.O./Job No.: 65400811.122633

Originated by: D.S. Messinger, P.E. *D.S. Messinger* Date: 11-23-07Checked by: David S. McShane, P.E. *DS. McShane* Date: 11-23-07

Fluor Hanford, Inc.

ENGINEERING
ANALYSIS

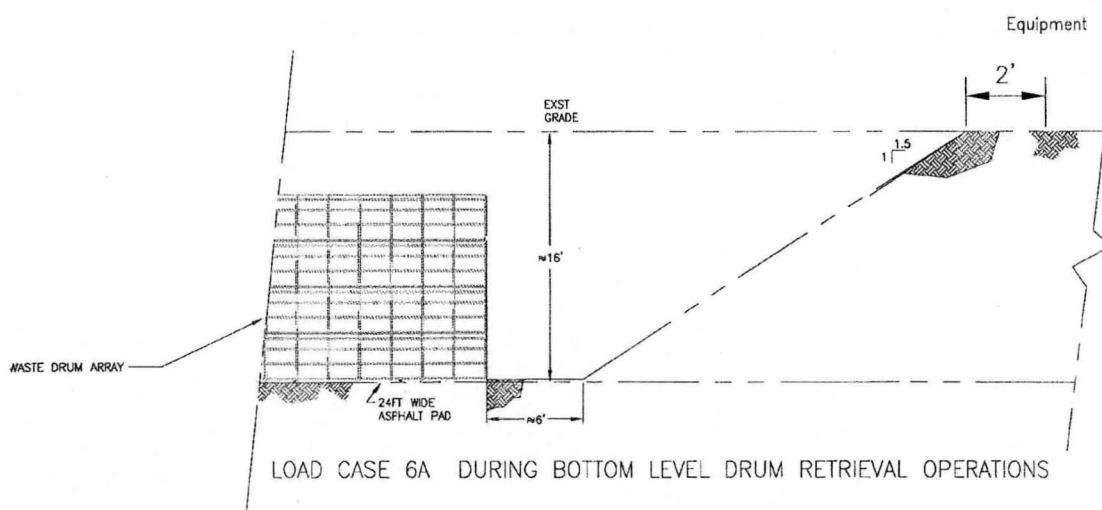
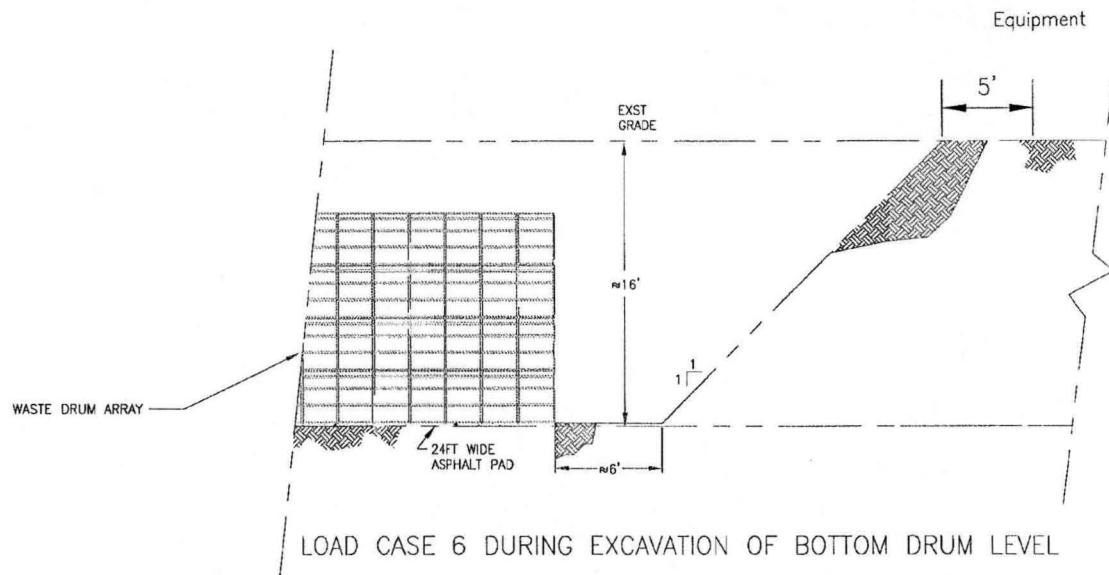
Calc. No.:122633-C-004

Revision: 0

Page No.: 9 of 9

Client: Fluor Hanford
 Subject: Slope Stability Analysis for
 Excavation Work At CWC Trench
 Location: CWC 200W

T.O./Job No.: 65400811.122633

Originated by: D.S. Messinger, P.E. *D.J. Messinger* Date: 11-23-07Checked by: David S. McShane, P.E. *DS McShane* Date: 11-23-07

APPENDIX A

(STB2006 SOLUTIONS)

ENGINEERING COMPUTER PROGRAM APPROVED FOR USE		
Program Name <u>STB2006</u>	Using Discipline(s) <u>CIVIL</u>	
Program Version Identifier <u>NA</u>	Release Date <u>2006</u>	
Program Source <u>Delft University of Technology-Vervuitt, Arnold (freeware)</u>		
Approved For Engineering Use By <u>Keith D. Messinger / Dr. Susan Farnsworth</u> Discipline Manager/Chief Design Engineer	Date <u>10/9/07</u>	
Principal User <u>David S. Messinger</u>	Location <u>1200 Jadwin-2nd floor</u>	Phone <u>(509) 372-3805</u>
<p>Program Abstract</p> <p>STB2006 is a revised version of the approved program STB2004. There is no noticeable difference in the "look and feel" of the two programs. Both STB2006 and STB2004 have a tendency to lock up or freeze if certain improper graphical inputs are made to the problem, such as placing the center of the failure circle in a position that can not present a rational solution to the problem. Verification of the current version is based on re-running the same set of problems used in the verification and validation of STB2004. Copies of the hand solved problems from the STB2004 V&V package are included as a part of this V&V package.</p> <p>STB is a program for the analysis of stability of a slope, using Bishop's simplified method, with some modifications introduced at GeoDelft and the Delft University. In this manual the various parts of the program are described, together with the basic methods and the definitions of the parameters. The manual can also be printed, by clicking the PRINT button of the screen 'Manual'. The program can be downloaded free of charge from the internet, from the page <http://geo.vervruyt.net>. Updated versions of the program will be announced on this page. The program may be used, copied and distributed without limitation, provided that it is not modified in any way. No responsibility is accepted by the author or the distributor of the program for any errors or for losses or damages incurred by using the program.</p> <p>In Bishop's method the safety factor of a slope is determined by comparing the moment of the weight of a soil wedge about the center of a slip circle, with the resisting moment provided by the shear stresses along the slip surface. The two moments are calculated by subdividing the sliding wedge into a large number of vertical slices. It is assumed that on the vertical side planes of the slices only horizontal (normal) stresses are acting, and no shear stresses. The first basic equation is Coulomb's equation for the shear stress along the lower part of a slice, $t = [c + (s - p) \tan(\phi)]/F$, here t is the shear stress, c is the cohesion, s is the total stress normal to the sliding plane, p is the pore water pressure, ϕ is the angle of internal friction, and F is the safety factor. The second basic equation is the equation of vertical equilibrium of a slice, which gives $W \cdot h = s \cdot t \cdot \tan(\alpha)$, where W is the (average) unit weight of the slice, h is its height, and α is the slope of the slip surface at the slice considered.</p> <p>Equilibrium of moments with respect to the center of the circle leads to a formula from which the safety factor can be calculated, iteratively. The program STB2006 also contains the three refinements of Bishop's method.</p> <p>1 - The first refinement is that care is taken that the direction of the shear stress along the slip surface is always opposing the sliding mechanism. This is achieved by cutting off the value of α at a minimum value of $\phi/2 - \pi/4$. This refinement is due to A. W. Koppejan of GeoDelft.</p> <p>2 - The second refinement is that the shearing resistance is reduced if the coefficient of neutral horizontal stress is so small that slip would occur along a plane perpendicular to the slip surface, combined with a local rotation, in agreement with a double sliding model, as suggested by G. de Josselin de Jong. This refinement is effective only if the coefficient of neutral horizontal stress (K_0) is smaller than 1.</p> <p>3 - The third refinement is that it is possible to introduce a horizontal body force, away from the slope. This may be used to simulate the stability of the slope during an earthquake. The horizontal forces acting on the slices increases the moment of the weight of the slices with respect to the center of the slip circle, thus reducing the stability factor.</p> <p>(See Appendix Sheets for limitations, verification problems and hard copy of the manual.)</p>		

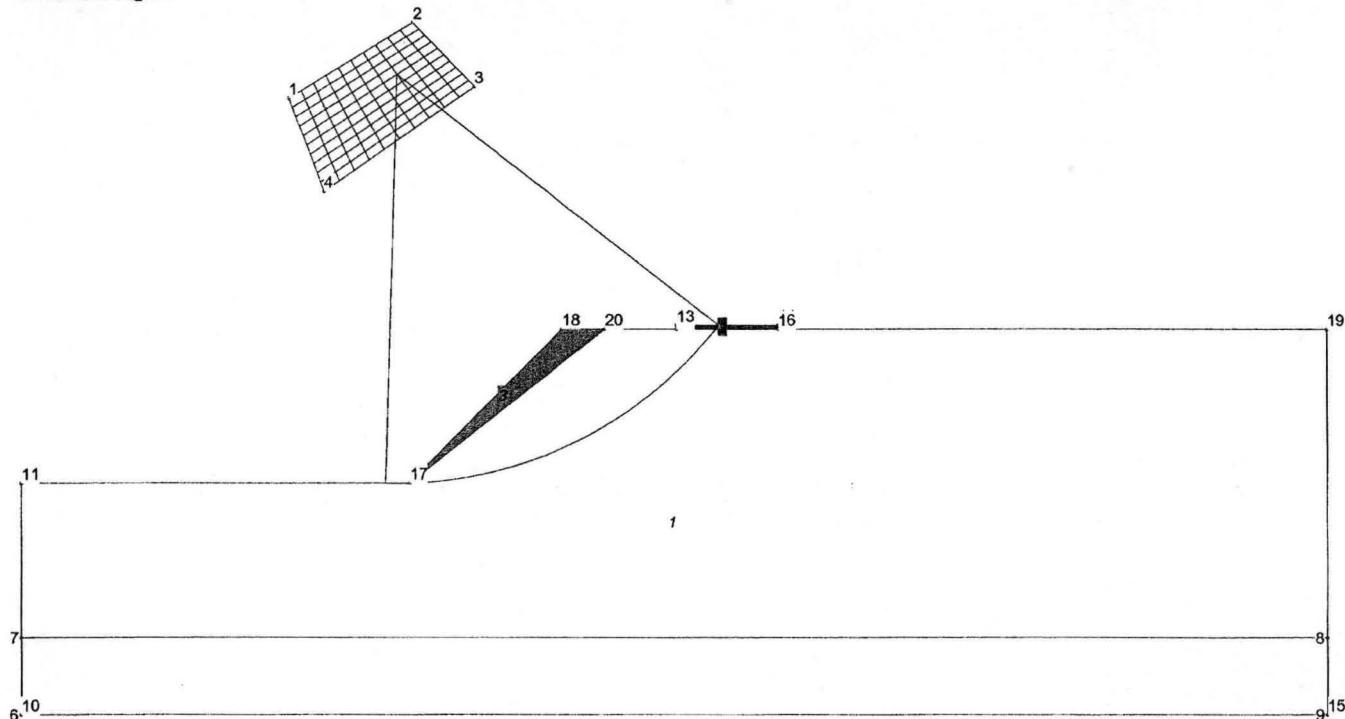
ENGINEERING COMPUTER PROGRAM APPROVED FOR USE		
Program Name <u>STB2006</u>	Using Discipline(s) <u>CIVIL</u>	
Program Version Identifier <u>NA</u>	Release Date <u>2006</u>	
Program Source <u>Delft University of Technology-Vervuitt, Arnold (freeware)</u>		
Approved For Engineering Use By <u>David S. Messinger / Susan Farnsworth</u> Discipline Manager/Chief Design Engineer	Date <u>10/9/07</u>	
Principal User <u>David S. Messinger</u>	Location <u>1200 Jadwin-2nd floor</u>	Phone <u>(509) 372-3805</u>
<p>Program Abstract</p> <p>(Continued from page 1)</p> <p>Limitations: The program limitations may be found within the programs help files and embedded electronic manual.</p> <p>Verification: Three verification test problems are supplied with the software. These problems are listed as test 1 through test 3 and demonstrate the various capabilities of the program. In addition, the FGG verifier (David S. Messinger, P.E.) produced five additional test problems based on published slope stability problems and answers. These additional verification problems were worked by hand, during the verification of STB2004 compared to the published answers and compared to the program generated output for STB2006. The program generated acceptable results in all cases. The verification problems are attached.</p> <p>STB2006 is authorized for use only after verification by each user on stand-alone personal computers. Authorization is granted only on the specific personal computers on which the verification have been run. Verification must be documented on the ACCEPTANCE TEST RECORD form and submitted to the FGG STB2006 Principal User.</p> <p>STB2006 is authorized for use by professional engineers who possess an understanding of soil mechanics. Users must verify the accuracy and applicability of the results.</p>		

David S. Messinger, P.E., Load Case 2

General Data

Licensed User : David S. Messinger, P.E.
File Name : CWC02.stb
Problem Name : Load Case 2
Number of Slices : 1000
Relative Horizontal Force: 0.000000
Number of Nodes : 20
Number of Soil Polygons : 3

Definition Figure



Input Data

Coordinates of Nodes

Node	x	y
1	6.828	16.020
2	10.092	17.985
3	11.724	16.320
4	7.761	13.589
5	10.000	6.000
6	0.000	0.000
7	0.000	2.000
8	33.974	2.000
9	33.974	0.000
10	0.000	0.000
11	0.000	6.000
12	16.986	10.000
13	16.986	10.100
14	19.611	10.100
15	33.974	0.000
16	19.611	10.000
17	10.000	6.000
18	14.000	10.000
19	33.974	10.000
20	15.120	10.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

ons Nodas 10 11 13 20

1: No
2: No

2:	Nodes	12	13	14	16
3:	Nodes	17	18	20	

STB2006 - Slope Stability

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 5 of 43

Page 2

David S. Messinger, P.E., Load Case 2

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P	0.000	
2	12571.430	12571.430	1.000	0.000	0.000	P	0.000	
3	110.000	110.000	1.000	0.000	38.000	P	0.000	

Output Data

Safety Factors

1.241	1.247	1.255	1.262	1.274	1.283	1.318	1.347	1.379	1.412	1.447
1.202	1.208	1.213	1.218	1.223	1.234	1.240	1.247	1.254	1.266	1.274
1.173	1.176	1.179	1.183	1.187	1.190	1.199	1.202	1.207	1.212	1.217
1.169	1.168	1.169	1.169	1.165	1.167	1.171	1.172	1.180	1.181	1.184
1.194	1.190	1.188	1.185	1.184	1.184	1.186	1.187	1.187	1.188	1.190
1.319	1.315	1.305	1.289	1.285	1.282	1.282	1.284	1.287	1.287	1.294
1.483	1.472	1.463	1.456	1.451	1.448	1.447	1.447	1.449	1.453	1.458
1.468	1.448	1.429	1.413	1.398	1.385	1.373	1.364	1.356	1.349	1.345
1.495	1.467	1.439	1.413	1.389	1.366	1.344	1.324	1.305	1.288	1.272
1.581	1.545	1.511	1.477	1.444	1.413	1.382	1.352	1.324	1.296	1.270
1.746	1.708	1.670	1.633	1.596	1.560	1.525	1.491	1.457	1.424	1.392

Critical slip circle

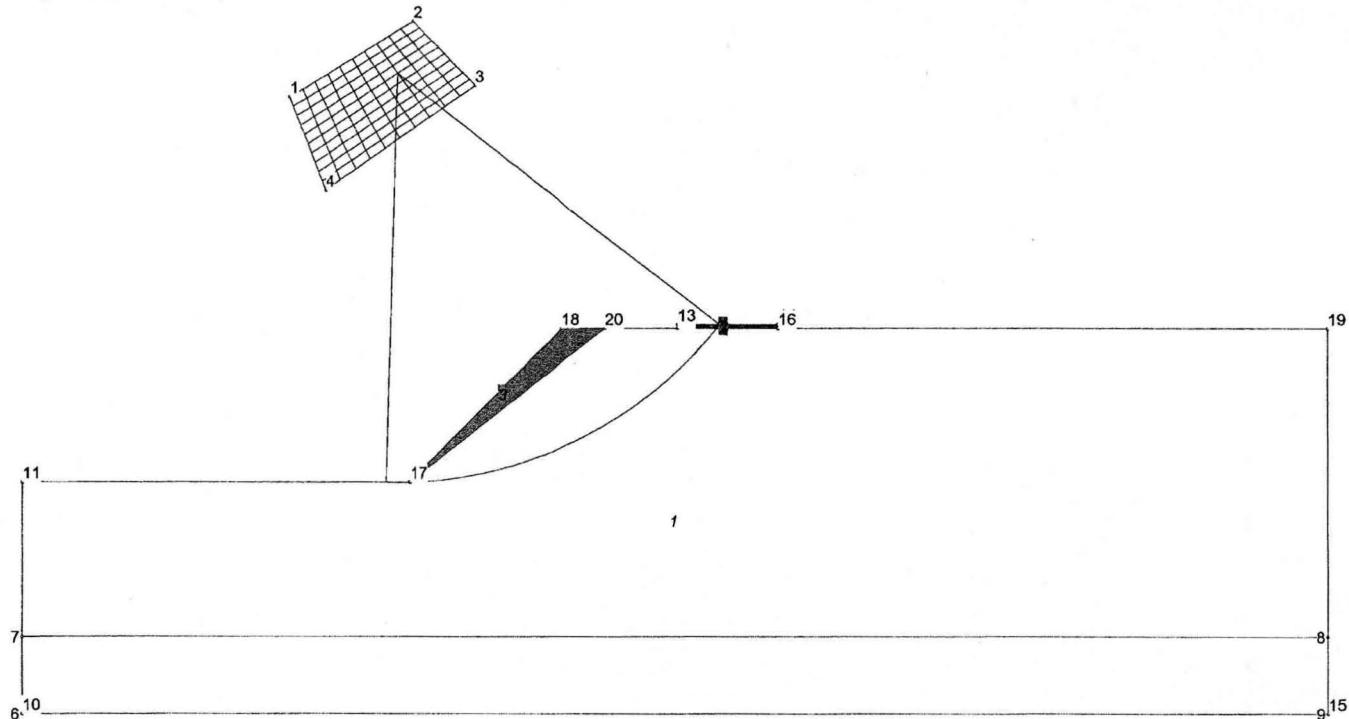
Center: x = 9.682, y = 16.638, Radius: 10.642, F = 1.165

David S. Messinger, P.E., Load Case 2A

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC02A.stb
 Problem Name : Load Case 2A
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Figure with critical slip circle



$$F = 1.165$$

Input Data

Coordinates of Nodes

Node	x	y
1	6.828	16.020
2	10.092	17.985
3	11.724	16.320
4	7.761	13.589
5	10.000	6.000
6	0.000	0.000
7	0.000	2.000
8	33.974	2.000
9	33.974	0.000
10	0.000	0.000
11	0.000	6.000
12	16.986	10.000
13	16.986	10.100
14	19.611	10.100
15	33.974	0.000
16	19.611	10.000
17	10.000	6.000
18	14.000	10.000
19	33.974	10.000
20	15.120	10.000

STB2006 - Slope Stability

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 7 of 43

Page 2

David S. Messinger, P.E., Load Case 2A

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1 : Nodes 10 11 17 20 12 19 15
2 : Nodes 12 13 14 16
3 : Nodes 17 18 20

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko —	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P	0.000	
2	12571.430	12571.430	1.000	0.000	0.000	P	0.000	
3	110.000	110.000	1.000	0.000	38.000	P	0.000	

Output Data

Safety Factors

1.241	1.247	1.255	1.262	1.274	1.283	1.318	1.347	1.379	1.412	1.446
1.202	1.208	1.213	1.218	1.223	1.233	1.240	1.247	1.254	1.266	1.274
1.173	1.176	1.179	1.183	1.187	1.190	1.199	1.203	1.207	1.212	1.217
1.169	1.168	1.169	1.169	1.165	1.167	1.171	1.172	1.180	1.181	1.184
1.194	1.190	1.188	1.185	1.184	1.184	1.186	1.187	1.187	1.188	1.190
1.319	1.315	1.305	1.289	1.285	1.282	1.282	1.284	1.288	1.287	1.294
1.483	1.472	1.463	1.456	1.451	1.448	1.447	1.447	1.449	1.453	1.458
1.468	1.448	1.429	1.413	1.398	1.385	1.373	1.364	1.356	1.350	1.345
1.495	1.467	1.439	1.413	1.389	1.366	1.344	1.324	1.305	1.288	1.272
1.581	1.545	1.511	1.477	1.444	1.413	1.382	1.352	1.323	1.296	1.270
1.746	1.708	1.670	1.633	1.596	1.560	1.525	1.491	1.457	1.424	1.392

Critical slip circle

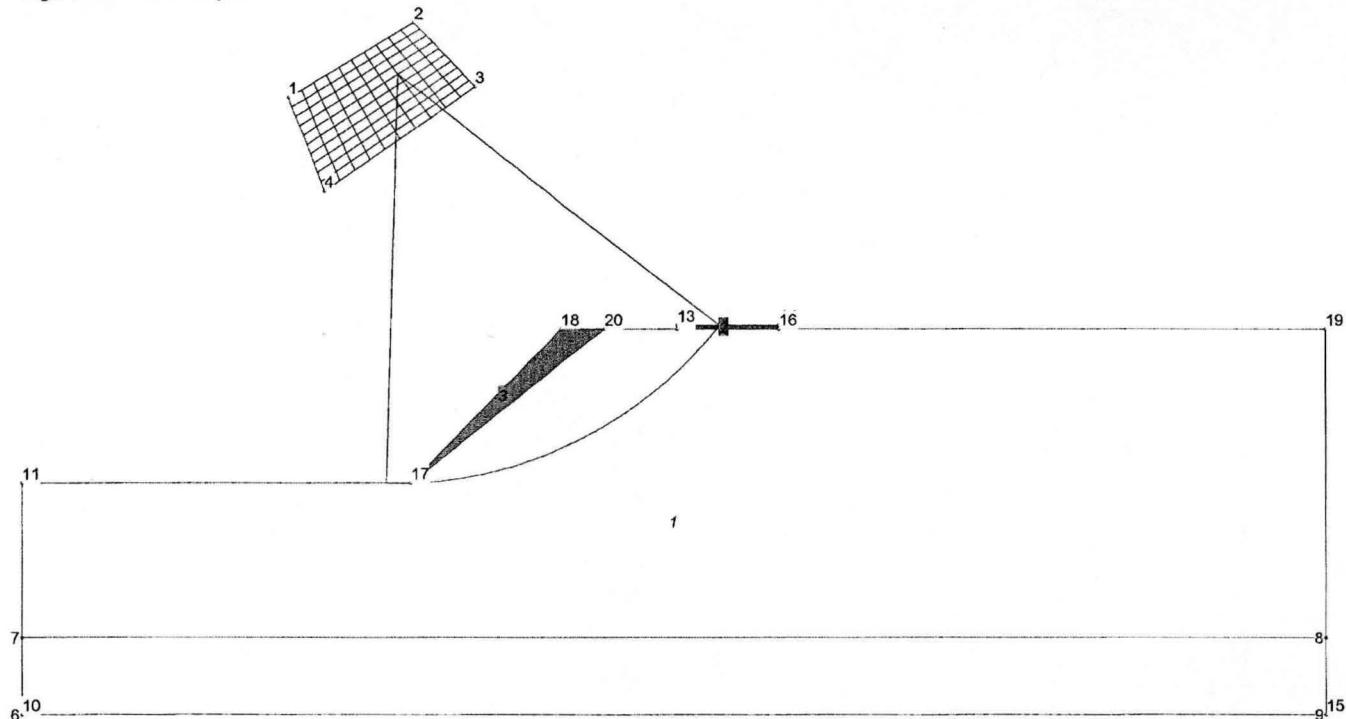
Center: x = 9.682, y = 16.638, Radius: 10.642, F = 1.165

David S. Messinger, P.E., Load Case 2B

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC02B.stb
 Problem Name : Load Case 2B
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Figure with critical slip circle



$$F = 1.090$$

Input Data

Coordinates of Nodes

Node	x	y
1	6.828	16.020
2	10.092	17.985
3	11.724	16.320
4	7.761	13.589
5	10.000	6.000
6	0.000	0.000
7	0.000	2.000
8	33.974	2.000
9	33.974	0.000
10	0.000	0.000
11	0.000	6.000
12	16.986	10.000
13	16.986	10.100
14	19.611	10.100
15	33.974	0.000
16	19.611	10.000
17	10.000	6.000
18	14.000	10.000
19	33.974	10.000
20	15.120	10.000

STB2006 - Slope Stability

David S. Messinger, P.E., Load Case 2B

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 9 of 43

Page 2

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1 :	Nodes	10	11	17	20	12	19	15
2 :	Nodes	12	13	14	16			
3 :	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko --	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P		0.000
2	12571.430	12571.430	1.000	0.000	0.000	P		0.000
3	0.000	10.000	1.000	0.000	0.000	P		0.000

Output Data

Safety Factors

1.183	1.189	1.196	1.204	1.217	1.225	1.260	1.288	1.318	1.349	1.383
1.140	1.146	1.151	1.156	1.161	1.172	1.178	1.185	1.192	1.205	1.213
1.105	1.108	1.112	1.116	1.120	1.123	1.132	1.136	1.140	1.145	1.151
1.094	1.093	1.094	1.095	1.090	1.092	1.096	1.097	1.106	1.107	1.109
1.111	1.107	1.105	1.101	1.100	1.100	1.102	1.103	1.103	1.104	1.106
1.239	1.232	1.221	1.200	1.195	1.191	1.190	1.191	1.194	1.193	1.199
1.457	1.441	1.428	1.417	1.408	1.401	1.397	1.395	1.395	1.397	1.400
1.481	1.454	1.429	1.407	1.387	1.369	1.354	1.340	1.329	1.320	1.314
1.576	1.539	1.503	1.469	1.438	1.408	1.380	1.355	1.331	1.310	1.291
1.782	1.738	1.695	1.653	1.613	1.575	1.538	1.503	1.469	1.438	1.408
2.175	2.136	2.099	2.063	2.030	1.999	1.970	1.941	1.911	1.881	1.850

Critical slip circle

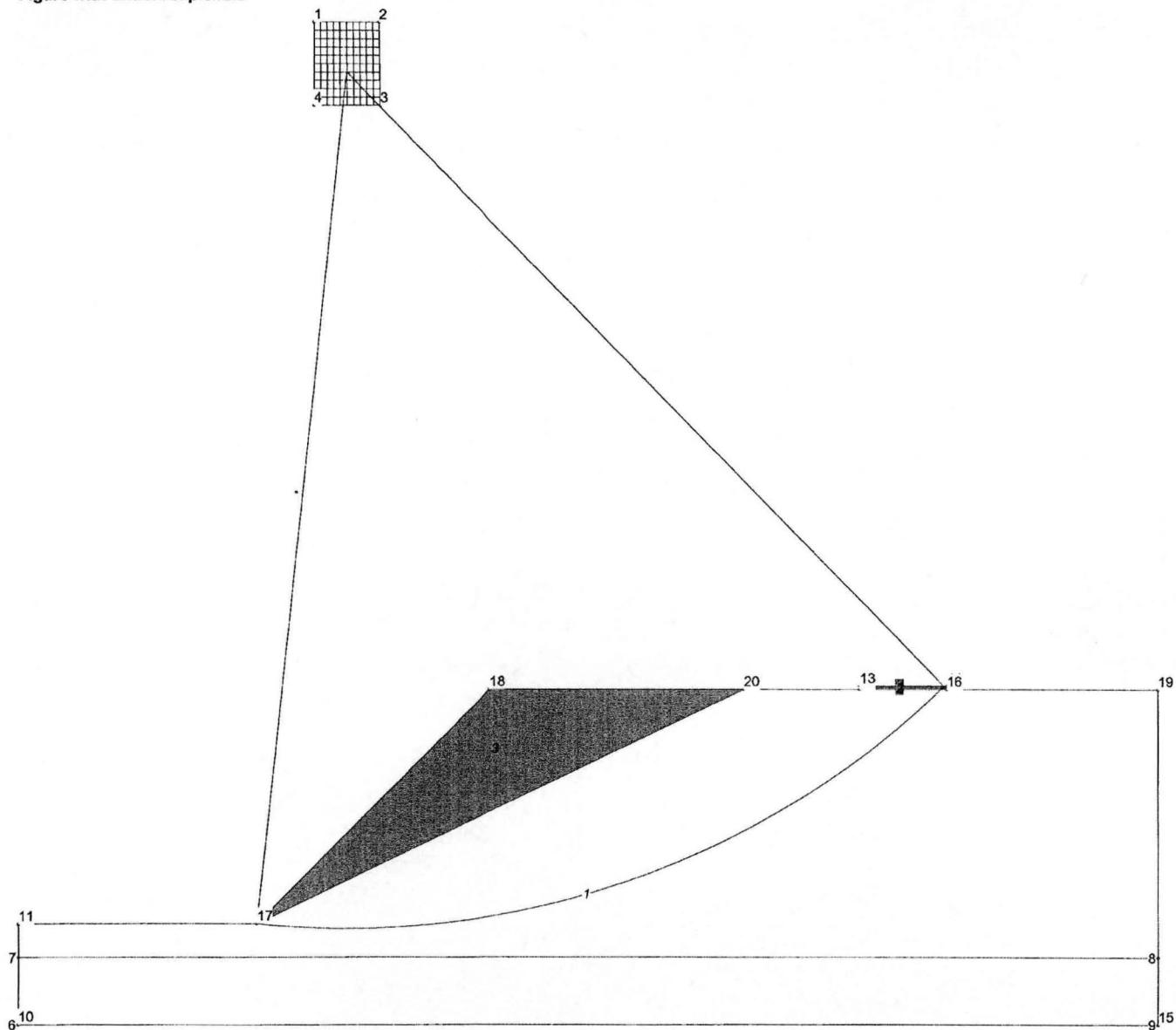
Center: x = 9.682, y = 16.638, Radius: 10.642, F = 1.090

David S. Messinger, Load Case 3

General Data

Licensed User : David S. Messinger
File Name : CWC03.stb
Problem Name : Load Case 3
Number of Slices : 1000
Relative Horizontal Force: 0.000000
Number of Nodes : 20
Number of Soil Polygons : 3

Figure with critical slip circle



$$F = 1.332$$

Input Data

Coordinates of Nodes

Node	x	y
1	8.711	29.877
2	10.668	29.877
3	10.708	27.390
4	8.711	27.390

STB2006 - Slope Stability

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 11 of 43

Page 2

David S. Messinger, Load Case 3

5	7.000	3.000
6	0.000	0.000
7	0.000	2.000
8	33.974	2.000
9	33.974	0.000
10	0.000	0.000
11	0.000	3.000
12	24.986	10.000
13	24.986	10.100
14	27.611	10.100
15	33.974	0.000
16	27.611	10.000
17	7.000	3.000
18	14.000	10.000
19	33.974	10.000
20	21.560	10.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1:	Nodes	10	11	17	20	12	19	15
2:	Nodes	12	13	14	16			
3:	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko —	c kN/m ²	phl degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	32.000	P		0.000
2	12571.430	12571.430	1.000	0.000	0.000	P		0.000
3	0.000	10.000	1.000	0.000	0.000	P		0.000

Output Data

Safety Factors

1.546	1.535	1.522	1.510	1.502	1.489	1.476	1.463	1.453	1.442	1.432
1.516	1.503	1.492	1.480	1.469	1.459	1.446	1.436	1.422	1.410	1.397
1.486	1.474	1.463	1.451	1.440	1.427	1.416	1.404	1.391	1.380	1.367
1.458	1.445	1.432	1.421	1.409	1.398	1.385	1.375	1.362	1.341	1.337
1.427	1.414	1.404	1.393	1.380	1.370	1.357	1.334	1.338	1.338	1.339
1.399	1.389	1.376	1.363	1.346	1.336	1.332	1.332	1.339	1.340	1.342
1.370	1.359	1.336	1.333	1.333	1.333	1.334	1.335	1.338	1.339	1.344
1.335	1.333	1.333	1.333	1.333	1.336	1.337	1.339	1.343	1.345	1.351
1.333	1.333	1.335	1.336	1.337	1.341	1.342	1.347	1.350	1.356	1.363
1.335	1.336	1.337	1.341	1.342	1.347	1.350	1.356	1.362	1.367	1.375
1.337	1.341	1.343	1.347	1.350	1.356	1.362	1.367	1.375	1.384	1.395

Critical slip circle

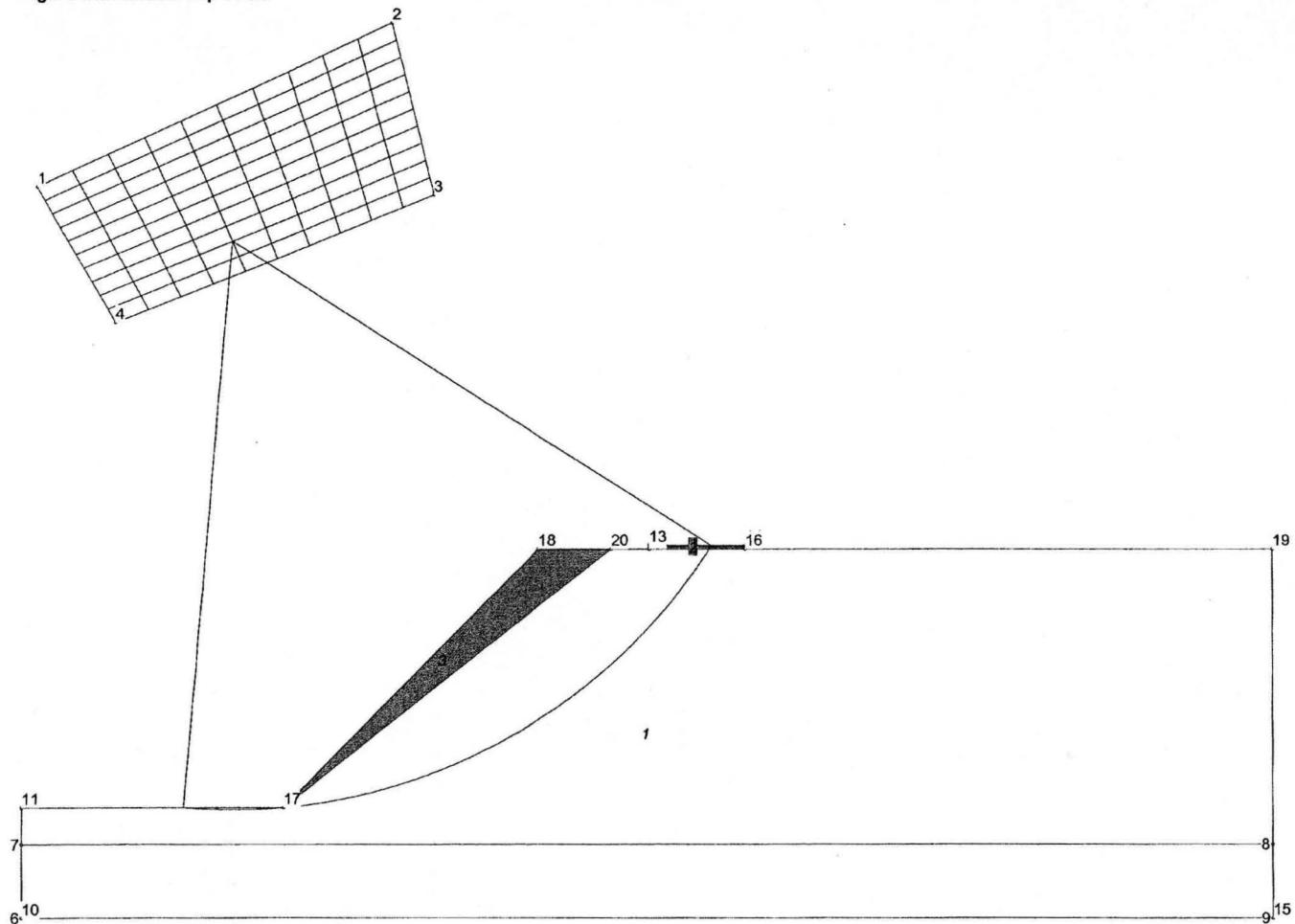
Center: x = 9.701, y = 28.385, Radius: 25.528, F = 1.332

David S. Messinger, P.E., Load Case 3

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC03A.stb
 Problem Name : Load Case 3
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Figure with critical slip circle

 $F = 1.076$

Input Data

Coordinates of Nodes

Node	x	y
1	0.461	19.840
2	10.044	24.292
3	11.176	19.613
4	2.574	16.142
5	7.000	3.000
6	0.000	0.000
7	0.000	2.000
8	33.974	2.000
9	33.974	0.000
10	0.000	0.000
11	0.000	3.000
12	16.986	10.000
13	16.986	10.100
14	19.611	10.100
15	33.974	0.000

16	19.611	10.000
17	7.000	3.000
18	14.000	10.000
19	33.974	10.000
20	15.959	10.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1 :	Nodes	10	11	17	20	12	19	15
2 :	Nodes	12	13	14	16			
3 :	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko --	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P		0.000
2	12571.430	12571.430	1.000	0.000	0.000	P		0.000
3	110.000	110.000	1.000	0.000	38.000	P		0.000

Output Data**Safety Factors**

2.394	2.389	2.387	2.383	2.381	2.378	2.375	2.372	2.373	2.372	2.370
2.102	2.098	2.092	2.087	2.082	2.080	2.076	2.069	2.066	2.063	2.061
1.843	1.836	1.830	1.824	1.818	1.811	1.805	1.800	1.794	1.790	1.784
1.609	1.602	1.594	1.587	1.581	1.572	1.565	1.559	1.552	1.544	1.537
1.400	1.393	1.385	1.376	1.367	1.360	1.350	1.342	1.335	1.327	1.318
1.219	1.209	1.198	1.189	1.179	1.169	1.160	1.151	1.141	1.124	1.125
1.106	1.100	1.094	1.088	1.084	1.081	1.079	1.077	1.076	1.078	1.079
1.125	1.115	1.109	1.103	1.098	1.095	1.092	1.093	1.095	1.099	1.107
1.290	1.293	1.296	1.322	1.325	1.353	1.336	1.312	1.290	1.269	1.250
1.498	1.488	1.476	1.463	1.449	1.432	1.413	1.392	1.367	1.340	1.309
1.577	1.577	1.576	1.574	1.571	1.565	1.556	1.546	1.533	1.517	1.498

Critical slip circle

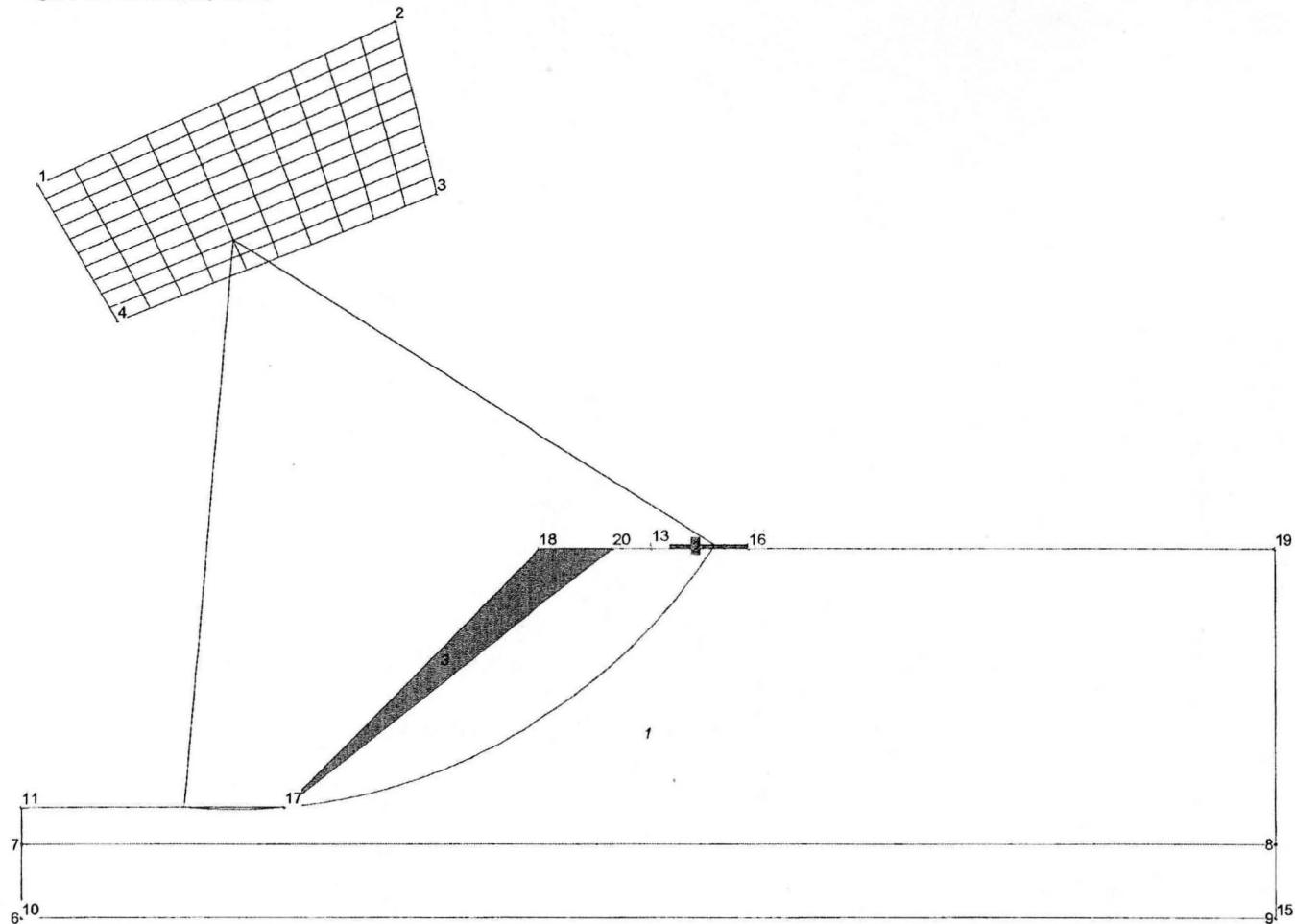
Center: x = 5.671, y = 18.349, Radius: 15.406, F = 1.076

David S. Messinger, P.E., Load Case 3

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC03A.slb
 Problem Name : Load Case 3
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Figure with critical slip circle

**F = 1.076****Input Data****Coordinates of Nodes**

Node	x	y
1	0.461	19.840
2	10.044	24.292
3	11.176	19.613
4	2.574	16.142
5	7.000	3.000
6	0.000	0.000
7	0.000	2.000
8	33.974	2.000
9	33.974	0.000
10	0.000	0.000
11	0.000	3.000
12	16.986	10.000
13	16.986	10.100
14	19.611	10.100
15	33.974	0.000

David S. Messinger, P.E., Load Case 3

16	19.611	10.000
17	7.000	3.000
18	14.000	10.000
19	33.974	10.000
20	15.959	10.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1:	Nodes	10	11	17	20	12	19	15
2:	Nodes	12	13	14	16			
3:	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P		0.000
2	12571.430	12571.430	1.000	0.000	0.000	P		0.000
3	110.000	110.000	1.000	0.000	38.000	P		0.000

Output Data**Safety Factors**

2.394	2.389	2.387	2.383	2.381	2.378	2.375	2.372	2.373	2.372	2.370
2.102	2.098	2.092	2.087	2.082	2.080	2.076	2.069	2.066	2.063	2.061
1.843	1.836	1.830	1.824	1.818	1.811	1.805	1.800	1.794	1.790	1.784
1.609	1.602	1.594	1.587	1.581	1.572	1.565	1.559	1.552	1.544	1.537
1.400	1.393	1.385	1.376	1.367	1.360	1.350	1.342	1.335	1.327	1.318
1.219	1.209	1.198	1.189	1.179	1.169	1.160	1.151	1.141	1.124	1.125
1.106	1.100	1.094	1.088	1.084	1.081	1.079	1.077	1.076	1.078	1.079
1.125	1.115	1.109	1.103	1.098	1.095	1.092	1.093	1.095	1.099	1.107
1.290	1.293	1.296	1.322	1.325	1.353	1.336	1.312	1.290	1.269	1.250
1.498	1.488	1.476	1.463	1.449	1.432	1.413	1.392	1.367	1.340	1.309
1.577	1.577	1.576	1.574	1.571	1.565	1.556	1.546	1.533	1.517	1.498

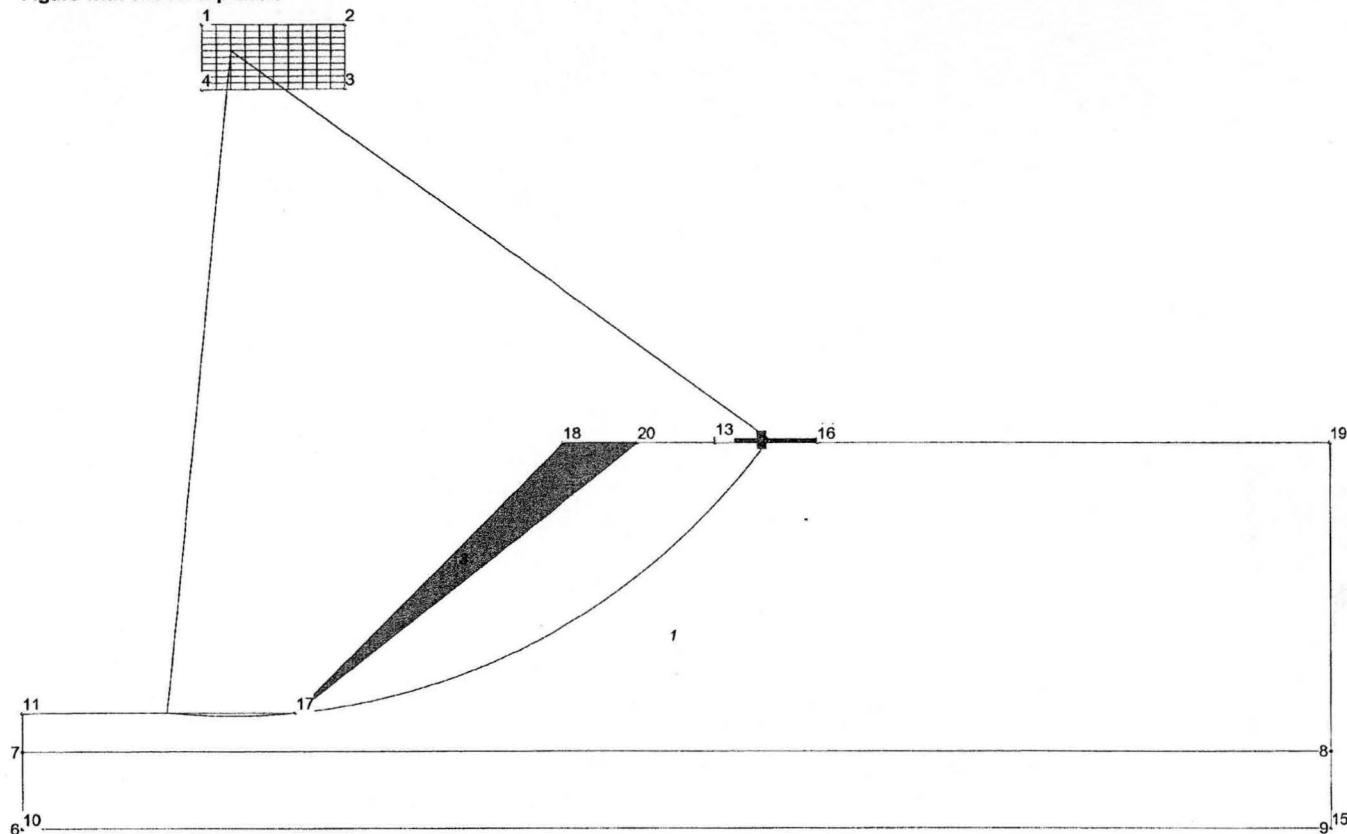
Critical slip circle

Center: x = 5.671, y = 18.349, Radius: 15.406, F = 1.076

David S. Messinger, P.E., Load Case 3B

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC03B.stb
 Problem Name : Load Case 3B
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Figure with critical slip circle

$$F = 1.104$$

Input Data**Coordinates of Nodes**

Node	x	y
1	4.596	20.897
2	8.327	20.897
3	8.327	19.231
4	4.596	19.198
5	7.000	3.000
6	0.000	0.000
7	0.000	2.000
8	33.974	2.000
9	33.974	0.000
10	0.000	0.000
11	0.000	3.000
12	17.986	10.000
13	17.986	10.100
14	20.611	10.100
15	33.974	0.000
16	20.611	10.000
17	7.000	3.000
18	14.000	10.000
19	33.974	10.000
20	15.959	10.000

STB2006 - Slope Stability

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 17 of 43

Page 2

David S. Messinger, P.E., Load Case 3B

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1 : Nodes 10 11 17 20 12 19 15

2 : Nodes 12 13 14 16

3 : Nodes 17 18 20

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	--	0.000	38.000	P		0.000
2	12571.430	12571.430	1.000	0.000	0.000	P		0.000
3	0.000	10.000	1.000	0.000	0.000	P		0.000

Output Data

Safety Factors

1.511	1.499	1.484	1.470	1.456	1.442	1.428	1.414	1.400	1.386	1.371
1.437	1.423	1.409	1.395	1.381	1.368	1.353	1.339	1.324	1.311	1.296
1.365	1.352	1.339	1.324	1.310	1.296	1.282	1.268	1.254	1.240	1.226
1.298	1.284	1.271	1.257	1.242	1.228	1.214	1.200	1.187	1.173	1.149
1.234	1.219	1.206	1.193	1.179	1.165	1.143	1.139	1.135	1.131	1.128
1.172	1.152	1.139	1.134	1.130	1.126	1.122	1.125	1.122	1.119	1.117
1.133	1.129	1.125	1.122	1.119	1.116	1.113	1.111	1.109	1.109	1.108
1.118	1.114	1.111	1.109	1.107	1.106	1.105	1.104	1.104	1.106	1.107
1.108	1.107	1.104	1.104	1.104	1.104	1.105	1.107	1.111	1.115	1.122
1.106	1.106	1.108	1.110	1.113	1.116	1.121	1.127	1.133	1.142	1.155
1.116	1.120	1.125	1.129	1.136	1.144	1.155	1.167	1.186	1.204	1.224

Critical slip circle

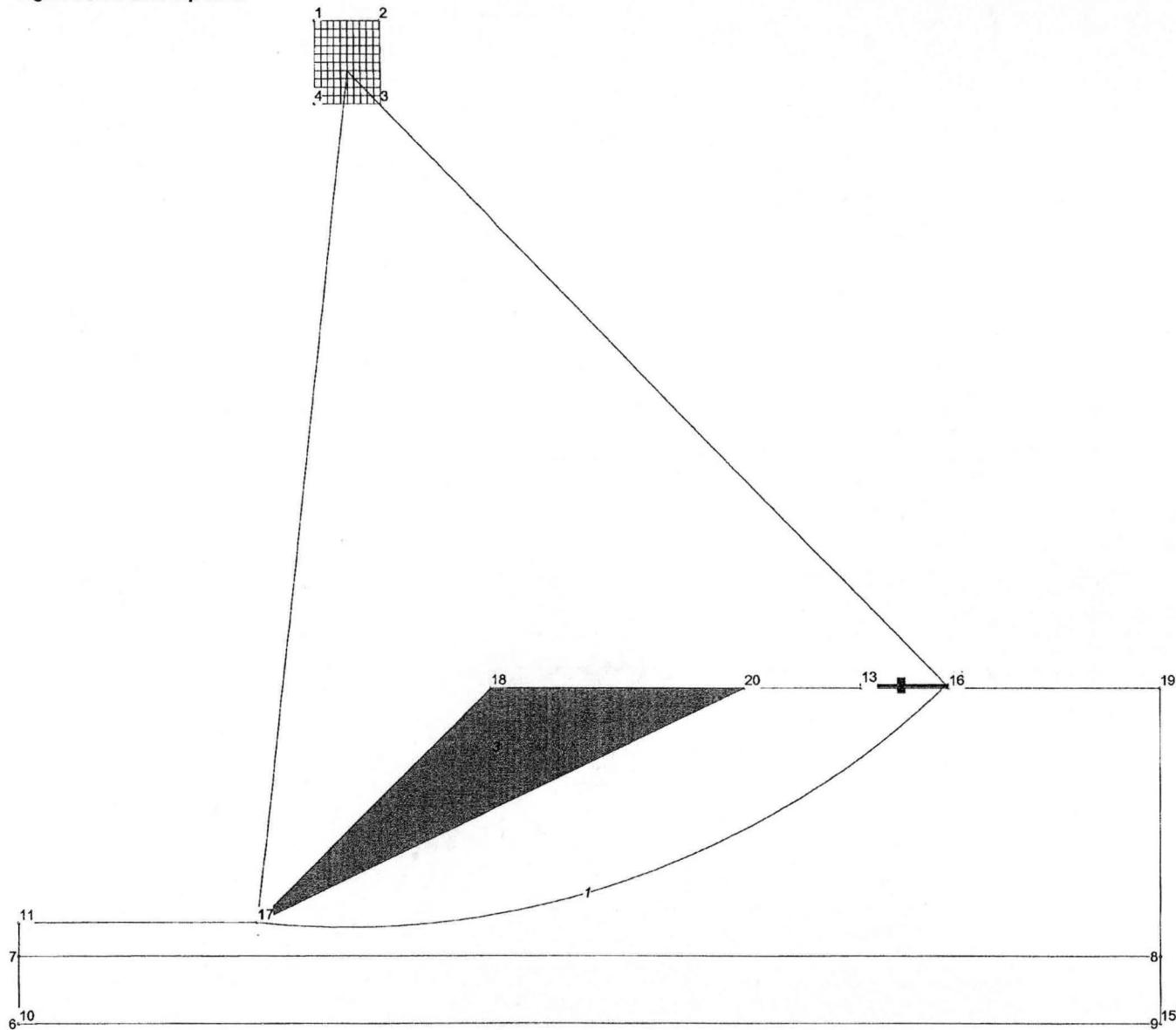
Center: x = 5.343, y = 20.220, Radius: 17.300, F = 1.104

David S. Messinger, Load Case 4

General Data

Licensed User : David S. Messinger
 File Name : CWC04.stb
 Problem Name : Load Case 4
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Figure with critical slip circle



$$F = 1.332$$

Input Data

Coordinates of Nodes

Node	x	y
1	8.711	29.877
2	10.668	29.877
3	10.708	27.390
4	8.711	27.390

David S. Messinger, Load Case 4

5	7.000	3.000
6	0.000	0.000
7	0.000	2.000
8	33.974	2.000
9	33.974	0.000
10	0.000	0.000
11	0.000	3.000
12	24.986	10.000
13	24.986	10.100
14	27.611	10.100
15	33.974	0.000
16	27.611	10.000
17	7.000	3.000
18	14.000	10.000
19	33.974	10.000
20	21.560	10.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1:	Nodes	10	11	17	20	12	19	15
2:	Nodes	12	13	14	16			
3:	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko —	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	32.000	P		0.000
2	12571.430	12571.430	1.000	0.000	0.000	P		0.000
3	0.000	10.000	1.000	0.000	0.000	P		0.000

Output Data**Safety Factors**

1.546	1.535	1.522	1.510	1.502	1.489	1.476	1.465	1.453	1.442	1.432
1.516	1.503	1.492	1.480	1.469	1.459	1.446	1.436	1.422	1.410	1.397
1.486	1.474	1.463	1.451	1.440	1.427	1.416	1.404	1.391	1.380	1.367
1.458	1.445	1.432	1.421	1.409	1.398	1.385	1.375	1.362	1.341	1.337
1.427	1.414	1.404	1.393	1.380	1.370	1.357	1.334	1.338	1.338	1.339
1.399	1.389	1.376	1.363	1.346	1.336	1.332	1.332	1.339	1.340	1.342
1.370	1.359	1.336	1.333	1.333	1.333	1.334	1.335	1.338	1.339	1.344
1.335	1.333	1.333	1.333	1.333	1.336	1.337	1.339	1.343	1.345	1.351
1.333	1.333	1.335	1.336	1.337	1.341	1.342	1.347	1.350	1.356	1.363
1.335	1.336	1.337	1.341	1.342	1.347	1.350	1.356	1.362	1.367	1.375
1.337	1.341	1.343	1.347	1.350	1.356	1.362	1.367	1.375	1.384	1.395

Critical slip circle

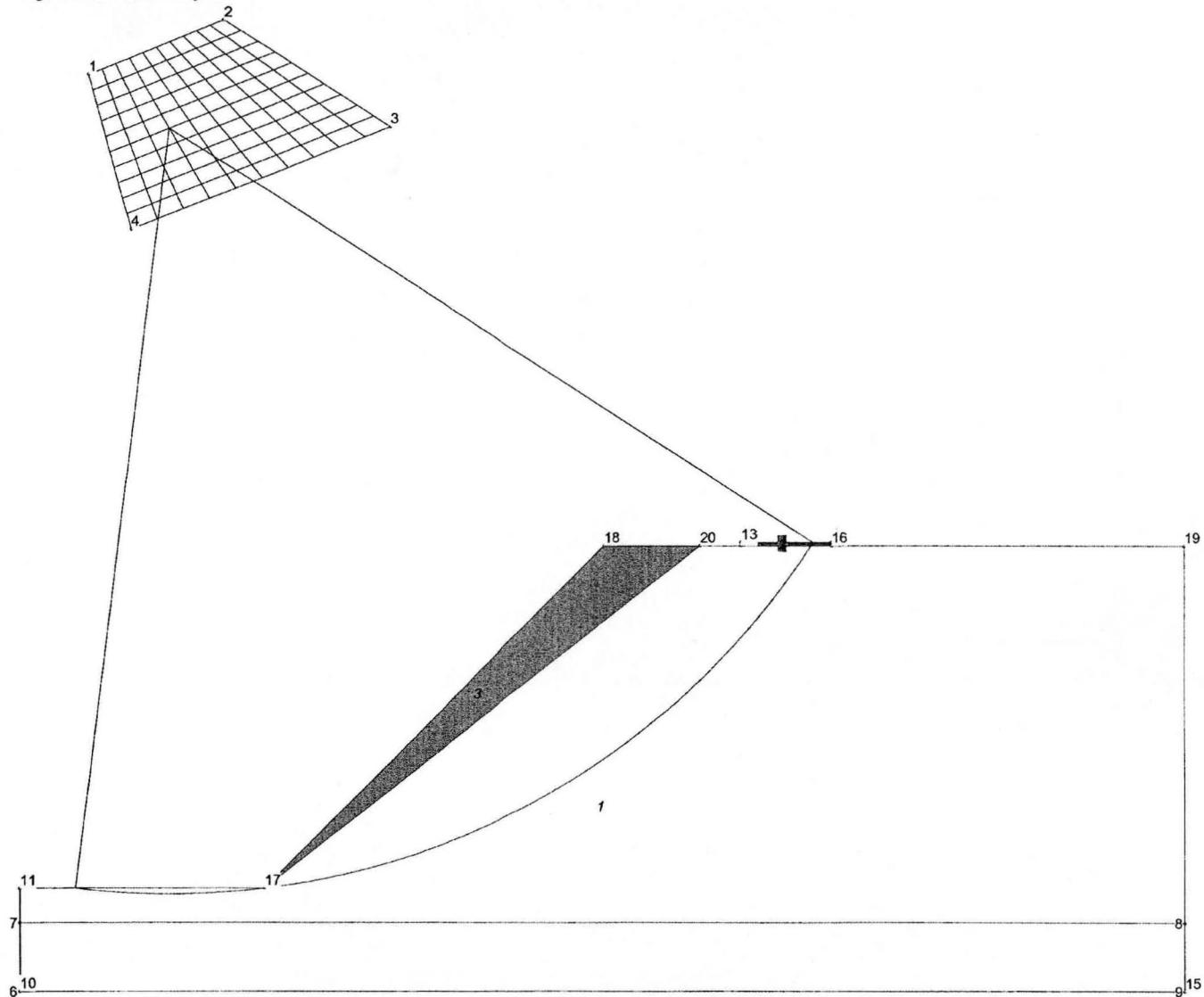
Center: x = 9.702, y = 28.385, Radius: 25.528, F = 1.332

David S. Messinger, P.E., Load Case 4A

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC04A.stb
 Problem Name : Load Case 4A
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Figure with critical slip circle



$$F = 1.121$$

Input Data

Coordinates of Nodes

Node	x	y
1	2.000	26.734
2	5.868	28.333
3	10.702	25.228
4	3.207	22.212
5	7.000	3.000
6	0.000	0.000
7	0.000	2.000

8	34.000	2.000
9	34.000	0.000
10	0.000	0.000
11	0.000	3.000
12	21.000	13.000
13	21.000	13.100
14	23.625	13.100
15	34.000	0.000
16	23.625	13.000
17	7.000	3.000
18	17.000	13.000
19	34.000	13.000
20	19.799	13.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1:	Nodes	10	11	17	20	12	19	15
2:	Nodes	12	13	14	16			
3:	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P	0.000	0.000
2	12571.430	12571.430	1.000	0.000	0.000	P	0.000	0.000
3	110.000	110.000	1.000	0.000	38.000	P	0.000	0.000

Output Data**Safety Factors**

1.423	1.464	1.510	1.558	1.609	1.664	1.719	1.780	1.846	1.915	1.989
1.370	1.403	1.440	1.479	1.522	1.566	1.612	1.661	1.716	1.771	1.831
1.319	1.344	1.373	1.404	1.438	1.473	1.509	1.549	1.592	1.635	1.683
1.270	1.288	1.310	1.333	1.358	1.384	1.412	1.442	1.474	1.508	1.544
1.223	1.235	1.249	1.264	1.280	1.299	1.320	1.341	1.364	1.389	1.414
1.181	1.186	1.194	1.200	1.210	1.221	1.233	1.247	1.260	1.276	1.291
1.136	1.133	1.133	1.135	1.140	1.149	1.152	1.157	1.164	1.170	1.178
1.133	1.129	1.132	1.129	1.122	1.121	1.121	1.122	1.124	1.125	1.129
1.136	1.133	1.137	1.135	1.132	1.131	1.128	1.129	1.132	1.133	1.136
1.146	1.145	1.152	1.154	1.154	1.158	1.161	1.165	1.171	1.179	1.191
1.161	1.166	1.174	1.190	1.203	1.218	1.235	1.262	1.256	1.244	1.231

Critical slip circle

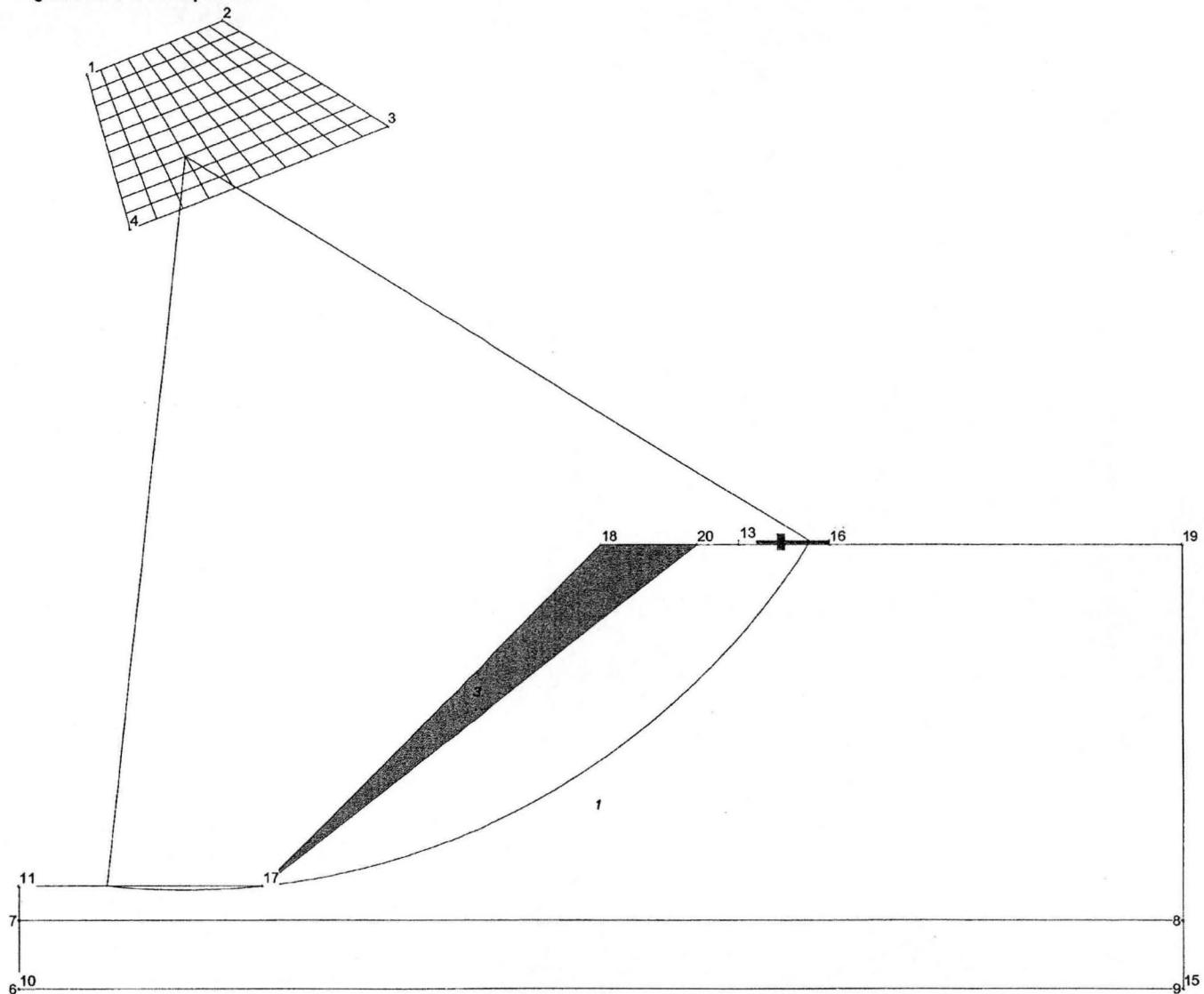
Center: x = 4.308, y = 25.165, Radius: 22.328, F = 1.121

David S. Messinger, P.E., Load Case 4B

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC04B.stb
 Problem Name : Load Case 4B
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Figure with critical slip circle



$F = 1.048$

Input Data

Coordinates of Nodes

Node	x	y
1	2.000	26.734
2	5.868	28.333
3	10.702	25.228
4	3.207	22.212
5	7.000	3.000
6	0.000	0.000
7	0.000	2.000

8	34.000	2.000
9	34.000	0.000
10	0.000	0.000
11	0.000	3.000
12	21.000	13.000
13	21.000	13.100
14	23.625	13.100
15	34.000	0.000
16	23.625	13.000
17	7.000	3.000
18	17.000	13.000
19	34.000	13.000
20	19.799	13.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1 :	Nodes	10	11	17	20	12	19	15
2 :	Nodes	12	13	14	16			
3 :	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko --	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P	0.000	0.000
2	12571.430	12571.430	1.000	0.000	0.000	P	0.000	0.000
3	0.000	10.000	1.000	0.000	0.000	P	0.000	0.000

Output Data**Safety Factors**

1.344	1.383	1.426	1.471	1.519	1.570	1.622	1.679	1.740	1.802	1.870
1.293	1.325	1.359	1.396	1.436	1.477	1.520	1.566	1.617	1.668	1.723
1.244	1.267	1.295	1.324	1.355	1.388	1.422	1.460	1.499	1.539	1.584
1.197	1.213	1.233	1.255	1.278	1.303	1.329	1.357	1.387	1.418	1.452
1.152	1.162	1.174	1.189	1.203	1.221	1.241	1.260	1.282	1.304	1.328
1.111	1.115	1.121	1.126	1.135	1.145	1.156	1.169	1.181	1.195	1.210
1.067	1.063	1.061	1.062	1.066	1.075	1.077	1.081	1.087	1.092	1.099
1.067	1.062	1.064	1.060	1.051	1.049	1.048	1.048	1.049	1.049	1.052
1.076	1.071	1.076	1.073	1.069	1.067	1.064	1.064	1.067	1.069	1.072
1.094	1.094	1.103	1.107	1.108	1.113	1.120	1.127	1.137	1.150	1.169
1.121	1.130	1.144	1.170	1.192	1.220	1.253	1.305	1.307	1.300	1.294

Critical slip circle

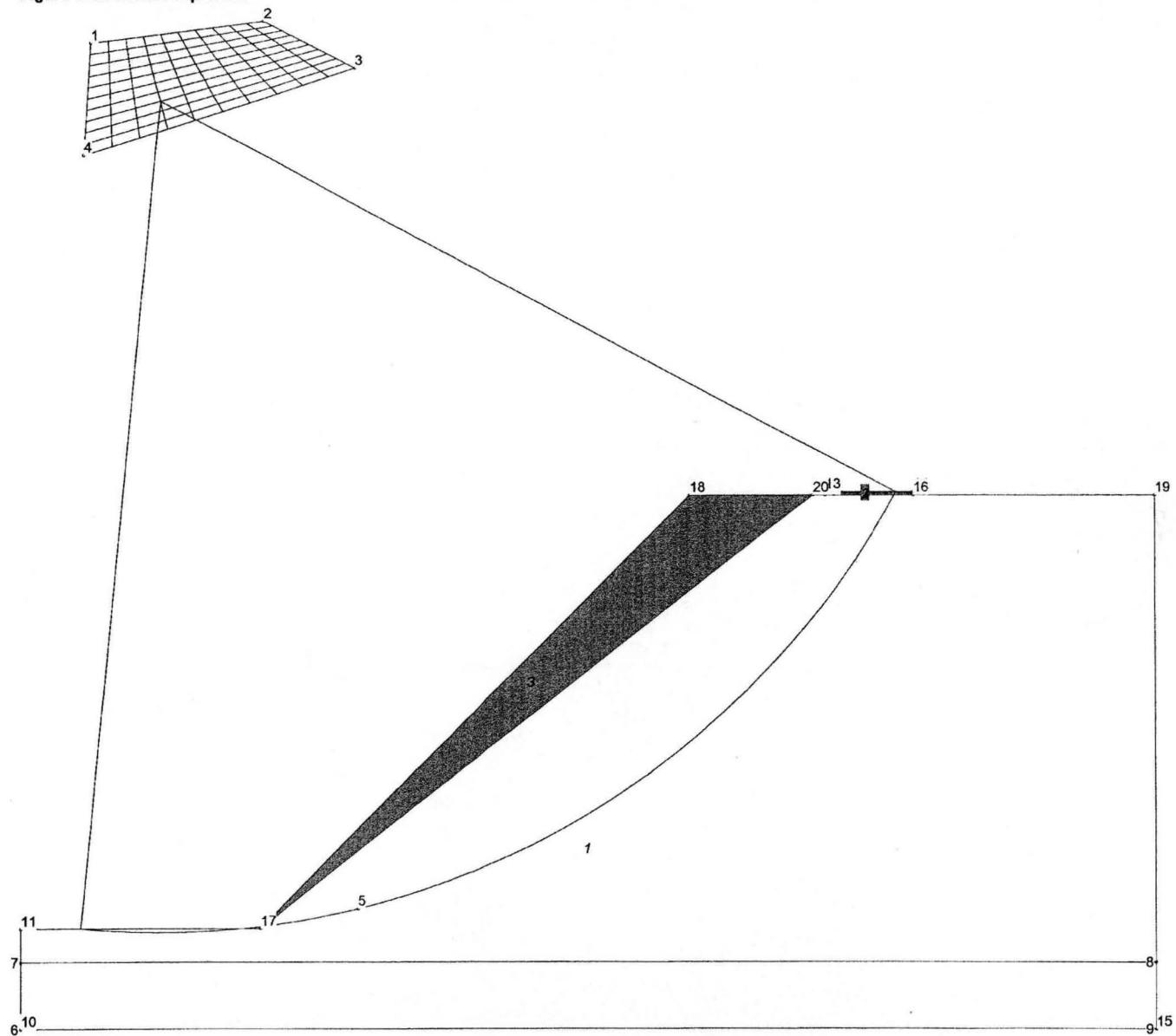
Center: x = 4.767, y = 24.346, Radius: 21.462, F = 1.048

David S. Messinger, P.E., Load Case 5A

General Data

Licensed User : David S. Messinger, P.E.
File Name : CWC05A.stb
Problem Name : Load Case 5A
Number of Slices : 1000
Relative Horizontal Force: 0.000000
Number of Nodes : 20
Number of Soil Polygons : 3

Figure with critical slip circle



F = 1.088

David S. Messinger, P.E., Load Case 5A

Input Data**Coordinates of Nodes**

Node	x	y
1	2.111	29.560
2	7.127	30.226
3	9.935	28.822
4	1.905	26.189
5	10.000	3.600
6	0.000	0.000
7	0.000	2.000
8	34.000	2.000
9	34.000	0.000
10	0.000	0.000
11	0.000	3.000
12	24.000	16.000
13	24.000	16.100
14	26.625	16.100
15	34.000	0.000
16	26.625	16.000
17	7.000	3.000
18	20.000	16.000
19	34.000	16.000
20	23.639	16.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1:	Nodes	10	11	17	20	12	19	15
2:	Nodes	12	13	14	16			
3:	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko —	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P	0.000	0.000
2	12571.430	12571.430	1.000	0.000	0.000	P	0.000	0.000
3	110.000	110.000	1.000	0.000	38.000	P	0.000	0.000

Output Data**Safety Factors**

1.410	1.432	1.453	1.476	1.500	1.524	1.548	1.573	1.600	1.628	1.654
1.360	1.376	1.393	1.410	1.427	1.446	1.465	1.485	1.504	1.524	1.547
1.312	1.322	1.335	1.348	1.359	1.372	1.385	1.400	1.414	1.429	1.443
1.264	1.272	1.281	1.288	1.296	1.304	1.312	1.320	1.330	1.339	1.348
1.221	1.223	1.228	1.230	1.235	1.238	1.241	1.247	1.250	1.255	1.258
1.178	1.178	1.178	1.177	1.177	1.176	1.177	1.176	1.176	1.176	1.176
1.139	1.135	1.131	1.126	1.124	1.119	1.115	1.111	1.104	1.103	1.105
1.111	1.103	1.091	1.090	1.089	1.089	1.089	1.088	1.089	1.089	1.091
1.097	1.096	1.096	1.097	1.096	1.097	1.099	1.103	1.106	1.109	1.121
1.104	1.106	1.108	1.112	1.116	1.120	1.128	1.135	1.146	1.157	1.178
1.114	1.121	1.127	1.134	1.145	1.158	1.170	1.190	1.210	1.226	1.222

Critical slip circle

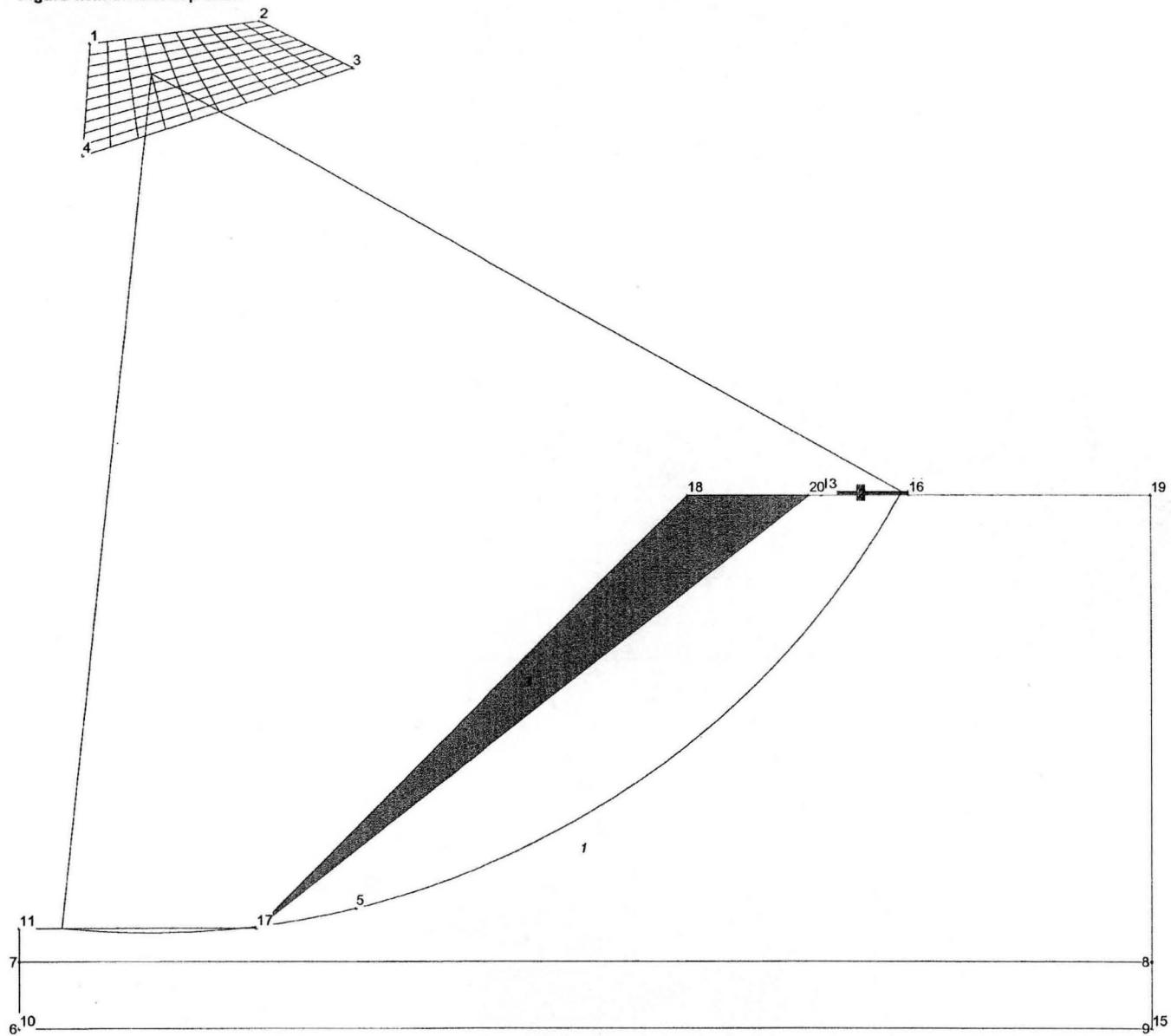
Center: x = 4.104, y = 27.813, Radius: 24.921, F = 1.088

David S. Messinger, P.E., Load Case 5B

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC05B.stb
 Problem Name : Load Case 5B
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Figure with critical slip circle



$$F = 1.022$$

Input Data**Coordinates of Nodes**

Node	x	y
1	2.111	29.560
2	7.127	30.226
3	9.935	28.822
4	1.905	26.189
5	10.000	3.600
6	0.000	0.000
7	0.000	2.000
8	34.000	2.000
9	34.000	0.000
10	0.000	0.000
11	0.000	3.000
12	24.000	16.000
13	24.000	16.100
14	26.625	16.100
15	34.000	0.000
16	26.625	16.000
17	7.000	3.000
18	20.000	16.000
19	34.000	16.000
20	23.639	16.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1:	Nodes	10	11	17	20	12	19	15
2:	Nodes	12	13	14	16			
3:	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko —	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P	0.000	
2	12571.430	12571.430	1.000	0.000	0.000	P	0.000	
3	0.000	10.000	1.000	0.000	0.000	P	0.000	

Output Data**Safety Factors**

1.320	1.341	1.360	1.380	1.402	1.424	1.445	1.468	1.492	1.517	1.541
1.275	1.289	1.304	1.319	1.334	1.351	1.368	1.387	1.404	1.421	1.442
1.230	1.239	1.250	1.262	1.271	1.283	1.295	1.308	1.320	1.334	1.347
1.185	1.192	1.199	1.206	1.213	1.220	1.227	1.234	1.243	1.250	1.258
1.145	1.146	1.150	1.152	1.156	1.158	1.161	1.165	1.168	1.172	1.175
1.104	1.104	1.103	1.102	1.101	1.101	1.100	1.099	1.099	1.098	1.098
1.068	1.064	1.060	1.054	1.051	1.047	1.042	1.038	1.030	1.029	1.031
1.045	1.037	1.023	1.022	1.022	1.023	1.023	1.023	1.025	1.027	1.030
1.038	1.038	1.040	1.042	1.043	1.047	1.052	1.059	1.066	1.073	1.094
1.055	1.061	1.067	1.075	1.085	1.097	1.113	1.129	1.151	1.177	1.218
1.080	1.094	1.108	1.124	1.148	1.175	1.205	1.249	1.298	1.342	1.353

Critical slip circle

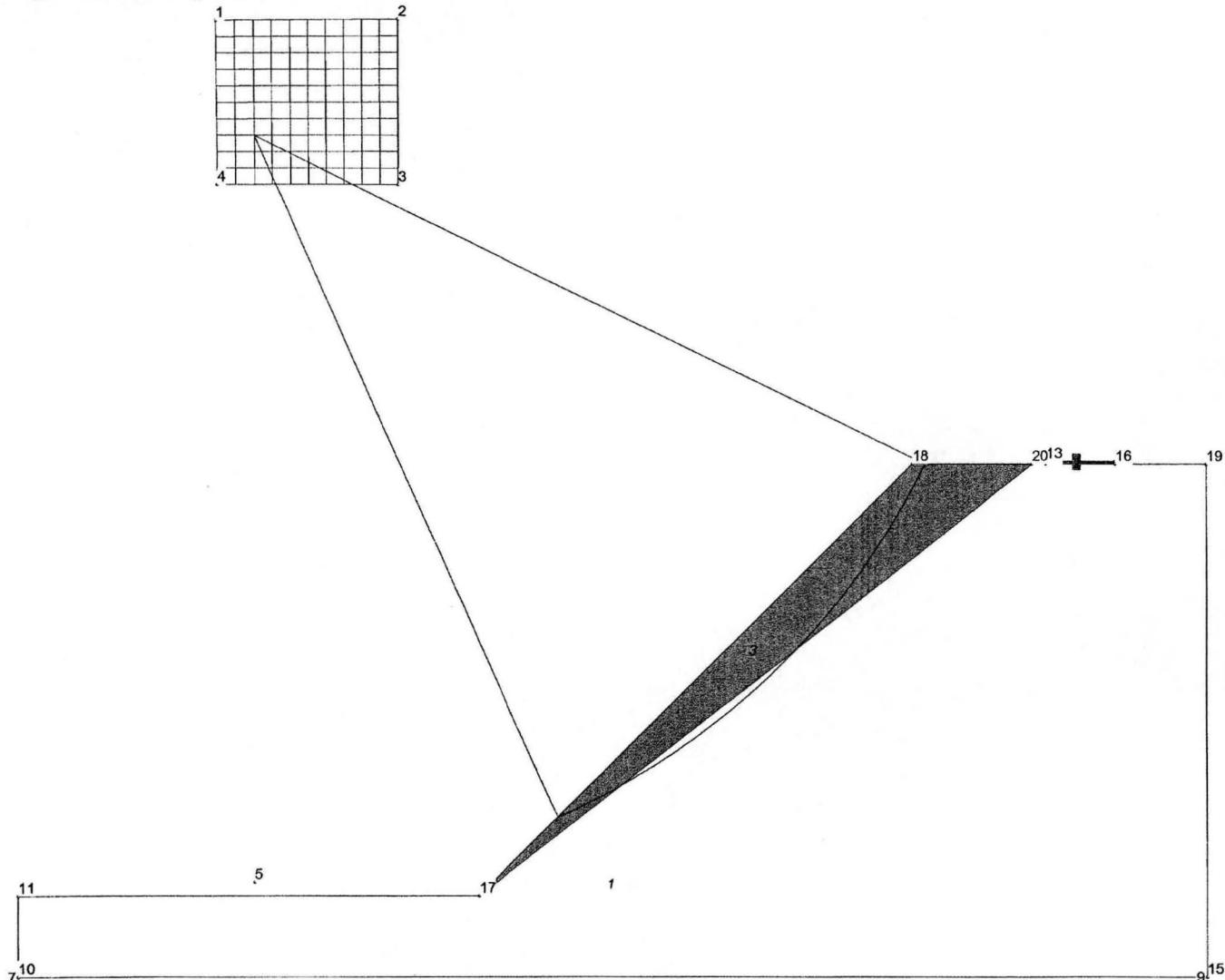
Center: x = 3.895, y = 28.648, Radius: 25.781, F = 1.022

David S. Messinger, P.E., Load Case 6A

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC06A.stb
 Problem Name : Load Case 6A
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Figure with critical slip circle



$$F = 1.000$$

Input Data

Coordinates of Nodes

Node	x	y
1	7.192	35.486
2	14.013	35.541
3	14.013	29.385
4	7.247	29.385
5		3.500
6	0.000	0.000
7	0.000	0.000
8	44.000	0.000
9	44.000	0.000

10	0.000	0.000
11	0.000	3.000
12	38.000	19.000
13	38.000	19.100
14	40.625	19.100
15	44.000	0.000
16	40.625	19.000
17	17.000	3.000
18	33.000	19.000
19	44.000	19.000
20	37.479	19.000

Structure of Polygons

Window of centers of slip circles
Nodes 1 2 3 4

Deepest point of all slip circles
Node 5

Water Polygon
Nodes 6 7 8 9

Soil Polygons

1:	Nodes	10	11	17	20	12	19	15
2:	Nodes	12	13	14	16			
3:	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko --	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	12.300	38.000	P		0.000
2	12571.430	12571.430	1.000	0.000	0.000	P		0.000
3	110.000	110.000	1.000	12.300	38.000	P		0.000

Output Data**Safety Factors**

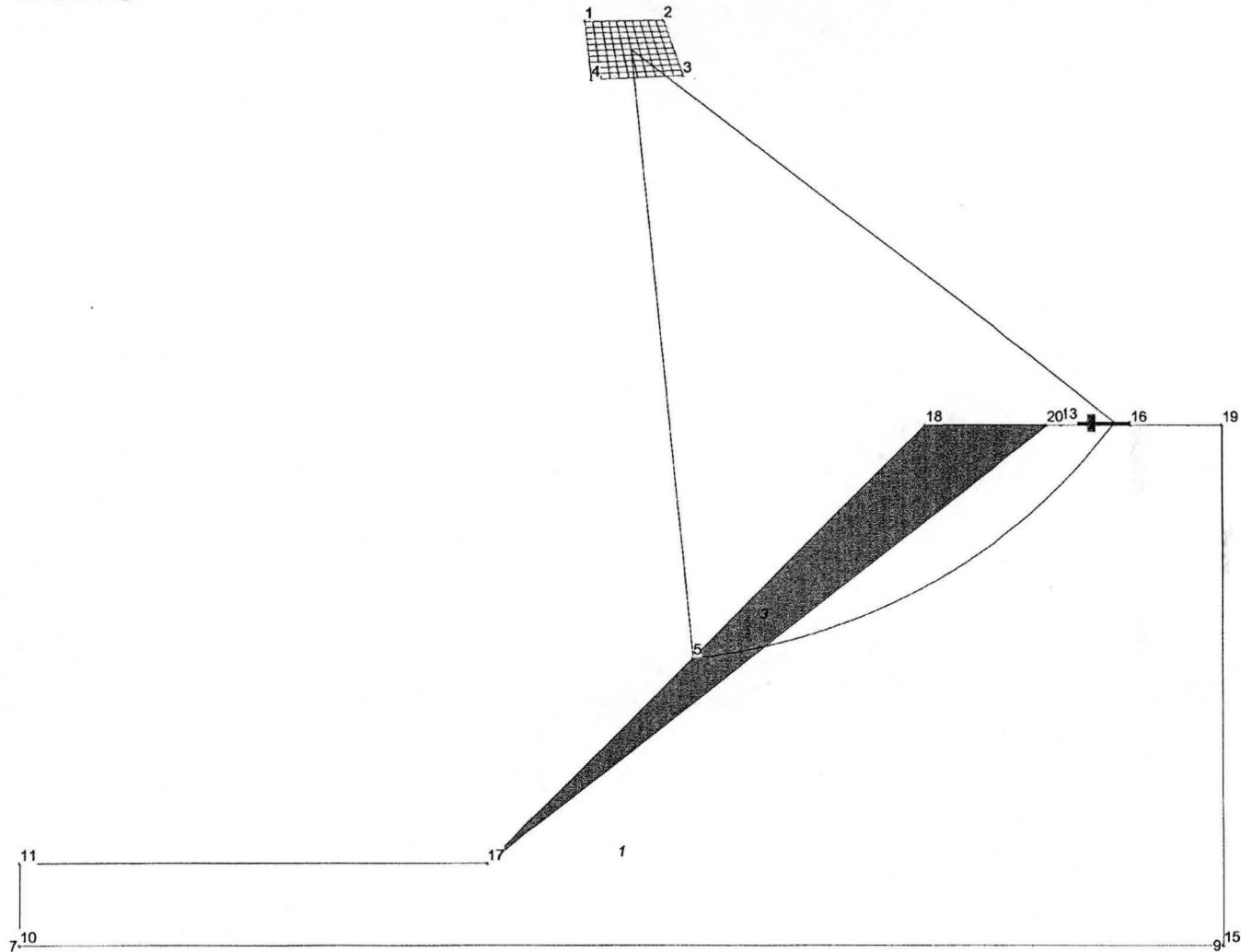
1.215	1.193	1.171	1.156	1.155	1.158	1.163	1.172	1.183	1.201	1.205
1.164	1.147	1.146	1.147	1.151	1.157	1.168	1.183	1.196	1.179	1.162
1.138	1.138	1.140	1.146	1.155	1.168	1.189	1.172	1.155	1.138	1.122
1.133	1.138	1.146	1.158	1.179	1.165	1.148	1.131	1.115	1.100	1.085
1.138	1.149	1.166	1.159	1.142	1.126	1.110	1.094	1.079	1.064	1.051
1.154	1.154	1.138	1.121	1.105	1.089	1.074	1.059	1.046	1.034	1.023
1.133	1.117	1.101	1.085	1.070	1.055	1.042	1.029	1.019	1.010	1.005
1.098	1.082	1.067	1.052	1.039	1.027	1.016	1.007	1.002	1.001	1.006
1.065	1.050	1.037	1.024	1.014	1.005	1.000	1.000	1.006	1.017	1.032
1.036	1.024	1.013	1.005	1.000	1.000	1.008	1.024	1.043	1.073	1.117
1.014	1.006	1.002	1.004	1.014	1.034	1.061	1.101	1.165	1.278	1.517

Critical slip circle

Center: x = 8.587, y = 31.219, Radius: 27.719, F = 1.000

David S. Messinger, P.E., Load Case 6B**General Data**

Licensed User : David S. Messinger, P.E.
 File Name : CWC06B.stb
 Problem Name : Load Case 6B
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 20
 Number of Soil Polygons : 3

Definition Figure**Input Data****Coordinates of Nodes**

Node	x	y
1	20.630	33.790
2	23.479	33.845
3	24.191	31.818
4	20.850	31.653
5	24.500	10.500
6	0.000	0.000
7	0.000	0.000
8	44.000	0.000
9	44.000	0.000
10	0.000	0.000
11	0.000	3.000
12	38.000	19.000
13	38.000	19.100
14	40.625	19.100
15	44.000	0.000

STB2006 - Slope Stability

David S. Messinger, P.E., Load Case 6B

16	40.625	19.000
17	17.000	3.000
18	33.000	19.000
19	44.000	19.000
20	37.479	19.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1:	Nodes	10	11	17	20	12	19	15
2:	Nodes	12	13	14	16			
3:	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko —	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	12.300	38.000	P		0.000
2	12571.430	12571.430	1.000	0.000	0.000	P		0.000
3	0.000	10.000	1.000	0.000	0.000	P		0.000

Output Data

Safety Factors

1.185	1.182	1.177	1.173	1.169	1.164	1.161	1.156	1.151	1.146	1.143
1.149	1.144	1.139	1.134	1.129	1.124	1.119	1.113	1.106	1.101	1.097
1.115	1.108	1.102	1.095	1.090	1.082	1.077	1.071	1.064	1.053	1.042
1.079	1.072	1.065	1.047	1.044	1.041	1.039	1.036	1.033	1.031	1.029
1.039	1.036	1.033	1.035	1.033	1.030	1.027	1.024	1.022	1.020	1.018
1.028	1.025	1.022	1.018	1.016	1.013	1.017	1.016	1.013	1.012	1.011
1.018	1.016	1.013	1.010	1.008	1.006	1.003	1.002	1.001	1.008	1.007
1.011	1.008	1.006	1.004	1.003	1.001	1.002	1.001	1.002	1.002	1.005
1.006	1.004	1.003	1.002	1.002	1.003	1.003	1.005	1.008	1.013	1.017
1.004	1.004	1.005	1.006	1.008	1.011	1.014	1.022	1.029	1.040	1.050
1.006	1.009	1.011	1.015	1.021	1.031	1.038	1.052	1.069	1.090	1.117

Critical slip circle

Center: x = 21.742, y = 32.334, Radius: 22.386, F = 1.001

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 31 of 43

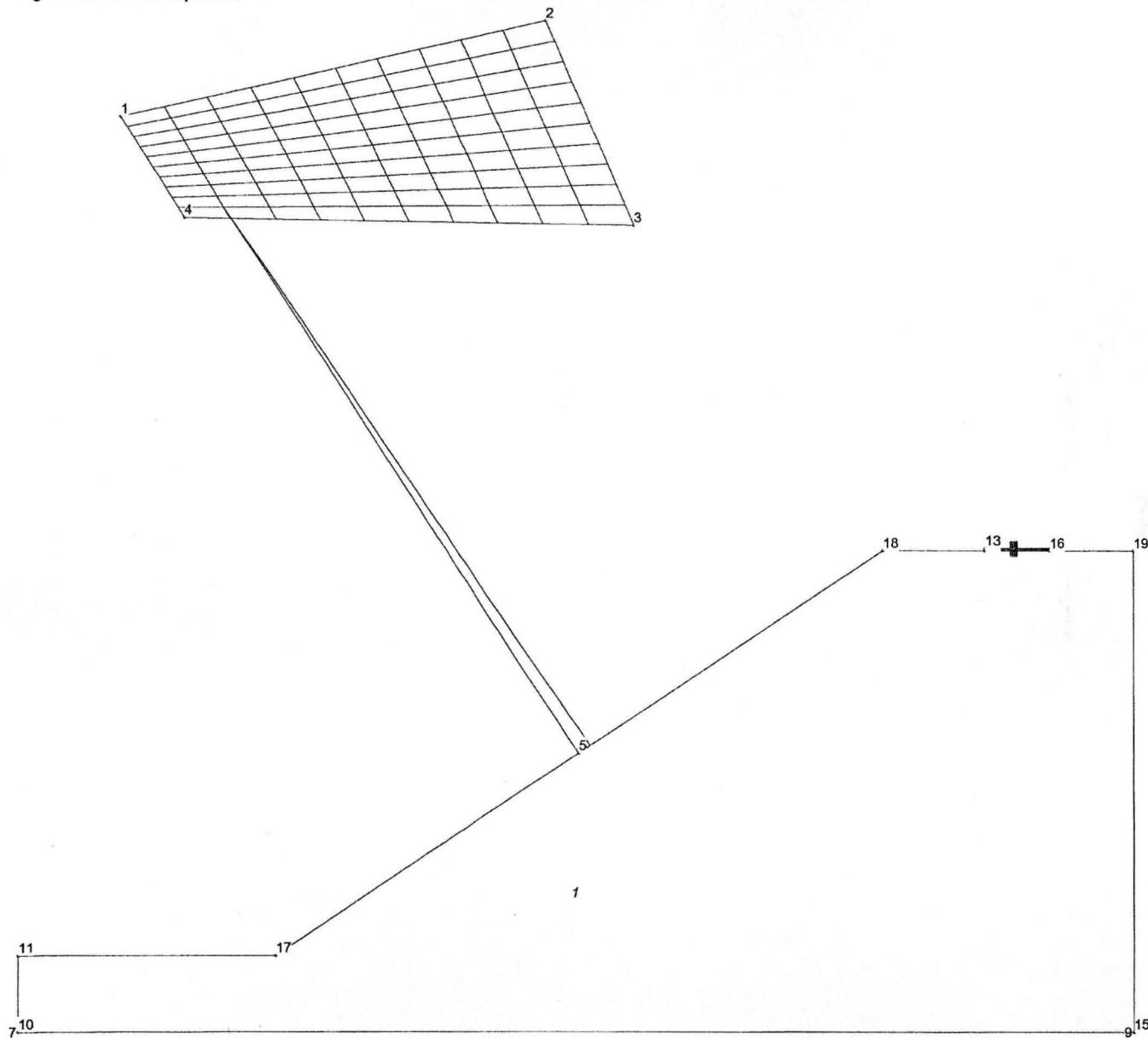
Page 2

David S. Messinger, P.E., Load Case 6C

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC06C.stb
 Problem Name : Load Case 6C
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 19
 Number of Soil Polygons : 2

Figure with critical slip circle



F = 1.172

STB2006 - Slope Stability

David S. Messinger, P.E., Load Case 6C

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 33 of 43

Page 2

Input Data

Coordinates of Nodes

Node	x	y
1	4.013	36.115
2	20.767	39.931
3	24.191	31.818
4	6.469	32.117
5	22.000	11.000
6	0.000	0.000
7	0.000	0.000
8	44.000	0.000
9	44.000	0.000
10	0.000	0.000
11	0.000	3.000
12	38.000	19.000
13	38.000	19.100
14	40.625	19.100
15	44.000	0.000
16	40.625	19.000
17	10.000	3.000
18	34.000	19.000
19	44.000	19.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1: Nodes 10 11 17 18 12 19 15
2: Nodes 12 13 14 16

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko —	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P	0.000	0.000
2	12571.430	12571.430	1.000	0.000	0.000	P	0.000	0.000

Output Data

Safety Factors

1.466	1.463	1.462	1.461	1.461	1.462	1.465	1.465	1.468	1.473	1.478
1.412	1.415	1.416	1.420	1.422	1.427	1.433	1.437	1.452	1.462	1.473
1.519	1.518	1.518	1.518	1.518	1.519	1.520	1.522	1.524	1.526	1.530
1.378	1.376	1.374	1.372	1.371	1.369	1.368	1.367	1.367	1.367	1.368
1.268	1.267	1.265	1.263	1.262	1.261	1.261	1.261	1.261	1.262	1.263
1.213	1.214	1.215	1.216	1.217	1.218	1.220	1.221	1.222	1.224	1.225
1.196	1.196	1.196	1.197	1.197	1.198	1.198	1.199	1.199	1.200	1.201
1.183	1.183	1.183	1.183	1.184	1.184	1.184	1.184	1.184	1.184	1.184
1.175	1.175	1.175	1.175	1.175	1.175	1.175	1.175	1.175	1.175	1.175
1.172	1.172	1.172	1.172	1.172	1.172	1.172	1.172	1.172	1.172	1.172
1.173	1.173	1.173	1.173	1.173	1.173	1.173	1.173	1.173	1.173	1.173

Critical slip circle

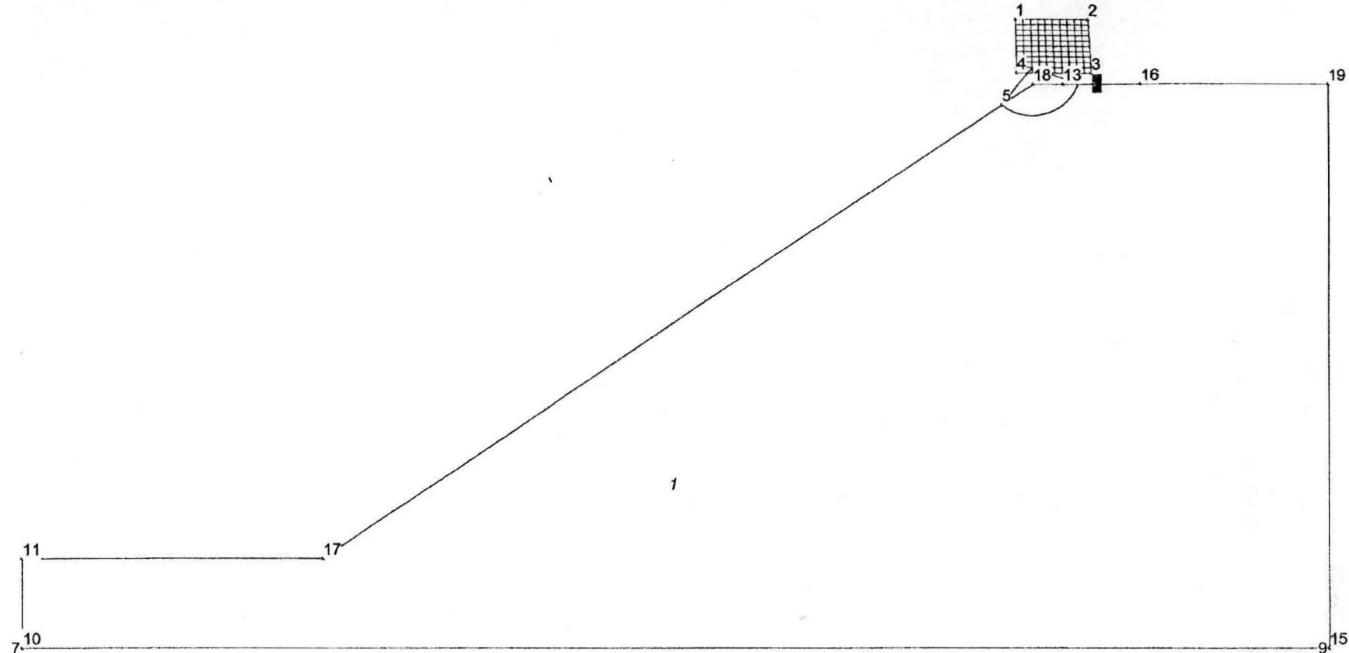
Center: x = 7.986, y = 32.528, Radius: 25.688, F = 1.172

David S. Messinger, P.E., Load Case 6C1

General Data

Licensed User : David S. Messinger, P.E.
 File Name : CWC06C1.stb
 Problem Name : Load Case 6C1
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 19
 Number of Soil Polygons : 2

Figure with critical slip circle



$F = 1.109$

Input Data

Coordinates of Nodes

Node	x	y
1	33.431	21.190
2	35.847	21.190
3	35.976	19.378
4	33.475	19.387
5	32.950	18.300
6	0.000	0.000
7	0.000	0.000
8	44.000	0.000
9	44.000	0.000
10	0.000	0.000
11	0.000	3.000
12	35.000	19.000
13	35.000	19.010
14	37.625	19.010
15	44.000	0.000
16	37.625	19.000
17	10.000	3.000
18	34.000	19.000
19	44.000	19.000

Structure of Polygons

Window of centers of slip circles
 Nodes 1 2 3 4
 Fixed point of all slip circles
 Node 5
 Water Polygon
 Nodes 6 7 8 9
 Soil Polygons
 1: Nodes 10 11 17 18 12 19 15
 2: Nodes 12 13 14 16

STB2006 - Slope Stability

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 35 of 43

Page 2

David S. Messinger, P.E., Load Case 6C1

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko --	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	11.500	38.000	P	0.000	0.000
2	125714.300	125714.300	1.000	0.000	0.000	P	0.000	0.000

Output Data

Safety Factors

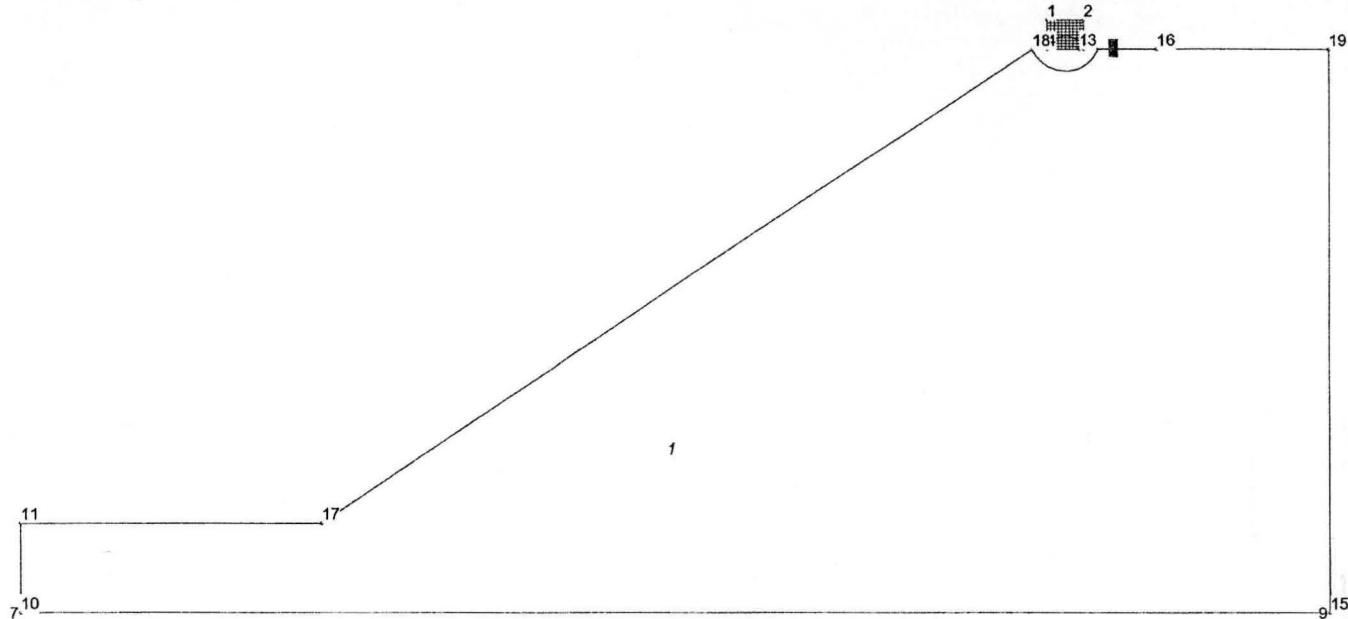
8.308	8.339	8.497	8.569	8.729	8.858	9.084	9.367	9.628	10.024	10.512
5.210	5.164	5.149	5.144	5.128	5.149	5.142	5.200	5.254	5.334	5.472
3.631	3.582	3.518	3.480	3.429	3.387	3.376	3.356	3.349	3.367	3.382
2.686	2.627	2.567	2.510	2.454	2.406	2.370	2.334	2.314	2.302	2.306
2.098	2.059	2.022	1.989	1.960	1.932	1.915	1.909	1.909	1.921	1.948
1.923	1.883	1.837	1.797	1.758	1.723	1.694	1.670	1.662	1.661	1.675
1.764	1.714	1.664	1.619	1.573	1.530	1.493	1.460	1.434	1.426	1.427
1.621	1.567	1.513	1.460	1.408	1.358	1.317	1.276	1.242	1.220	1.222
1.492	1.436	1.380	1.331	1.278	1.227	1.179	1.138	1.117	1.109	1.137
1.393	1.342	1.292	1.245	1.202	1.170	1.164	1.188	1.295	1.674	3.995
1.346	1.318	1.306	1.326	1.427	1.866	3.691	3.602	3.522	3.458	3.421

Critical slip circle

Center: x = 33.969, y = 19.566, Radius: 1.625, F = 1.109

David S. Messinger, P.E., Load Case 6C2**General Data**

Licensed User : David S. Messinger, P.E.
 File Name : CWC06C2.stb
 Problem Name : Load Case 6C2
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 19
 Number of Soil Polygons : 2

Definition Figure**Input Data****Coordinates of Nodes**

Node	x	y
1	34.500	20.000
2	35.760	20.000
3	35.761	19.000
4	34.500	19.000
5	34.000	19.000
6	0.000	0.000
7	0.000	0.000
8	44.000	0.000
9	44.000	0.000
10	0.000	0.000
11	0.000	3.000
12	35.600	19.000
13	35.600	19.010
14	38.225	19.010
15	44.000	0.000
16	38.225	19.000
17	10.000	3.000
18	34.000	19.000
19	44.000	19.000

Structure of Polygons

Window of centers of slip circles
 Nodes 1 2 3 4
 Fixed point of all slip circles
 Node 5
 Water Polygon
 Nodes 6 7 8 9
 Soil Polygons
 1: Nodes 10 11 17 18 12 19 15
 2: Nodes 12 13 14 16

STB2006 - Slope Stability

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 37 of 43

Page 2

David S. Messinger, P.E., Load Case 6C2

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P	0.000	
2	125714.300	125714.300	1.000	0.000	0.000	P	0.000	

Output Data

Safety Factors

2.187	2.155	2.130	2.112	2.102	2.100	2.107	2.123	2.146	2.178	---
2.027	1.991	1.961	1.937	1.922	1.916	1.919	1.932	1.954	1.983	---
1.864	1.826	1.791	1.762	1.742	1.730	1.734	1.743	1.759	1.788	---
1.715	1.666	1.624	1.595	1.569	1.552	1.542	1.554	1.571	1.600	---
1.572	1.519	1.471	1.429	1.402	1.380	1.369	1.374	1.391	1.420	---
1.445	1.383	1.327	1.285	1.244	1.221	1.206	1.205	1.227	1.260	---
1.344	1.274	1.219	1.164	1.126	1.093	1.082	1.084	1.115	1.163	---
1.341	1.287	1.224	1.189	1.164	1.145	1.164	1.222	1.298	1.453	---
---	---	---	---	---	---	---	---	---	---	---
---	---	---	---	---	---	---	---	---	---	---
---	---	---	---	---	---	---	---	---	---	---

Critical slip circle

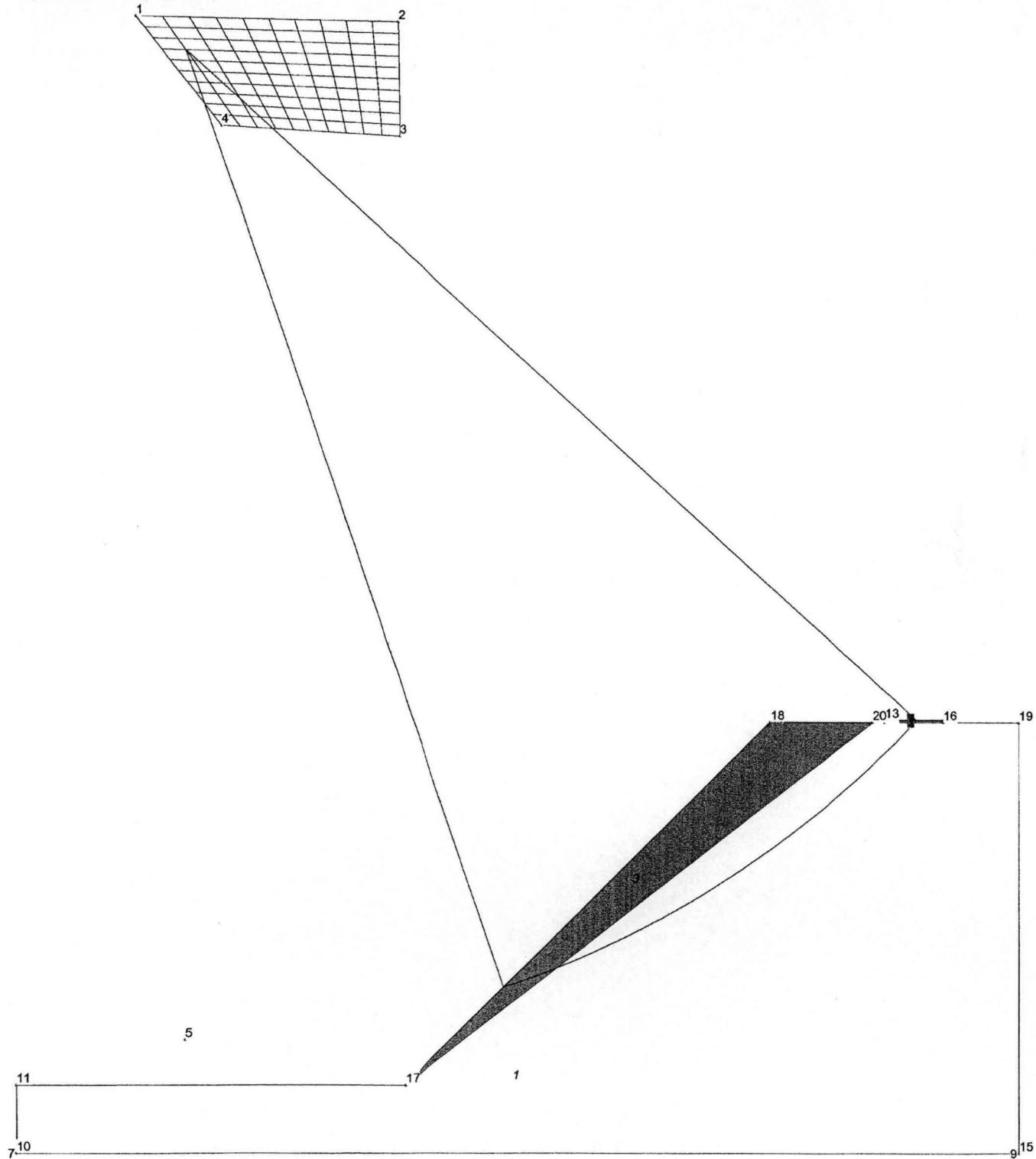
Center: x = 35.004, y = 19.400, Radius: 1.236, F = 1.082

David S. Messinger, P.E., Load Case 6C3

General Data

Licensed User : David S. Messinger, P.E.
File Name : CWC06C3.stb
Problem Name : Load Case 6C3
Number of Slices : 1000
Relative Horizontal Force: 0.000000
Number of Nodes : 20
Number of Soil Polygons : 3

Figure with critical slip circle



Input Data**Coordinates of Nodes**

Node	x	y
1	5.155	50.062
2	16.642	49.844
3	16.717	44.784
4	8.846	45.245
5		5.000
6	0.000	0.000
7	0.000	0.000
8	44.000	0.000
9	44.000	0.000
10	0.000	0.000
11	0.000	3.000
12	38.000	19.000
13	38.000	19.100
14	40.625	19.100
15	44.000	0.000
16	40.625	19.000
17	17.000	3.000
18	33.000	19.000
19	44.000	19.000
20	37.479	19.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Deepest point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1:	Nodes	10	11	17	20	12	19	15
2:	Nodes	12	13	14	16			
3:	Nodes	17	18	20				

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko —	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	11.500	38.000	P	0.000	0.000
2	12571.430	12571.430	1.000	0.000	0.000	P	0.000	0.000
3	0.000	10.000	1.000	0.000	38.000	P	0.000	0.000

Output Data**Safety Factors**

1.784	1.763	1.742	1.722	1.701	1.681	1.660	1.640	1.620	1.599	1.579
1.664	1.648	1.632	1.616	1.600	1.584	1.568	1.552	1.536	1.520	1.504
1.556	1.544	1.532	1.521	1.509	1.497	1.485	1.473	1.461	1.449	1.437
1.458	1.450	1.442	1.434	1.425	1.417	1.407	1.398	1.390	1.381	1.373
1.369	1.364	1.359	1.353	1.348	1.341	1.336	1.331	1.326	1.320	1.314
1.286	1.284	1.281	1.278	1.275	1.272	1.269	1.266	1.263	1.260	1.256
1.203	1.202	1.202	1.202	1.201	1.201	1.200	1.199	1.199	1.198	1.198
1.116	1.120	1.121	1.124	1.126	1.128	1.130	1.132	1.134	1.136	1.138
1.043	1.043	1.043	1.044	1.044	1.044	1.045	1.061	1.067	1.072	1.077
1.038	1.033	1.030	1.026	1.033	1.031	1.030	1.030	1.031	1.030	1.032
1.383	1.289	1.149	1.102	1.072	1.052	1.040	1.043	1.035	1.031	1.028

Critical slip circle

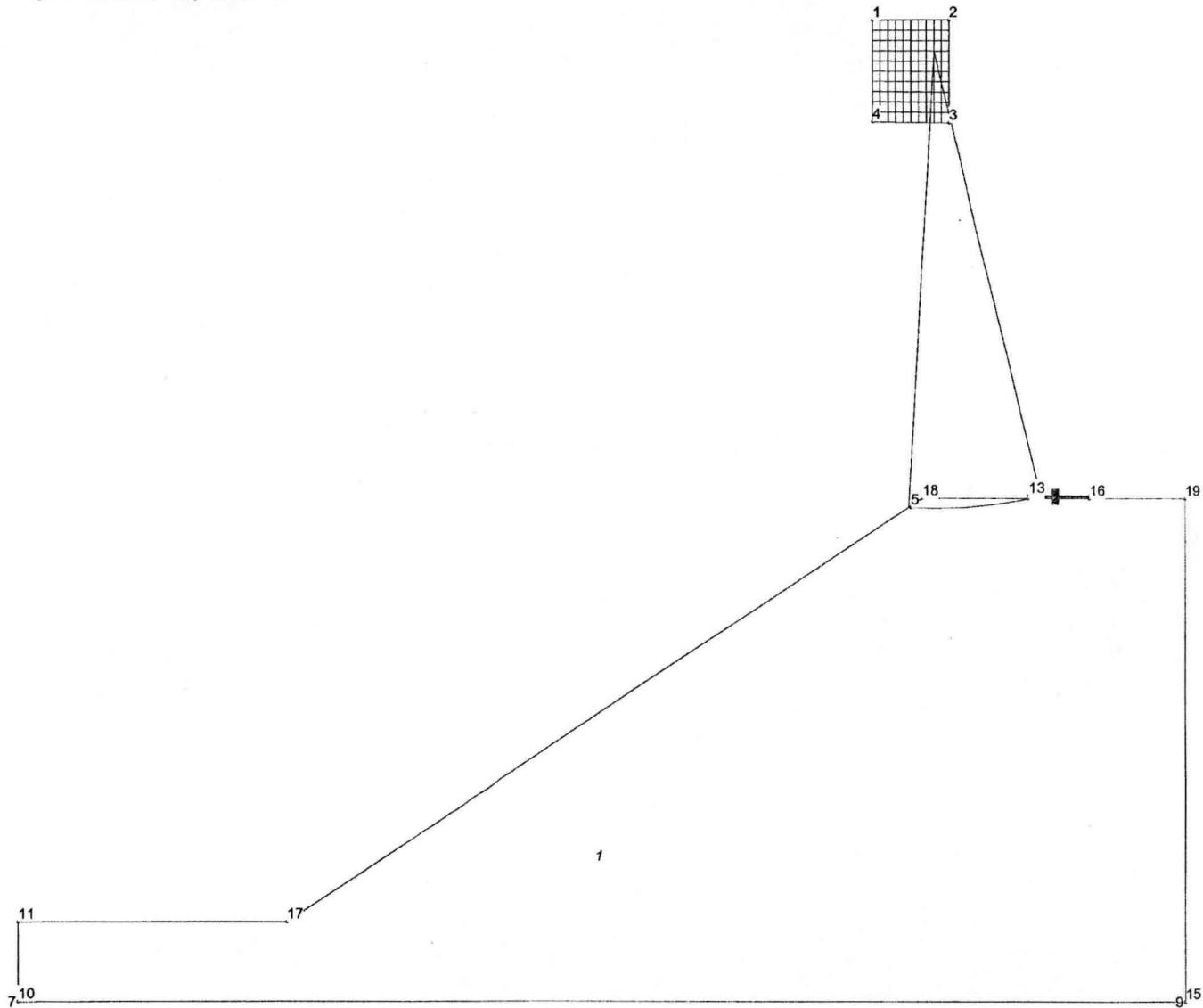
Center: x = 7.303, y = 48.588, Radius: 43.588, F = 1.026

David S. Messinger, P.E., Load Case 6D

General Data

Licensed User : David S. Messinger, P.E.
 File Name : WC06D.stb
 Problem Name : Load Case 6D
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 19
 Number of Soil Polygons : 2

Figure with critical slip circle



$$F = 1.057$$

Input Data

Coordinates of Nodes

Node	x	y
1	32.156	37.086
2	35.042	37.086
3	35.042	33.184
4	32.156	33.237
5	33.572	18.655
6	0.000	0.000

STB2006 - Slope Stability

David S. Messinger, P.E., Load Case 6D

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 41 of 43

Page 2

7	0.000	0.000
8	44.000	0.000
9	44.000	0.000
10	0.000	0.000
11	0.000	3.000
12	38.000	19.000
13	38.000	19.100
14	40.333	19.100
15	44.000	0.000
16	40.333	19.000
17	10.000	3.000
18	34.000	19.000
19	44.000	19.000

Structure of Polygons

Window of centers of slip circles

Nodes 1 2 3 4

Fixed point of all slip circles

Node 5

Water Polygon

Nodes 6 7 8 9

Soil Polygons

1:	Nodes	10	11	17	18	12	19	15
2:	Nodes	12	13	14	16			

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko --	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P	0.000	0.000
2	18214.290	18214.290	1.000	0.000	0.000	P	0.000	0.000

Output Data

Safety Factors

3.460	3.403	3.330	3.273	3.193	3.135	3.054	2.995	2.906	2.846	2.754
2.980	2.917	2.812	2.713	2.633	2.514	2.375	2.279	2.117	1.984	1.784
1.913	1.662	1.403	1.057	1.077	1.301	1.584	1.972	2.561	3.441	5.003
2.636	3.479	4.562	6.598	9.143	11.871	12.315	12.197	12.077	11.956	11.833
11.499	11.386	11.272	11.156	11.039	10.920	10.801	10.679	10.557	10.433	10.307
10.211	10.098	9.983	9.868	9.751	9.633	9.513	9.392	9.270	9.146	9.020
9.106	8.995	8.882	8.769	8.654	8.538	8.421	8.302	8.182	8.060	7.937
8.159	8.051	7.942	7.832	7.721	7.608	7.494	7.379	7.263	7.145	7.026
7.348	7.243	7.138	7.032	6.925	6.817	6.708	6.598	6.486	6.374	6.260
6.650	6.550	6.450	6.349	6.246	6.143	6.039	5.934	5.828	5.721	5.613
6.049	5.954	5.859	5.762	5.665	5.567	5.468	5.369	5.268	5.167	5.064

Critical slip circle

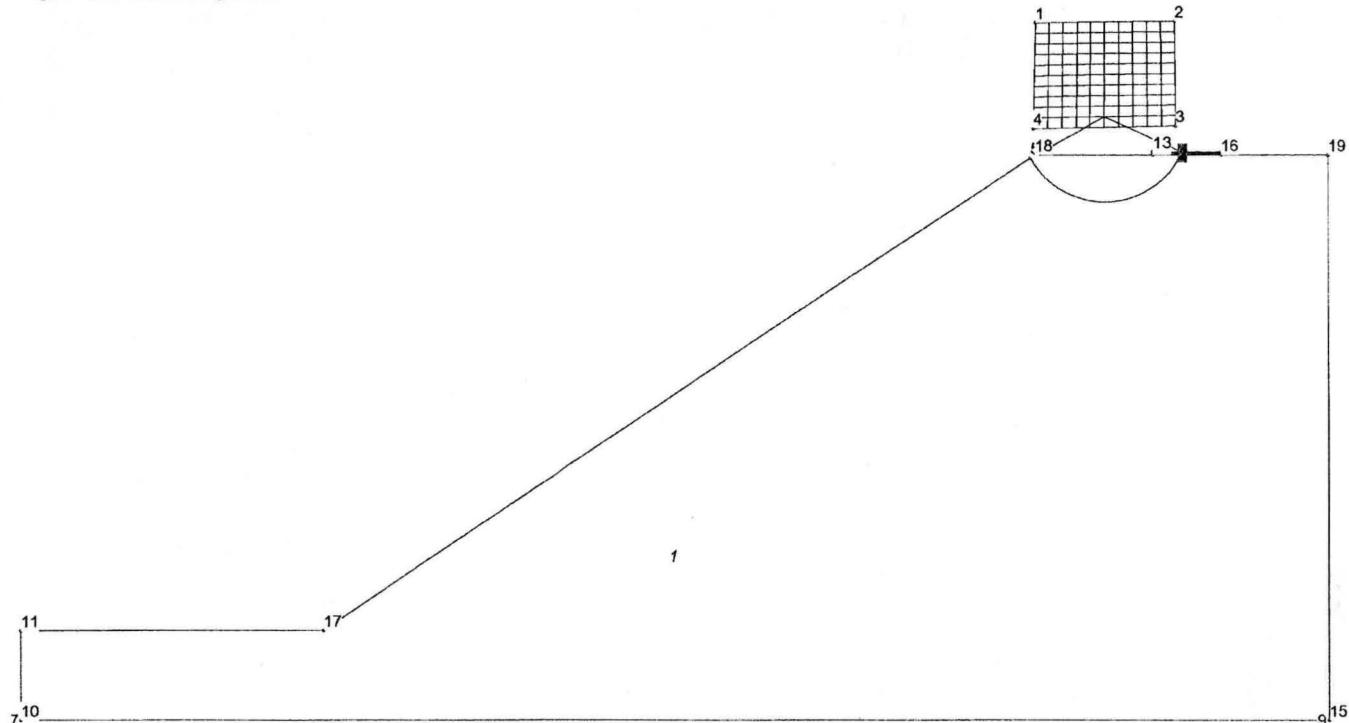
Center: x = 34.465, y = 35.918, Radius: 17.286, F = 1.057

David S. Messinger, P.E., Load Case 6D1

General Data

Licensed User : David S. Messinger, P.E.
 File Name : WC06D1.stb
 Problem Name : Load Case 6D1
 Number of Slices : 1000
 Relative Horizontal Force: 0.000000
 Number of Nodes : 19
 Number of Soil Polygons : 2

Figure with critical slip circle



$$F = 1.277$$

Input Data

Coordinates of Nodes

Node	x	y
1	34.035	23.471
2	38.737	23.514
3	38.780	19.977
4	33.949	19.891
5	33.850	18.900
6	0.000	0.000
7	0.000	0.000
8	44.000	0.000
9	44.000	0.000
10	0.000	0.000
11	0.000	3.000
12	38.000	19.000
13	38.000	19.100
14	40.333	19.100
15	44.000	0.000
16	40.333	19.000
17	10.000	3.000
18	34.000	19.000
19	44.000	19.000

Structure of Polygons

Window of centers of slip circles
 Nodes 1 2 3 4
 Fixed point of all slip circles
 Node 5

STB2006 - Slope Stability

APPENDIX A
CALC. NO. 122633-C-004

Appendix Page 43 of 43

Page 2

David S. Messinger, P.E., Load Case 6D1

Water Polygon

Nodes	6	7	8	9				
1 :	Nodes	10	11	17	18	12	19	15
2 :	Nodes	12	13	14	16			

Properties of Soils

Soil	Wd kN/m ³	Ws kN/m ³	Ko —	c kN/m ²	phi degrees	P/F	p=0 m	cap m
1	110.000	110.000	1.000	0.000	38.000	P	0.000	
2	18214.290	18214.290	1.000	0.000	0.000	P	0.000	

Output Data

Safety Factors

17.202	17.277	17.175	17.125	17.135	17.220	17.392	17.670	18.072	18.624	19.564
7.253	7.111	6.984	6.876	6.750	6.693	6.642	6.658	6.720	6.839	6.976
4.202	4.066	3.954	3.829	3.739	3.643	3.586	3.537	3.507	3.529	3.574
2.758	2.647	2.536	2.426	2.331	2.249	2.175	2.116	2.069	2.052	2.058
2.017	1.921	1.836	1.747	1.670	1.598	1.530	1.481	1.447	1.435	1.451
1.848	1.759	1.670	1.586	1.504	1.436	1.369	1.314	1.286	1.277	1.317
1.629	1.615	1.587	1.611	1.681	1.817	2.110	2.707	4.155	11.519	411.090
100.969	103.688	106.927	110.829	115.633	121.679	129.501	140.017	154.783	176.775	211.777
53.135	53.684	54.396	55.319	56.529	58.137	60.322	63.392	67.915	75.041	87.389
25.479	25.091	24.717	24.362	24.036	23.754	23.542	23.445	23.547	24.038	25.406
11.851	11.302	10.736	10.153	9.548	8.919	8.260	7.564	6.822	6.019	5.128

Critical slip circle

Center: x = 36.367, y = 20.290, Radius: 2.875, F = 1.277

Attachment B
Calculation 300013-C-001

Calculation Cover Sheet and Revision Summary

Section 1: Identification							
1. Project Number 300013	2. Modification Description Title/Subject Crane Stability Analysis			3. Page i of ii			
4. Use of Form							
<input checked="" type="checkbox"/> Calculation <input type="checkbox"/> Engineering Analysis <input type="checkbox"/> Software Installation <input type="checkbox"/> Technical Basis <input type="checkbox"/> Other							
5. Job Title Crane Stability Analysis		6. WBS Number NA	7. Department/Organization CWC				
8. Calculation Number 300013-C-001		9. Affected Building Numbers NA	10. Room		11. Floor		
12. Independent Verification Required?		13. Performance Category (PC)					
<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No		<input type="checkbox"/> 0 <input type="checkbox"/> 1 <input type="checkbox"/> 2 <input type="checkbox"/> 3 <input checked="" type="checkbox"/> N/A					
Section 2: Preparation, Review, and Approval							
14. Rev. No.	15.	16. Originator	17. Checker	18. Independent Verifier	19. Approver: Design Authority or System Engineer	20. Supersedes Calc. No. or Rev. No.	21. Field Confirmation Required?
0	Print Name Sign Date	David S. Messinger <i>DS Messinger</i>	D.S. McShane <i>D.S. McShane</i>				<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No
	Print Name Sign Date						<input type="checkbox"/> Yes <input type="checkbox"/> No
	Print Name Sign Date						<input type="checkbox"/> Yes <input type="checkbox"/> No
22. Rev. No.	23. Description/Reason for Revision					24. Affected Pages	
Registration Stamp (as applicable): Signature/Date: _____				Classification Review: Signature/Date: _____			

Calculation/Technical Basis Sheet

Section 1: Identification				
1. Project Number 300013	2. Modification Description Title/Subject Crane Stability Analysis	3. Page ii of ii		
4. Calculation Number 300013-C-001		5. Rev. No. 0		
Section 2: Scope, Objective, Method, Assumptions, Technical Basis				
6. Scope and Objective This calculation was performed to support safe crane operations connected with drum retrieval in trench's at the Central Waste Facility in the 200 West Area. There are two cranes considered in this analysis. The first is a Manitowoc Model AT1100G12.4T0287 and the second is a Link-Belt Model LS-518.				
7. Method Hand calculation using Mathcad 14.0 MO11 The approach recommended by Bowles in Ref. 2, Section 4-9 will be followed.				
8. Assumptions and Technical Basis <ol style="list-style-type: none"> 1. The soil weight is assumed to be about 110pcf. This is a reasonable for the area of interest. 2. The angle of internal friction for the soil is 38°. (Ref. 1) 3. The dry soil is cohesion less but has an apparent cohesion of between 11psf and 14psf when freshly excavated allowing the newly excavated slope to stand at 1H:1V. 4. The Link Belt Crane tracks are 44 inches wide, and exert a maximum ground loading of 20.9 psi. 5. Determine the margin of safety for soil failure for the maximum crane loads. 				
Section 3: Design Inputs/References				
9. Ref. No.	10. Inputs/References (with Revision and/or Date or Source)			
1	Calculation No. 122633-C-004			
2	Foundation Analysis and Design, 5th Edition, Bowles, McGraw Hill			
Section 4: Conclusions				
<ol style="list-style-type: none"> 1. The Manitowoc crane was found to have a margin of safety of 12% for the maximum crane load and 15% for the maximum expected service load at the trench. 2. The Link-Belt crane was found to have a margin of safety of 52% with regard to maximum service load at the trench. No maximum crane loading was provided. 				

Checkee's Note: THE crane, MODEL AT1100G12.4T0287 IS
 NOT A MANITOWOC CRANE. THIS CRANE IS A ~~GROVE~~^{DEM} CRANE
 MANUFACTURED BY GROVE CRANES, WHICH IS A
 SUBSIDIARY OF MANITOWOC CRANES. DEM 12-30-08

Fluor Hanford, Inc.

ENGINEERING
ANALYSISCalc. No.:300013-C-001
Revision: 0
Page No.: 1 of 3

Client: CHPRC

Subject: Crane Stability Analysis for
Retrieval Work At CWC Trench
Location: CWC 200W

T.O./Job No.: 300013

Originated by: D.S. Messinger, P.E.

Date: 12/29/08

Checked by: David S. McShane, P.E.

Date: 12/30/08

OBJECTIVE:

This calculation was performed to support safe crane operations connected with drum retrieval in trench's at the Central Waste Facility in the 200 West Area. There are two cranes considered in this analysis. The first is a Manitowoc Model AT1100G12.4T0287 and the second is a Link-Belt Model LS-518.

REFERENCE:

1. Calculation No. 122633-C-004
2. Foundation Analysis and Design, 5th Edition, Bowles, McGraw Hill

METHOD:

Hand calculation using Mathcad 14.0 MO11

The approach recommended by Bowels in Ref. 2, Section 4-9 will be followed.

ASSUMPTIONS AND LIMITATIONS:

1. The soil weight is assumed to be about 110pcf. This is a reasonable for the area of interest.
2. The angle of internal friction for the soil is 38°. (Ref. 1)
3. The dry soil is cohesion less but has an apparent cohesion of between 11psf and 14psf when freshly excavated allowing the newly excavated slope to stand at 1H:1V.
4. The Link Belt Crane tracks are 44 inches wide, and exert a maximum ground loading of 20.9 psi.
5. Determine the margin of safety for soil failure for the maximum crane loads.

CONCLUSION:

1. The Manitowoc crane was found to have a margin of safety of 12% for the maximum crane load and 15% for the maximum expected service load at the trench.
2. The Link-Belt crane was found to have a margin of safety of 52% with regard to maximum service load at the trench. No maximum crane loading was provided.

CALCULATIONS:**COULOMB EARTH PRESSURES COEFFICIENTS:**

(Passive Pressure)

$$K_p(\phi, \alpha, \beta, \delta) := \frac{\sin(\alpha - \phi)^2}{\sin(\alpha)^2 \cdot \sin(\alpha + \delta) \left(1 - \sqrt{\frac{\sin(\phi + \delta) \sin(\phi + \beta)}{\sin(\alpha + \delta) \sin(\alpha + \beta)}} \right)^2}$$

$K_p(38\text{deg}, 90\text{deg}, 10\text{deg}, 20\text{deg}) = 21.636$ Values check against table 11-1 & 11-2 of Bowles. (Ref. 2)

Fluor Hanford, Inc.

ENGINEERING
ANALYSISCalc. No.:300013-C-001
Revision: 0
Page No.: 2 of 3Client: CHPRC
Subject: Crane Stability Analysis for
Retrieval Work At CWC Trench
Location: CWC 200W

T.O./Job No.: 300013

Originated by: D.S. Messinger, P.E. D.S. Messinger Date: 12/29/08Checked by: David S. McShane, P.E. David McShane Date: 12/30/08Trench with 1-1/2:1 slope:

$$\phi := 38\text{deg}$$

$$K_p(38\text{deg}, 90\text{deg}, 33.7\text{deg}, 38\text{deg}) = 22.924 \quad K_{p\max} := K_p(38\text{deg}, 90\text{deg}, 33.7\text{deg}, 38\text{deg})$$

$$K_p(38\text{deg}, 90\text{deg}, -33.7\text{deg}, 38\text{deg}) = 1.772 \quad K_{p\min} := K_p(38\text{deg}, 90\text{deg}, -33.7\text{deg}, 38\text{deg})$$

$$R := \frac{K_{p\min}}{K_{p\max}} = 0.077 \quad \text{From Table 4-4, } N_{\gamma H} := 56.1 \quad b := 4\text{ft} \quad B := 4\text{ft} \quad L := B$$

$$\gamma := 110 \frac{\text{lbf}}{\text{ft}^3} \quad c := 11 \frac{\text{lbf}}{\text{ft}^2} \quad N'_q := 48.93 \quad N'_c := 45.23 \quad (\text{See Ref. 2, Sec. 4-9, Par. 2, subpar. 1-4})$$

Inclination factors:

$$i_c := 1 \quad i_q := 1 \quad i_\gamma := 1 \quad \text{No inclination}$$

Shape factors:

$$s_c := 1 + \frac{N'_q}{N'_c} = 2.082 \quad s_q := 1 + \frac{B}{L} \cdot \sin(\phi) = 1.616 \quad \text{for a square footing at the surface, } s_\gamma := 0.6$$

$$q_{\bar{B}} := 0 \quad \text{since } d = 0B \quad \text{Compute } N'_\gamma := \frac{N_{\gamma H}}{2} + \frac{N_{\gamma H}}{2} \left[R + \frac{b}{2B} \cdot (1 - R) \right] = 43.159$$

$$q_{ult} := c \cdot N'_c \cdot s_c \cdot i_c + q_{\bar{B}} \cdot N'_q \cdot s_q \cdot i_q + \frac{1}{2} \gamma \cdot B \cdot N'_\gamma \cdot s_\gamma \cdot i_\gamma = 6.733 \frac{\text{kip}}{\text{ft}^2}$$

$$q_{ult} \cdot B^2 = 107.724 \text{ kip}$$

The maximum crane jack load, $Jack_{max} := 94.820 \text{ kip}$ $Load_{max} := 91.2 \text{ kip}$

The margin of safety for the crane with 4'x4' jack pad set back 4' from the top of slope is:

$$1 - \frac{Jack_{max}}{(q_{ult} \cdot B^2)} = 11.979\% \quad \text{Therefore for the assumed conditions the crane would reach its maximum capacity before the soil under the jack pad would fail.}$$

$$1 - \frac{Load_{max}}{(q_{ult} \cdot B^2)} = 15.339\% \quad \frac{q_{ult} \cdot B^2}{Load_{max}} = 1.181$$

Fluor Hanford, Inc.

ENGINEERING
ANALYSIS

Calc. No.:300013-C-001

Revision: 0

Page No.: 3 of 3

Client: CHPRC

Subject: Crane Stability Analysis for
Retrieval Work At CWC Trench
Location: CWC 200W

T.O./Job No.: 300013

Originated by: D.S. Messinger, P.E.

Date: 12/29/08Checked by: David S. McShane, P.E. David McShane Date: 12/30/08

Considering the Link-Belt LS-518 tracked crane, only the width and length of track differ from the above condition.

$$B := 44\text{in} \quad L := 15\text{ft} \quad \text{effective length @ 20.9 psi} \quad s_q := 1 + \frac{B}{L} \cdot \sin(\phi) = 1.15$$

Maximum track pressure, $\text{track}_{\max} := 20.9\text{psi}$

$$q_{ult} := c \cdot N_c \cdot s_c \cdot i_c + q_{bar} \cdot N_q \cdot s_q \cdot i_q + \frac{1}{2} \gamma \cdot B \cdot N_g \cdot s_g \cdot i_g = 2.013 \times 10^5 \text{lb} \cdot \text{s}^{-2} \cdot \text{ft}^{-1}$$

$$q_{ult} \cdot B^2 = 2.707 \times 10^6 \text{lb} \cdot \text{s}^{-2} \cdot \text{ft}$$

Assume the margin of safety for the tracked crane is $1 - \frac{\text{track}_{\max}}{q_{ult}} = 51.908\%$

or the factor of safety is $\frac{q_{ult}}{\text{track}_{\max}} = 2.079$

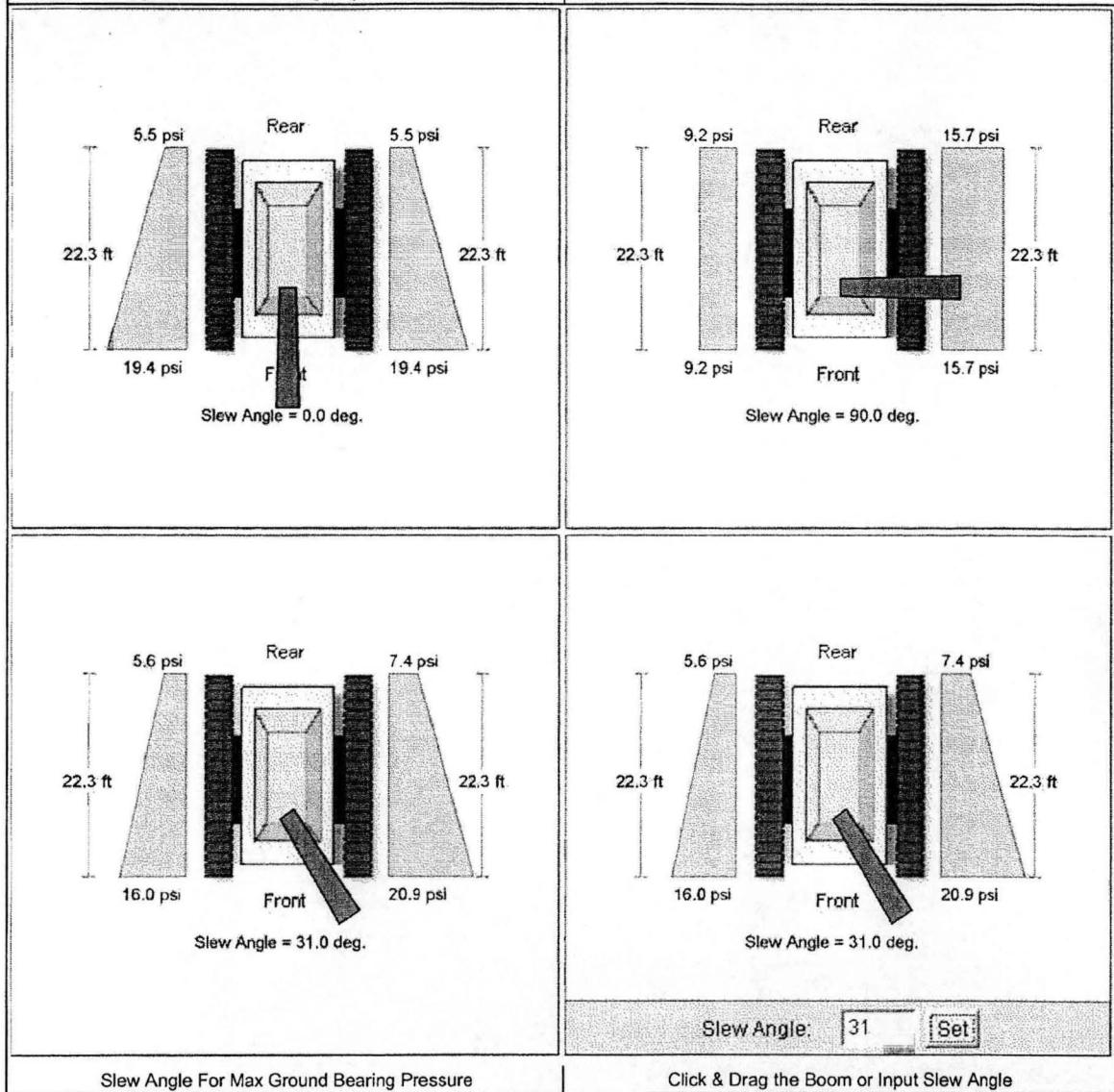
Appendix Follows

Link-Belt Construction Equipment Co., Lexington, Kentucky - LS-518

Model LS-518 - std 44 inch shoes - incl wt of shoes on ground

AB (90000#) ctwt - open throat tube boom (62 x 70)

30000 lbs load @ 55 ft radius, pick from Boom	140 ft main boom
68.9° boom angle	
293,358 lbs gross vehicle weight (GVW)	Date: 12/9/2008- v 1.0



CAUTION: DO NOT USE FOR CAPACITIES!

User to consult the crane rating manual or capacity plate supplied with the machine for input values used in these ground bearing reaction calculations. Do not exceed allowable lifting capacities for configured machine setup.

<- Go Back

Print this page

Messinger, David S

From: McShane, David S
Sent: Thursday, December 11, 2008 1:02 PM
To: Messinger, David S
Subject: FW: Crane Pad Loads

Attachments: at1100g12.4to287 (sm).pdf



at1100g12.4to
7 (sm).pdf (4 M)

-----Original Message-----

From: Yanochko, Ronald M (Ron)
Sent: Wednesday, September 24, 2008 12:20 PM
To: McShane, David S
Subject: FW: Crane Pad Loads

For your use.

-----Original Message-----

From: Myers, Douglas W [mailto:Douglas.Myers@manitowoc.com]
Sent: Wednesday, September 24, 2008 12:13 PM
To: Yanochko, Ronald M (Ron)
Subject: FW: Crane Pad Loads

Ron,

This took a long time, but below is information on your AT1100 pad load request.

Regards,

Doug Myers
Product Specialist
Grove
Manitowoc Crane Group
Phone 717-593-5035
Fax 717-593-5104
Email - douglas.myers@manitowoc.com

-----Original Message-----

From: Hollingshead, Neil J
Sent: Wednesday, September 24, 2008 1:49 PM
To: Myers, Douglas W
Subject: Fw: Crane Pad Loads

Not sure if you got this

Neil

Neil Hollingshead
Manitowoc Cranes
Global Product Manager
All Terrain & Rough Terrain Cranes
+44 (0)7894 595484 (mobile)
+44 (0)191 5201013 (office)
neil.hollingshead@manitowoc.com

Message sent by BlackBerry

----- Original Message -----

From: Borsberry, David J
To: Bage, Michael T; Hollingshead, Neil J
Sent: Tue Sep 23 13:47:49 2008
Subject: RE: Crane Pad Loads

CALC 300013-C-001

APPENDIX Page 3 of 10

Mike,

The attached jack load graph for this model shows a max jack point load of 43 tonnes (94820 pounds).

This is with the 27300 pound ballast lifting 25698 pounds at 55 feet radius.

The graphical method is only a guide and is not an exact result.

It will usually err to the high side but is the only info we can officially provide.

For your info a calculation programme devised by Sunderland Engineering is available.

This was never officially issued but it is OK to help verify the graphical results.

The calculation for this crane shows the worst position is over the rear corner.

The rear corner jack point load for the 25662 pounds load at 55 feet radius is 41.31 tonnes (91100 pounds).

The same jack with the 25698 pounds load at 55 feet radius is 41.37 tonnes (91200 pounds).

Dave

From: Bage, Michael T
Sent: 21 September 2008 08:54
To: Hollingshead, Neil J
Cc: Borsberry, David J
Subject: RE: Crane Pad Loads

Neil,

I guess the only person that can help with this is Dave Borsberry.

I copy him to see what can be done.

Thanks and regards, Mike.

From: Hollingshead, Neil J
Sent: 21 September 2008 11:39
To: Bage, Michael T
Subject: FW: Crane Pad Loads

CALC 300013-C-001

APPENDIX Page 4 of 10

Try again as first mail bounced back.

Best regards

Neil

Neil Hollingshead
Manitowoc Cranes
Global Product Manager
All Terrain & Rough Terrain Cranes
+44 (0)7894 595484 (mobile)
+44 (0)191 5201013 (office)
neil.hollingshead@manitowoc.com

From: Hollingshead, Neil J
Sent: 18 September 2008 13:29
To: Bage, Mike
Cc: Myers, Douglas W
Subject: FW: Crane Pad Loads

Mike

Do you know how this will be handled ?

Neil Hollingshead
Manitowoc Cranes
Global Product Manager
All Terrain & Rough Terrain Cranes
+44 (0)7894 595484 (mobile)
+44 (0)191 5201013 (office)
neil.hollingshead@manitowoc.com

From: Myers, Douglas W

Sent: 18 September 2008 13:19
To: Hollingshead, Neil J
Subject: FW: Crane Pad Loads

CALC 300013-C-001

APPENDIX Page 5 of 10

Neil,

I know this is not your area, but who is left that can help with a pad load request for an AT1100. All the guys in the UK are gone. Any idea??

Doug Myers

Product Specialist

Grove

Manitowoc Crane Group

Phone 717-593-5035

Fax 717-593-5104

Email - douglas.myers@manitowoc.com <mailto:douglas.myers@manitowoc.com>

From: Yanochko, Ronald M (Ron) [mailto:Ronald_M_Ron_Yanochko@RL.gov]
Sent: Tuesday, September 16, 2008 6:30 PM
To: Myers, Douglas W
Cc: McShane, David S
Subject: Crane Pad Loads

This is a follow-up to our telephone conversation, earlier this day.

Please provide outrigger pad loads for the following crane information, as discussed.

Crane Description: Grove All-Terrain 110 Ton

Crane Model No.: AT1100 All-terrain hydraulic crane

Serial No.: 850430

Case No. 1 Case No. 2

Counter weight to be used: 27,300 lbs.
27,300 lbs.

CALC 300013-C-001

APPENDIX Page 6 of 10

Outrigger Spans:
25' - 0" x 25' - 0"

25' - 0" x 25' - 0"

Boom Length:
100' - 4"

Boom Angle:
48 degrees

57.3 degrees

Radius:
55' - 0"

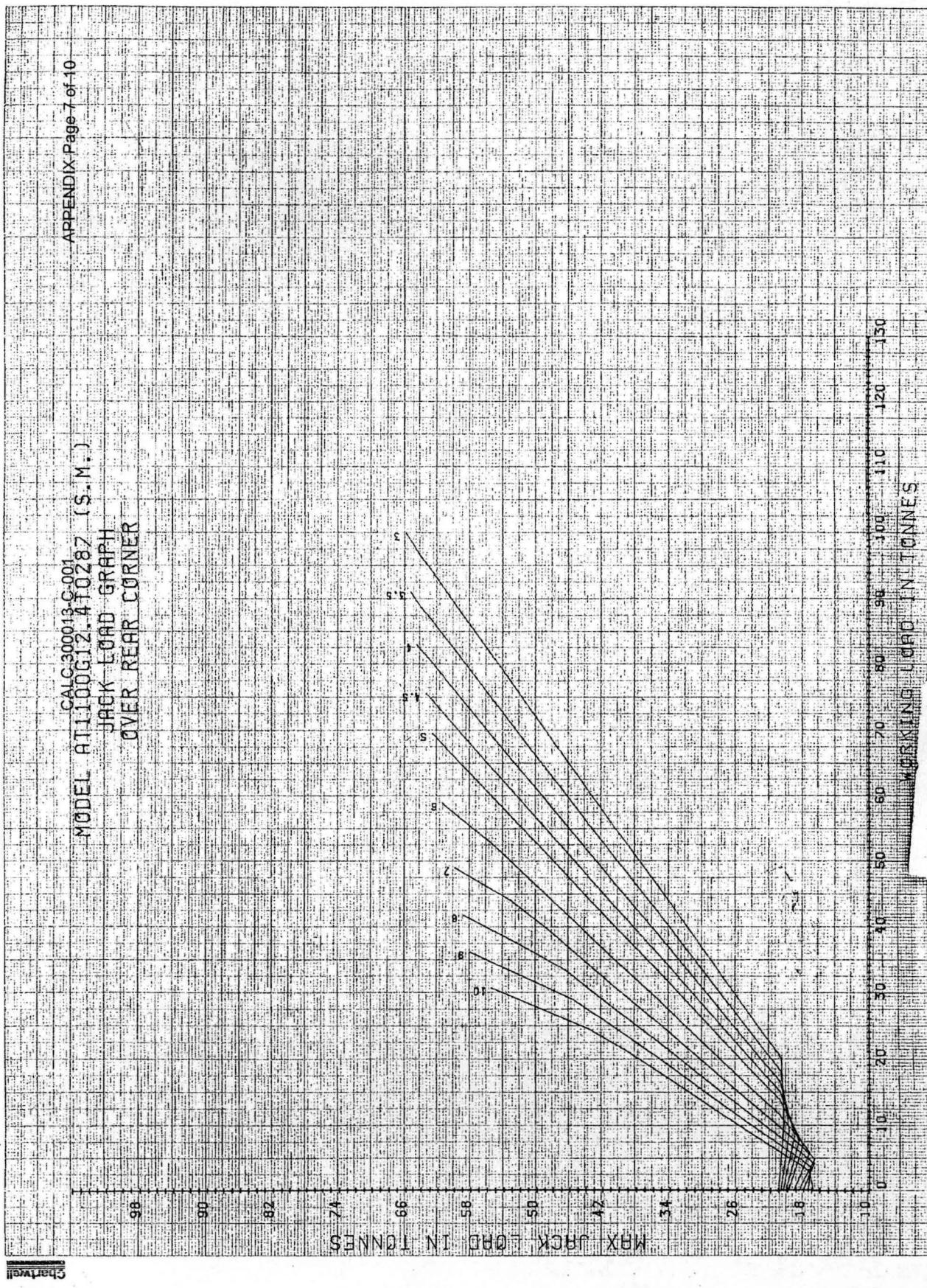
55' - 0"

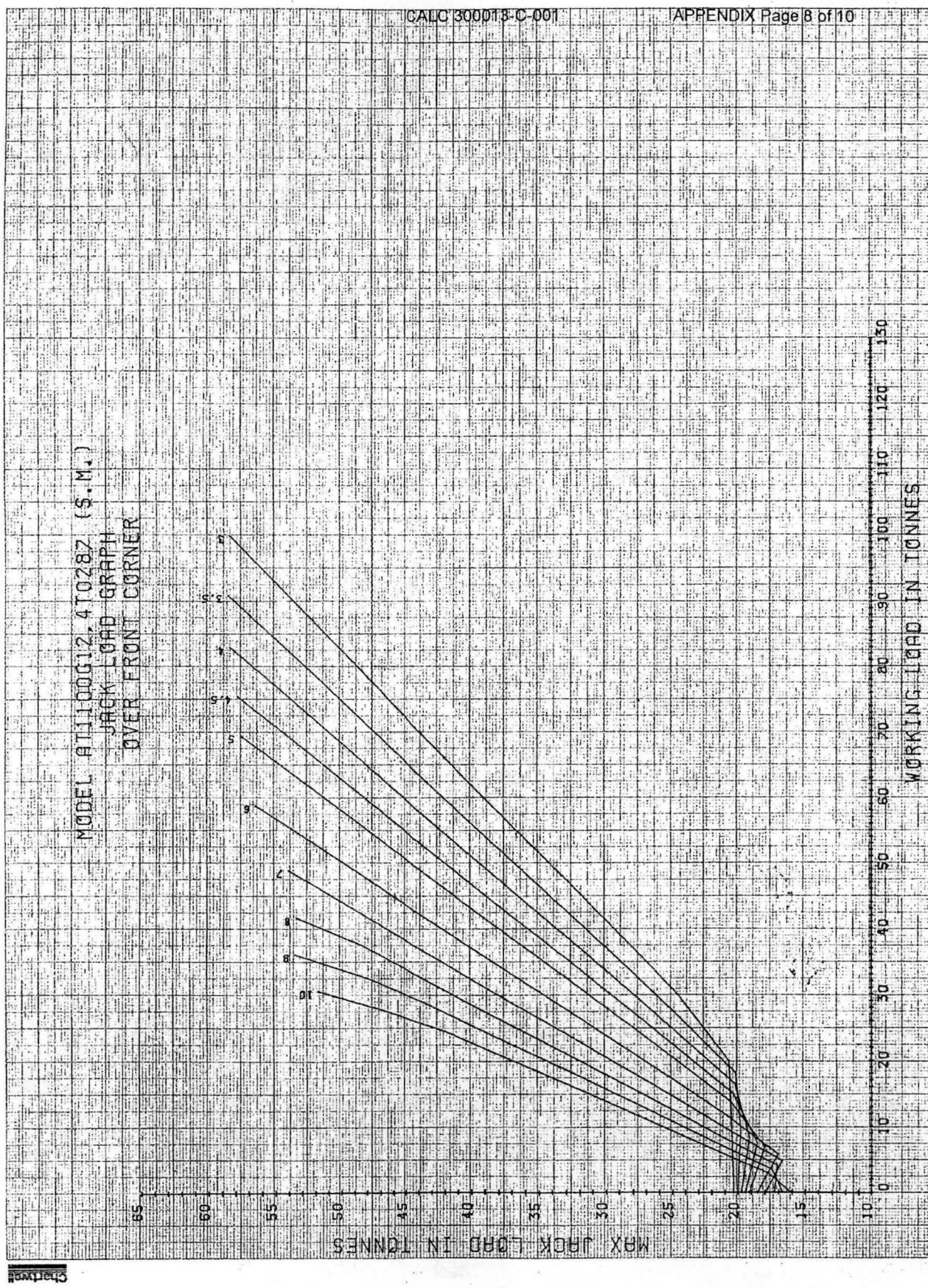
Lifted Load:
25,662.0 lbs.

25, 698 lbs.

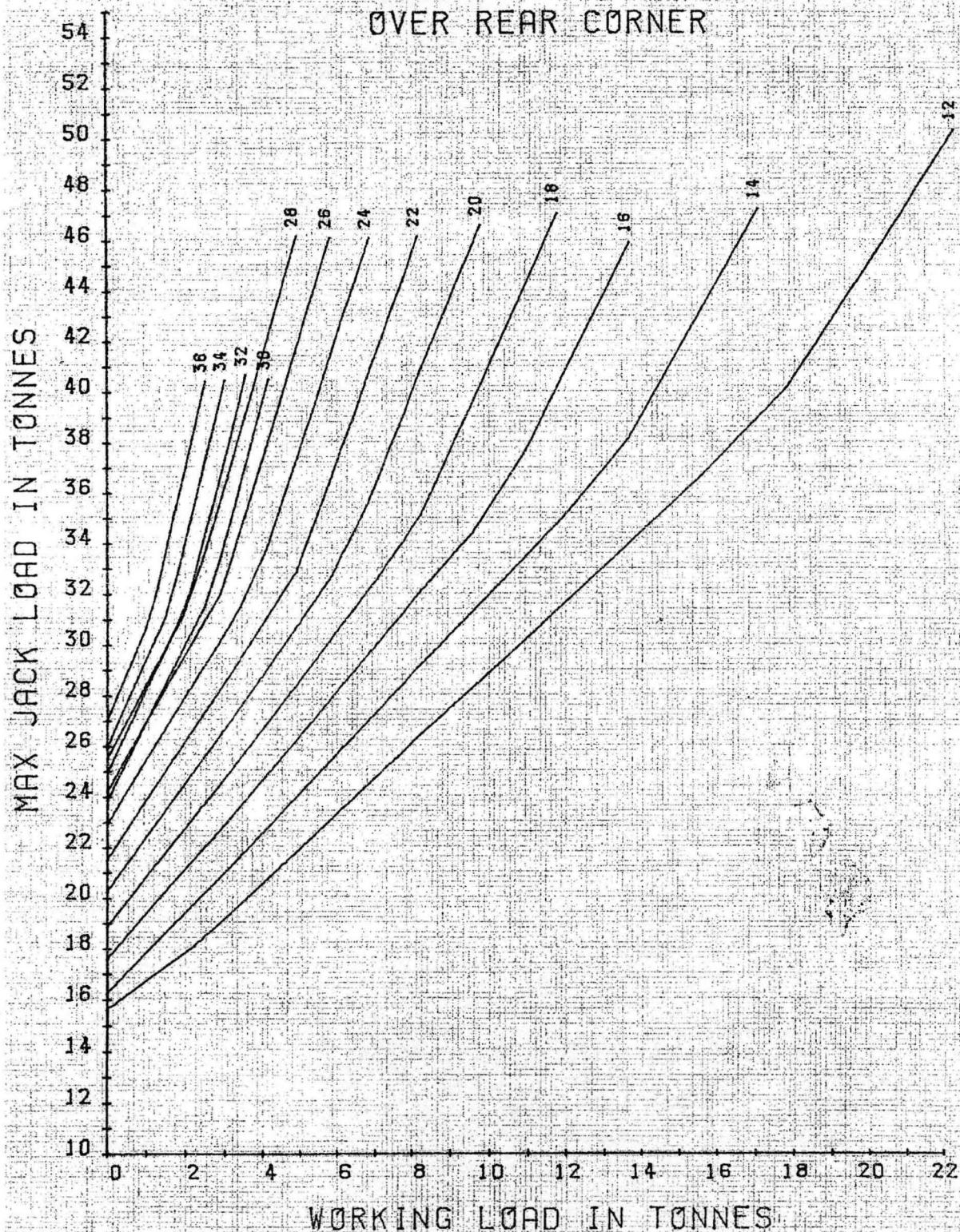
Pad Diameter:
28 inches

28 inches





MODEL AT1100G12.4T0287 (S.M.)
JACK LOAD GRAPH
OVER REAR CORNER



MODEL AT1100G12.4T0287 (S.M.)
JACK LOAD GRAPH
OVER FRONT CORNER

