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TEMP-STRESS – A Thermomechanical Finite Element Program for the Analysis of Plane and Axisymmetric Reinforced/Prestressed Concrete Structures – User's Manual

by James M. Kennedy,
Phillip A. Pfeiffer, and
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Argonne National Laboratory, Argonne, Illinois 60439
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Applied Technology

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Concrete Structures - User's Manual

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James M. Kennedy, Phillip A. Pfeiffer and Algirdas H. Marchertas*

January 1989

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ABSTRACT

TEMP-STRESS has been developed to improve the understanding of the behavior of concrete subjected to mechanical loadings and high temperatures simulating the effects of coolant spills, molten debris, etc. The capability to model concrete structures subjected to static and dynamic overpressures, such as LWR and LMR containments with complex axisymmetric geometries, can be solved. The computer code is a finite element program which has a weakly coupled thermomechanical formulation. It can handle transient and steady state problems through the use of explicit time integration and dynamic relaxation. There is a plane or axisymmetric continuum element and flexural beam and shell elements for concrete discretization. The continuum element is a four node quadrilateral using numerical integration and elastic hourglass control. Variable material properties as a function of temperature are available. Thermal and/or mechanical loading can be handled. The concrete material model has the following characteristics: a) elastic-plastic response, b) variable loading surface capability, c) cracking normal to maximum principal strain at specified failure surface, d) post-failure element treatment, and e) variable temperature dependence. Concrete can be reinforced and/or prestressed.

1. INTRODUCTION AND OVERVIEW

This report provides the user guidance for a two-dimensional nonlinear finite element computer program for transient and steady-state (dynamic relaxation) analysis of plane and axisymmetric configurations. The program has a capability to study the behavior of concrete structures with a weakly coupled thermomechanical formulation. The finite element method employed is valid for large rotations, large strains and material nonlinearities. A corotational coordinate technique is used (each element being associated with a coordinate system which rotates with that element).

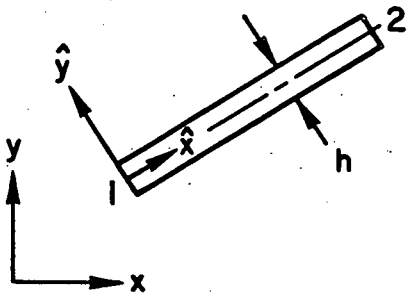
The governing stress equations are solved using an explicit time integration scheme. A conservative stability limitation is imposed on the time-step. Explicit integration is most appropriate for impulsive problems or problems where the loads are of relatively short duration. The explicit scheme is extremely efficient and permits large problems to be solved rapidly. The explicit integration algorithm can also be used in conjunction with dynamic relaxation for steady-state problems. This is advantageous for models where the stiffness matrix is not available or very difficult to program as in the case of the highly nonlinear material behavior of concrete. Dynamic relaxation simulations achieve system equilibrium by iterating until a dual criteria of force and displacement are met. When explicit time integration is performed, lumped masses are used to represent the inertial properties. Slidelines, which are useful for modeling interfaces between materials, such as the liner-concrete interface are an additional feature of explicit time integration. A rigid interface option is included to provide a means of connecting two dissimilar meshes. Energy balance checks and energy partition output can be used to check the fidelity of computations and to assess the distribution of damage and energy absorption in simulations.

An explicit technique is also employed to determine the thermal field in the concrete structure since it is an integral part of the program desired for thermal stress analysis.

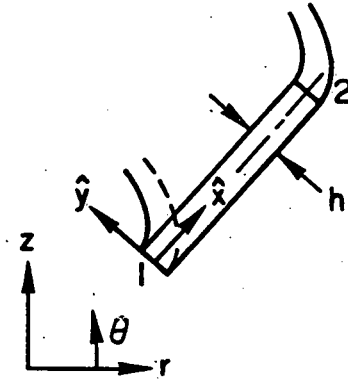
The structural finite elements available in the program, except for the thermal elements, are shown in Fig. 1.1 and listed in Table 1.1. These elements are subdivided into two groups:

1. Flexural elements
2. Continuum elements

FLEXURAL ELEMENTS

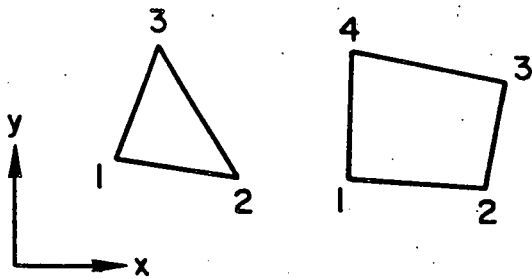


BEAM

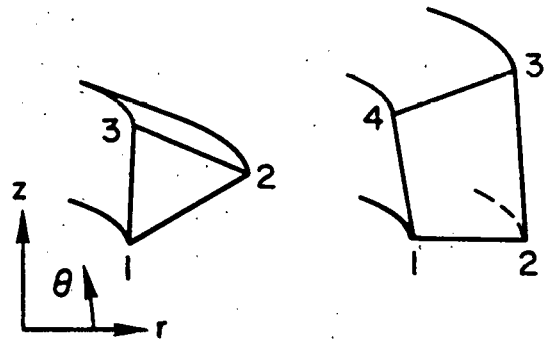


AXISYMMETRIC CONICAL SHELL

CONTINUUM ELEMENTS



PLANE



AXISYMMETRIC

Figure 1.1 Finite Element Library

Table 1.1 Element Library

Flexural Elements

beam with bar option

axisymmetric conical shell with membrane and ring options

Continuum Elements

plane continuum with quadrilateral and triangular* geometry

axisymmetric continuum with quadrilateral and triangular* geometry

* triangular shape is achieved by collapsing node 4 into node 3

Concrete flexural and continuum elements are the primary elements of the TEMP-STRESS program. Concrete can be reinforced and/or prestressed. Flexural and continuum elements that represent other media are available to facilitate modeling of mixed media configurations. The DYNAPCON program, Marchertas (1982), a forerunner of TEMP-STRESS, had provided the mechanical response of reinforced and/or prestressed concrete structures. The basic element features and architecture of the program are those of the STRAW and WHAMS programs, Kennedy (1974), Belytschko and Mullen (1978) and Kennedy et al. (1985).

The beam and axisymmetric conical shell are the flexural elements used for modeling thin-walled structures. A bar option is available in the beam element while membrane and ring options are available in the shell element for modeling simplifications. Plane and axisymmetric solids can be modeled by use of the continuum elements. The continuum elements are basically quadrilateral in cross-section with triangular shapes achieved by collapsing node 4 into node 3. An arbitrary number of Gauss quadrature points from 1 to $n \times n$ can be used in the continuum elements; for one-point quadrature, a consistent hourglass control algorithm is included. In the continuum elements, the coordinate system at each quadrature point is rotated with the material, so arbitrary anisotropic materials can be handled properly when large rotations occur. In the continuum elements, a Jaumann rate is used to maintain proper frame invariance in the material law, but the use of the Jaumann rate limits its application to isotropic materials.

All of these elements (concrete and other media) can be combined in any reasonable manner to model plane two-dimensional problems or axisymmetric three-dimensional problems.

An elastic-plastic law with an associated flow rule is the constitutive relation used for continuum and flexural elements. The non-concrete media employs an isotropic strain hardening material law. The concrete material model includes a mixed isotropic-kinematic strain-hardening formulation, a variable loading surface capability, cracking normal to maximum principal strain at specified failure surface, post-failure treatment, and variable temperature dependence. For the sake of efficiency, options for uniaxial, biaxial or triaxial states of stress or strain within the general subroutine have been developed for use with the flexural and continuum elements. Linear elastic materials can also be handled.

An analysis of the structural integrity of flexural and continuum structures subjected to surface heat fluxes or varying temperature conditions will often involve a thermal stress analysis of the structure. In order to perform either a steady state or transient thermal stress analysis, a thermal analysis must be made for the temperature distribution. Coupled thermomechanical problems can be solved using the flexural and continuum elements. In coupled problems, the thermal response is always transient. However, the stress (mechanical) response can be either transient (dynamic) or quasi-static. A quasi-static response is obtained through the dynamic relaxation technique, and is valid for solutions in which inertial effects are negligible or nonexistent.

The heat conduction (thermal) finite elements are developed to accommodate heat flow through flexural and continuum structural finite elements. To account for thermal stresses arising from temperature variations through the thickness of flexural elements, layered two-dimensional rectangular elements are used. For modeling the temperature variation in the structural continuum elements, quadrilateral thermal elements with a nodal connectivity identical to that of the structure are used. Prescribed temperature, heat flux and film-coefficient (convection) are the thermal boundary conditions available for the thermal field solutions. An internal heat source capability is also available.

Extensive methods are also available for automatically generating mechanical loads. The following load types are available:

- a) initial velocities prescribed on nodes
- b) pressures applied as functions of time or the volume enclosed by a line of nodes
- c) forces applied on nodes

Gravity and special condition (e.g. prestressing) loads can be included if necessary in the simulation.

Displacement boundary conditions which can be treated are quite arbitrary. At any node or any group of nodes, each of the components of displacement in an orthogonal coordinate system may be specified to vanish or have a selected time history.

2. PROGRAM FEATURES

2.1 Elements

2.1.1 Flexural Elements

Two types of flexural elements are included in the program. A plane flexural element which is called a beam element and an axisymmetric flexural element which is used for axisymmetric shell analysis. The cross section of both elements is assumed to be homogeneous and, for the beam, the thickness is assumed to be constant in the entire cross section. The elements are shown in Fig. 1.1. Both elements are initially straight.

In both elements, the in-plane displacements are linear, whereas the transverse displacements are cubic functions of the chord length. The elements are formulated in terms of the usual Bernoulli-Euler hypothesis, so that normals to the midline are constrained to remain straight and normal. In the plane beam element, the strain consequently varies linearly from end to end and linearly across the thickness; in the axisymmetric element the variation is more complex. If the material response is nonlinear, the variation of the stresses through the elements can be quite complex, so that numerical integration is necessary to determine the nodal forces in the element. In the beam element, this integration is accomplished by using the trapezoidal rule through the thickness and along the length, with two points used in the integration along the length. An arbitrary number of integration points through the thickness may be specified by the user. In the axisymmetric element, Gauss integration is used through the thickness with the number of integration points specified by the data; Gauss integration is also used along the chord length (usually two).

The formulation of the flexural elements is based on a modification of the corotational or rigid-convected formulation described by Belytschko and Hsieh (1973). In the present formulation, the strain corresponds to the logarithmic strain in that it is defined by

$$\hat{\epsilon}_x = \ln \left(\frac{l}{l_0} \right) \quad (2.1)$$

where l is the current length of the element and l_0 the original length. The stress used in this element corresponds to σ , which is called the physical or Cauchy stress, and is given by

$$\sigma_x = \frac{P}{A} \quad (2.2)$$

where A is the current cross-sectional area and P is the load. The stress-strain data that is input into the program should correspond to this definition of stress and strain. Experience indicates that the program is reasonably accurate for strains of up to 20% for non-concrete media. The exact limits on the magnitudes of strain that can be handled by this element are not clear at this time. Arbitrarily large rotations can be treated.

In the program, the lumped mass matrix is used in conjunction with explicit integration. The lumped mass is quite accurate for problems involving the large displacement, elastic-plastic response of structures.

A flexural element requires three degrees of freedom at each end point: two translational degrees of freedom and one rotation. The program allows for two or three degrees of freedom per node. In order to accommodate the number of degrees of freedom required by a flexural element when using two degrees of freedom per node, a total of four nodes is required for each element. The first two nodes are used for the translational degrees of freedom at the end points. Rotations at each of the end points are associated with two additional nodes called dummy nodes. For each of the dummy nodes, the second degree of freedom is not used at all. Therefore, in the data for flexural elements, one can conceptually treat them as quadrilaterals with two nodes at each end. If several beam elements are connected in a row, both nodes of adjacent elements must be shared in order to assure compatibility of displacements and rotation across the cross section. The use of the dummy node concept enables pin connections to be arbitrarily inserted in a beam, for if two adjacent elements do not share the same dummy node, rotational compatibility is not enforced across the interface, so that a pinned connection results at that point. The two degrees of freedom per node option also allows intermixing with the continuum elements.

In the data for the coordinates of the node of the flexural element, the first two nodes should correspond to the location of the middle surface of the beam or shell at the ends of the element. These are the only nodes necessary if using three degrees of freedom per node. If using two degrees of freedom per node, the third node (first dummy node) corresponds in location with the second node, while the fourth node corresponds in location

with the first node. No coordinate data is needed for the dummy nodes. Instead, the dummy node coordinates serve as an optional means for the input of the thickness of the beam. This also allows the thickness of the beam to be varied without new material input data. Similarly, any output for the deformed coordinates of the dummy nodes should be interpreted as meaningless.

The element data are given for these elements in Section 3. The stations at which stresses and strains can be output depends on the number of integration points specified; if NPD (through the thickness) integration points are specified, each end of the element has NPD stations for a total of $NPD \times 2$. The order of the stations is illustrated in Fig. 2.1; to locate the bottom station of the beam, note that the \hat{x} axis is positive from node 1 to node 2, and the \hat{y} axis is positive counterclockwise from the \hat{x} axis. The bottom station is that with the lowest \hat{y} value.

A special case that is often useful is the bar option of the beam element, in which the bending stiffness is neglected so that the element only has axial stiffness; these elements are often called bars. This option is achieved by simply specifying one integration point through the thickness with two integration points along the length of the element.

Two special cases of the flexural shell element that are useful in modeling are the membrane and ring options. The membrane option is achieved by specifying one integration point through the thickness and two points along the length. In the membrane option, the flexural stiffness is eliminated so that the element only has resistance to forces tangent to the surface of the shell. Another way of explaining this is to note that in the membrane option, the axial and hoop stresses are constant through the thickness of the shell.

The ring option is obtained by choosing one point integration along the length and through the thickness and making the two nodes coincident by giving them the same node number. As can be seen from Fig. 1.1, when using the ring option, the element becomes a circumferential ring with no bending stiffness.

The flexural beam and shell elements that represent concrete media have uniaxial reinforcement that has been superposed on the parent element. The uniaxial reinforcement will be assumed to be embedded parallel to the respective neutral axis/surface of the element. The cross-sectional area, the normal distance of the reinforcement from the neutral axis/surface of the element and the stress-strain data of the reinforcement material are the

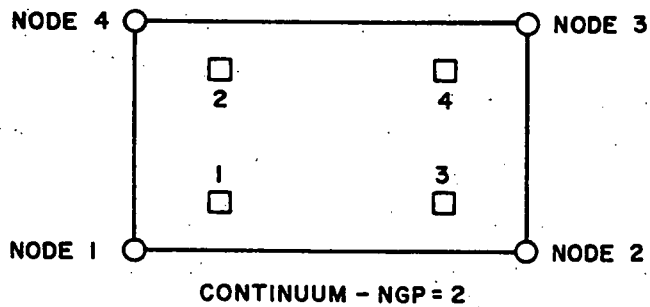
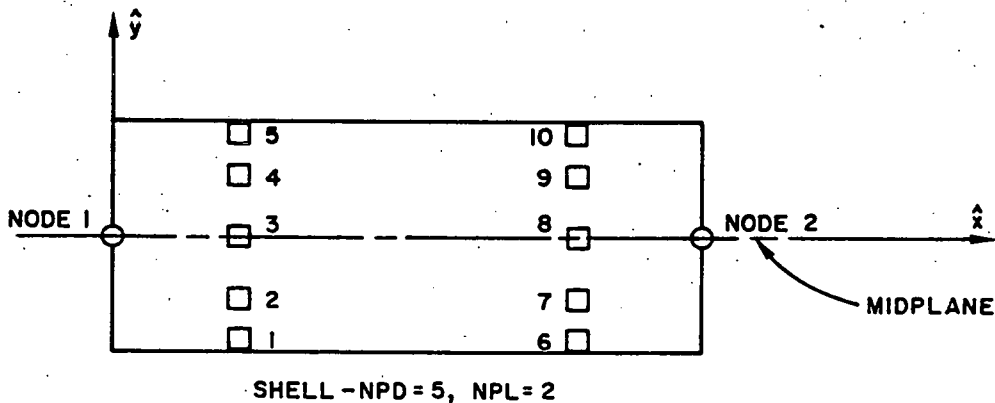
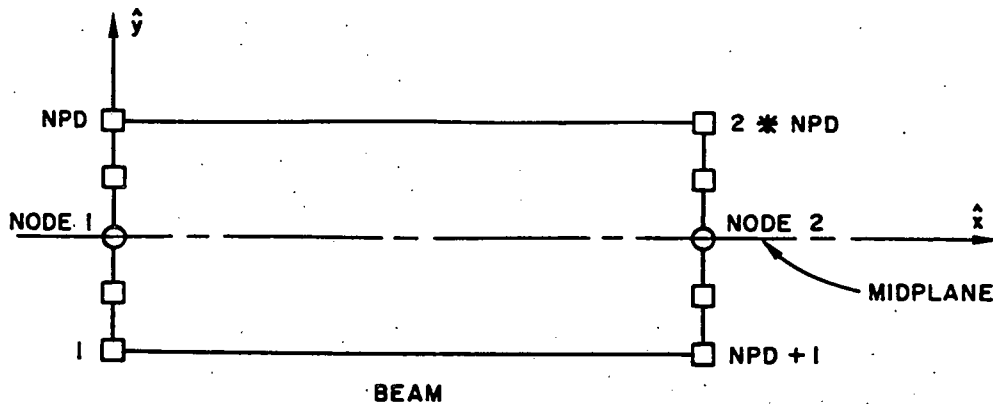


Figure 2.1 Location and Numbering of Integration Stations for Flexural and Continuum Elements

specified input data. Axial and shear tie are the two types of reinforcement that can be specified at various layers in the beam element; meridional, hoop, helical (seismic) and shear tie are those which can be specified at the various layers in the shell element. Reinforcement options in the shell element are shown in Fig. 2.2. Subscripts refer to various reinforcement layers.

Prestressing is modeled by simple elements, such as rods, rings, and membranes. The rod and the ring elements are used to simulate individual tendons. Specifically, a ring element is utilized for circumferential prestress, and a rod element is used for axial prestress. The axisymmetric membrane (shell without bending resistance) element is used for smearing the prestressing. The membrane element (with Poisson's ratio $\nu = 0$) effectively represents a net of prestress tendons.

The sliding option is used in conjunction with prestressing in order that the tendons can move along prescribed nodes of the concrete analytical model. Sliding along the prescribed sequence of concrete nodes enables the tendons to adjust themselves with respect to the concrete. This sliding feature simulates the actual behavior of the tendons during the prestressing operation and also during the loading of the structure. The prestress load is prescribed by input given in Section 2.6.

2.1.2 Continuum Elements

A linear displacement 4-node quadrilateral with straight sides is the principal element. Triangular elements are achieved by collapsing node 4 into node 3. Plane or axisymmetric geometries can be modeled. A corotational velocity strain-corotational stress formulation is used which is applicable to large strains, large rotations and anisotropic materials.

In the velocity strain formulation, deformation is measured by the velocity strain, which is given in terms of the velocity components v_x , v_y by

$$\begin{aligned} \eta_x &= \frac{\partial v_x}{\partial x} \\ \eta_y &= \frac{\partial v_y}{\partial y} \\ \eta_{xy} &= \frac{1}{2} \left(\frac{\partial v_x}{\partial y} + \frac{\partial v_y}{\partial x} \right) \end{aligned} \tag{2.3}$$

SEISMIC REINFORCEMENT

SHEAR TIE

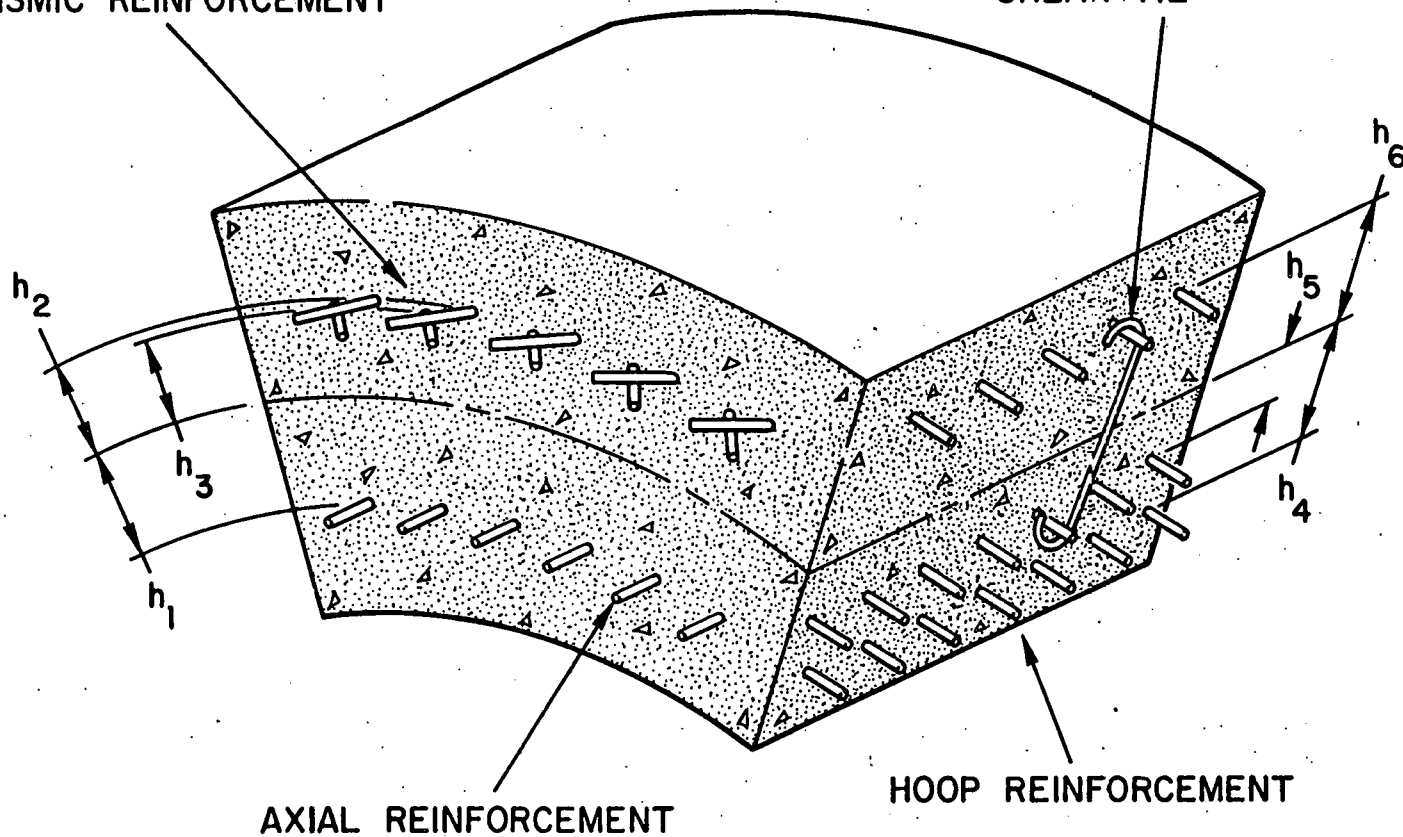


Figure 2.2 Representation of Reinforcement in Concrete of the Axisymmetric Shell Element

Note that the tensor velocity shear strain is stored and used in the program. This strain rate is not integrable, so when the velocity strain formulation is used, the total strain for a general biaxial state of strain has no rigorous meaning. However, in the uniaxial case, the integral of the velocity strain yields the logarithmic strain; thus the stress-strain data for these elements should be given in terms of physical stress versus logarithmic strain.

In the corotational velocity strain formulation, each element is associated with a coordinate system which rotates with the elements (\hat{x}, \hat{y}). The deformation of the element is then expressed in terms of the velocity strain expressed in the corotational components, viz.

$$\begin{aligned}\hat{\eta}_x &= \frac{\partial \hat{v}_x}{\partial \hat{v}} \\ \hat{\eta}_y &= \frac{\partial \hat{v}_y}{\partial \hat{v}} \\ \hat{\eta}_{xy} &= \frac{1}{2} \left(\frac{\partial \hat{v}_x}{\partial \hat{y}} + \frac{\partial \hat{v}_y}{\partial \hat{x}} \right)\end{aligned}\tag{2.4}$$

The stresses are expressed in the same corotational coordinates. Thus the components of the stress tensor are $\hat{\sigma}_x$, $\hat{\sigma}_y$ and $\hat{\sigma}_{xy}$. The alignment of the corotational coordinates relative to the global system (x, y) can be determined at any instant from the angle θ , which is measured counterclockwise from x to \hat{x} . The stress-strain law, when expressed in the corotational coordinates, then gives the increment in the corotational stress in terms of the corotational velocity strain in the following form

$$\Delta \hat{\sigma} = f(\hat{\sigma}, \hat{\eta})\tag{2.5}$$

Since the rate of the corotational stress is frame invariant, no corrections for the rotation are needed. Furthermore, since the constitutive law evaluation is made in the corotational coordinate system, the formulation is applicable to anisotropic materials. Unless otherwise specified, it is assumed that the angle between the corotational coordinate system and the global coordinate system θ is initially zero.

For the continuum elements, the diagonal lumped mass matrix is always used in conjunction with explicit integration.

Figure 2.1 illustrates the number of integration points (two) along one direction for a 4-node quadrilateral element. For this element, 1, 2 and 3 integration points along one direction respectively yield a total of 1, 4 and 9 integration points.

The stations at which element variables can be output correspond to the integration points and are in the order shown in Fig. 2.1. Station 1 is the integration point closest to node 1, and the station numbers are then incremented along the line of integration points parallel to the side defined by nodes 1 and 4. For each station, the order of the variables is the same.

The continuum elements that represent concrete media have reinforcement that is modeled by homogenization; the stress-strain law of the reinforcement is embedded in the stress-strain law of the elements. This approach can account for the direction, the position and the percentage of reinforcement. The reinforcement is assumed to remain rigidly bonded to the concrete; debonding of rebars and concrete is not considered. Reinforcement can be specified in the hoop direction and arbitrary orthogonal directions in the r,z or the x,y plane. The reinforcement arrangement is shown in Fig. 2.3. In addition to that specified within the continuum element, reinforcement can also be modeled by means of discrete rod and ring elements.

2.2 Material Laws

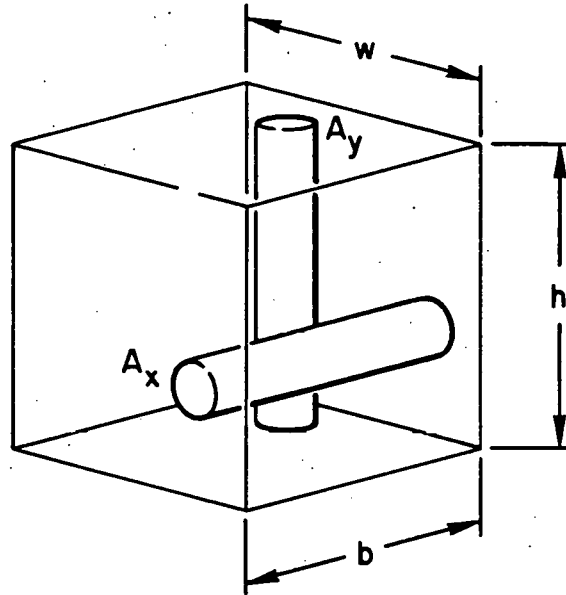
The following elastic-plastic material laws are available for representing non-concrete media:

1. Uniaxial stress for the plane stress beam element.
2. Biaxial stress for plane strain beam, plane stress continuum and axisymmetric shell elements.
3. Triaxial stress for plane strain and axisymmetric continuum elements.

Poisson's ratio should be set to zero when using the bar option of the beam element and the ring option for the shell element.

All of the elastic-plastic laws include isotropic strain hardening, which is isotropic in that the yield value σ_y in compression and tension grows equally when plastic stresses occur. The current yield value σ_y for each station is an element variable which may be output.

A_x - % OF AREA (hw)
 A_y - % OF AREA (bw)



A_r - % OF AREA (hw)
 A_z - % OF AREA (bw)
 A_c - % OF AREA (bh)

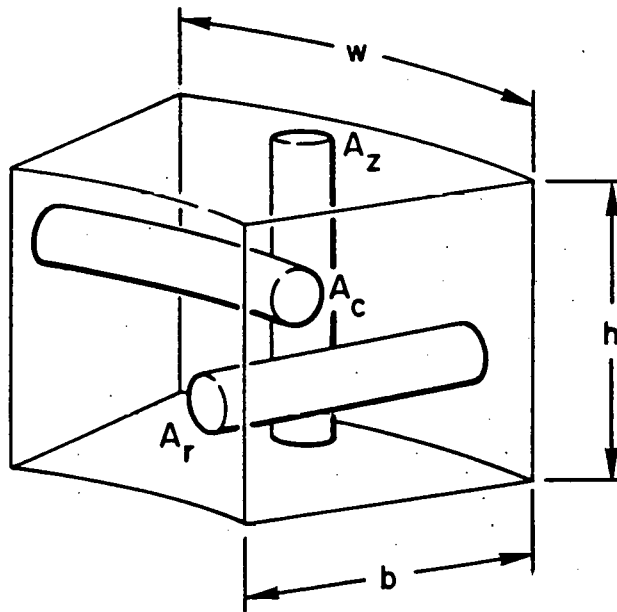


Figure 2.3 Representation of Reinforcement in Concrete for Plane and Axisymmetric Continuum Elements

In the uniaxial law, the stress in the plastic regime is a piecewise linear function of the strain, as shown in Fig. 2.4. The piecewise linear function is described through the points on the stress-strain curve: σ_i and ϵ_i . The first stress point σ_1 corresponds to the yield stress; the stresses in all subsequent points should be monotonically increasing values. If during the course of the computation the strain exceeds the last strain point which is input, the simulation will terminate with a message indicating the element where the strain was exceeded. The material related element group data for Cards IVb, IVc and IVd are described in Table 2.1.

The biaxial and triaxial laws are based on a Mises yield condition, an associated flow law and isotropic strain hardening; the stress in the plastic regime is a piecewise linear function of the strain, as shown in Fig. 2.4. The input data for these materials is given in Table 2.1. For continuum elements, the strains in the elements are the time integrals of the corresponding velocity strains. Therefore, for large strain problems, these strains are not tensors; however, they are indicative of the magnitudes of the deformation. Whenever a corotational formulation is used, the stress and strain output is in terms of corotational components.

The concrete material law formulation uses a uniaxial relationship for the plane stress beam. A biaxial relationship is used with the plane strain beam, plane stress continuum and axisymmetric shell elements. Finally, a triaxial relationship is used for the plane strain and axisymmetric continuum elements. The material law uses an elastic-plastic stress-strain relationship which includes strain hardening, Takahashi (1983), that can vary from isotropic to kinematic representations, Marchertas et al. (1985). Both associated and non-associated flow rules can be used with either Prager's or Ziegler's formulations. The stress in the plastic regime is a piecewise linear function of strain, as shown in Fig. 2.4. The previous comments about velocity strains and large strains with respect to non-concrete continuum are the same for concrete continuum elements. Two types of constitutive models are available in the code. The difference between the two models is the post-failure response in compression and tension.

In the first constitutive model (preferred) the strength capacity of the concrete in multiaxial stress space is characterized by a four parameter failure surface described by Hsieh, Ting and Chen (1983). The response after

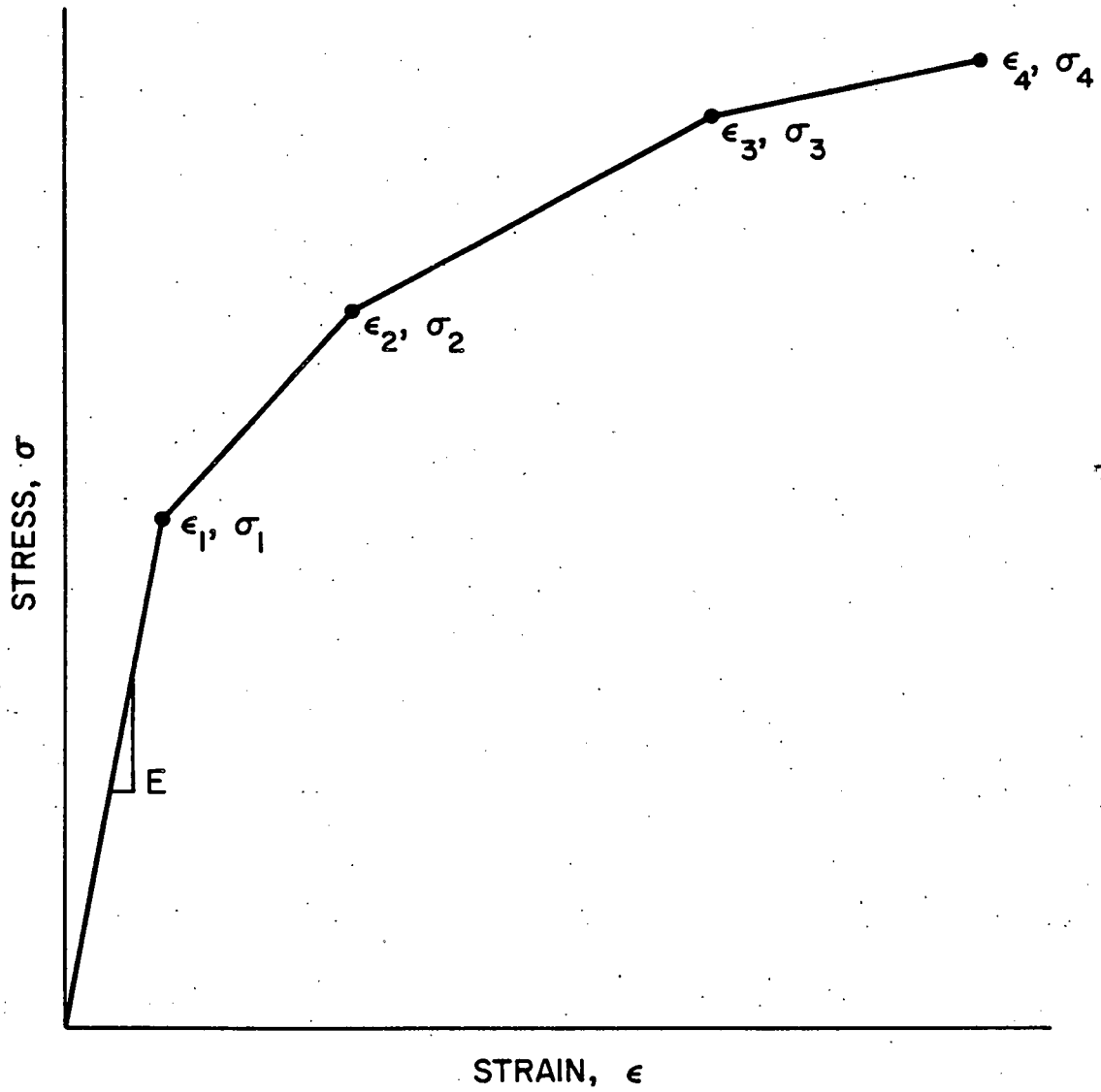


Figure 2.4 Stress-Strain Law for Elastic-Plastic Material

Table 2.1 Summary of Material Data Allocation

LTYP	1	2	3	4	11	12	13	14
element	beam	plane continuum	plane concrete	concrete beam	shell	axi- symmetric continuum	axi- symmetric concrete	concrete shell
Card IVb								
E(1)	ρ	ρ	ρ	ρ	ρ	ρ	ρ	ρ
E(2)			ν	ν			ν	ν
E(3)	α	α	α	α	α	α	α	α
E(4)	α	α	α_s	α_s	α	α	α_s	α_s
E(5)	k	k	k	k	k	k	k	k
E(6)	c	c	c	c	c	c	c	c
E(7)	NTP	NTP	NTP	NTP	NTP	NTP	NTP	NTP
E(8)	NSP	NSP	NSP	NSP	NSP	NSP	NSP	NSP
Card IVc								
E(9)	w	t	t	w				
E(10)	h	α_H	α_H	h	h	α_H	α_H	h
E(11)	α_a	α_a	α_a	α_a	α_a	α_a	α_a	α_a
E(12)	\hat{A}	\hat{A}	RM	RM	\hat{A}	\hat{A}	RM	RM
E(13)	\bar{e}_o^i	\bar{e}_o^i	ASC	ASC	\bar{e}_o^i	\bar{e}_o^i	ASC	ASC
E(14)	ICE	ICE	ICE	ICE	ICE	ICE	ICE	ICE
E(15)	NPL		FSF	FSF	NPL		FSF	FSF
E(16)	NPD	NGP	NGP	NGP	NPD	NGP	NGP	NGP

Table 2.1 Summary of Material Data Allocation (cont.)

LTyp	1	2	3	4	11	12	13	14
element	beam	plane continuum	plane concrete	concrete beam	shell	axi- symmetric continuum	axi- symmetric concrete	concrete shell
Card IVd (repeat for each temperature curve; enter data only to needed pair)								
E(17)	T	T	T	T	T	T	T	T
E(18)	E	E	E	E	E	E	E	E
E(19)	ϵ_1	ϵ_1	ϵ_1	ϵ_1	ϵ_1	ϵ_1	ϵ_1	ϵ_1
E(20)	σ_1	σ_1	σ_1	σ_1	σ_1	σ_1	σ_1	σ_1
E(21)	ϵ_2	ϵ_2	ϵ_2	ϵ_2	ϵ_2	ϵ_2	ϵ_2	ϵ_2
E(22)	σ_2	σ_2	σ_2	σ_2	σ_2	σ_2	σ_2	σ_2
E(23)	ϵ_3	ϵ_3	ϵ_3	ϵ_3	ϵ_3	ϵ_3	ϵ_3	ϵ_3
E(24)	σ_3	σ_3	σ_3	σ_3	σ_3	σ_3	σ_3	σ_3
Card IVe			x	x			x	x
Card IVf			x	x			x	x
Card IVg			x	x			x	x
Card IVh			x	x			x	x
Card IVi			x	x			x	x
Card IVj			x	x			x	x
Card IVk				x				x
Card IVl				x				x

Notation for Table 2.1

ρ	:	density
ν	:	Poisson's ratio
α	:	coefficient of thermal-expansion
k	:	thermal conductivity
c	:	specific heat
NTP	:	number of temperature curves input for this material
NSP	:	number of stress-strain pairs defining piecewise linear curve
w	:	beam width
h	:	beam height and shell thickness
α_a	:	artificial viscosity
\hat{A}	:	effective strain-rate amplitude parameter
$\bar{\epsilon}_0^i$:	reference strain-rate
ICE	:	material law flag
NPL	:	number of integration points along length
NPD	:	number of integration points through thickness
t	:	plane continuum or hydrodynamic element thickness
α_H	:	hourglass viscosity
NGP	:	number of Gaussian integration points along one direction
α_S	:	stress-dependent thermal expansion coefficient
RM	:	mixed hardening parameter
ASC	:	flow rule parameter
FSF	:	failure surface flag
T	:	temperature
E	:	Young's modulus
ϵ_j	:	i -th strain on piecewise linear stress-strain curve, see Fig. 2.4
σ_j	:	i -th stress on piecewise linear stress-strain curve, see Fig. 2.4

tensile failure is simulated using the element-size-independent cracking criterion established by Bazant and Oh (1983). For tensile failure the cracks are assumed to be normal to the maximum principal strain and are smeared into the element by reducing the stiffness to zero normal to the cracks. Strain softening in the model is available through an energy release term (element size independent feature) or a specified strain at complete failure, which allows the tensile stress normal to the crack to be reduced to zero gradually. For compression failure, the concrete is allowed to crush which essentially sets all the stresses to zero. Strain softening for crushing in the model is also available through an energy release term or a specified strain at complete compression failure. This allows the stresses to be reduced to zero gradually.

The second constitutive model (still under development) is based on an elastic-plastic formulation coupled with the critical state theory used in soils. This has been described by Lin et al. (1987). The elastic-plastic model described earlier is applied up to the maximum strength capacity of material. Beyond this point, the modified critical state theory is used to simulate the material response. In this way, the softening (or hardening) of the material can be taken into account. The softening (or hardening) of the material is facilitated by the shrinkage (or expansion) of a limiting surface in the stress space. This surface is assumed to coincide with the failure surface at the maximum strength of material. It is derived with the help of the five-parameter Willam and Warnke (1974) failure surface. The shape of the limiting surface has the same angular dependence as the failure surface in the deviatoric plane. In the meridian plane, the shape is assumed to be that of a slanted ellipse. It is applicable to concrete, geomaterials, and other materials which have different strengths in tension and compression.

2.3 Time Integration

2.3.1 Dynamic Relaxation

For processes where the stiffness matrix is not available or very difficult to program as in the case of highly nonlinear materials, quasi-steady state and steady state (static) solutions may be attained by dynamic relaxation. Dynamic relaxation is a process in which sufficient damping is added to the system so that through the solution of the equations of motion, the final equilibrium position is reached. Both mass-proportional and

stiffness-proportional damping are available. For steady state solutions, only mass-proportional damping is used (90% is recommended).

In the computer program, the external loads are prescribed by input. To facilitate dynamic relaxation, this loading is applied incrementally, i.e., in load steps of specified magnitude. Hence, dynamic relaxation for each of the load increments permits readjustment of the structure while approaching static equilibrium asymptotically, as shown in Fig. 2.5. Static equilibrium is attained at definite points as indicated in Fig. 2.5.

Equilibrium of the system is established by the imposition of two criteria. One of the criterion is equivalent to the comparison of internal forces with the external forces at the same iteration step. The other criterion is tantamount to the comparison of the change of displacements at successive iteration steps.

2.3.2 Stable Time Step

The explicit integration is carried out by the central difference method. The stability properties (structural) of this integration procedure limits the time step by

$$\Delta t \leq \frac{2}{\omega_{\max}} \left((1 + \mu^2)^{1/2} - \mu \right) \quad (2.6)$$

where ω_{\max} is the maximum eigenvalue (the highest frequency) of the system and μ is the fraction of critical damping in the highest frequency. The formulas for the stability calculations are summarized in Table 2.2.

The program has provisions for automatically calculating Δt (structurally) for each simulation using Eq. (2.6) with a reduction factor (0.8 to 0.9 of the stable Δt is recommended) to be specified by the user. The user usually should check for conservation of energy in the computation. An energy imbalance greater than a few percent usually indicates that a numerical instability has occurred during the course of the computation. Energy output can be obtained as described on Card Xc.

When the explicit integration technique is applied to solve a transient problem, the time parameter used pertains to the actual time of the physical process. In quasi-steady state and steady state problems the time parameter in explicit integration loses its original meaning; it no longer means real time, but rather an integration parameter proportional to time.

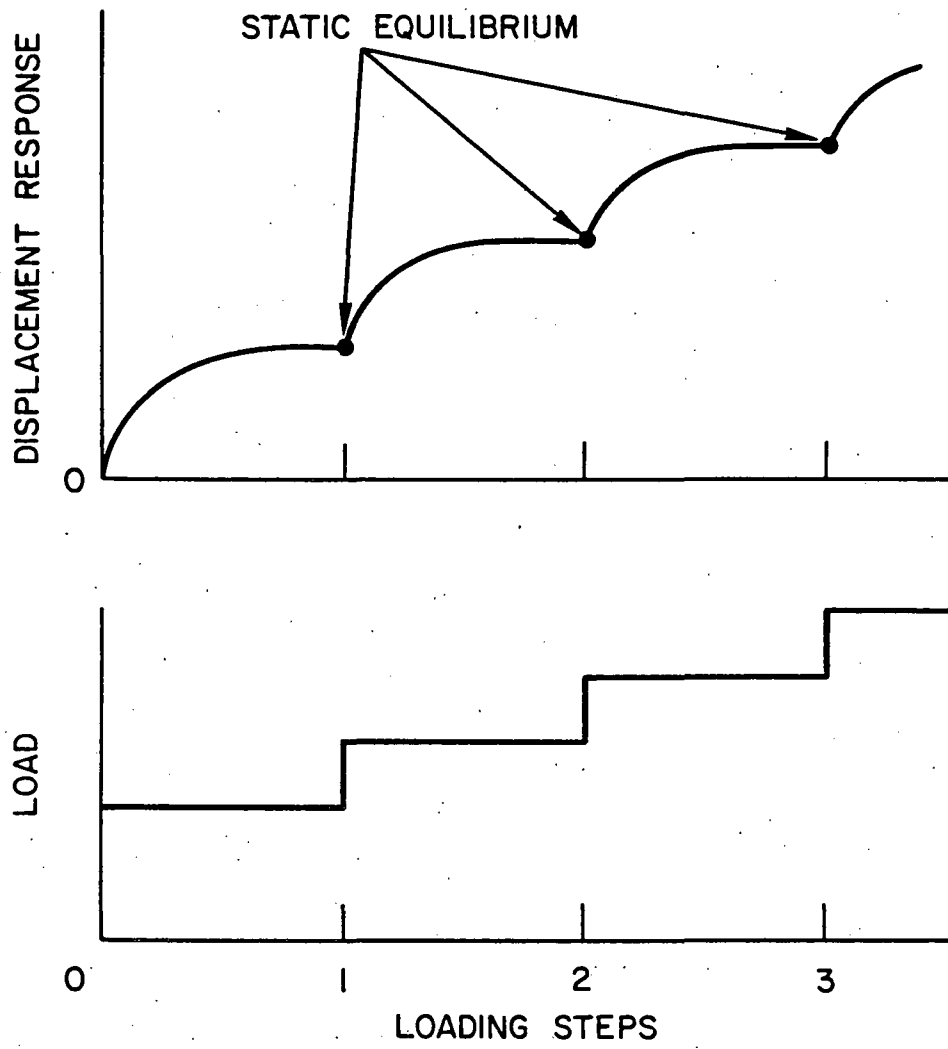


Figure 2.5 Loading Method for Dynamic Relaxation

Table 2.2 Guidelines for Stable Time Step in Explicit Calculations

$$\Delta t \leq \frac{2}{\omega_{\max}} \left((1 + \mu^2)^{1/2} - \mu \right)$$

Square of Maximum Frequency

beam and shell $\omega_{\max}^2 = \frac{4c^2}{l^2}$ or $\frac{16c^2 h^2}{4}$ (whichever is larger)

continuum $\omega_{\max}^2 = \frac{2c^2}{A} \left(\gamma_1 + (\gamma_1 - \bar{\nu} \gamma_2)^{1/2} \right)$

$$\gamma_1 = \frac{1}{2} (y_{24}^2 + y_{31}^2 + x_{42}^2 + x_{13}^2) \quad \bar{\nu} = \frac{4(1 - 2\nu)}{1 - \nu^2}$$

$$\gamma_2 = \frac{1}{4} (y_{24}^2 + y_{31}^2)(x_{42}^2 + x_{13}^2) - \frac{1}{4} (y_{24}x_{42} + y_{31}x_{13})^2$$

Square of Elastic Wavespeed

beam and shell $c^2 = \frac{E}{\rho} \left(\begin{array}{c} \text{uniaxial} \\ \text{plane} \\ \text{stress} \end{array} \right)$ or $\frac{E}{\rho(1 - \nu^2)} \left(\begin{array}{cc} \text{uniaxial} & \text{biaxial} \\ \text{plane} & \text{plane} \\ \text{stress} & \text{stress} \end{array} \right)$

continuum $c^2 = \frac{E}{\rho(1 - \nu^2)} \left(\begin{array}{c} \text{plane} \\ \text{stress} \end{array} \right)$ or $\frac{E(1 - \nu)}{\rho(1 + \nu)(1 - 2\nu)} \left(\begin{array}{c} \text{plane} \\ \text{strain} \end{array} \right)$

Δt = stable time step
 ω_{\max} = maximum frequency
 μ = critical damping fraction
 c = elastic wavespeed
 l = length of element
 h = thickness of beam or shell

A = cross-sectional area
 E = Young's modulus
 ρ = mass density
 ν = Poisson's ratio
 x_{ij} = x-coordinate difference
 y_{ij} = y-coordinate difference

For purely static problems this time parameter becomes solely an integration parameter.

From Table 2.2, it can be seen that an inefficient time step (too small) may result if there are large differences in element sizes or large differences between translational and/or rotational frequencies which, for static solutions, will cause excessive computation time. This inefficiency may be overcome by using a concept of mass-adjustment. The principal feature of this method is to use a constant Δt (usually $\Delta t = 1.0$ for convenience). This is done by adjusting the mass density, ρ , for both translational and rotational frequencies, utilizing the stability time limit relationship given in Eq. (2.6). From experience, by utilizing the adjusted mass scheme, a considerable saving in computational time may result, depending on the type of structure and discretization used.

For thermal solutions, the critical time step is

$$\Delta t = \frac{\rho c \ell}{2k} \quad (2.7)$$

where ℓ is the shortest distance between nodes, ρ is the mass density, c is the specific heat, and k is the conductivity.

2.3.3 Artificial Viscosities

In the integration of the finite element equations of motion with small time steps, such as is generally the case in explicit integration, high frequency oscillations which are called "spurious oscillations" or "aliasing" will appear in an undamped system. The severity of these oscillations tends to increase if the mesh is rather heterogeneous. These oscillations can be reduced and sometimes eliminated by the use of a suitable artificial viscosity, which is really a numerical damping.

The program has a provision for including a linear artificial viscosity. For a uniaxial stress-strain law this artificial viscosity is of the form

$$\hat{\sigma}_{vis} = \frac{2\mu E}{\omega_{max}} \hat{\eta} \quad (2.8)$$

where μ is a constant input by the user, E is Young's modulus and ω_{max} is the maximum eigenvalue of the system. For constant strain type elements, which

include the continuum elements and the membrane part in the beam element, μ corresponds to the fraction of critical damping in the highest element mode. For most cases, a value of μ between 0.02 and 0.10 is recommended. Note that according to Eq. (2.6) the use of an artificial viscosity reduces the stability limit on the time step.

2.4 Mesh Generation

Nodal coordinates may be a mix of cartesian and polar systems with polar systems permitted to be defined in local coordinates. Some mesh generation features are included. Intermediate node coordinate data can be skipped if:

1. The coordinates can be generated by equispacing these nodes.
2. The coordinates between two nodes can be generated by changing the distance between each sequential pair of nodes by a specified ratio.

The ratio of distances between automatically generated nodes can be specified by using KONTRL(3) on Card III.

Intermediate element node data can be skipped if the elements have the same material type number, MTYP, and are of the same element type, LTYP, and the nodes can be generated by:

1. Adding 1 to the node numbers of the previous element.
2. Adding a number INCR to the node numbers of the previous element.

2.5 Boundary Conditions/Support Conditions

Prescribed displacement boundary conditions/support conditions which can be treated by the program are quite arbitrary. At any node or for any group of nodes, either one or two components of the displacements in an (\hat{x}, \hat{y}) coordinate system may be specified to vanish or to have a selected time history. The (\hat{x}, \hat{y}) coordinate system may be aligned arbitrarily in space relative to the global coordinate system, in that case, the angle θ from x to \hat{x} , positive in a counter-clockwise sense, has to be input. If no angle θ is input, the (\hat{x}, \hat{y}) system is assumed to coincide with the global coordinate system. The rotation nodes or component can also have prescribed conditions. Since the formulation is based on a displacement type of finite element method, no conditions need to be prescribed on stress free boundaries.

If displacements are prescribed nonzero values, these values can be arbitrary piecewise linear functions of time, which are input through the load line data, Cards VIII.

2.6 Loading

The program has provisions for several types of loading on the model:

1. Initial values of velocities on a line of nodes specified by the user.
2. Pressure loads applied on a line of nodes where the pressure may be an arbitrary piecewise linear function of time or volume; this function is specified by the data f_i and t_i as shown in Fig. 2.6.
3. Forces, displacements or thermal loads applied on an arbitrary set of nodes, where the forces, displacements thermals may be arbitrary piecewise linear functions of time.
4. Prestressing an arbitrary set of elements; this loading condition is applied prior to the desired simulation.
5. Gravity loading specified by using KONTRL(6) on Card III and the detailed data of acceleration of gravity rate and the direction cosines for the gravity on Card XI.

A load line is here defined as a set of node points; for initial velocity and pressure loadings, the load line must consist of a sequence of adjacent node points (elements). For applied forces and displacements, the load line may be an arbitrary set of nodes. For thermal loads, temperatures are applied to an arbitrary set of nodes (minimum of two) while heat fluxes and film coefficients are applied to a line of nodes (elements). Prestressing and internal heat sources are applied to an arbitrary set of elements. Nodes and elements specified for thermal loads are those of the thermal mesh.

Whenever pressure, initial normal or tangential velocities or heat flux are prescribed, it is necessary that a positive direction be defined for these quantities. This is accomplished as shown in Fig. 2.7. First, a positive direction is defined for the load line as that direction obtained by tracing a line in the order that the nodes are numbered for the load line. The positive normal direction is then obtained by rotating clockwise 90° from the positive load line direction. A positive pressure, positive normal initial velocity or positive heat flux is applied in that direction, whereas a negative pressure, negative normal velocity or negative heat flux is applied opposite to the direction shown. The positive tangential direction corresponds to the direction of the load line.

Load lines are used to input a specified prestress to the tendon elements. In quasi-static problems, prestressing is prescribed by means of

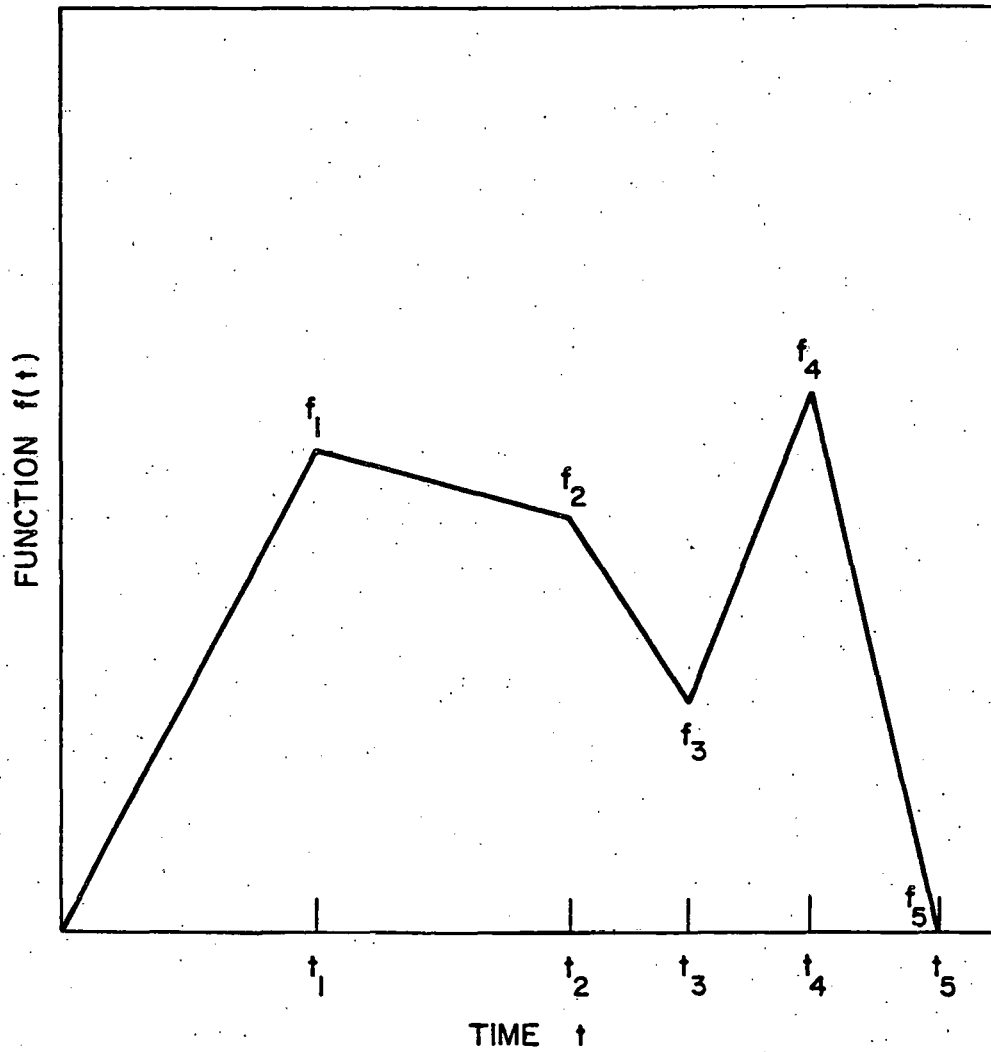


Figure 2.6 Functions $f(t)$ as Defined by f_i and t_i ; $f(t)$ may be a Pressure, Nodal Force, Displacement or Thermal

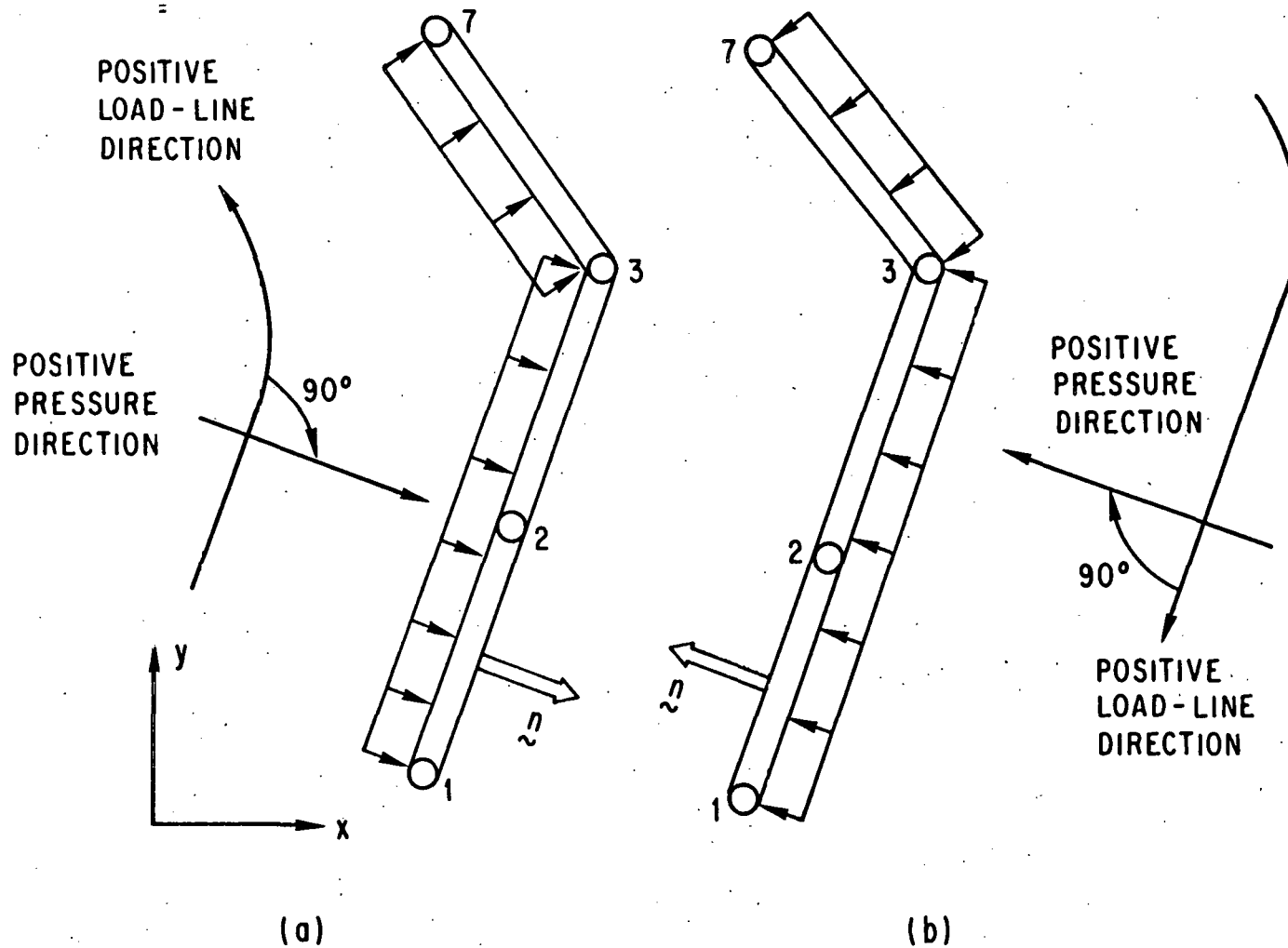


Figure 2.7 Sign Convention for Load Lines and Applied Loads; in Case (a): the Nodes are Numbered in the Order 1, 2, 3, 7; Case (b): 7, 3, 2, 1

one step load. This is possible because of large structural damping (as much as 90%) associated with the quasi-static type problems. For dynamic problems, prestressing would be specified by a gradual ramp load so as not to generate inertial effects (which could cause premature cracking in the concrete). Prestressing is a preload which precedes the thermal and/or mechanical loading for the simulation. These loadings (prestress load followed by thermo-mechanical) are applied in a sequential manner.

2.7 Sliding Interface

For the purpose of treating interfaces between two solids that are not physically bonded and regions where severe discontinuities in tangential displacements are expected, sliding interfaces have been included. The sliding interfaces may be straight or piecewise linear curves, but sharp corners should be avoided. The model included here cannot treat debonding, so the regions on both sides of the interface must remain in contact. For purposes of modeling tangential forces across the interface, simple Coulomb friction and viscous force models have been included. Only small relative displacements can be handled on the slide line.

2.8 Rigid Interface

The rigid interface option provides a means of connecting two dissimilar meshes. Three types of rigid interfaces are included, one for joining continuum meshes of different size elements, the second for joining continuum elements to flexural elements and the third for connecting multiple sets of flexural elements. These will be called the continuum, flexural and flexural offset interfaces, respectively. While meshes of two different sizes can be interconnected through triangles, the rigid sideline is more convenient because the resulting meshes are more easily generated in an automatic fashion.

In using a rigid continuum interface, one side must be designated as master, the second as slave. The option works best when the side with the more massive nodes, which is usually the side with the larger elements, is chosen to be the master side.

A second application for the rigid interface is for joining flexural elements, such as the beam or axisymmetric shell, to continuum elements, which is called the rigid flexural interface. The rigid interface should be perpendicular to the midline of the flexural element.

In this option, the flexural node should always be the master node. The interaction between the flexural master node and the continuum slave nodes is based on the assumption that the entire slave line remains straight and rotates with the flexural node, i.e., the Kirchhoff assumption is imposed on the continuum nodes.

The rigid interface can also be used to represent two flexural elements that act together and have noncommon midplanes (e.g. lined structures) by letting one set of flexural nodes be the master nodes, the second set be slave nodes. Generally the more massive structure should be assigned to be the master nodes. In using this option, the spacing of the slave nodes and master nodes should be approximately equal. This feature can only be used in the two degree of freedom per node option. The liner may also be a membrane without flexural resistance. Multiple layering of elements can also be handled with multiple sets of rigid interface data that have the same set of master nodes with different sets of slave nodes. Thus sandwich shells may be modeled effectively and efficiently without the penalty of a small stable time step that results when layers are connected with stiff, deformable elements. Only the translational nodes are necessary for nodal input data. The rotational nodes are generated internally for those elements with flexural resistance.

2.9 Output

The program has options for two types of output:

1. Time histories of selected displacement, velocity, and acceleration components, and time histories of selected element variables, such as stresses or strains.
2. At particular time steps either displacements, velocities or accelerations at all nodes and/or element variables in all stations in all elements may be printed.

The time histories may be printed with a frequency specified by the user, and, in addition, they may be plotted with a printer plotter or with DISSPLA. For any time histories which are selected for plotting, the variables at the selected frequency are stored in core, so that the number of time histories selected influences the core storage requirements of the program.

The method for selecting particular displacement, velocity or acceleration components is quite straightforward: the user simply specifies the node number, whether he desires a displacement, velocity or acceleration, and the

choice of plotting and/or printout. The selection of element variables for time history output is more complex, for the user must specify the index of the variable he selects in addition to the element number. The indexing procedure for variables depends on the element type, the material type, and the arrangements of the output station, which correspond to the integration points used in the element. Therefore, in order to specify the index, the user must know how the stations are arranged in a particular element, and how the variables are arranged in each station, which depends on the material type.

3. PROGRAM INPUT

3.1 Input Format Overview

The program uses a fixed input format, so all data must be input exactly according to the format specified for each card, and the card groups must appear in sequence according to their number. Some variables, which are commonly considered integers, such as E(7) on Card IVb-1, are input as floating point numbers; they should not be input as integers.

An outline of the data is given in Table 3.1. This table gives the order in which the data appears and when it can be omitted. The details of the data cards are given in the following. Prior to constructing his own data sets, the user is advised to peruse Section 4 which gives data sets for some sample problems.

Table 3.1 Input Data Structure Overview

<u>Card</u>	<u>Description</u>	<u>When Needed</u>
I	Title	for all data sets
II	Parameter	for all data sets
III	Control	for all data sets
IV	Material property	
IVa-1 (a to d)	Beam element	} when element appears in mesh.
IVa-2 (a to d)	Plane continuum element	
IVa-3 (a to j)	Plane concrete element	
IVa-4 (a to l)	Concrete beam element	
IVa-11 (a to d)	Shell element	
IVa-12 (a to d)	Axisymmetric continuum element	
IVa-13 (a to j)	Axisymmetric concrete element	
IVa-14 (a to l)	Concrete shell element	
V	Node	for all data sets
VI	Element	for all data sets
VII	Prescribed displacement	only if NUMDIS > 0

Table 3.1 Input Data Structure Overview (cont.)

VIIIa	Load line (parameters)	for all data sets
VIIIb-1	Load line (time history)	only if IVOL(1) > 0
VIIIb-2	Load line (initial velocity)	only if IVOL(1) = 0
VIIIc	Load line (nodes/elements)	for all data sets
IXa	Sliding/rigid interface (parameters)	only if NSLID > 0
IXb	Sliding/rigid interface (nodes)	only if NSLID > 0
IXc	Sliding/rigid interface (nodes)	only if NSLID > 0
Xa	Output	for all data sets
Xb	Motion output	only if NPRU > 0
Xc	Element output	only if NPRS > 0
Xd	Picture output	only if NPIC > 0
XI	Gravity loading	only if KONTRL(6) = 1

3.2 Restart Procedure

The user has the option to prescribe a time step number in which data for a restart will be written on Unit 12. In order to use this restart data, it is only necessary to provide the data written on Unit 12 and Cards I and II from the input stream described in the following section. Two modifications to the input are also available during a restart. Include these cards only if changes are to be made. A load line and the time step or load increment for global output may be modified as long as there is no change in the original array dimensioning. Parameters such as the number of nodes on the load line, the number of points defining the loading function and the number of output pictures may not be altered. Nodes, loading function values and picture time steps, however, can be modified. If there are changes, all Cards VIII and Xd must be included; even unmodified load lines. Other modifications to various parameters (e.g., the maximum number of iterations per load increment value on the control card) can be achieved by recompiling appropriate subroutines. Again, array dimensioning must be maintained. Restart files, if requested,

are automatically written every load step when using the dynamic relaxation analysis option.

3.3 Input Specifications

INCLUDE FOR ALL DATA SETS

Card Type I	Title Card (20A4)						
1	6	11	16	21	26	31	36
TITLE							
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-80	TITLE	Any 80 alphanumeric characters to identify the run. These characters will be printed as a heading to the output. If the word "RESTART" is in the first seven columns, the job is assumed to be a restart.

INCLUDE FOR ALL DATA SETS

Card Type II Parameter Card (6I5,E10.6,2I5,E10.6,3I5)

1	6	11	16	21	26	31	36
NUMNP	NUMEL	NUMMAT	NUMDIS	MXSTEP	NDGREE	DELTA	
41	46	51	56	61	66	71	76
NPRES	NSLID	TIMEND		NPRO	NEXPC	NXSTEP	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	NUMNP	Number of nodes in the mesh.
6-10	NUMEL	Number of elements in the mesh.
11-15	NUMMAT	Number of different material types; if beam and shell elements are of the same material, they must still be associated with different material types.
16-20	NUMDIS	Number of nodes at which any displacement components are specified a zero or nonzero value. Also indicates number of Type VII cards needed.
21-25	MXSTEP	Number of time steps or load increments. Not necessary if DELT is to be computed by the code.
26-30	NDGREE	Number of degrees of freedom per node. (Currently, can be 2 or 3).
31-40	DELTA	Time step, sec or load increment. If the code is to compute the time step, input a negative value (-0.8 to -0.9 recommended) for DELT that will represent the reduced value of the stable time step.
41-45	NPRES	Number of different load lines of the mesh.
46-50	NSLID	Number of sliding interfaces for the mesh. (Not operational in implicit solution).
51-60	TIMEND	Duration of simulation. Use only when DELT is to be computed by the code.
61-65	NPRO	Number of prestress tendons in the mesh.
66-70	NEXPC	Number of elements in the mesh expected to crack. (Default is NUMEL).

(Continue)

Card Type II (Continued)

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
71-75	NXSTEP	Time step number from which restart will begin. Necessary only if the job is a restart. Usually equal to MXSTEP from any previously saved job restart file.

INCLUDE FOR ALL DATA SETS

Card Type III

Control Card (16I5)

1	6	11	16	21	26	31	36
KON(1)	KON(2)	KON(3)	KON(4)	KON(5)	KON(6)	KON(7)	KON(8)
41	46	51	56	61	66	71	76
KON(9)	KON(10)	KON(11)	KON(12)	KON(13)	KON(14)	KON(15)	KON(16)

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	KONTRL(1)	Analysis option. KONTRL(1)=0: initial velocity analysis option. KONTRL(1)=1: transient analysis option. KONTRL(1)=2: dynamic relaxation analysis option.
6-10	KONTRL(2)	Integration solution option. KONTRL(2)=0: explicit solution.
11-15	KONTRL(3)	Nonequispaced mesh option. KONTRL(3)=0: any nodes for which node cards are not included will be equispaced. KONTRL(3)>0: nodes for which node cards are not included will be spaced so that the ratio of distances between consecutive nodes is KONTRL(3)/1000.
16-20	KONTRL(4)	Percent of stiffness-proportional damping.
21-25	KONTRL(5)	Percent of mass-proportional damping.
26-30	KONTRL(6)	Gravity load option. KONTRL(6)=0: no gravity loading. KONTRL(6)=1: include gravity loading.
31-35	KONTRL(7)	blank.

(Continue)

Card Type III (Continued)

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
36-40	KONTRL(8)	Option for reading or writing data for restart of a job on UNIT 12. KONTRL(8)=0: no restart write or read. KONTRL(8)=1: write a restart file from which only a read can be made with no further writes allowed. KONTRL(8)=2: write a restart file which permits reads and further writes to be made on an existing or new restart file. KONTRL(8)=3: write a restart file which permits modifications to the load lines and time steps for picture output plus reads and writes as in KONTRL(8)=2.
41-45	KONTRL(9)	First value defining time reduction integration parameter. KONTRL(9) times 10 to the KONTRL(10) power is the complete value. If left blank, it defaults to 1.
46-50	KONTRL(10)	If needed, the second value of the time reduction parameter.
51-55	KONTRL(11)	Request for graphics mesh plots (undeformed or deformed). KONTRL(11)=0: no mesh plots will be made. KONTRL(11)>0: mesh plots will be made.*
56-60	KONTRL(12)	KONTRL(12) = maximum number of iterations per load increment.# KONTRL(12), KONTRL(13) and KONTRL(14) are used only when employing the dynamic relaxation analysis option.
61-65	KONTRL(13)	KONTRL(13)/1.0E4 is the error force criterion allowed in the load increment. KONTRL(13)=100 to 200: (recommended).
66-70	KONTRL(14)	KONTRL(14)/1.0E4 is the displacement error criterion allowed in the load increment. KONTRL(14)=10 to 20: (recommended).

(Continue)

Card Type III (Continued)

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
71-75	KONTRL(15)	Option for printing crack information. KONTRL(15)=0: no printing of crack information. (stress limit exceeded cracking). KONTRL(15)=1: print crack information. (stress limit exceeded cracking). KONTRL(15)=2: no printing of crack information. (solution uses cracking control). KONTRL(15)=3: print crack information. (solution uses cracking control).
76-80	KONTRL(16)	blank.

*Undeformed and/or deformed mesh plots are available. Undeformed plots are given at the zero cycle picture output (see Card Type Xd). Deformed plots are given at certain requested cycles (see Card Type Xd). The scale for deformed plots is set by the undeformed (or first requested) plot so that they may be overlaid to view the deformation. If desired, all plotted displacements may be magnified by the value indicated in KONTRL(11); for no magnification, set KONTRL(11)=1.

#Most simulations will take a few hundred to a thousand iterations to meet the error force and displacement error criteria. Simulations which need to capture extreme nonlinear behavior could take several tens of thousands of iterations.

INCLUDE ONLY IF USING BEAM ELEMENTS

Card Type IVa-1 Material Property Card (215) Beam Element

1	6	11	16	21	26	31	36
MTYP	LTYP						
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	MTYP	Material type number.
6-10	LTYP	Element type number. LTYP=1: beam element.

INCLUDE ONLY IF USING BEAM ELEMENTS

Card Type IVb-1 Material Property Card (8E10.4) Beam Element

1	6	11	16	21	26	31	36
E(1)		E(2)		E(3)		E(4)	
41	46	51	56	61	66	71	76
E(5)		E(6)		E(7)		E(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(1)	Mass-density.
11-20	E(2)	blank.
21-30	E(3)	Poisson's ratio.
31-40	E(4)	Coefficient of thermal-expansion.
41-50	E(5)	Thermal conductivity.
51-60	E(6)	Specific heat.
61-70	E(7)	Number of temperature curves input for this material on Card Type IVd-1. If E(7) is left blank, default value is 1.0.
71-80	E(8)	Number of stress-strain pairs defining the piecewise linear stress-strain curve. Each temperature curve must have this number of pairs.

INCLUDE ONLY IF USING BEAM ELEMENTS

Card Type IVc-1 Material Property Card (8E10.4) Beam Element

1	6	11	16	21	26	31	36
E(9)		E(10)		E(11)		E(12)	
41	46	51	56	61	66	71	76
E(13)		E(14)		E(15)		E(16)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(9)	Beam width. If left blank, E(9) defaults to unit-width.
11-20	E(10)	Beam thickness (bar area). Input required for NDGREE=3 or bar option. For NDGREE=2 option, thickness can be either specified in dummy node data on Card Type V or input in E(10).
21-30	E(11)	Artificial viscosity.
31-40	E(12)	Effective strain-rate amplitude parameter.
41-50	E(13)	Reference strain-rate.
51-60	E(14)	Plane-stress or plane-strain flag. E(14)=1.0: plane-stress material law. E(14)=2.0: plane-strain material law.
61-70	E(15)	Number of integration points along length. If E(15) is left blank, default value is 2.0. (Currently only acceptable value). For bar option, set E(15)=2.0.
71-80	E(16)	Number of integration points through thickness. If E(16) is left blank, default value is 5.0. If not left blank, E(16)≥2.0. For bar option, set E(16)=1.0.

INCLUDE ONLY IF USING BEAM ELEMENTS

Card Type IVd-1 Material Property Card (8E10.4) Beam Element

1	6	11	16	21	26	31	36
E(17)		E(18)		E(19)		E(20)	
41	46	51	56	61	66	71	76
E(21)		E(22)		E(23)		E(24)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(17)	Temperature-value for curve 1 of piecewise linear stress-strain law.
11-20	E(18)	Young's modulus for temperature value 1.
21-30	E(19)	Strain value for point 1.
31-40	E(20)	Stress value for point 1.
41-50	E(21)	Strain value for point 2.
51-60	E(22)	Stress value for point 2.
61-70	E(23)	Strain value for point 3.
71-80	E(24)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVd-1 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

INCLUDE ONLY IF USING PLANE CONTINUUM ELEMENTS

Card Type IVa-2 Material Property Card (2I5) Plane Continuum Element

1	6	11	16	21	26	31	36
MTYP	LTP						
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	MTYP	Material type number.
6-10	LTP	Element type number. LTP=2: plane continuum element.

INCLUDE ONLY IF USING PLANE CONTINUUM ELEMENTS

Card Type IVb-2 Material Property Card (8E10.4) Plane Continuum Element

1	6	11	16	21	26	31	36
E(1)		E(2)		E(3)		E(4)	
41	46	51	56	61	66	71	76
E(5)		E(6)		E(7)		E(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(1)	Mass-density.
11-20	E(2)	blank.
21-30	E(3)	Poisson's ratio.
31-40	E(4)	Coefficient of thermal-expansion.
41-50	E(5)	Thermal conductivity.
51-60	E(6)	Specific heat.
61-70	E(7)	Number of temperature curves input for this material on Card Type IVd-2. If E(7) is left blank, default value is 1.0.
71-80	E(8)	Number of stress-strain pairs defining the piecewise linear stress-strain curve. Each temperature curve must have this number of pairs.

INCLUDE ONLY IF USING PLANE CONTINUUM ELEMENTS

Card Type IVc-2 Material Property Card (8E10.4) Plane Continuum Element

1	6	11	16	21	26	31	36
E(9)		E(10)		E(11)		E(12)	
41	46	51	56	61	66	71	76
E(13)		E(14)		E(15)		E(16)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(9)	Plane continuum width. If left blank, E(9) defaults to unit-width.
11-20	E(10)	Hourglass viscosity.
21-30	E(11)	Artificial viscosity.
31-40	E(12)	Effective strain-rate amplitude parameter.
41-50	E(13)	Reference strain-rate.
51-60	E(14)	Plane-stress or plane-strain flag. E(14)=11.0: plane-stress material law. E(14)=12.0: plane-strain material law.
61-70	E(15)	blank.
71-80	E(16)	Number of Gaussian integration points along one direction. If E(16) is left blank, default value is 1.0.

INCLUDE ONLY IF USING PLANE CONTINUUM ELEMENTS

Card Type IVd-2 Material Property Card (8E10.4) Plane Continuum Element

1	6	11	16	21	26	31	36
E(17)		E(18)		E(19)		E(20)	
41	46	51	56	61	66	71	76
E(21)		E(22)		E(23)		E(24)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(17)	Temperature-value for curve 1 of piecewise linear stress-strain law.
11-20	E(18)	Young's modulus for temperature value 1.
21-30	E(19)	Strain value for point 1.
31-40	E(20)	Stress value for point 1.
41-50	E(21)	Strain value for point 2.
51-60	E(22)	Stress value for point 2.
61-70	E(23)	Strain value for point 3.
71-80	E(24)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVd-2 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

INCLUDE ONLY IF USING PLANE CONCRETE CONTINUUM ELEMENTS

Card Type IVa-3 Material Property Card (215) Plane Concrete Element

1 6 11 16 21 26 31 36

| MTYP | LTYT |

41 46 51 56 61 66 71 76

Columns

Variable

Description

1-5

MTYP

Material type number.

6-10

LTYT

Element type number.
LTYT=3: plane concrete element.

INCLUDE ONLY IF USING PLANE CONCRETE CONTINUUM ELEMENTS

Card Type IVb-3 Material Property Card (8E10.4) Plane Concrete Element

1	6	11	16	21	26	31	36
E(1)		E(2)		E(3)		E(4)	
41	46	51	56	61	66	71	76
E(5)		E(6)		E(7)		E(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(1)	Mass-density.
11-20	E(2)	Poisson's ratio.
21-30	E(3)	Coefficient of thermal-expansion.*
31-40	E(4)	Stress-dependent thermal expansion coefficient.
41-50	E(5)	Thermal conductivity.*
51-60	E(6)	Specific heat.*
61-70	E(7)	Number of temperature curves input for this material on Card Type IVd-3. If E(7) is left blank, default value is 1.0.
71-80	E(8)	Number of stress-strain pairs defining the piecewise linear stress-strain curve. Each temperature curve must have this number of pairs.

*Property variation with temperature may be imposed by prescribing a negative value in the E-field and providing the temperature-property data at the end of the material cards (Card Type IVi-3 and IVj-3).

INCLUDE ONLY IF USING PLANE CONCRETE CONTINUUM ELEMENTS

Card Type IVC-3 Material Property Card (8E10.4) Plane Concrete Element

1	6	11	16	21	26	31	36
E(9)		E(10)		E(11)		E(12)	
41	46	51	56	61	66	71	76
E(13)		E(14)		E(15)		E(16)	

Columns	Variable	Description
1-10	E(9)	Plane concrete width. If left blank, E(9) defaults to unit-width.
11-20	E(10)	Hourglass viscosity (0.13 recommended for the Belytschko et al. method and any negative value for the Kosloff and Frazier method).
21-30	E(11)	Artificial viscosity.
31-40	E(12)	Mixed hardening parameter. E(12)=0.0: kinematic hardening. 0.0<E(12)<1.0: mixed hardening. E(12)=1.0: isotropic hardening.
41-50	E(13)	Flow rule parameter. E(13)=0.0: associated flow rule (Prager's). E(13)=1.0: nonassociated flow rule (Prager's). E(13)=2.0: associated flow rule (Ziegler's). E(13)=3.0: nonassociated flow rule (Ziegler's).
51-60	E(14)	Plane-stress or plane-strain flag. E(14)=11.0: plane-stress material (first constitutive model). E(14)=12.0: plane-strain material (first constitutive model). E(14)=21.0: plane-stress material (second constitutive model). E(14)=22.0: plane-strain material (second constitutive model).

(Continue)

Card Type IVc-3 (Continued)

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
61-70	E(15)	<p>Failure surface flag.</p> <p>E(15)=0.0: default values (FC=1.0, FT=0.1, FBC=1.15, FPC=0.8, FCC=4.2, GFT=0.0* and GFC=0.0* for the first constitutive model while FC=1.0, FT=0.1, FBC=1.15, FHC=1.3, MUMAX=0.65, LAM1=140 and LAM3/LAM2=10.0 for the second constitutive model).</p> <p>E(15)=1.0: values to be specified at the end of stress-strain pairs (Card Type IVE-3 or IVf-3).</p> <p>E(15)=2.0: pertains to condition E(15)=0.0 with reinforcement (Card Type IVg-3 and IVh-3).</p> <p>E(15)=3.0: pertains to condition E(15)=1.0 with reinforcement (Card Type IVE-3 or IVf-3 with IVg-3 and IVh-3).</p>
71-80	E(16)	<p>Number of Gaussian integration points along one direction (1.0 recommended for cracking).#</p> <p>If E(16) is left blank, default value is 1.0.</p>

*These values imply sudden cracking and crushing for the constitutive model.

#Extensively cracked concrete has problems with hourglass control for one point Gaussian integration and stress locking for two point. This is presently handled by specifying as input one point Gaussian integration and zero hourglass viscosity. This stipulates that an average (one point) material stiffness is employed with two point Gaussian integration to avoid the hourglassing and stress locking conditions.

INCLUDE ONLY IF USING PLANE CONCRETE CONTINUUM ELEMENTS

Card Type IVd-3 Material Property Card (8E10.4) Plane Concrete Element

1	6	11	16	21	26	31	36
E(17)		E(18)		E(19)		E(20)	
41	46	51	56	61	66	71	76
E(21)		E(22)		E(23)		E(24)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(17)	Temperature-value for curve 1 of piecewise linear stress-strain law.
11-20	E(18)	Young's modulus for temperature value 1.
21-30	E(19)	Strain value for point 1.
31-40	E(20)	Stress value for point 1.
41-50	E(21)	Strain value for point 2.
51-60	E(22)	Stress value for point 2.
61-70	E(23)	Strain value for point 3.
71-80	E(24)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVd-3 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

If the first constitutive model is used, set the last stress (corresponding to very large strain) equal to the maximum strength of the material under uniaxial compression; if the second constitutive model is used, read in the stress-strain coordinates of the softening portion of the material under uniaxial compression (the program will automatically calculate the exponential coefficient LAM2).

INCLUDE ONLY IF USING PLANE CONCRETE ELEMENTS WITH $E(14) < 20.0$ AND $E(15) = 1.0$

Card Type IVE-3 Material Property Card (7E10.4) Plane Concrete Element

1	6	11	16	21	26	31	36
FC		FT		FBC		FPC	
41	46	51	56	61	66	71	76
FCC		GFT		GFC			

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	FC	Normalized uniaxial compressive strength. ($S1=S2=0.0$ and $S3=-FC$).
11-20	FT	Normalized uniaxial tensile strength. ($S1=FT$ and $S2=S3=0.0$).
21-30	FBC	Normalized biaxial compressive strength. ($S1=0.0$ and $S2=S3=-FBC$).
31-40	FPC	Normalized triaxial compressive strength. ($S1=S2=-FPC$ and $S3=-FPC$ with $ FCC >> FPC $).
41-50	FCC	Normalized triaxial compressive strength. ($S1=S2=-FPC$ and $S3=-FCC$ with $ FCC >> FPC $).
51-60	GFT	Fracture energy (force/length) in tension.*
61-70	GFC	Fracture energy (force/length) in compression.*

Absolute values for compression and tension are used in this input. Normalization is with respect to uniaxial compressive strength; i.e., $FC=1.0$. $S1$, $S2$ and $S3$ are the principal stresses.

*By inputting negative values for GFT and/or GFC (GFT and GFC are now defined as the complete failure strains), the strains for complete failure can be input directly instead of being calculated by the code from the fracture energies. Typical values are approximately five times the peak strains.

INCLUDE ONLY IF USING PLANE CONCRETE ELEMENTS WITH $E(14) > 20.0$ AND $E(15) = 1.0$

Card Type IVf-3 Material Property Card (7E10.4) Plane Concrete Element

1	6	11	16	21	26	31	36
FC		FT		FBC		FHC	
41	46	51	56	61	66	71	76
MUMAX		LAM1		LAM3/LAM2			

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	FC	Uniaxial compressive strength. ($S_1=S_2=0.0$ and $S_3=-FC$. Program will calculate FC from the stress-strain input).
11-20	FT	Normalized uniaxial tensile strength. ($S_1=FT$ and $S_2=S_3=0.0$).
21-30	FBC	Normalized biaxial compressive strength. ($S_1=0.0$ and $S_2=S_3=-FBC$).
31-40	FHC	Normalized elastic limit for hydrostatic compression. ($S_1=S_2=S_3=-FHC$).
41-50	MUMAX	Ratio defining the maximum point on the ellipse in the meridian plane. (Use $0.5 < MUMAX \leq 0.66$).
51-60	LAM1	Hardening coefficient.
61-70	LAM3/LAM2	Ratio for uniaxial tension/compression softening.

Normalization is with respect to uniaxial compressive strength (S_1 , S_2 and S_3 are the principal stresses).

INCLUDE ONLY IF USING PLANE CONCRETE CONTINUUM ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVg-3 Material Property Card (4E10.4) Plane Concrete Element

1	6	11	16	21	26	31	36
DENS		THEXP		NTR		NSR	
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	DENS	Mass-density for the reinforcement.
11-20	THEXP	Coefficient of thermal-expansion* for the reinforcement.
21-30	NTR	Number of temperature curves for the reinforcement.
31-40	NSR	Number of strain-stress pairs.

*Property variation with temperature may be imposed by prescribing a negative value for THEXP and providing the temperature-property data at the end of the material cards (Card Type IVi-3 and IVj-3).

INCLUDE ONLY IF USING PLANE CONCRETE CONTINUUM ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVh-3 Material Property Card (8E10.4) Plane Concrete Element

1	6	11	16	21	26	31	36
ER(1)		ER(2)		ER(3)		ER(4)	
41	46	51	56	61	66	71	76
ER(5)		ER(6)		ER(7)		ER(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	ER(1)	Temperature-value for curve 1 of piecewise linear stress-strain law representing the reinforcement.
11-20	ER(2)	Young's modulus for temperature value 1.
21-30	ER(3)	Strain value for point 1.
31-40	ER(4)	Stress value for point 1.
41-50	ER(5)	Strain value for point 2.
51-60	ER(6)	Stress value for point 2.
61-70	ER(7)	Strain value for point 3.
71-80	ER(8)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVh-3 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

INCLUDE ONLY IF USING PLANE CONCRETE CONTINUUM ELEMENTS WITH THERMAL VARIATION

Card Type IVi-3 Material Property Card (E10.4) Plane Concrete Element

1	6	11	16	21	26	31	36

NPP							

41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	NPP	Number of temperature-property points.

INCLUDE ONLY IF USING PLANE CONCRETE CONTINUUM ELEMENTS WITH THERMAL VARIATION

Card Type IVj-3 Material Property Card (8E10.4) Plane Concrete Element

1	6	11	16	21	26	31	36
EP(1)		EP(2)		EP(3)		EP(4)	
41	46	51	56	61	66	71	76
EP(5)		EP(6)		EP(7)		EP(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	EP(1)	Temperature value for point 1.
11-20	EP(2)	Property value for point 1.
21-30	EP(3)	Temperature value for point 2.
31-40	EP(4)	Property value for point 2.
41-50	EP(5)	Temperature value for point 3.
51-60	EP(6)	Property value for point 3.
61-70	EP(7)	Temperature value for point 4.
71-80	EP(8)	Property value for point 4.

If less than four points desired, leave the field blank.

Additional Type IVj-3 cards may be used if more temperature-property points are desired to describe the property.

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS

Card Type IVa-4 Material Property Card (2I5) Concrete Beam Element

1 6 11 16 21 26 31 36

MTYP	LTYP						
------	------	--	--	--	--	--	--

41 46 51 56 61 66 71 76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	MTYP	Material type number.
6-10	LTYP	Element type number. LTYP=4: concrete beam element.

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS

Card Type IVb-4 Material Property Card (8E10.4) Concrete Beam Element

1	6	11	16	21	26	31	36
E(1)		E(2)		E(3)		E(4)	
41	46	51	56	61	66	71	76
E(5)		E(6)		E(7)		E(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(1)	Mass-density.
11-20	E(2)	Poisson's ratio.
21-30	E(3)	Coefficient of thermal-expansion.*
31-40	E(4)	Stress-dependent thermal expansion coefficient.
41-50	E(5)	Thermal conductivity.*
51-60	E(6)	Specific heat.*
61-70	E(7)	Number of temperature curves input for this material on Card Type IVd-4. If E(7) is left blank, default value is 1.0.
71-80	E(8)	Number of stress-strain pairs defining the piecewise linear stress-strain curve. Each temperature curve must have this number of pairs.

*Property variation with temperature may be imposed by prescribing a negative value in the E-field and providing the temperature-property data at the end of the material cards (Card Type IVk-4 and IVl-4).

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS

Card Type IVc-4 Material Property Card (8E10.4) Concrete Beam Element

1	6	11	16	21	26	31	36
E(9)		E(10)		E(11)		E(12)	
41	46	51	56	61	66	71	76
E(13)		E(14)		E(15)		E(16)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(9)	Beam width. If left blank, E(9) defaults to unit-width.
11-20	E(10)	Beam thickness (bar area). Input required for NDGREE=3 or bar option. For NDGREE=2 option, thickness can be either specified in dummy node data on Card Type V or input in E(10).
21-30	E(11)	Artificial viscosity.
31-40	E(12)	Mixed hardening parameter. E(12)=0.0: kinematic hardening. 0.0<E(12)<1.0: mixed hardening. E(12)=1.0: isotropic hardening.
41-50	E(13)	Flow rule parameter. E(13)=0.0: associated flow rule (Prager's). E(13)=1.0: nonassociated flow rule (Prager's). E(13)=2.0: associated flow rule (Ziegler's). E(13)=3.0: nonassociated flow rule (Ziegler's).
51-60	E(14)	Plane-stress or plane-strain flag. E(14)=11.0: plane-stress material (first constitutive model). E(14)=12.0: plane-strain material (first constitutive model). E(14)=21.0: plane-stress material (second constitutive model). E(14)=22.0: plane-strain material (second constitutive model).

(Continue)

Card Type IVc-4 (Continued)

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
61-70	E(15)	<p>Failure surface flag.</p> <p>E(15)=0.0: default values (FC=1.0, FT=0.1, FBC=1.15, FPC=0.8, FCC=4.2, GFT=0.0* and GFC=0.0* for the first constitutive model while FC=1.0, FT=0.1, FBC=1.15, FHC=1.3, MUMAX=0.65, LAM1=140 and LAM3/LAM2=10.0 for the second constitutive model).</p> <p>E(15)=1.0: values to be specified at the end of stress-strain pairs (Card Type IVe-4 or IVf-4).</p> <p>E(15)=2.0: pertains to condition E(15)=0.0 with reinforcement (Card Type IVg-4 and IVh-4).</p> <p>E(15)=3.0: pertains to condition E(15)=1.0 with reinforcement (Card Type IVe-4 or IVf-4 with IVg-4 and IVh-4).</p>
71-80	E(16)	<p>Number of integration points through thickness.</p> <p>If E(16) is left blank, default value is 5.0.</p> <p>If not left blank, $E(16) \geq 2.0$.</p> <p>For bar option, set $E(16)=1.0$.</p>

*These values imply sudden cracking and crushing for the constitutive model.

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS

Card Type IVd-4 Material Property Card (8E10.4) Concrete Beam Element

1	6	11	16	21	26	31	36
E(17)		E(18)		E(19)		E(20)	
41	46	51	56	61	66	71	76
E(21)		E(22)		E(23)		E(24)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(17)	Temperature-value for curve 1 of piecewise linear stress-strain law.
11-20	E(18)	Young's modulus for temperature value 1.
21-30	E(19)	Strain value for point 1.
31-40	E(20)	Stress value for point 1.
41-50	E(21)	Strain value for point 2.
51-60	E(22)	Stress value for point 2.
61-70	E(23)	Strain value for point 3.
71-80	E(24)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVd-4 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

If the first constitutive model is used, set the last stress (corresponding to very large strain) equal to the maximum strength of the material under uniaxial compression; if the second constitutive model is used, read in the stress-strain coordinates of the softening portion of the material under uniaxial compression (the program will automatically calculate the exponential coefficient LAM2).

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS WITH $E(14) < 20.0$ AND $E(15) = 1.0$

Card Type IVe-4 Material Property Card (7E10.4) Concrete Beam Element

1	6	11	16	21	26	31	36
FC		FT		FBC		FPC	
41	46	51	56	61	66	71	76
FCC		GFT		GFC			

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	FC	Normalized uniaxial compressive strength. ($S1=S2=0.0$ and $S3=-FC$).
11-20	FT	Normalized uniaxial tensile strength. ($S1=FT$ and $S2=S3=0.0$).
21-30	FBC	Normalized biaxial compressive strength. ($S1=0.0$ and $S2=S3=-FBC$).
31-40	FPC	Normalized triaxial compressive strength. ($S1=S2=-FPC$ and $S3=-FCC$ with $ FCC >> FPC $).
41-50	FCC	Normalized triaxial compressive strength. ($S1=S2=-FPC$ and $S3=-FCC$ with $ FCC >> FPC $).
51-60	GFT	Fracture energy (force/length) in tension.*
61-70	GFC	Fracture energy (force/length) in compression.*

Absolute values for compression and tension are used in this input. Normalization is with respect to uniaxial compressive strength; i.e., $FC=1.0$. $S1$, $S2$ and $S3$ are the principal stresses.

*By inputting negative values for GFT and/or GFC (GFT and GFC are now defined as the complete failure strains), the strains for complete failure can be input directly instead of being calculated by the code from the fracture energies. Typical values are approximately five times the peak strains.

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS WITH $E(14) > 20.0$ AND $E(15) = 1.0$

Card Type IVf-4 Material Property Card (7E10.4) Concrete Beam Element

1	6	11	16	21	26	31	36
FC		FT		FBC		FHC	
41	46	51	56	61	66	71	76
MUMAX		LAM1		LAM3/LAM2			

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	FC	Uniaxial compressive strength. ($S1=S2=0.0$ and $S3=-FC$. Program will calculate FC from the stress-strain input).
11-20	FT	Normalized uniaxial tensile strength. ($S1=FT$ and $S2=S3=0.0$).
21-30	FBC	Normalized biaxial compressive strength. ($S1=0.0$ and $S2=S3=-FBC$).
31-40	FHC	Normalized elastic limit for hydrostatic compression. ($S1=S2=S3=-FHC$).
41-50	MUMAX	Ratio defining the maximum point on the ellipse in the meridian plane. (Use $0.5 < MUMAX \leq 0.66$).
51-60	LAM1	Hardening coefficient.
61-70	LAM3/LAM2	Ratio for uniaxial tension/compression softening.

Normalization is with respect to uniaxial compressive strength ($S1$, $S2$ and $S3$ are the principal stresses).

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVg-4 Material Property Card (4E10.4) Concrete Beam Element

1	6	11	16	21	26	31	36
DENS		THEXP		NTR		NSR	
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	DENS	Mass-density for the reinforcement.
11-20	THEXP	Coefficient of thermal-expansion* for the reinforcement.
21-30	NTR	Number of temperature curves for the reinforcement.
31-40	NSR	Number of strain-stress pairs.

*Property variation with temperature may be imposed by prescribing a negative value for THEXP and providing the temperature-property data at the end of the material cards (Card Type IVk-4 and IVl-4).

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVh-4 Material Property Card (8E10.4) Concrete Beam Element

1	6	11	16	21	26	31	36
ER(1)		ER(2)		ER(3)		ER(4)	
41	46	51	56	61	66	71	76
ER(5)		ER(6)		ER(7)		ER(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	ER(1)	Temperature-value for curve 1 of piecewise linear stress-strain law representing the reinforcement.
11-20	ER(2)	Young's modulus for temperature value 1.
21-30	ER(3)	Strain value for point 1.
31-40	ER(4)	Stress value for point 1.
41-50	ER(5)	Strain value for point 2.
51-60	ER(6)	Stress value for point 2.
61-70	ER(7)	Strain value for point 3.
71-80	ER(8)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVh-4 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVi-4 Material Property Card (E10.4) Concrete Beam Element

1 6 11 16 21 26 31 36

| NRB

41 46 51 56 61 66 71 76

Columns

Variable

Description

1-10

NRB

Number of reinforcing layers.

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVj-4 Material Property Card (3E10.4) Concrete Beam Element

1	6	11	16	21	26	31	36
AREA		ARM		ANG			
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	AREA	Area of reinforcing bar in the specified layer. (Use AREA=-AREA if this layer represents shear ties).
11-20	ARM	Distance of specified reinforcing layer from local x-axis.* (Use negative value if the layer is in a negative direction from the local x-axis).
21-30	ANG	Identification of reinforcement. ANG=0.0: reinforcement parallel to local x-axis. If negative AREA, angular alignment of shear tie layer with respect to the beam's local x-axis. 0.0<ANG<90.0: angular alignment of shear tie.

*The local x-axis is defined by the element connectivity (see Card Type Xc).

Use as many cards as there are layers specified by NRB on Card Type IVi-4.

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS WITH THERMAL VARIATION

Card Type IVk-4 Material Property Card (E10.4) Concrete Beam Element

1 6 11 16 21 26 31 36

+-----+
| NPP |
+-----+

41 46 51 56 61 66 71 76

+-----+
|
+-----+ColumnsVariableDescription

1-10

NPP

Number of temperature-property points.

INCLUDE ONLY IF USING CONCRETE BEAM ELEMENTS WITH THERMAL VARIATION

Card Type IV1-4 Material Property Card (8E10.4) Concrete Beam Element

1	6	11	16	21	26	31	36
+-----+-----+-----+-----+		+-----+-----+-----+-----+		+-----+-----+-----+-----+		+-----+-----+-----+-----+	
EP(1)		EP(2)		EP(3)		EP(4)	
+-----+-----+-----+-----+		+-----+-----+-----+-----+		+-----+-----+-----+-----+		+-----+-----+-----+-----+	
41	46	51	56	61	66	71	76
+-----+-----+-----+-----+		+-----+-----+-----+-----+		+-----+-----+-----+-----+		+-----+-----+-----+-----+	
EP(5)		EP(6)		EP(7)		EP(8)	
+-----+-----+-----+-----+		+-----+-----+-----+-----+		+-----+-----+-----+-----+		+-----+-----+-----+-----+	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	EP(1)	Temperature value for point 1.
11-20	EP(2)	Property value for point 1.
21-30	EP(3)	Temperature value for point 2.
31-40	EP(4)	Property value for point 2.
41-50	EP(5)	Temperature value for point 3.
51-60	EP(6)	Property value for point 3.
61-70	EP(7)	Temperature value for point 4.
71-80	EP(8)	Property value for point 4.

If less than four points desired, leave the field blank.

Additional Type IV1-4 cards may be used if more temperature-property points are desired to describe the property.

INCLUDE ONLY IF USING SHELL ELEMENTS

Card Type IVa-11 Material Property Card (2I5) Shell Element

1	6	11	16	21	26	31	36
MTYP	LTP						
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	MTYP	Material type number.
6-10	LTP	Element type number. LTP=11: shell element.

INCLUDE ONLY IF USING SHELL ELEMENTS

Card Type IVb-11 Material Property Card (8E10.4) Shell Element

1	6	11	16	21	26	31	36
E(1)		E(2)		E(3)		E(4)	
41	46	51	56	61	66	71	76
E(5)		E(6)		E(7)		E(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(1)	Mass-density.
11-20	E(2)	blank.
21-30	E(3)	Poisson's ratio.
31-40	E(4)	Coefficient of thermal-expansion.
41-50	E(5)	Thermal conductivity.
51-60	E(6)	Specific heat.
61-70	E(7)	Number of temperature curves input for this material on Card Type IVd-11. If E(7) is left blank, default value is 1.0.
71-80	E(8)	Number of stress-strain pairs defining the piecewise linear stress-strain curve. Each temperature curve must have this number of pairs.

INCLUDE ONLY IF USING SHELL ELEMENTS

Card Type IVc-11 Material Property Card (8E10.4) Shell Element

1	6	11	16	21	26	31	36
E(9)		E(10)		E(11)		E(12)	
41	46	51	56	61	66	71	76
E(13)		E(14)		E(15)		E(16)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(9)	blank.
11-20	E(10)	Shell thickness (ring area). Input required for NDGREE=3, membrane or ring option. For NDGREE=2 option, thickness can be either specified in dummy node data on Card Type V or input in E(10).
21-30	E(11)	Artificial viscosity.
31-40	E(12)	Effective strain-rate amplitude parameter.
41-50	E(13)	Reference strain-rate.
51-60	E(14)	Axisymmetric flag. E(14)=3.0: axisymmetric material law.
61-70	E(15)	Number of integration points along length. If E(15) is left blank, default value is 2.0. For ring option, set E(15)=1.0. For membrane option, set E(15)=2.0.
71-80	E(16)	Number of integration points through thickness. If E(16) is left blank, default value is 5.0. If not left blank, E(16)≥2.0. For ring or membrane option, set E(16)=1.0.

INCLUDE ONLY IF USING SHELL ELEMENTS

Card Type IVd-11 Material Property Card (8E10.4) Shell Element

1	6	11	16	21	26	31	36
E(17)		E(18)		E(19)		E(20)	
41	46	51	56	61	66	71	76
E(21)		E(22)		E(23)		E(24)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(17)	Temperature-value for curve 1 of piecewise linear stress-strain law.
11-20	E(18)	Young's modulus for temperature value 1.
21-30	E(19)	Strain value for point 1.
31-40	E(20)	Stress value for point 1.
41-50	E(21)	Strain value for point 2.
51-60	E(22)	Stress value for point 2.
61-70	E(23)	Strain value for point 3.
71-80	E(24)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVd-11 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

INCLUDE ONLY IF USING AXISYMMETRIC CONTINUUM ELEMENTS

Card Type IVa-12 Material Property Card (215) Axisymmetric Continuum Element

1 6 11 16 21 26 31 36

| MTYP | LTYP |

41 46 51 56 61 66 71 76

Columns

Variable

Description

1-5

MTYP

Material type number.

6-10

LTYP

Element type number.
LTYP=12: axisymmetric continuum element.

INCLUDE ONLY IF USING AXISYMMETRIC CONTINUUM ELEMENTS

Card Type IVb-12 Material Property Card (8E10.4) Axisymmetric Continuum Element

1	6	11	16	21	26	31	36
E(1)		E(2)		E(3)		E(4)	
41	46	51	56	61	66	71	76
E(5)		E(6)		E(7)		E(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(1)	Mass-density.
11-20	E(2)	blank.
21-30	E(3)	Poisson's ratio.
31-40	E(4)	Coefficient of thermal-expansion.
41-50	E(5)	Thermal conductivity.
51-60	E(6)	Specific heat.
61-70	E(7)	Number of temperature curves input for this material on Card Type IVd-12. If E(7) is left blank, default value is 1.0.
71-80	E(8)	Number of stress-strain pairs defining the piecewise linear stress-strain curve. Each temperature curve must have this number of pairs.

INCLUDE ONLY IF USING AXISYMMETRIC CONTINUUM ELEMENTS

Card Type IVc-12 Material Property Card (8E10.4) Axisymmetric Continuum Element

1	6	11	16	21	26	31	36
E(9)		E(10)		E(11)		E(12)	
41	46	51	56	61	66	71	76
E(13)		E(14)		E(15)		E(16)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(9)	blank.
11-20	E(10)	Hourglass viscosity.
21-30	E(11)	Artificial viscosity.
31-40	E(12)	Effective strain-rate amplitude parameter.
41-50	E(13)	Reference strain-rate.
51-60	E(14)	Axisymmetric flag. E(14)=13.0: axisymmetric material law.
61-70	E(15)	blank.
71-80	E(16)	Number of Gaussian integration points along one direction. If E(16) is left blank, default value is 1.0.

INCLUDE ONLY IF USING AXISYMMETRIC CONTINUUM ELEMENTS

Card Type IVd-12 Material Property Card (8E10.4) Axisymmetric Continuum Element

1	6	11	16	21	26	31	36
E(17)		E(18)		E(19)		E(20)	
41	46	51	56	61	66	71	76
E(21)		E(22)		E(23)		E(24)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(17)	Temperature-value for curve 1 of piecewise linear stress-strain law.
11-20	E(18)	Young's modulus for temperature value 1.
21-30	E(19)	Strain value for point 1.
31-40	E(20)	Stress value for point 1.
41-50	E(21)	Strain value for point 2.
51-60	E(22)	Stress value for point 2.
61-70	E(23)	Strain value for point 3.
71-80	E(24)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVd-12 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

INCLUDE ONLY IF USING AXISYMMETRIC CONCRETE CONTINUUM ELEMENTS

Card Type IVa-13 Material Property Card (215) Axisymmetric Concrete Element

1 6 11 16 21 26 31 36

| MTYP | LTPY |

41 46 51 56 61 66 71 76

Columns

Variable

Description

1-5

MTYP

Material type number.

6-10

LTPY

Element type number.
LTPY=13: axisymmetric concrete element.

INCLUDE ONLY IF USING AXISYMMETRIC CONCRETE CONTINUUM ELEMENTS

Card Type IVb-13 Material Property Card (8E10.4) Axisymmetric Concrete Element

1	6	11	16	21	26	31	36
E(1)		E(2)		E(3)		E(4)	
41	46	51	56	61	66	71	76
E(5)		E(6)		E(7)		E(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(1)	Mass-density.
11-20	E(2)	Poisson's ratio.
21-30	E(3)	Coefficient of thermal-expansion.*
31-40	E(4)	Stress-dependent thermal expansion coefficient.
41-50	E(5)	Thermal conductivity.*
51-60	E(6)	Specific heat.*
61-70	E(7)	Number of temperature curves input for this material on Card Type IVd-13. If E(7) is left blank, default value is 1.0.
71-80	E(8)	Number of stress-strain pairs defining the piecewise linear stress-strain curve. Each temperature curve must have this number of pairs.

*Property variation with temperature may be imposed by prescribing a negative value in the E-field and providing the temperature-property data at the end of the material cards (Card Type IVi-13 and IVj-13).

INCLUDE ONLY IF USING AXISYMMETRIC CONCRETE CONTINUUM ELEMENTS

Card Type IVc-13 Material Property Card (8E10.4) Axisymmetric Concrete Element

1	6	11	16	21	26	31	36
E(9)		E(10)		E(11)		E(12)	
41	46	51	56	61	66	71	76
E(13)		E(14)		E(15)		E(16)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(9)	blank.
11-20	E(10)	Hourglass viscosity (0.13 recommended for the Belytschko et al. method and any negative value for the Kosloff and Frazier method).
21-30	E(11)	Artificial viscosity.
31-40	E(12)	Mixed hardening parameter. E(12)=0.0: kinematic hardening. 0.0<E(12)<1.0: mixed hardening. E(12)=1.0: isotropic hardening.
41-50	E(13)	Flow rule parameter. E(13)=0.0: associated flow rule (Prager's). E(13)=1.0: nonassociated flow rule (Prager's). E(13)=2.0: associated flow rule (Ziegler's). E(13)=3.0: nonassociated flow rule (Ziegler's).
51-60	E(14)	Axisymmetric flag. E(14)=13.0: axisymmetric material law (first constitutive model). E(14)=23.0: axisymmetric material law (second constitutive model).

(Continue)

Card Type IVc-13 (Continued)

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
61-70	E(15)	<p>Failure surface flag.</p> <p>E(15)=0.0: default values (FC=1.0, FT=0.1, FBC=1.15, FPC=0.8, FCC=4.2, GFT=0.0* and GFC=0.0* for the first constitutive model while FC=1.0, FT=0.1, FBC=1.15, FHC=1.3, MUMAX=0.65, LAM1=140 and LAM3/LAM2=10.0 for the second constitutive model).</p> <p>E(15)=1.0: values to be specified at the end of stress-strain pairs (Card Type IVE-13 or IVf-13).</p> <p>E(15)=2.0: pertains to condition E(15)=0.0 with reinforcement (Card Type IVg-13 and IVh-13).</p> <p>E(15)=3.0: pertains to condition E(15)=1.0 with reinforcement (Card Type IVE-13 or IVf-13 with IVg-13 and IVh-13).</p>
71-80	E(16)	<p>Number of Gaussian integration points along one direction (1.0 recommended for cracking).#</p> <p>If E(16) is left blank, default value is 1.0.</p>

*These values imply sudden cracking and crushing for the constitutive model.

#Extensively cracked concrete has problems with hourglass control for one point Gaussian integration and stress locking for two point. This is presently handled by specifying as input one point Gaussian integration and zero hourglass viscosity. This stipulates that an average (one point) material stiffness is employed with two point Gaussian integration to avoid the hourglassing and stress locking conditions.

INCLUDE ONLY IF USING AXISYMMETRIC CONCRETE CONTINUUM ELEMENTS

Card Type IVd-13 Material Property Card (8E10.4) Axisymmetric Concrete Element

1	6	11	16	21	26	31	36
E(17)		E(18)		E(19)		E(20)	
41	46	51	56	61	66	71	76
E(21)		E(22)		E(23)		E(24)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(17)	Temperature-value for curve 1 of piecewise linear stress-strain law.
11-20	E(18)	Young's modulus for temperature value 1.
21-30	E(19)	Strain value for point 1.
31-40	E(20)	Stress value for point 1.
41-50	E(21)	Strain value for point 2.
51-60	E(22)	Stress value for point 2.
61-70	E(23)	Strain value for point 3.
71-80	E(24)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVd-13 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

If the first constitutive model is used, set the last stress (corresponding to very large strain) equal to the maximum strength of the material under uniaxial compression; if the second constitutive model is used, read in the stress-strain coordinates of the softening portion of the material under uniaxial compression (the program will automatically calculate the exponential coefficient LAM2).

INCLUDE ONLY IF USING AXISYM CONCRETE ELEMENTS WITH $E(14) < 20.0$ AND $E(15) = 1.0$

Card Type IVE-13 Material Property Card (7E10.4) Axisymmetric Concrete Element

1	6	11	16	21	26	31	36
FC		FT		FBC		FPC	
41	46	51	56	61	66	71	76
FCC		GFT		GFC			

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	FC	Normalized uniaxial compressive strength. ($S1=S2=0.0$ and $S3=-FC$).
11-20	FT	Normalized uniaxial tensile strength. ($S1=FT$ and $S2=S3=0.0$).
21-30	FBC	Normalized biaxial compressive strength. ($S1=0.0$ and $S2=S3=-FBC$).
31-40	FPC	Normalized triaxial compressive strength. ($S1=S2=-FPC$ and $S3=-FCC$ with $ FCC >> FPC $).
41-50	FCC	Normalized triaxial compressive strength. ($S1=S2=-FPC$ and $S3=-FCC$ with $ FCC >> FPC $).
51-60	GFT	Fracture energy (force/length) in tension.*
61-70	GFC	Fracture energy (force/length) in compression.*

Absolute values for compression and tension are used in this input. Normalization is with respect to uniaxial compressive strength; i.e., $FC=1.0$. $S1$, $S2$ and $S3$ are the principal stresses.

*By inputting negative values for GFT and/or GFC (GFT and GFC are now defined as the complete failure strains), the strains for complete failure can be input directly instead of being calculated by the code from the fracture energies. Typical values are approximately five times the peak strains.

INCLUDE ONLY IF USING AXISYM CONCRETE ELEMENTS WITH $E(14) > 20.0$ AND $E(15) = 1.0$

Card Type IVf-13 Material Property Card (7E10.4) Axisymmetric Concrete Element

1	6	11	16	21	26	31	36
FC		FT		FBC		FHC	
41	46	51	56	61	66	71	76
MUM		LAM1		LAM3/LAM2			

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	FC	Uniaxial compressive strength. ($S1=S2=0.0$ and $S3=-FC$. Program will calculate FC from the stress-strain input).
11-20	FT	Normalized uniaxial tensile strength. ($S1=FT$ and $S2=S3=0.0$).
21-30	FBC	Normalized biaxial compressive strength. ($S1=0.0$ and $S2=S3=-FBC$).
31-40	FHC	Normalized elastic limit for hydrostatic compression. ($S1=S2=S3=-FHC$).
41-50	MUMAX	Ratio defining the maximum point on the ellipse in the meridian plane. (Use $0.5 < MUMAX \leq 0.66$).
51-60	LAM1	Hardening coefficient.
61-70	LAM3/LAM2	Ratio for uniaxial tension/compression softening.

Normalization is with respect to uniaxial compressive strength ($S1$, $S2$ and $S3$ are the principal stresses).

INCLUDE ONLY IF USING AXISYM CONCRETE CONTINUUM ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVg-13 Material Property Card (4E10.4) Axisymmetric Concrete Element

1	6	11	16	21	26	31	36
DENS		THEXP		NTR		NSR	
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	DENS	Mass-density for the reinforcement.
11-20	THEXP	Coefficient of thermal-expansion* for the reinforcement.
21-30	NTR	Number of temperature curves for the reinforcement.
31-40	NSR	Number of strain-stress pairs.

*Property variation with temperature may be imposed by prescribing a negative value for THEXP and providing the temperature-property data at the end of the material cards (Card Type IVi-13 and IVj-13).

INCLUDE ONLY IF USING AXISYM CONCRETE CONTINUUM ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVh-13 Material Property Card (8E10.4) Axisymmetric Concrete Element

1	6	11	16	21	26	31	36
ER(1)		ER(2)		ER(3)		ER(4)	
41	46	51	56	61	66	71	76
ER(5)		ER(6)		ER(7)		ER(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	ER(1)	Temperature-value for curve 1 of piecewise linear stress-strain law representing the reinforcement.
11-20	ER(2)	Young's modulus for temperature value 1.
21-30	ER(3)	Strain value for point 1.
31-40	ER(4)	Stress value for point 1.
41-50	ER(5)	Strain value for point 2.
51-60	ER(6)	Stress value for point 2.
61-70	ER(7)	Strain value for point 3.
71-80	ER(8)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVh-13 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

INCLUDE ONLY IF USING AXISYM CONCRETE CONTINUUM ELEMENTS WITH THERMAL VARIATION

Card Type IVi-13 Material Property Card (E10.4) Axisymmetric Concrete Element

1	6	11	16	21	26	31	36
NPP							
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	NPP	Number of temperature-property points.

INCLUDE ONLY IF USING AXISYM CONCRETE CONTINUUM ELEMENTS WITH THERMAL VARIATION

Card Type IVj-13 Material Property Card (8E10.4) Axisymmetric Concrete Element

1	6	11	16	21	26	31	36
EP(1)		EP(2)		EP(3)		EP(4)	
41	46	51	56	61	66	71	76
EP(5)		EP(6)		EP(7)		EP(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	EP(1)	Temperature value for point 1.
11-20	EP(2)	Property value for point 1.
21-30	EP(3)	Temperature value for point 2.
31-40	EP(4)	Property value for point 2.
41-50	EP(5)	Temperature value for point 3.
51-60	EP(6)	Property value for point 3.
61-70	EP(7)	Temperature value for point 4.
71-80	EP(8)	Property value for point 4.

If less than four points desired, leave the field blank.

Additional Type IVj-13 cards may be used if more temperature-property points are desired to describe the property.

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS

Card Type IVa-14 Material Property Card (2I5) Concrete Shell Element

1	6	11	16	21	26	31	36
MTYP		LTYP					
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	MTYP	Material type number.
6-10	LTYP	Element type number. LTYP=14: concrete shell element.

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS

Card Type IVb-14 Material Property Card (8E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
E(1)		E(2)		E(3)		E(4)	
41	46	51	56	61	66	71	76
E(5)		E(6)		E(7)		E(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(1)	Mass-density.
11-20	E(2)	Poisson's ratio.
21-30	E(3)	Coefficient of thermal-expansion.*
31-40	E(4)	Stress-dependent thermal expansion coefficient.
41-50	E(5)	Thermal conductivity.*
51-60	E(6)	Specific heat.*
61-70	E(7)	Number of temperature curves input for this material on Card Type IVd-14. If E(7) is left blank, default value is 1.0.
71-80	E(8)	Number of stress-strain pairs defining the piecewise linear stress-strain curve. Each temperature curve must have this number of pairs.

*Property variation with temperature may be imposed by prescribing a negative value in the E-field and providing the temperature-property data at the end of the material cards (Card Type IVk-14 and IVl-14).

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS

Card Type IVc-14 Material Property Card (8E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
E(9)		E(10)		E(11)		E(12)	
41	46	51	56	61	66	71	76
E(13)		E(14)		E(15)		E(16)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(9)	blank.
11-20	E(10)	Shell thickness (ring area). Input required for NDGREE=3, membrane or ring option. For NDGREE=2 option, thickness can be either specified in dummy node data on Card Type V or input in E(10).
21-30	E(11)	Artificial viscosity.
31-40	E(12)	Mixed hardening parameter. E(12)=0.0: kinematic hardening. 0.0<E(12)<1.0: mixed hardening. E(12)=1.0: isotropic hardening.
41-50	E(13)	Flow rule parameter. E(13)=0.0: associated flow rule (Prager's). E(13)=1.0: nonassociated flow rule (Prager's). E(13)=2.0: associated flow rule (Ziegler's). E(13)=3.0: nonassociated flow rule (Ziegler's).
51-60	E(14)	Axisymmetric flag. E(14)=13.0: axisymmetric material law (first constitutive model). E(14)=23.0: axisymmetric material law (second constitutive model).

(Continue)

Card Type IVc-14 (Continued)

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
61-70	E(15)	<p>Failure surface flag.</p> <p>E(15)=0.0: default values (FC=1.0, FT=0.1, FBC=1.15, FPC=0.8, FCC=4.2, GFT=0.0* and GFC=0.0* for the first constitutive model while FC=1.0, FT=0.1, FBC=1.15, FHC=1.3, MUMAX=0.65, LAM1=140 and LAM3/LAM2=10.0 for the second constitutive model).</p> <p>E(15)=1.0: values to be specified at the end of stress-strain pairs (Card Type IVe-14 or IVf-14).</p> <p>E(15)=2.0: pertains to condition E(15)=0.0 with reinforcement (Card Type IVg-14 and IVh-14).</p> <p>E(15)=3.0: pertains to condition E(15)=1.0 with reinforcement (Card Type IVe-14 or IVf-14 with IVg-14 and IVh-14).</p>
71-80	E(16)	<p>Number of integration points through thickness.</p> <p>If E(16) is left blank, default value is 5.0.</p> <p>If not left blank, E(16)≥2.0.</p> <p>For ring or membrane option, set E(16)=1.0.</p>

*These values imply sudden cracking and crushing for the constitutive model.

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS

Card Type IVd-14 Material Property Card (8E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
E(17)		E(18)		E(19)		E(20)	
41	46	51	56	61	66	71	76
E(21)		E(22)		E(23)		E(24)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	E(17)	Temperature-value for curve 1 of piecewise linear stress-strain law.
11-20	E(18)	Young's modulus for temperature value 1.
21-30	E(19)	Strain value for point 1.
31-40	E(20)	Stress value for point 1.
41-50	E(21)	Strain value for point 2.
51-60	E(22)	Stress value for point 2.
61-70	E(23)	Strain value for point 3.
71-80	E(24)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVd-14 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

If the first constitutive model is used, set the last stress (corresponding to very large strain) equal to the maximum strength of the material under uniaxial compression; if the second constitutive model is used, read in the stress-strain coordinates of the softening portion of the material under uniaxial compression (the program will automatically calculate the exponential coefficient LAM2).

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS WITH $E(14) < 20.0$ AND $E(15) = 1.0$

Card Type IVe-14 Material Property Card (7E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
FC		FT		FBC		FPC	
41	46	51	56	61	66	71	76
FCC		GFT		GFC			

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	FC	Normalized uniaxial compressive strength. ($S1=S2=0.0$ and $S3=-FC$).
11-20	FT	Normalized uniaxial tensile strength. ($S1=FT$ and $S2=S3=0.0$).
21-30	FBC	Normalized biaxial compressive strength. ($S1=0.0$ and $S2=S3=-FBC$).
31-40	FPC	Normalized triaxial compressive strength. ($S1=S2=-FPC$ and $S3=-FPC$ with $ FCC \gg FPC $).
41-50	FCC	Normalized triaxial compressive strength. ($S1=S2=-FPC$ and $S3=-FCC$ with $ FCC \gg FPC $).
51-60	GFT	Fracture energy (force/length) in tension.*
61-70	GFC	Fracture energy (force/length) in compression.*

Absolute values for compression and tension are used in this input. Normalization is with respect to uniaxial compressive strength; i.e., $FC=1.0$. $S1$, $S2$ and $S3$ are the principal stresses.

*By inputting negative values for GFT and/or GFC (GFT and GFC are now defined as the complete failure strains), the strains for complete failure can be input directly instead of being calculated by the code from the fracture energies. Typical values are approximately five times the peak strains.

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS WITH $E(14) > 20.0$ AND $E(15) = 1.0$

Card Type IVf-14 Material Property Card (7E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
FC		FT		FBC		FHC	
41	46	51	56	61	66	71	76
MUMAX		LAM1		LAM3/LAM2			

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	FC	Uniaxial compressive strength. ($S1=S2=0.0$ and $S3=-FC$. Program will calculate FC from the stress-strain input).
11-20	FT	Normalized unaxial tensile strength. ($S1=FT$ and $S2=S3=0.0$).
21-30	FBC	Normalized biaxial compressive strength. ($S1=0.0$ and $S2=S3=-FBC$).
31-40	FHC	Normalized elastic limit for hydrostatic compression. ($S1=S2=S3=-FHC$).
41-50	MUMAX	Ratio defining the maximum point on the ellipse in the meridian plane. (Use $0.5 < MUMAX \leq 0.66$).
51-60	LAM1	Hardening coefficient.
61-70	LAM3/LAM2	Ratio for uniaxial tension/compression softening.

Normalization is with respect to uniaxial compressive strength ($S1$, $S2$ and $S3$ are the principal stresses).

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVg-14 Material Property Card (4E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
DENS		THEXP		NTR		NSR	
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	DENS	Mass-density for the reinforcement.
11-20	THEXP	Coefficient of thermal-expansion* for the reinforcement.
21-30	NTR	Number of temperature curves for the reinforcement.
31-40	NSR	Number of strain-stress pairs.

*Property variation with temperature may be imposed by prescribing a negative value for THEXP and providing the temperature-property data at the end of the material cards (Card Type IVk-14 and IVl-14).

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVh-14 Material Property Card (8E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
ER(1)		ER(2)		ER(3)		ER(4)	
41	46	51	56	61	66	71	76
ER(5)		ER(6)		ER(7)		ER(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	ER(1)	Temperature-value for curve 1 of piecewise linear stress-strain law representing the reinforcement.
11-20	ER(2)	Young's modulus for temperature value 1.
21-30	ER(3)	Strain value for point 1.
31-40	ER(4)	Stress value for point 1.
41-50	ER(5)	Strain value for point 2.
51-60	ER(6)	Stress value for point 2.
61-70	ER(7)	Strain value for point 3.
71-80	ER(8)	Stress value for point 3.

If less than three points desired, leave the field blank.

Additional Type IVh-14 cards may be used if more stress-strain points are desired to describe the material or if more than one temperature curve is desired. No limit on number of points or temperature curves.

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVi-14 Material Property Card (E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
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NRB

41	46	51	56	61	66	71	76
----	----	----	----	----	----	----	----

Columns

Variable

Description

1-10

NRB

Number of reinforcing layers.

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS WITH $E(15) = 2.0$ OR 3.0

Card Type IVj-14 Material Property Card (3E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
AREA		ARM		ANG			
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	AREA	Area of reinforcing bar in the specified layer. (Use AREA=-AREA if this layer represents shear ties).
11-20	ARM	Distance of specified reinforcing layer from local x-axis.* (Use negative value if the layer is in a negative direction from the local x-axis).
21-30	ANG	Identification of reinforcement within the meridional-circumferential plane. ANG=0.0: meridional reinforcement. 0.0< ANG <90.0: 'seismic' reinforcement. ANG=90.0: hoop reinforcement. If negative AREA, angular alignment of shear tie layer with respect to the shell's local x-axis. 0.0<ANG<90.0: angular alignment of shear tie.

*The local x-axis is defined by the element connectivity (see Card Type Xc).

Use as many cards as there are layers specified by NRB on Card Type IVi-14.

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS WITH THERMAL VARIATION

Card Type IVk-14 Material Property Card (E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
+-----+-----+-----+-----+-----+-----+-----+-----+							
	NPP						
+-----+-----+-----+-----+-----+-----+-----+-----+							
41	46	51	56	61	66	71	76
+-----+-----+-----+-----+-----+-----+-----+-----+							
+-----+-----+-----+-----+-----+-----+-----+-----+							

Columns

Variable

Description

1-10

NPP

Number of temperature-property points.

INCLUDE ONLY IF USING CONCRETE SHELL ELEMENTS WITH THERMAL VARIATION

Card Type IV1-14 Material Property Card (8E10.4) Concrete Shell Element

1	6	11	16	21	26	31	36
EP(1)		EP(2)		EP(3)		EP(4)	
41	46	51	56	61	66	71	76
EP(5)		EP(6)		EP(7)		EP(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	EP(1)	Temperature value for point 1.
11-20	EP(2)	Property value for point 1.
21-30	EP(3)	Temperature value for point 2.
31-40	EP(4)	Property value for point 2.
41-50	EP(5)	Temperature value for point 3.
51-60	EP(6)	Property value for point 3.
61-70	EP(7)	Temperature value for point 4.
71-80	EP(8)	Property value for point 4.

If less than four points desired, leave the field blank.

Additional Type IV1-14 cards may be used if more temperature-property points are desired to describe the property.

INCLUDE FOR ALL DATA SETS

Card Type V Node Card (2I5,4E10.4)

1	6	11	16	21	26	31	36
N	M	XC(N)		YC(N)		XSHIFT	
41	46	51	56	61	66	71	76
YSHIFT							

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	N	Node number.
6-10	M	Coordinate system used for this sequence of nodes. Input may be a mix of cartesian and polar systems. M=0: cartesian (x, y) system. M=1: polar (r, θ) system.
11-20	XC(N)	x-coordinate (r-coordinate if polar).
21-30	YC(N)	y-coordinate (θ -coordinate if polar).
31-40	XSHIFT	x-coordinate value defining the local origin if using polar system input.
41-50	YSHIFT	y-coordinate value defining the local origin if using polar system input.

Nodal coordinates for nodes equispaced between two nodes will be automatically generated if the data cards for intermediate nodes are skipped. If nonequispaced nodes are to be generated, use KONTRL(3) (see Card Type III).

Coordinate values specified are those for the mid-plane surface of the beam or shell elements.

When using 2 degrees of freedom per node option, a 'dummy' node (for beam or shell elements only) is needed to store the rotation associated with the translation node. The value input for the coordinates would thus be left blank unless one wishes to input the thickness (see Card Type IVC-1) of the beam or shell here instead. The thickness would be specified through the XC(N) coordinate. In that case, the beam or shell thickness used will be the average of the thickness prescribed by the beam's or shell's two connecting nodes. If the thickness is specified here, it will override that specified on the material card (see Card Type IVC-1).

When using 3 degrees of freedom per node option (for beam or shell element only), the rotations at this node are the 3rd degree of freedom. The thickness of the beam element must then be stored in E(10) on the material card.

INCLUDE FOR ALL DATA SETS

Card Type VI

Element Card (12I5)

1	6	11	16	21	26	31	36
M	IX(1,M)	IX(2,M)	IX(3,M)	IX(4,M)	IX(5,M)	IX(6,M)	IX(7,M)
41	46	51	56	61	66	71	76
IX(8,M)	MTYP	LTYP	INCR				

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	M	Element number.
6-10	IX(1,M)	
11-15	IX(2,M)	
16-20	IX(3,M)	
21-25	IX(4,M)	Nodes describing the element (see element description below).
26-30	IX(5,M)	
31-35	IX(6,M)	
36-40	IX(7,M)	
41-45	IX(8,M)	See notes below.
46-50	MTYP	Material type number. See Card Type IVa.
51-55	LTYP	Element type number.
56-60	INCR	Increment to generate IX(J,M) for subsequent elements. If INCR is left blank, default is 1.

Beam or shell elements: IX(1,M) and IX(2,M) are displacement nodes always. For ring option, set IX(2,M)=IX(1,M). IX(3,M) and IX(4,M) are 'dummy' nodes which store rotations for IX(2,M) and IX(1,M), respectively, for NDGREE=2 option. IX(8,M) contains the load line number of a prestressing element specified by Card Type VIIa.

(Continue)

Card Type VI (Continued)

Continuum elements: IX(1,M), IX(2,M), IX(3,M) and IX(4,M) are displacement nodes. Element has only the 2 degrees of freedom per node option. Basically a quadrilateral shape. Triangular shape can be achieved by setting IX(4,M)=IX(3,M). Nodes describing element are connected counterclockwise. IX(5,M), IX(6,M), and IX(7,M) store the reinforcement ratios (multiplied by 10000) in the directions xi, eta, theta, respectively. IX(8,M) stores the angle (in degrees) of the xi-reinforcement with respect to the r-axis (xi, eta, theta are orthogonal axes of reinforcement).

Element cards which can be generated by adding INCR to all node numbers (those describing the element) of the previous element need not be included because they will be generated automatically. The last element card is needed however.

INCLUDE ONLY IF NUMDIS > 0 ON CARD TYPE II

Card Type VII Prescribed Displacement Card (I10,E10.4)

1	7	9	11	16	21	26	31	36
N	I	I	I	ANGLE				
41	46	51	56	61	66	71	76	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-6	N	Node number at which one or more displacement components are specified. Essential boundary conditions/support conditions.
7		blank.
8	I	x-displacement component.
9	I	y-displacement component.
10	I	Blank for 2 degree of freedom nodes. Angular-displacement component for 3 degree of freedom nodes. I=0: displacement component has no constraint. I=1: displacement component is zero. I=2: displacement component is prescribed a value in subroutine FREEFD.
11-20	ANGLE	Angle of rotation (in degrees), right-hand rule, of the node before application of the constraint. Angle of rotation is used if a displacement component other than the x or y component prescribed. Do not rotate rotation nodes that are to be constrained for NDGREE=2.

For NDGREE=2 beam or shell element 'dummy' nodes, constrain both components whenever the node is to be constrained against rotation.

Internal incrementing by one is done between two prescribed displacement nodes by placing a minus sign in column preceding the last node number.

Use as many cards as there are nodes having prescribed displacements.

INCLUDE FOR ALL DATA SETS

Card Type VIIIA Load Line Card (6I5,E10.4)

1	6	11	16	21	26	31	36
I	NDNOD	IVOL(1)	IVOL(2)	INT1	NPT	VINT	
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	I	Load line number.
6-10	NDNOD	Number of nodes* on load line.
11-15	IVOL(1)	External loading condition or property variation. IVOL(1)=0: initial velocity input. IVOL(1)=1: pressure-time (or increment) input. IVOL(1)=2: pressure-volume# input. IVOL(1)=3: force-time input (x-component). IVOL(1)=4: force-time input (y-component). IVOL(1)=5: displacement-time input (x-component). IVOL(1)=6: displacement-time input (y-component). IVOL(1)=7: temperature-time input. IVOL(1)=8: heat flux-time input. IVOL(1)=9: film coefficient-time input. IVOL(1)=10: internal heat source-time input. IVOL(1)=11: prestressing-time input.\$
16-20	IVOL(2)	Geometry of external loading. IVOL(2)=0: plane geometry (all elements). IVOL(2)=1: axisymmetric geometry (velocity-strain concrete or continuum element, 1 point Gaussian). IVOL(2)=2: axisymmetric geometry (velocity-strain concrete or continuum element, 2 point Gaussian). IVOL(2)=3: axisymmetric geometry (shell element).
21-25	INT1	Node interval: if the node numbers of a load line can be generated by adding INT1 each time, read in INT1 and skip all node data but the first node on Card Type VIIIC, otherwise INT1=0 or blank.

(Continue)

Card Type VIIIa (Continued)

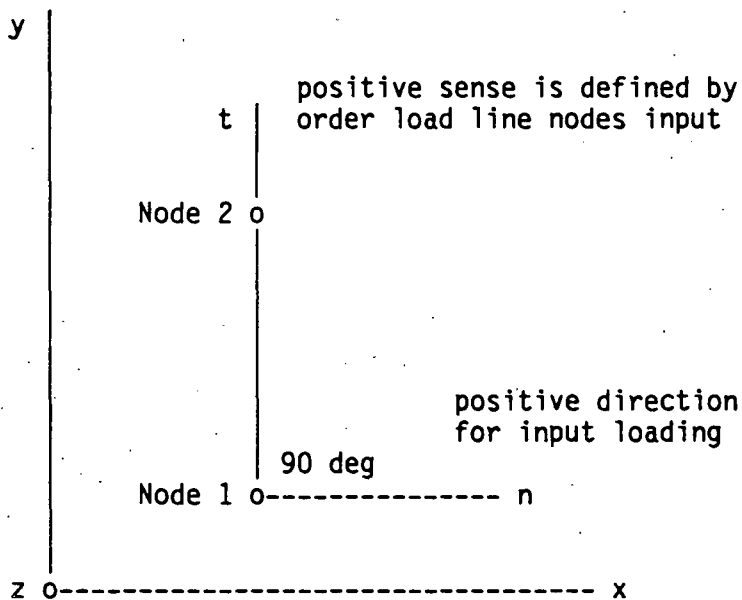
<u>Columns</u>	<u>Variable</u>	<u>Description</u>
26-30	NPT	Number of points used to define the piecewise linear function for pressure, force, displacement or thermal input on Card Type VIIIb-1 if not using IVOL(1)=0.
31-40	VINT	Initial volume of load line if using IVOL(1)=2. Angle of heat flux to x-axis if using IVOL(1)=8. Far field fluid temperature if using IVOL(1)=9.

*Number of elements on load line if using IVOL(1)=10 or 11.

#Volume is volume change divided by initial volume.

\$This loading condition is applied prior to the desired simulation.

See following figure indicating positive direction for input loading.



The positive direction for the impulse, pressure or thermal input of the load line is defined by rotating clockwise 90 degrees from the positive sense of the load line. The positive sense of the load line is defined by the order in which the node numbers are read as input.

INCLUDE ONLY IF IVOL(1) > 0 ON CARD TYPE VIIIa

Card Type VIIIb-1 Load Line Card (8E10.4)

1	6	11	16	21	26	31	36
PT(1)		PT(2)		PT(3)		PT(4)	
41	46	51	56	61	66	71	76
PT(5)		PT(6)		PT(7)		PT(8)	

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	PT(1)	Time, point 1.
11-20	PT(2)	Pressure,* point 1.
21-30	PT(3)	Time, point 2.
31-40	PT(4)	Pressure, point 2.
41-50	PT(5)	Time, point 3.
51-60	PT(6)	Pressure, point 3.
61-70	PT(7)	Time, point 4.
71-80	PT(8)	Pressure, point 4.

If $p(0)=0.0$, then (0,0) need not be input.

If less than three points desired, leave the field blank.

Additional Type VIIIb-1 cards may be used if more points are desired to describe the load line input. No limit on number of points.

*The sign convention is stated on Card Type VIIIa.

INCLUDE ONLY IF IVOL(1) = 0 ON CARD TYPE VIIIa

Card Type VIIIb-2 Load Line Card (4E10.4)

1	6	11	16	21	26	31	36
PT(1)		PT(2)		PT(3)		PT(4)	
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	PT(1)	Prescribed component of velocity* at time=0, tangent to load line.
11-20	PT(2)	Prescribed component of velocity at time=0, normal to load line.
21-30	PT(3)	First node factor. Factor by which impulse is multiplied at first node on load line (should be 2.0 if first node is on line of symmetry). If left blank, impulse at first node is not changed.
31-40	PT(4)	Last node factor. Same condition as above.

*The sign convention is stated on Card Type VIIIa.

INCLUDE FOR ALL DATA SETS

Card Type VIIIc Load Line Card (16I5)

1	6	11	16	21	26	31	36
KPR(1)	KPR(2)	KPR(3)	KPR(4)	etc.			
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
----------------	-----------------	--------------------

1-5	KPRES(1)	Nodes* on load line.
-----	----------	----------------------

6-10	KPRES(2)	
------	----------	--

etc.

If all nodes can be generated for KPRES(J), J=2 to NDNOD, by incrementing by INT1, only KPRES(1) need be included.

Additional Type VIIIc cards may be used if this card is inadequate to describe all the nodes on the load line.

*When using temperature-, heat flux- and film coefficient-time (or increment) input the nodes on the load line are those associated with the thermal mesh. When using internal heat source-time input the thermal elements are input to the load line instead of nodes. Structural mesh bar and ring elements are input to the load line instead of nodes when using prestressing-time input.

INCLUDE ONLY IF NSLID > 0 ON CARD TYPE II

Card Type IXa Sliding/Rigid Interface Card (6I5,3E10.2)

1	6	11	16	21	26	31	36
M	ND1	ND2	INT1	INT2	NRM	SLIPAR(1)	
41	46	51	56	61	66	71	76
SLIPAR(2)		SLIPAR(3)					

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	M	Sliding/rigid interface number.
6-10	ND1	Number of master nodes* for interface.
11-15	ND2	Number of slave nodes* for interface.
16-20	INT1	Automatic generator for the nodes on master side. If INT1>0, only first node on master side need be input.
21-25	INT2	Automatic generator for the nodes on slave side. If INT2>0, only first node on slave side need be input.
26-30	NRM	Sliding/rigid mode option. NRM=0: sliding interface. NRM=1: rigid continuum interface. NRM=2: rigid flexural interface. NRM=3: flexural offset rigid interface.
31-40	SLIPAR(1)	Coulomb friction: static friction coefficient. Viscous friction: dynamic viscosity coefficient.
41-50	SLIPAR(2)	Coulomb friction: dynamic friction coefficient. Viscous friction: boundary layer thickness.
51-60	SLIPAR(3)	Coulomb friction: SLIPAR(3)=0.0. Viscous friction: SLIPAR(3)=1.0.

*On lines of symmetry and for most cases involving boundary/support nodes, nodes should not exist for sliding interfaces. For rigid interfaces, only the translational nodes are necessary for nodal input data. The rotational nodes are generated internally for those elements with flexural resistance.

INCLUDE ONLY IF NSLID > 0 ON CARD TYPE II

Card Type IXb Sliding/Rigid Interface Card (16I5)

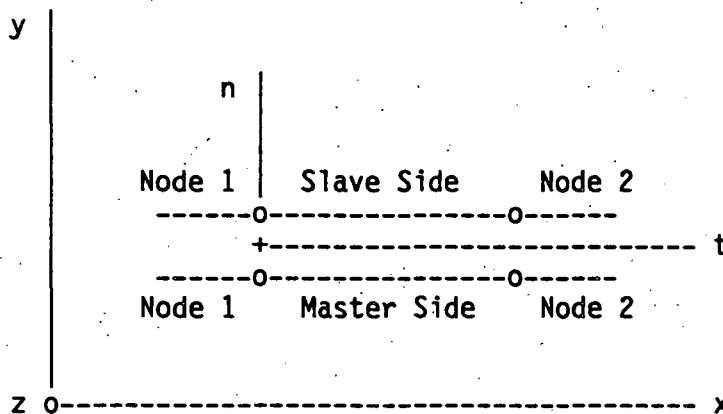
1	6	11	16	21	26	31	36
KS(1,1)	KS(1,2)	KS(1,3)	KS(1,4)	etc.			
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	KSLID(1,1)	First node on master side of interface.
6-10	KSLID(1,2)	Second node on master side of interface.
etc.		

If all nodes can be generated for KSLID(1,J), J=2 to ND1, by incrementing by INT1, only KSLID(1,1) need be included.

Additional Type IXb cards may be used if this card is inadequate to describe all the nodes on the load line.

See following figure describing sliding interface pairs.



Each node pair must have the same coordinate values. KSLID(1,J) to KSLID(2,J) defines the n-local direction; t-local is normal to n and defined by the order in which the node numbers are read as input. At corners, t is the tangent average.

See the theory manual for a description of rigid interface master-slave nodes.

INCLUDE ONLY IF NSLID > 0 ON CARD TYPE II

Card Type IXc Sliding/Rigid Interface Card (16I5)

1	6	11	16	21	26	31	36
KS(2,1)	KS(2,2)	KS(2,3)	KS(2,4)	etc.			
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-5	KSLID(2,1)	First node on slave side of interface.
6-10	KSLID(2,2)	Second node on slave side of interface.
etc.		

If all nodes can be generated for KSLID(2,J), J=2 to ND2, by incrementing by INT2, only KSLID(1,1) need be included.

Additional Type IXc cards may be used if this card is inadequate to describe all the nodes on the load line.

INCLUDE FOR ALL DATA SETS

Card Type Xa Output Card (4I10)

1	6	11	16	21	26	31	36
NPFREQ		NPRU		NPRS		NPIC	
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	NPFREQ	Frequency of output. Whatever output is desired will be printed every NPFREQ step.
11-20	NPRU	Number of motion* output records.
21-30	NPRS	Number of element# output records.
31-40	NPIC	Number of complete (different motion and/or element records at a particular time step for all nodes and/or elements) output pictures.

*A motion record is a single displacement, velocity, acceleration, external force or internal force component of a node.

#An element record here means a single strain or stress component of an element; a volume change of a pressure line; an external loading; or one of several energy calculations.

INCLUDE ONLY IF NPRU > 0 ON CARD TYPE Xa

Card Type Xb Motion Output Card (I10,5A4,2E10.4)

1	7	9	11	16	21	26	31	36
+-----+-----+-----+-----+-----+-----+-----+-----+-----+								
N	J K L			Alphanumeric				
+-----+-----+-----+-----+-----+-----+-----+-----+-----+								
41	46	51	56	61	66	71	76	
+-----+-----+-----+-----+-----+-----+-----+-----+-----+								
+-----+-----+-----+-----+-----+-----+-----+-----+-----+								

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-6	N	Node number.
7		blank.
8	J	Component number. J=1: x-component. J=2: y-component. J=3: rotation component.
9	K	Motion record flag. K=0: displacement record. K=1: velocity record. K=2: acceleration record. K=3: external force record. K=4: internal force record.
10	L	Output type flag. L=0: printed values of motion record. L=1: printed values, printer plot and graphics plot of motion record. L=2: printed values, printer plot and punched cards of motion record. L=3: printed values, printer plot, graphics plot, and punched cards of motion record. L=4: printed values and printer plot of motion record.
11-30		Alphanumeric information to be written on ordinate axis and above the graphics plot.

Include the following if automatic scaling is to be overridden for graphics plot.

31-40	Scale of ordinate value per inch of plot.
41-50	Minimum value of ordinate.

INCLUDE ONLY IF NPRS > 0 ON CARD TYPE Xa

Card Type Xc Element Output Card (I10,5A4,2E10.4)

1	7	10 11	16	21	26	31	36
M	J	L	Alphanumeric				
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-6	M	Element number.
7-9	J	Component number (see following pages).
10	L	Output type flag. L=0: printed values of element record. L=1: printed values, printer plot and graphics plot of element record. L=2: printed values, printer plot and punched cards of element record. L=3: printed values, printer plot, graphics plot, and punched cards of element record. L=4: printed values and printer plot of motion record.
11-30		Alphanumeric information to be written on ordinate axis and above the graphics plot.
		Include the following if automatic scaling is to be overridden for graphics plot.
31-40		Scale of ordinate value per inch of plot.
41-50		Minimum value of ordinate.

(Continue)

Card Type Xc (Continued)

Beam or Shell Element Component Numbers - J

J-values for element component	at length point 1 for thickness points 1 to NPD	at length point NPL for thickness points NP-NPD+1 to NP
eps-x,mech	1 to NPD	NP-NPD+1 to NP
sig-x	NP+1 to NP+NPD	2*NP-NPD+1 to 2*NP
sig-yield	2*NP+1 to 2*NP+NPD	3*NP-NPD+1 to 3*NP
temperature	3*NP+1 to 3*NP+NPD	4*NP-NPD+1 to 4*NP
eps-z,mech	4*NP+1 to 4*NP+NPD	5*NP-NPD+1 to 5*NP
sig-z	5*NP+1 to 5*NP+NPD	6*NP-NPD+1 to 6*NP
eps-e,plas	6*NP+1 to 6*NP+NPD	7*NP-NPD+1 to 7*NP
eps-x,plas	7*NP+1 to 7*NP+NPD	8*NP-NPD+1 to 8*NP
eps-z,plas	8*NP+1 to 8*NP+NPD	9*NP-NPD+1 to 9*NP
temperature*	9*NP+1 to 9*NP+NPD	10*NP-NPD+1 to 10*NP

NPL - number of integration points along element length, specified in E(15) on material property Card Type IVb-1 or IVb-11.

NPD - number of integration points through element thickness, specified in E(16) on material property Card Type IVb-1 or IVb-11.

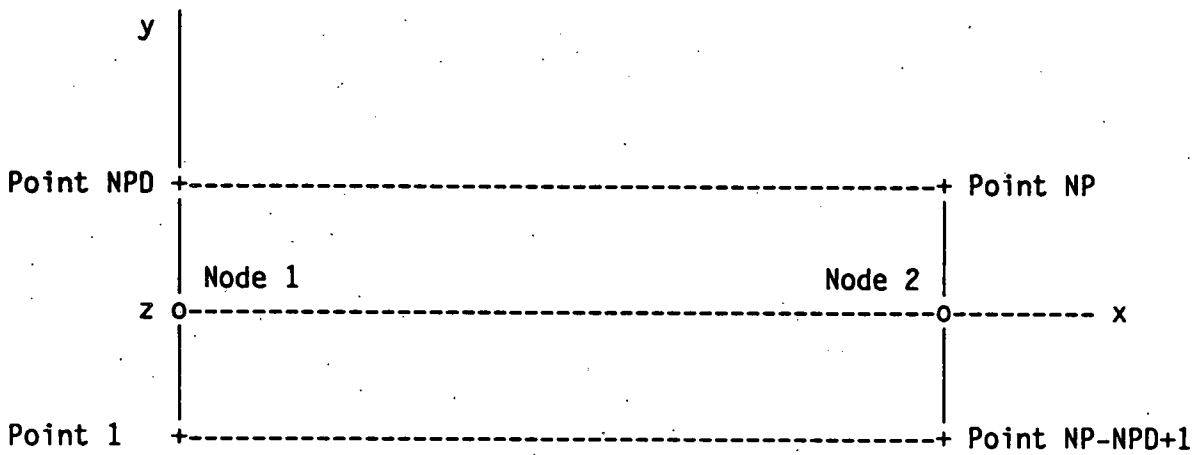
NP = NPL*NPD - total number of integration points.

*The shell element also has thermal node temperatures.

See following figure (local x, y, z coordinates) for location of components.

(Continue)

Card Type Xc (Continued)



x coordinate direction is determined by vector from Node 1 to Node 2 while y is determined by right-hand rule for indicating location of component numbers. The beam element uses the element interface locations for length integration points with thickness integration points being at equal intervals. The shell element uses Gaussian integration points along the length (usually 2) and through the thickness.

(Continue)

Card Type Xc (Continued)

Plane or Axisymmetric Continuum Element Component Numbers - J

J-values for element component	at Gaussian point 1	at Gaussian point NP
eps-x,mech	1	25*(NP-1)+1
eps-y,mech	2	25*(NP-1)+2
eps-xy,mech	3	25*(NP-1)+3
eps-z,mech	4	25*(NP-1)+4
sig-x	5	25*(NP-1)+5
sig-y	6	25*(NP-1)+6
sig-xy	7	25*(NP-1)+7
sig-z	8	25*(NP-1)+8
eps-x,plas	9	25*(NP-1)+9
eps-y,plas	10	25*(NP-1)+10
eps-xy,plas	11	25*(NP-1)+11
eps-z,plas	12	25*(NP-1)+12
eps-e,plas	13	25*(NP-1)+13
sig-yield	14	25*(NP-1)+14
temperature	15	25*(NP-1)+15

J-values for element component	at Node 1	at Node 2	at Node 3	at Node 4
temperature	22	23	24	25

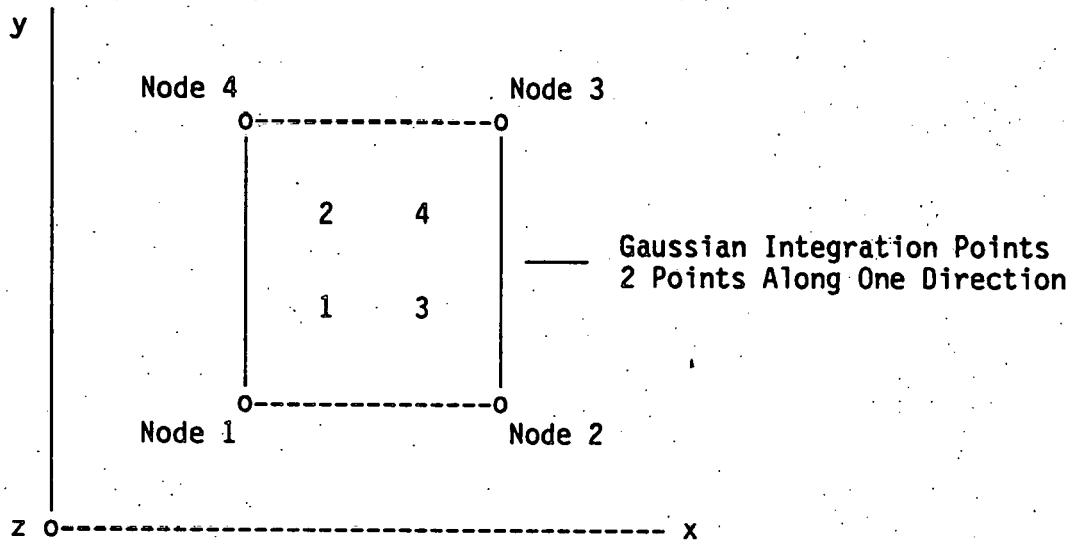
(Continue)

Card Type Xc (Continued)

NP - Gaussian integration point of element selected for output, maximum point would be $NGP \times NGP$.

NGP - number of Gaussian integration points along one direction, specified in E(16) on material property Card Type IVb-2 or IVb-12.

See following figure (global x, y, z coordinates) for location of components.



Output for the quadrilateral continuum element is given at the Gaussian integration points in the global coordinate system x, y, z. Shown here is the location of the integration stations for 2 Gaussian integration points along one direction.

(Continue)

Card Type Xc (Continued)

Plane or Axisymmetric Concrete Element Component Numbers - J

J-values for element component	at Gaussian point 1	at Gaussian point NP
eps-x,mech	1	NHV*(NP-1)+1
eps-y,mech	2	NHV*(NP-1)+2
eps-xy,mech	3	NHV*(NP-1)+3
eps-z,mech	4	NHV*(NP-1)+4
sig-x	5	NHV*(NP-1)+5
sig-y	6	NHV*(NP-1)+6
sig-xy	7	NHV*(NP-1)+7
sig-z	8	NHV*(NP-1)+8
eps-x,plas	9	NHV*(NP-1)+9
eps-y,plas	10	NHV*(NP-1)+10
eps-xy,plas	11	NHV*(NP-1)+11
eps-z,plas	12	NHV*(NP-1)+12
eps-e,plas	13	NHV*(NP-1)+13
sig-yield	14	NHV*(NP-1)+14
temperature	15	NHV*(NP-1)+15

J-values for element component	at Node 1	at Node 2	at Node 3	at Node 4
temperature	22	23	24	25

(Continue)

Card Type Xc (Continued)

J-values for element component	at Gaussian point 1	at Gaussian point NP
crack angl	30	$NHV*(NP-1)+30$
crack ang2	31	$NHV*(NP-1)+31$
crack type	32	$NHV*(NP-1)+32$
id-plas	33	$NHV*(NP-1)+33$

J-values for element (rbar) component	available only if one point
sig-xi	34
sig-eta	35
sig-theta	36
yield-x1	37
yield-eta	38
yield-theta	39

NGP - number of Gaussian integration points along one direction, specified in E(16) on material property Card Type IVb-3 or IVb-13.

NP - Gaussian integration point of element selected for output, maximum point would be $NGP*NGP$.

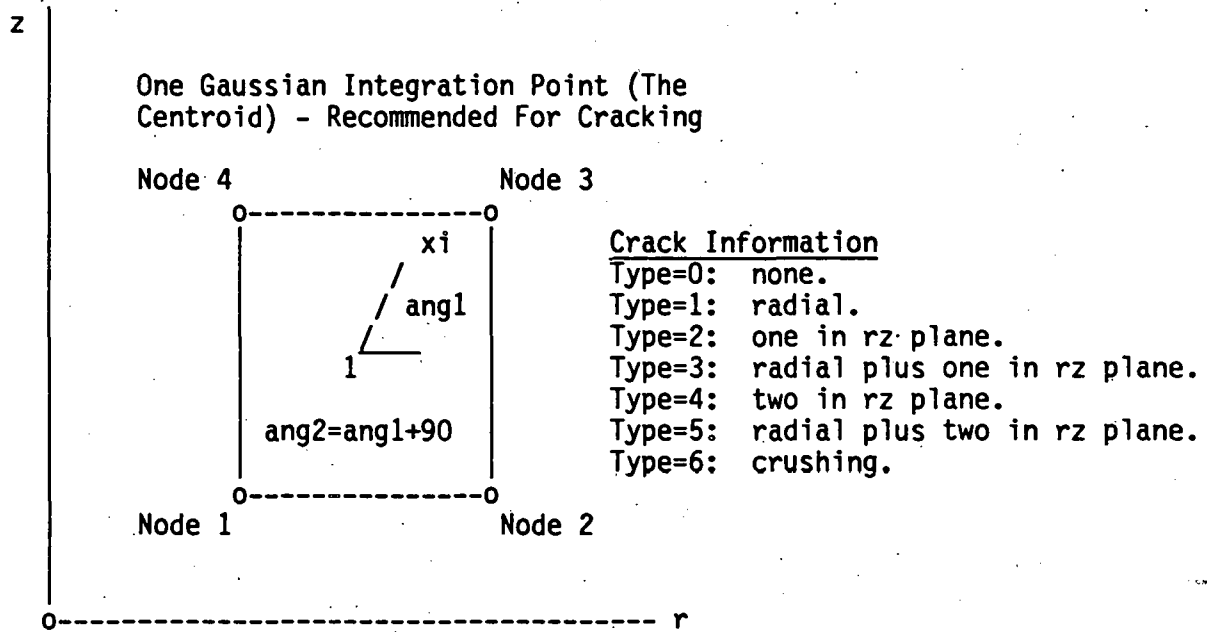
NHV = 48 - plane concrete element output values per integration point.

NHV = 51 - axisymmetric concrete element output values per integration point.

See following figure (global cylindrical coordinates) for location of components and crack output information.

(Continue)

Card Type Xc (Continued)



Output for the quadrilateral concrete element is given at the Gaussian integration points in global cylindrical coordinates. Shown here is the location of the integration station for one Gaussian integration point (the element centroid). For cracking, if using a global x, y, z coordinate system, the rz plane becomes the xy plane with an existing radial crack. Then, only Type=1, 3, 5 and 6 exist.

(Continue)

Card Type Xc (Continued)

Concrete Beam or Shell Element Component Numbers - J

J-values for element component	at length point 1 for thickness points 1 to NPD	at length point NPL for thickness points NP-NPD+1 to NP
eps-x,mech	1 to NPD	NP-NPD+1 to NP
sig-x	NP+1 to NP+NPD	2*NP-NPD+1 to 2*NP
sig-yield	2*NP+1 to 2*NP+NPD	3*NP-NPD+1 to 3*NP
temperature	3*NP+1 to 3*NP+NPD	4*NP-NPD+1 to 4*NP
eps-z,mech	4*NP+1 to 4*NP+NPD	5*NP-NPD+1 to 5*NP
sig-z	5*NP+1 to 5*NP+NPD	6*NP-NPD+1 to 6*NP
eps-e,plas	6*NP+1 to 6*NP+NPD	7*NP-NPD+1 to 7*NP
eps-x,plas	7*NP+1 to 7*NP+NPD	8*NP-NPD+1 to 8*NP
eps-z,plas	8*NP+1 to 8*NP+NPD	9*NP-NPD+1 to 9*NP
temperature*	9*NP+1 to 9*NP+NPD	10*NP-NPD+1 to 10*NP
alpha-x	10*NP+1 to 10*NP+NPD	11*NP-NPD+1 to 11*NP
alpha-z	11*NP+1 to 11*NP+NPD	12*NP-NPD+1 to 12*NP
icrack	12*NP+1 to 12*NP+NPD	13*NP-NPD+1 to 13*NP
id-plas	13*NP+1 to 13*NP+NPD	14*NP-NPD+1 to 14*NP
eps-rbar	NL+1 to NL+NRB	NL+MP-NRB+1 to NL+MP
sig-rbar	NL+MP+1 to NL+MP+NRB	NL+2*MP-NRB+1 to NL+2*MP
yield-rbar	NL+2*MP+1 to NL+2*MP+NRB	NL+3*MP-NRB+1 to NL+3*MP

NPL = 2 - number of integration points along element length.

NPD - number of integration points through element thickness, specified in E(16) on material property Card Type IVb-4 or IVb-14.

NP = 2*NPD - total number of integration points.

(Continue)

Card Type Xc (Continued)

NL = $17 \cdot NP$ - beam concrete element output values without reinforcement.

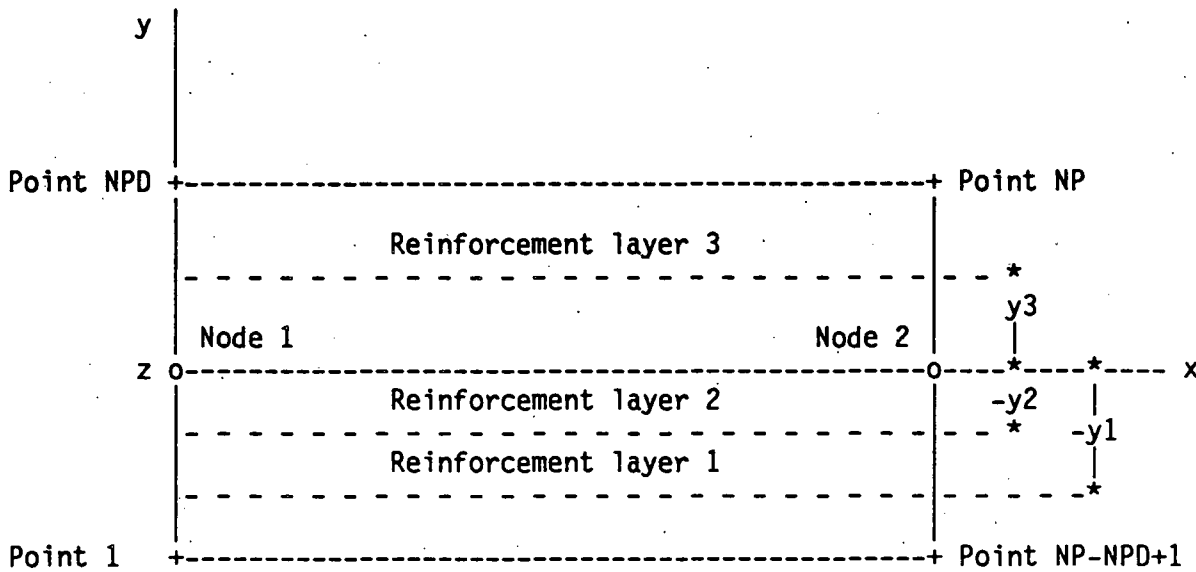
NL = $20 \cdot NP$ - shell concrete element output values without reinforcement.

NRB - number of reinforcing layers.

MP = $2 \cdot NRB$

*The shell element also has thermal node temperatures.

See following figure (local x, y, z coordinates) for location of components.



x coordinate direction is determined by vector from Node 1 to Node 2 while y is determined by right-hand rule for indicating location of component numbers. The beam element uses the element interface locations for length integration points with thickness integration points being at equal intervals. The shell element uses Gaussian integration points along the length (usually 2) and through the thickness.

(Continue)

Card Type Xc (Continued)

In addition, these values can be obtained:

<u>Special Element Associated Values</u>	<u>Element Number</u>	<u>J-Values</u>
Volume change of load line	M=NUMEL*+1	J=load line number.
External loading	M=NUMEL+2	J=load line number.
Internal energy of material	M=NUMEL+3	J=material type number.
External energy of load line	M=NUMEL+4	J=load line number.
Kinetic energy	M=NUMEL+5	J=1
Total internal energy	M=NUMEL+5	J=2
Total external energy	M=NUMEL+5	J=3
Total energy	M=NUMEL+5	J=4
Energy error, $\frac{\text{total ext} - \text{total}}{\text{total ext}}$	M=NUMEL+5	J=5
Estimated period of first mode associated with dynamic relaxation	M=NUMEL+5	J=6

*NUMEL = number of elements defined on Card Type II.

INCLUDE ONLY IF NPIC > 0 ON CARD TYPE Xa

Card Type Xd	Picture Output Card (2I10)						
1	6	11	16	21	26	31	36
NTSTEP		K					
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	NTSTEP	Time step or load increment at which complete global output is desired.
11-20	K	Output type flag. K=0: graphics plot only. K=1: output displacements at all nodes. K=2: output from K=1 plus coordinates of deformed structure (for NDGREE=2, output for rotational nodes is same as K=1). K=3: output from K=2 plus velocities and accelerations at all nodes. K=4: output from K=3 plus strains and stresses for all elements.

Deformed graphics plots of the mesh may be obtained at the various time steps for K=2, 3, or 4 by requesting a mesh plot on Card Type III.

INCLUDE ONLY IF KONTRL(6) = 1 ON CARD TYPE III

Card Type XI Gravity Loading Card (3E10.4)

1	6	11	16	21	26	31	36
ACCG		CXX		CYY			
41	46	51	56	61	66	71	76

<u>Columns</u>	<u>Variable</u>	<u>Description</u>
1-10	ACCG	Acceleration of gravity rate.
11-20	CXX	x-direction cosine for gravity.
21-30	CYY	y-direction cosine for gravity.

4. Sample Problems

4.1 Sample Problem 1: Thermally Loaded Axisymmetric Plate

Problem 1, shown in Fig. 4.1, is a tungsten axisymmetric plate whose upper surface is heated from an initial temperature of 482°F to 888°F. The bottom surface of the plate is prescribed to remain at 482°F. The thermal distribution occurs only through the plate. At the outer edge of the plate, the plate is assumed to be displacement restricted in the normal direction. Also shown in Fig. 4.1 is the 10 shell element stress mesh and the 60 rectangular element thermal mesh. The material properties and parameters needed for modeling are provided in Table 4.1; the material properties will remain constant over the temperature range. Table 4.2 provides the input data for the problem. The temperature distribution through the plate thickness is given in Fig. 4.2 for the analytical and theoretical cases. A very good agreement is achieved for this coarse discretization.

Table 4.1 Material Properties and Parameters for Axisymmetric Plate (Sample Problem 1)

Radius	$r = 10.0 \text{ in}$
Thickness	$t = 0.25 \text{ in}$
Density	$\rho = 1.806 \times 10^{-3} \text{ lb-sec}^2/\text{in}^4$
Young's modulus	$E = 5.2 \times 10^7 \text{ psi}$
Poisson's ratio	$\nu = 0.25$
Yield stress	$\sigma_y = 7.5 \times 10^4 \text{ psi}$
Plastic modulus	$E_p = 1.0 \times 10^5 \text{ psi}$
Thermal expansion coefficient	$\alpha = 5.17 \times 10^{-6} \text{ in/in/}^\circ\text{F}$
Thermal conductivity	$k = 5.29 \times 10^{-4} \text{ btu/sec/in/}^\circ\text{F}$
Specific heat	$c = 5.537 \text{ btu/lb/sec}^2/\text{in/}^\circ\text{F}$

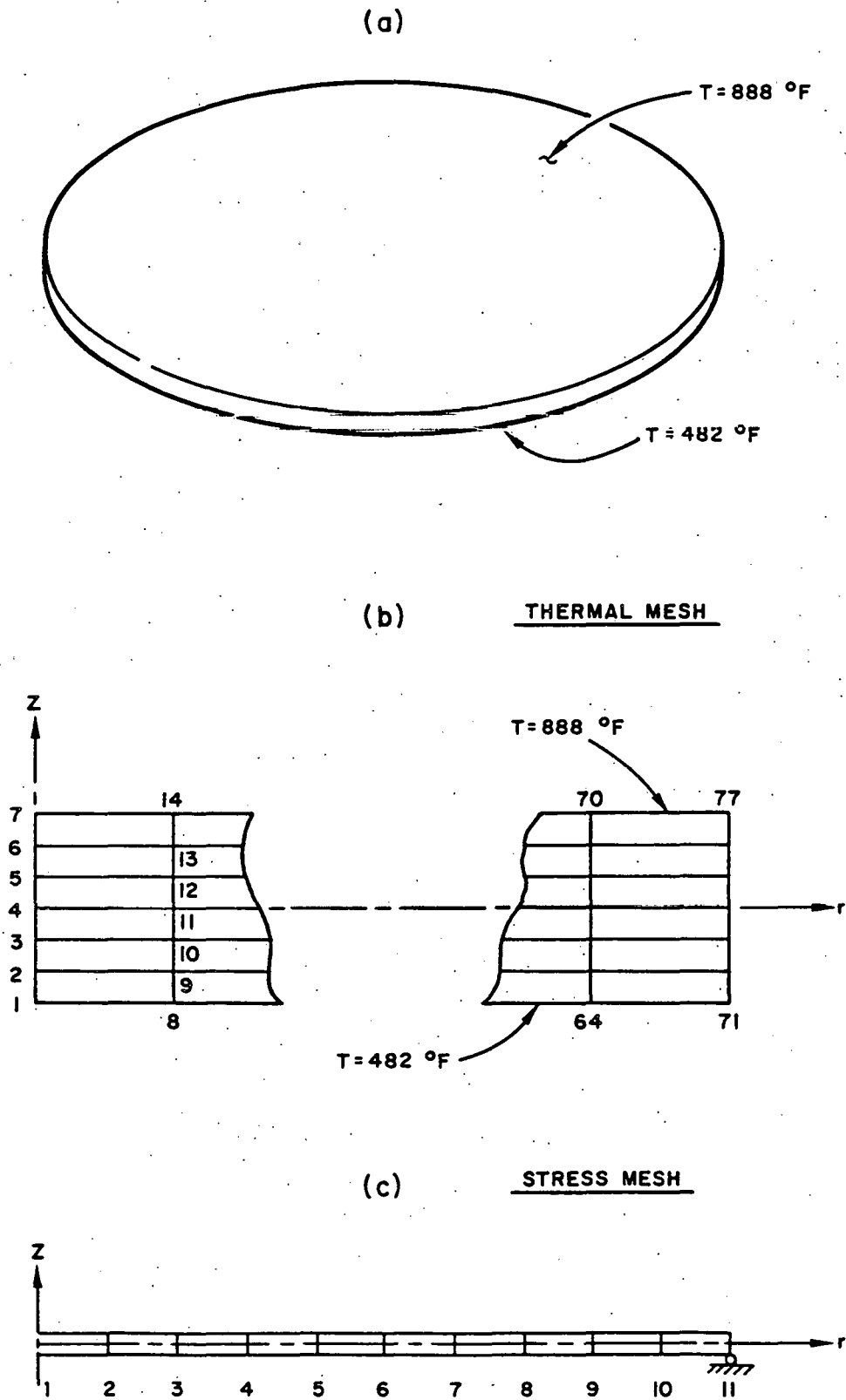


Figure 4.1 Thermal Loading and Finite Element Models for Axisymmetric Plate

Table 4.2 Input Cards for Sample Problem 1

	1	2	3	4	5	6	7	8		
1	1234567890123456789012345678901234567890123456789012345678901234567890								Card I	
2	PLATE PROBLEM - 10	STRESS ELEMENTS - 6	THERMAL ELEMENTS PER STRESS ELEMENT						Card II	
3	11	10	1	2	16	3	0.125	1	Card III	
4	2	0		90			2000	100	10	Card IVa-11
5	1	11	TUNGSTEN							Card IVb-11
6	1.806E-3			0.25	5.17E-6	0.000529	5.537099	1.0	2.0	Card IVc-11
7		1.0		0.0	0.0	0.0	3.0	2.0	7.0	Card IVd-11
8	482.0	52.0E+6	0.0014423	75000.0	0.5014423	125000.0				Card V
9	1	0.0	0.0							Card V
10	11	10.0	0.0							Card VI
11	1	1	2	0	0	0	0	1	11	Card VI
12	10	10	11	0	0	0	0	1	11	Card VII
13	10101									Card VII
14	110010									Card VIIa
15	1	11	7	3	7	3				Card VIIIb-1
16	0.0	482.0		2.0	888.0	50.0	888.0			Card VIIIc
17	7									Card Xa
18	1	2		9	1					Card Xb
19	10204Z-DISP NODE 1									Card Xc
20	110104R-DISP NODE 11									Card Xc
21	10014EMX AT STA 1 ELE 1									Card Xc
22	10074EMX AT STA 7 ELE 1									Card Xc
23	10154STX AT STA 1 ELE 1									Card Xc
24	10214STX AT STA 7 ELE 1									Card Xc
25	10434TEM AT STA 1 ELE 1									Card Xc
26	10464TEM AT STA 4 ELE 1									Card Xc
27	10494TEM AT STA 7 ELE 1									Card Xc
28	120014APPLIED TEMPERATURE									Card Xc
29	130014INTERNAL ENERGY									Card Xd
	16	4								

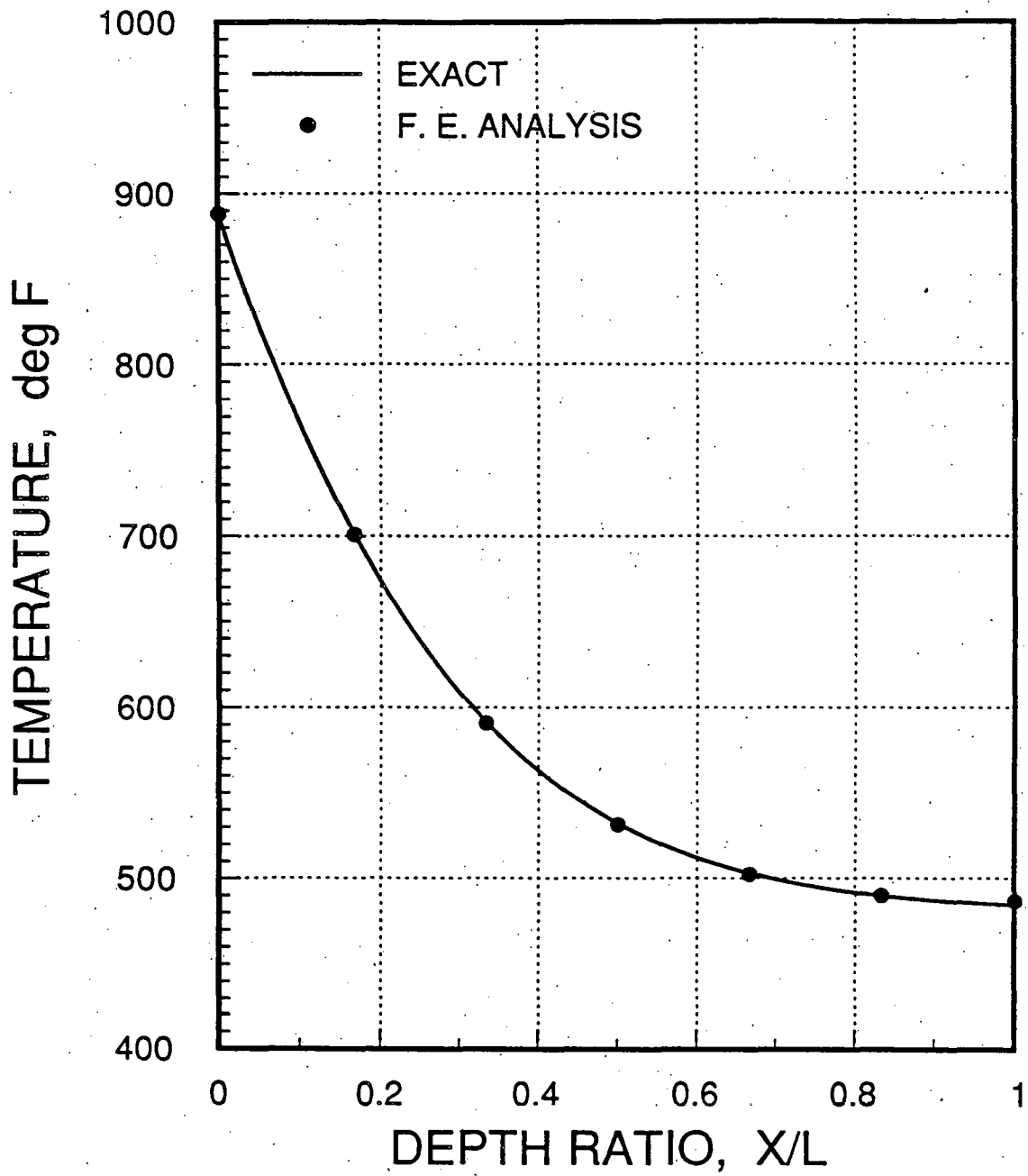


Figure 4.2 Temperature Distribution Through the Plate Thickness

4.2 Sample Problem 2: Thermal Analysis of Rectangular Plate

The analytical results of a rectangular plate problem described by plane quadrilateral continuum elements for a two-dimensional temperature field can be compared with the theoretical solution. Here, a sinusoidal temperature distribution is prescribed at one side, and zero temperature on the remaining sides. The theoretical steady-state temperature distribution along the line of symmetry is shown in Fig. 4.3. This figure also shows the analytical result, where the model uses one-half of the plate area. Four meshes were used to model this problem: 2 x 4 elements, 3 x 6 elements, 4 x 8 elements and 5 x 10 elements. A plot of contour isotherms for the four models are shown in Fig. 4.4. The case of 2 x 4 elements in the analytical model provides maximum deviation from the theoretical predictions, and the finer mesh models provide closer agreement. The overall agreement, however, appears very good. The input cards for the 50 element mesh are given in Table 4.4.

Table 4.3 Material Properties and Parameters for Rectangular Plate (Sample Problem 2)

Length	$L = 4.0 \text{ cm}$
Thickness	$t = 1.0 \text{ cm}$
Density	$\rho = 1.0 \text{ g/cm}^3$
Young's modulus	$E = 1.0 \times 10^{10} \text{ dynes/cm}$
Poisson's ratio	$\nu = 0.3$
Thermal expansion coefficient	$\alpha = 2.0 \times 10^{-5} \text{ cm/cm/}^\circ\text{C}$
Thermal conductivity	$k = 1.0 \text{ cal/sec/cm/}^\circ\text{C}$
Specific heat	$c = 1.0 \text{ cal/g/}^\circ\text{C}$

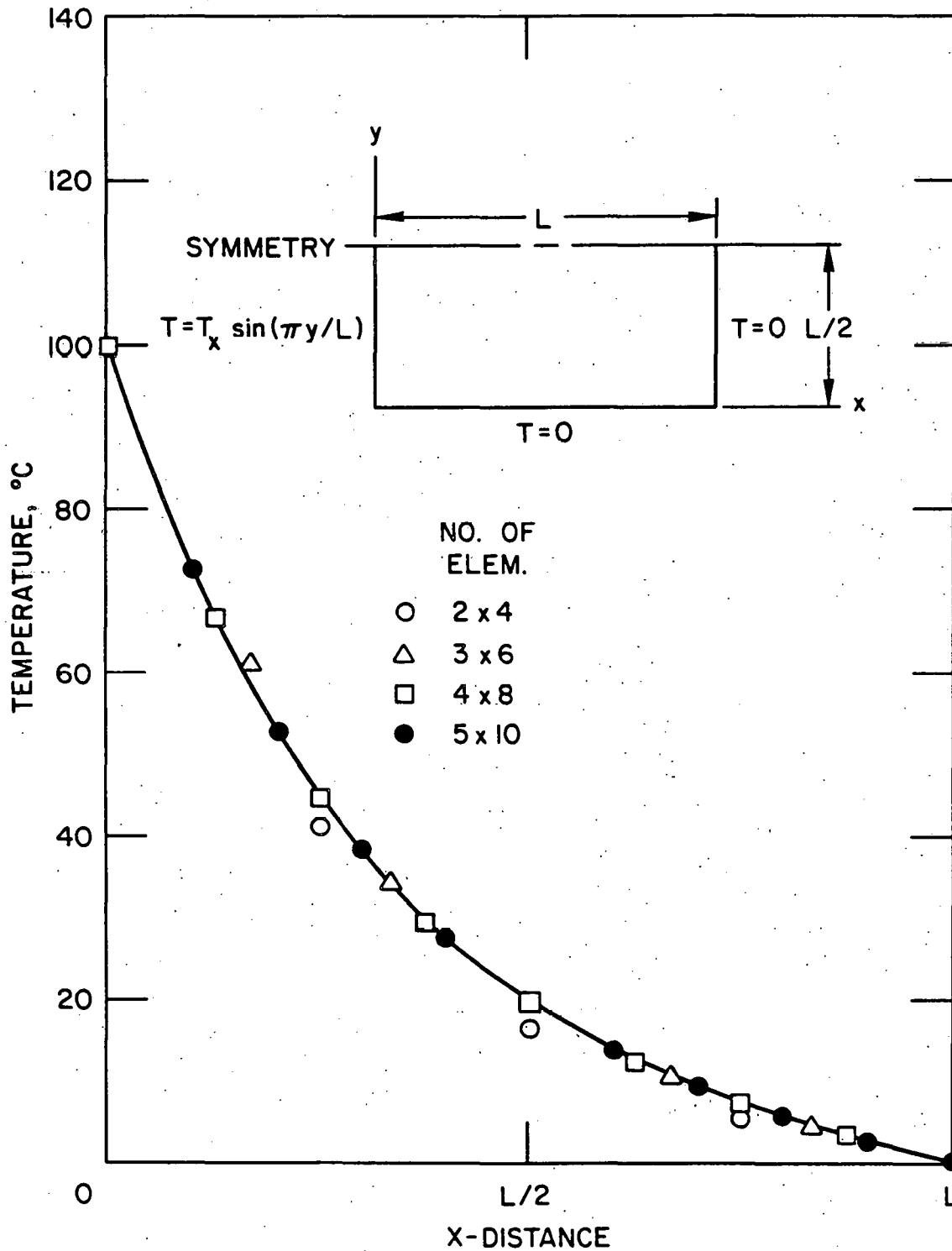


Figure 4.3 Temperature Variation Along the Line of Symmetry

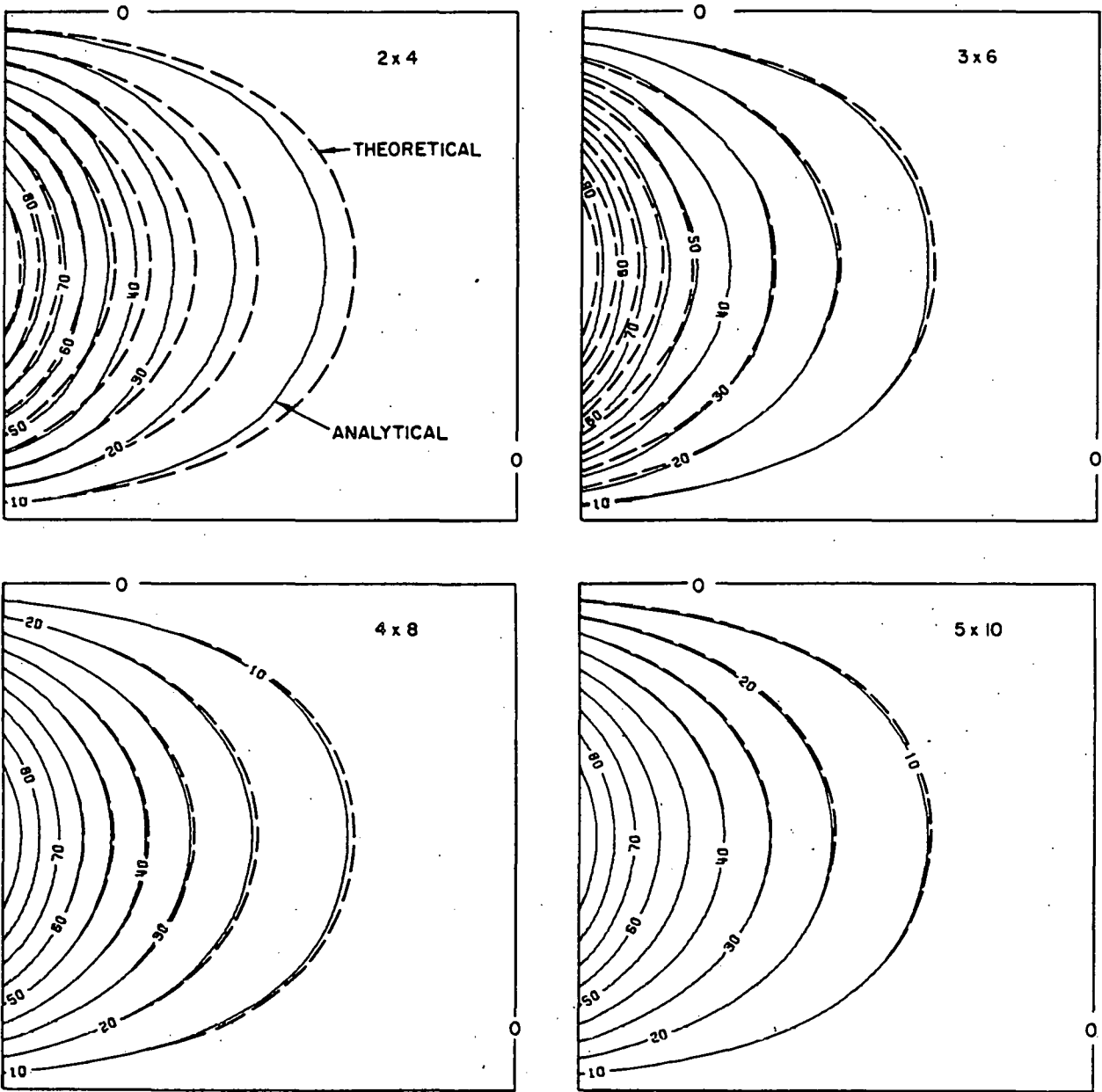


Figure 4.4 Contour Plots of Temperature Distribution

Table 4.4 Input Cards for Sample Problem 2

	1		2		3		4		5		6		7		8		
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	
1	50 ELEMENT MODEL OF RECTANGULAR PLATE - THERMAL ANALYSIS																Card I
2	66	50	1	2	130	2	5.0E-2	7									Card II
3	2	0			90						2000	100	10				Card III
4	1	2															Card IVa-2
5		1.0				0.3	2.0E-5	1.0	1.0	1.0	1.0	1.0	2.0				Card IVb-2
6		1.0		0.0		0.0	0.0	0.0	0.0	11.0			2.0				Card IVc-2
7		0.0		1.0E10		0.1	1.0E9	1.0	2.0E9								Card IVd-2
8	1			0.0		0.0											Card V
9	6			0.0		2.0											Card V
10	7			0.4		0.0											Card V
11	12			0.4		2.0											Card V
12	13			0.8		0.0											Card V
13	18			0.8		2.0											Card V
14	19			1.2		0.0											Card V
15	24			1.2		2.0											Card V
16	25			1.6		0.0											Card V
17	30			1.6		2.0											Card V
18	31			2.0		0.0											Card V
19	36			2.0		2.0											Card V
20	37			2.4		0.0											Card V
21	42			2.4		2.0											Card V
22	43			2.8		0.0											Card V
23	48			2.8		2.0											Card V
24	49			3.2		0.0											Card V
25	54			3.2		2.0											Card V
26	55			3.6		0.0											Card V
27	60			3.6		2.0											Card V
28	61			4.0		0.0											Card V
29	66			4.0		2.0											Card V
30	1	1	7	8	2			1	2	6							Card VI
31	11	2	8	9	3			1	2	6							Card VI
32	21	3	9	10	4			1	2	6							Card VI
33	31	4	10	11	5			1	2	6							Card VI
34	41	5	11	12	6			1	2	6							Card VI
35	50	59	65	66	60			1	2	6							Card VI

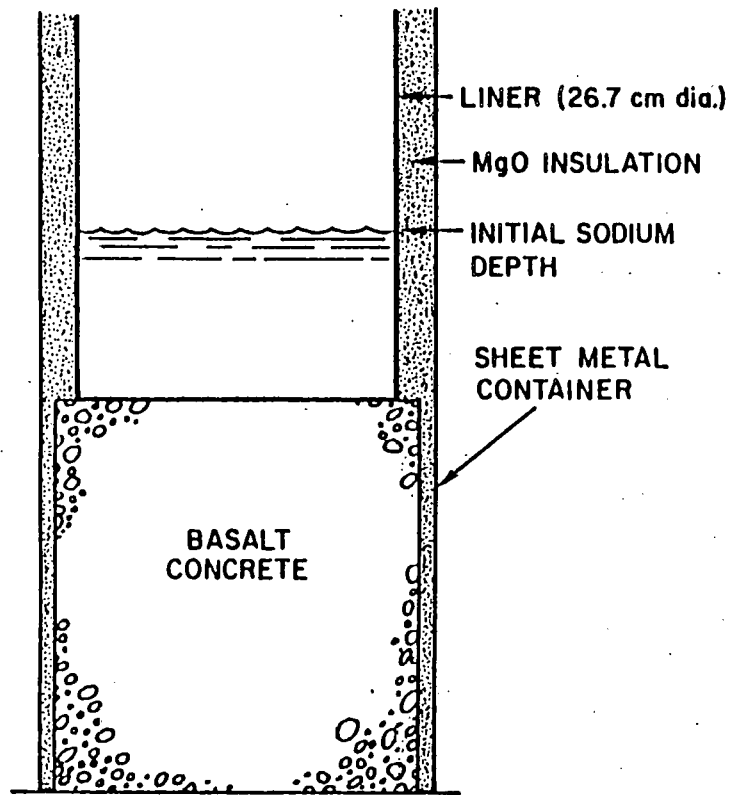
Table 4.4 Input Cards for Sample Problem 2 (cont.)

	1			2			3			4			5			6			7			8			
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890			
36	60010																						Card VII		
37	660111																						Card VII		
38	1 1	7 0	0 2																				Card VIIa		
39	2.0	30.9	20.0	30.9																			Card VIIb-1		
40	2																						Card VIIc		
41	2 1	7 0	0 2																				Card VIIa		
42	2.0	58.78	20.0	58.78																			Card VIIb-1		
43	3																						Card VIIc		
44	3 1	7 0	0 2																				Card VIIa		
45	2.0	80.9	20.0	80.9																			Card VIIb-1		
46	4																						Card VIIc		
47	4 1	7 0	0 2																				Card VIIa		
48	2.0	95.11	20.0	95.11																			Card VIIb-1		
49	5																						Card VIIc		
50	5 1	7 0	0 2																				Card VIIa		
51	2.0	100.0	20.0	100.0																			Card VIIb-1		
52	6																						Card VIIc		
53	6 16	7 0	0 2																				Card VIIa		
54	0.0	0.0	20.0	0.0																			Card VIIb-1		
55	1 7 13	19 25	31 37	43 49	55 61	62 63	64 65	66															Card VIIc		
56	7 11	8 0	0 2																				Card VIIa		
57	0.0	0.0	20.0	0.0																			Card VIIb-1		
58	6 12	18 24	30 36	42 48	54 60	66																	Card VIIc		
59	1	0	9	1																			Card Xa		
60	20244TEM	AT NODE	14																				Card Xc		
61	50244TEM	AT NODE	32																				Card Xc		
62	80244TEM	AT NODE	50																				Card Xc		
63	220244TEM	AT NODE	16																				Card Xc		
64	250244TEM	AT NODE	34																				Card Xc		
65	280244TEM	AT NODE	52																				Card Xc		
66	420244TEM	AT NODE	18																				Card Xc		
67	450244TEM	AT NODE	36																				Card Xc		
68	480244TEM	AT NODE	54																				Card Xc		
69	130	4																					Card Xd		

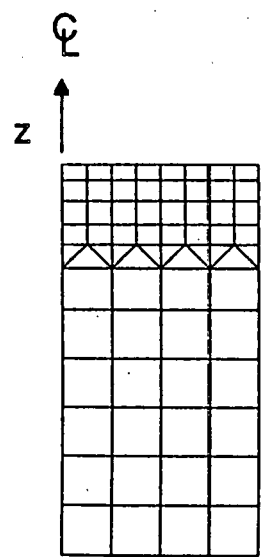
4.3 Sample Problem 3: Sandia Cylinder Test

A basalt concrete cylinder, which is unconstrained on the outside of the cylinder, is analytically examined here. Figure 4.5a illustrates the initial configuration of the free (unconstrained) test specimen used by Acton, et al. (1981). The analytical model of the concrete specimen used in this study was subdivided into finite elements as shown in Figs. 4.5b and 4.5c. Because of symmetry only one-half of the specimen is modeled. Two meshes are given in order to illustrate the use of the rigid connection as an alternative to triangular elements when combining elements of two different sizes. The temperature dependent material properties of basalt concrete used in this example are summarized in Table 4.5. The input cards for the two simulations are given in Tables 4.6 and 4.7. One Gaussian integration point is used for the quadrilateral elements and also for the collapsed triangular elements in the stress calculations. The bottom at the centerline of the concrete model is assumed fixed and the remainder of centerline nodes are constrained to move along the z-axis. The input temperature is applied on the top surface of the cylinder. Because of the MgO insulation of the unconstrained model, zero heat flux is assumed on the boundaries of the concrete model.

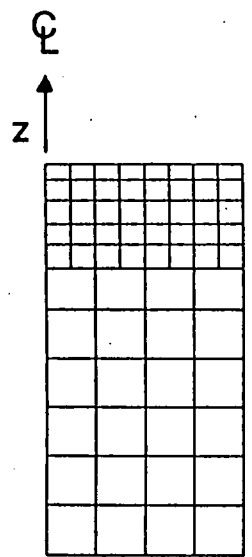
Figure 4.6 shows the thermocouple readings up to 20 minutes for the free concrete cylinder given in Fig. 4.5a. Analytical temperatures (identical for both meshes) for the model are also plotted in Fig. 4.6. These temperatures were calculated from the assumed temperature being specified at the initial surface of the respective analytical model. The cracking history of the model is given in Fig. 4.7 for both meshes at 5 minute intervals up to 20 minutes. The influence of the mesh choice on the cracking pattern seems to be minimal.



(a)



(b)



(c)

Figure 4.5 Cylinder Test Specimen Configuration and Analytical Models

Table 4.5 Basalt Concrete Properties for Cylinder
(Sample Problems 3 and 4)

Temperature (°C)	Modulus of Elasticity (GPa)	Yield Stress (MPa)
21	21.5	27.58
93	15.0	20.69
371	7.3	15.51
538	2.2	8.27
700	0.557	4.13
Density		$\rho = 2326 \text{ kg/m}^3$
Poisson's ratio		$\nu = 0.15$
Thermal expansion coefficient		$\alpha = 9.2 \times 10^{-6} \text{ m/m/}^\circ\text{C}$
Thermal conductivity		$k = 1.85 \text{ J/m/sec/}^\circ\text{C}$
Specific heat		$c = 949 \text{ J/kg/}^\circ\text{C}$

Table 4.6 Input Cards for Sample Problem 3 - 68 Element Model

	1		2		3		4		5		6		7		8		
	12345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	
1	SANDIA CYLINDER TEST (NO CONSTRAINT) - 68 ELEMENT MODEL																Card I
2	80	68	1	12	60	2	20.0	1									Card II
3	2	0			90					5000	200	100	1				Card III
4	1	13															Card IVa-13
5	2.326		0.15		9.2E-6			0.0185		0.949		5.0		5.0			Card IVb-13
6			0.0		0.0		1.0	0.0		13.0		0.0		1.0			Card IVc-13
7	21.0	25.00E10	0.00070		1.75E8		0.00125		2.45E8	0.00165		2.70E8					Card IVd-13
8	0.00200	2.80E8	2.00000		2.80E8												Card IVd-13
9	93.0	16.92E10	0.00065		1.10E8		0.00120		1.70E8	0.00175		2.00E8					Card IVd-13
10	0.00220	2.10E8	2.00000		2.10E8												Card IVd-13
11	371.0	8.42E10	0.00095		0.80E8		0.00160		1.20E8	0.00240		1.45E8					Card IVd-13
12	0.00310	1.55E8	2.00000		1.55E8												Card IVd-13
13	538.0	2.81E10	0.00160		0.45E8		0.00310		0.70E8	0.00450		0.80E8					Card IVd-13
14	0.00600	0.85E8	2.00000		0.85E8												Card IVd-13
15	700.0	0.92E10	0.00240		0.22E8		0.00460		0.35E8	0.00680		0.40E8					Card IVd-13
16	0.00900	0.42E8	2.00000		0.42E8												Card IVd-13
17	1		0.0		0.0												Card V
18	5		15.24		0.0												Card V
19	6		0.0		3.81												Card V
20	10		15.24		3.81												Card V
21	11		0.0		7.62												Card V
22	15		15.24		7.62												Card V
23	16		0.0		11.43												Card V
24	20		15.24		11.43												Card V
25	21		0.0		15.24												Card V
26	25		15.24		15.24												Card V
27	26		0.0		19.05												Card V
28	30		15.24		19.05												Card V
29	31		0.0		22.23												Card V
30	35		15.24		22.23												Card V
31	36		0.0		24.13												Card V
32	44		15.24		24.13												Card V
33	45		0.0		25.73												Card V
34	53		15.24		25.73												Card V
35	54		0.0		27.58												Card V

Table 4.6 Input Cards for Sample Problem 3 - 68 Element Model (cont.)

	1		2		3		4		5		6		7		8		
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	
36	62		15.24		27.58												Card V
37	63		0.0		29.21												Card V
38	71		15.24		29.21												Card V
39	72		0.0		30.48												Card V
40	80		15.24		30.48												Card V
41	1	1	2	7	6				1	13	1						Card VI
42	5	6	7	12	11				1	13	1						Card VI
43	9	11	12	17	16				1	13	1						Card VI
44	13	16	17	22	21				1	13	1						Card VI
45	17	21	22	27	26				1	13	1						Card VI
46	21	26	27	32	31				1	13	1						Card VI
47	25	31	37	36	36				1	13	1						Card VI
48	26	31	32	37	37				1	13	1						Card VI
49	27	32	38	37	37				1	13	1						Card VI
50	28	32	39	38	38				1	13	1						Card VI
51	29	32	33	39	39				1	13	1						Card VI
52	30	33	40	39	39				1	13	1						Card VI
53	31	33	41	40	40				1	13	1						Card VI
54	32	33	34	41	41				1	13	1						Card VI
55	33	34	42	41	41				1	13	1						Card VI
56	34	34	43	42	42				1	13	1						Card VI
57	35	34	35	43	43				1	13	1						Card VI
58	36	35	44	43	43				1	13	1						Card VI
59	37	36	37	46	45				1	13	1						Card VI
60	45	45	46	55	54				1	13	1						Card VI
61	53	54	55	64	63				1	13	1						Card VI
62	61	63	64	73	72				1	13	1						Card VI
63	68	70	71	80	79				1	13	1						Card VI
64	10110																Card VII
65	60100																Card VII
66	110100																Card VII
67	160100																Card VII
68	210100																Card VII
69	260100																Card VII
70	310100																Card VII

Table 4.6 Input Cards for Sample Problem 3 - 68 Element Model (cont.)

	1	2	3	4	5	6	7	8	
	12345678901	23456789012	34567890123	45678901234	56789012345	67890123456	78901234567	8901234567890	
71	360100								Card VII
72	450100								Card VII
73	540100								Card VII
74	630100								Card VII
75	720100								Card VII
76	1 8	7 1	1 5						Card VIIIa
77	0.0	21.0	1.2E+2	190.0	1.2E+3	360.0	2.4E+3	510.0	Card VIIIb-1
78	5.4E+3	550.0							Card VIIIb-1
79	72								Card VIIIc
80	1	2	14	1					Card Xa
81	720204Z-DISP NODE 72								Card Xb
82	800104R-DISP NODE 80								Card Xb
83	610014EMR AT STA 1 ELE 61								Card Xc
84	610024EMZ AT STA 1 ELE 61								Card Xc
85	610044EMT AT STA 1 ELE 61								Card Xc
86	530044EMT AT STA 1 ELE 53								Card Xc
87	240044EMT AT STA 1 ELE 24								Card Xc
88	610054STR AT STA 1 ELE 61								Card Xc
89	610064STZ AT STA 1 ELE 61								Card Xc
90	610084STT AT STA 1 ELE 61								Card Xc
91	680084STT AT STA 1 ELE 68								Card Xc
92	240084STT AT STA 1 ELE 24								Card Xc
93	610154TEM AT STA 1 ELE 61								Card Xc
94	680154TEM AT STA 1 ELE 68								Card Xc
95	240154TEM AT STA 1 ELE 24								Card Xc
96	710014INTERNAL ENERGY								Card Xc
99	60	4							Card Xd

Table 4.7 Input Cards for Sample Problem 3 - 64 Element Model

	1		2		3		4		5		6		7		8		
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	
1	SANDIA CYLINDER TEST (NO CONSTRAINT) - 64 ELEMENT MODEL																Card I
2	89	64	1	12	60	2	20.0	1	1								Card II
3	2	0			90						5000	200	100	1			Card III
4	1	13															Card IVa-13
5	2.326		0.15		9.2E-6				0.0185		0.949		5.0		5.0		Card IVb-13
6			0.0		0.0		1.0		0.0		13.0		0.0		1.0		Card IVc-13
7	21.0	25.00E10	0.00070		1.75E8		0.00125		2.45E8		0.00165		2.70E8				Card IVd-13
8	0.00200	2.80E8	2.00000		2.80E8												Card IVd-13
9	93.0	16.92E10	0.00065		1.10E8		0.00120		1.70E8		0.00175		2.00E8				Card IVd-13
10	0.00220	2.10E8	2.00000		2.10E8												Card IVd-13
11	371.0	8.42E10	0.00095		0.80E8		0.00160		1.20E8		0.00240		1.45E8				Card IVd-13
12	0.00310	1.55E8	2.00000		1.55E8												Card IVd-13
13	538.0	2.81E10	0.00160		0.45E8		0.00310		0.70E8		0.00450		0.80E8				Card IVd-13
14	0.00600	0.85E8	2.00000		0.85E8												Card IVd-13
15	700.0	0.92E10	0.00240		0.22E8		0.00460		0.35E8		0.00680		0.40E8				Card IVd-13
16	0.00900	0.42E8	2.00000		0.42E8												Card IVd-13
17	1		0.0		0.0												Card V
18	5		15.24		0.0												Card V
19	6		0.0		3.81												Card V
20	10		15.24		3.81												Card V
21	11		0.0		7.62												Card V
22	15		15.24		7.62												Card V
23	16		0.0		11.43												Card V
24	20		15.24		11.43												Card V
25	21		0.0		15.24												Card V
26	25		15.24		15.24												Card V
27	26		0.0		19.05												Card V
28	30		15.24		19.05												Card V
29	31		0.0		22.23												Card V
30	35		15.24		22.23												Card V
31	36		0.0		22.23												Card V
32	44		15.24		22.23												Card V
33	45		0.0		24.13												Card V
34	53		15.24		24.13												Card V
35	54		0.0		25.73												Card V

Table 4.7 Input Cards for Sample Problem 3 - 64 Element Model (cont.)

	1		2		3		4		5		6		7		8		
	12345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	
36	62		15.24		25.73												Card V
37	63		0.0		27.58												Card V
38	71		15.24		27.58												Card V
39	72		0.0		29.21												Card V
40	80		15.24		29.21												Card V
41	81		0.0		30.48												Card V
42	89		15.24		30.48												Card V
43	1	1	2	7	6				1	13	1						Card VI
44	5	6	7	12	11				1	13	1						Card VI
45	9	11	12	17	16				1	13	1						Card VI
46	13	16	17	22	21				1	13	1						Card VI
47	17	21	22	27	26				1	13	1						Card VI
48	21	26	27	32	31				1	13	1						Card VI
49	25	36	37	46	45				1	13	1						Card VI
50	33	45	46	55	54				1	13	1						Card VI
51	41	54	55	64	63				1	13	1						Card VI
52	49	63	64	73	72				1	13	1						Card VI
53	57	72	73	82	81				1	13	1						Card VI
54	64	79	80	89	88				1	13	1						Card VI
55	10110																Card VII
56	60100																Card VII
57	110100																Card VII
58	160100																Card VII
59	210100																Card VII
60	260100																Card VII
61	310100																Card VII
62	450100																Card VII
63	540100																Card VII
64	630100																Card VII
65	720100																Card VII
66	810100																Card VII
67	1	8	7	1	1	5											Card VIIa
68		0.0		21.0	1.2E+2	190.0	1.2E+3	360.0	2.4E+3	510.0							Card VIIb-1
69		5.4E+3		550.0													Card VIIb-1
70	81																Card VIIc

Table 4.7 Input Cards for Sample Problem 3 - 68 Element Model (cont.)

	1			2			3			4			5			6			7			8			
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890			
71	1	5	9	1	1	1																	Card IXa		
72	31																						Card IXb		
73	36																						Card IXc		
74		1		2			14			1													Card Xa		
75		810204Z-DISP NODE 81																				Card Xb			
76		890104R-DISP NODE 89																				Card Xb			
77		570014EMR AT STA 1 ELE 57																				Card Xc			
78		570024EMZ AT STA 1 ELE 57																				Card Xc			
79		570044EMT AT STA 1 ELE 57																				Card Xc			
80		490044EMT AT STA 1 ELE 49																				Card Xc			
81		240044EMT AT STA 1 ELE 24																				Card Xc			
82		570054STR AT STA 1 ELE 57																				Card Xc			
83		570064STZ AT STA 1 ELE 57																				Card Xc			
84		570084STT AT STA 1 ELE 57																				Card Xc			
85		490084STT AT STA 1 ELE 49																				Card Xc			
86		240084STT AT STA 1 ELE 24																				Card Xc			
87		570154TEM AT STA 1 ELE 57																				Card Xc			
88		640154TEM AT STA 1 ELE 64																				Card Xc			
89		240154TEM AT STA 1 ELE 24																				Card Xc			
90		670014INTERNAL ENERGY																				Card Xc			
91		60		4																			Card Xd		

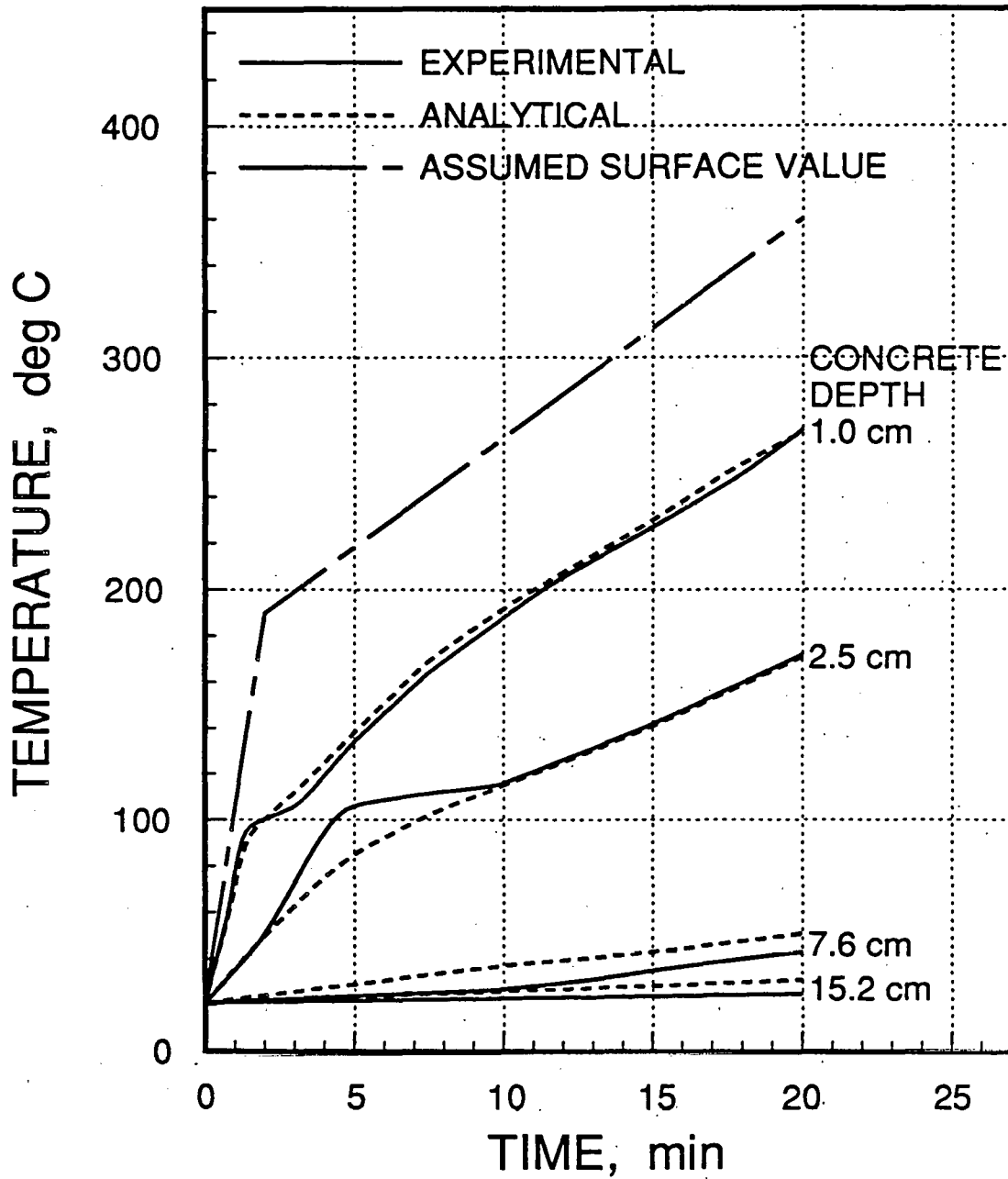


Figure 4.6 Analytical and Test Temperatures in Concrete Cylinder

/ CIRCUMFERENTIAL CRACKS

* RADIAL CRACKS

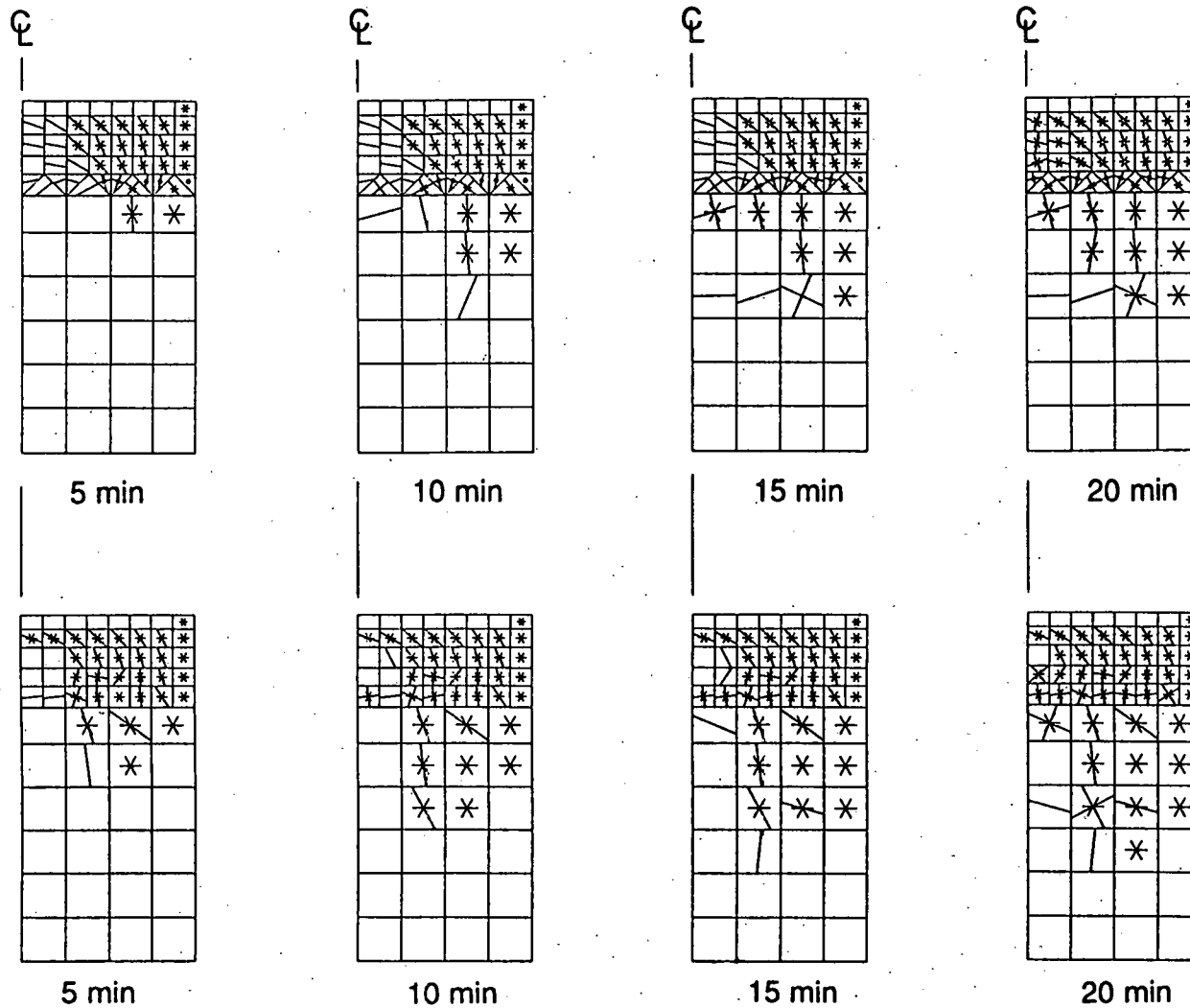


Figure 4.7 Cracking History of Concrete Cylinder Analytical Models

4.4 Sample Problem 4: Sandia Crucible Experiment

Dimensions of the test crucible employed by Acton, et al. (1979) are shown in Fig. 4.8. Liquid sodium of 600°C was dumped into the base cavity of the crucible resulting in a 13.2 cm deep pool. Measurements of temperature through the depth of the concrete slab were recorded and are reproduced in Fig. 4.9 which shows the temperature histories at different depths from the initial sodium-concrete interface. Posttest examination revealed an average surface erosion of approximately 5 cm deep in the crucible, as shown in Fig. 4.8. The cracking pattern of the crucible after the test is shown in Fig. 4.10.

The crucible model discretization used in the analysis is given in Fig. 4.11. The four-node quadrilateral continuum element with one point Gaussian integration is used. The temperature dependent material properties of basalt concrete used in this example are summarized in Table 4.5. Table 4.8 provides the input cards for this simulation.

The numerical solution yields temperature and stress results at each time step of the calculation. The temperature history of the crucible is compared with experimental results in Fig. 4.9. Cracking patterns are shown for times of 30 minutes, 60 minutes and 100 minutes in Fig. 4.11. Results of the cracking are in general, in good agreement with the observed cracks in Fig. 4.10.

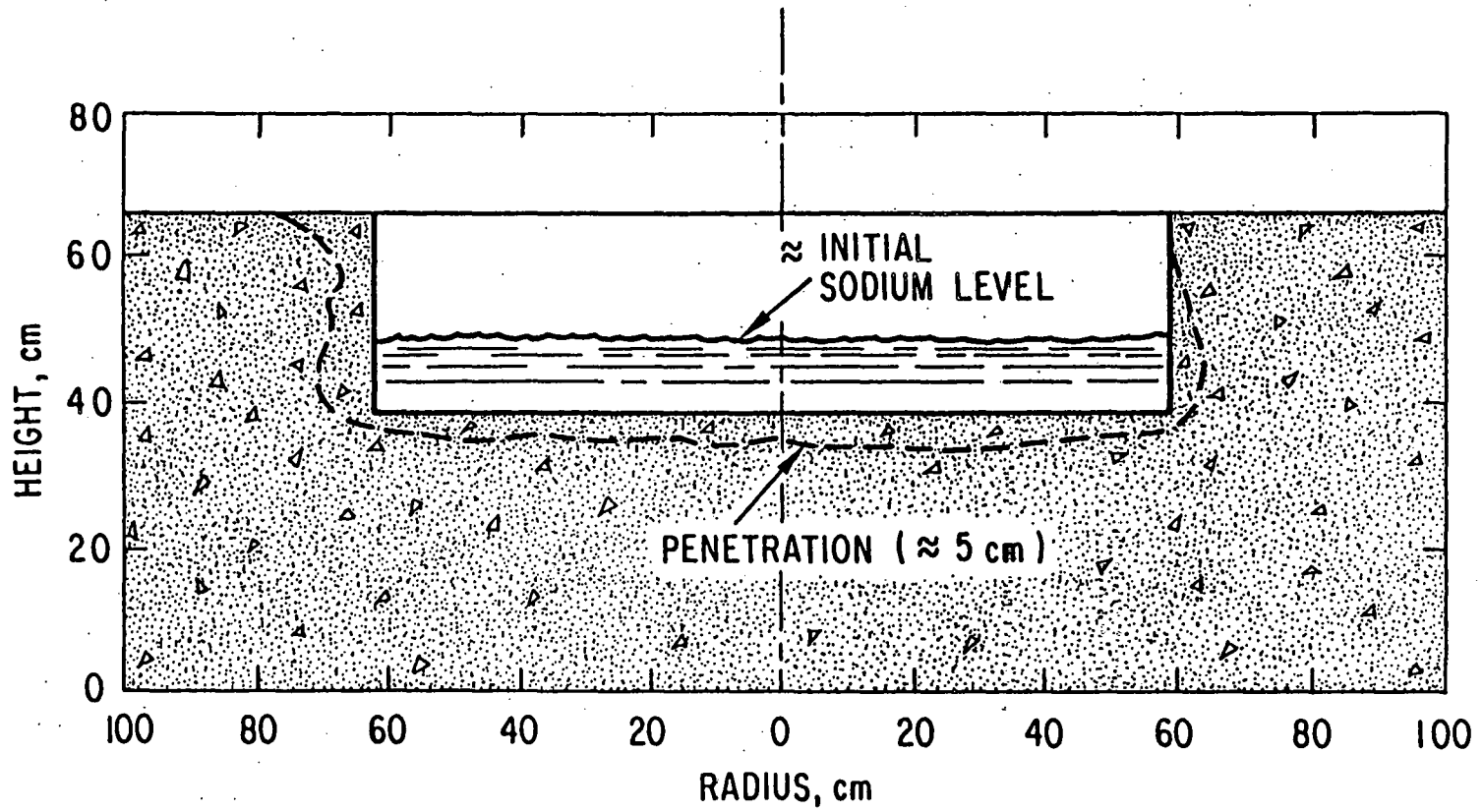


Figure 4.8 Test Crucible Configuration

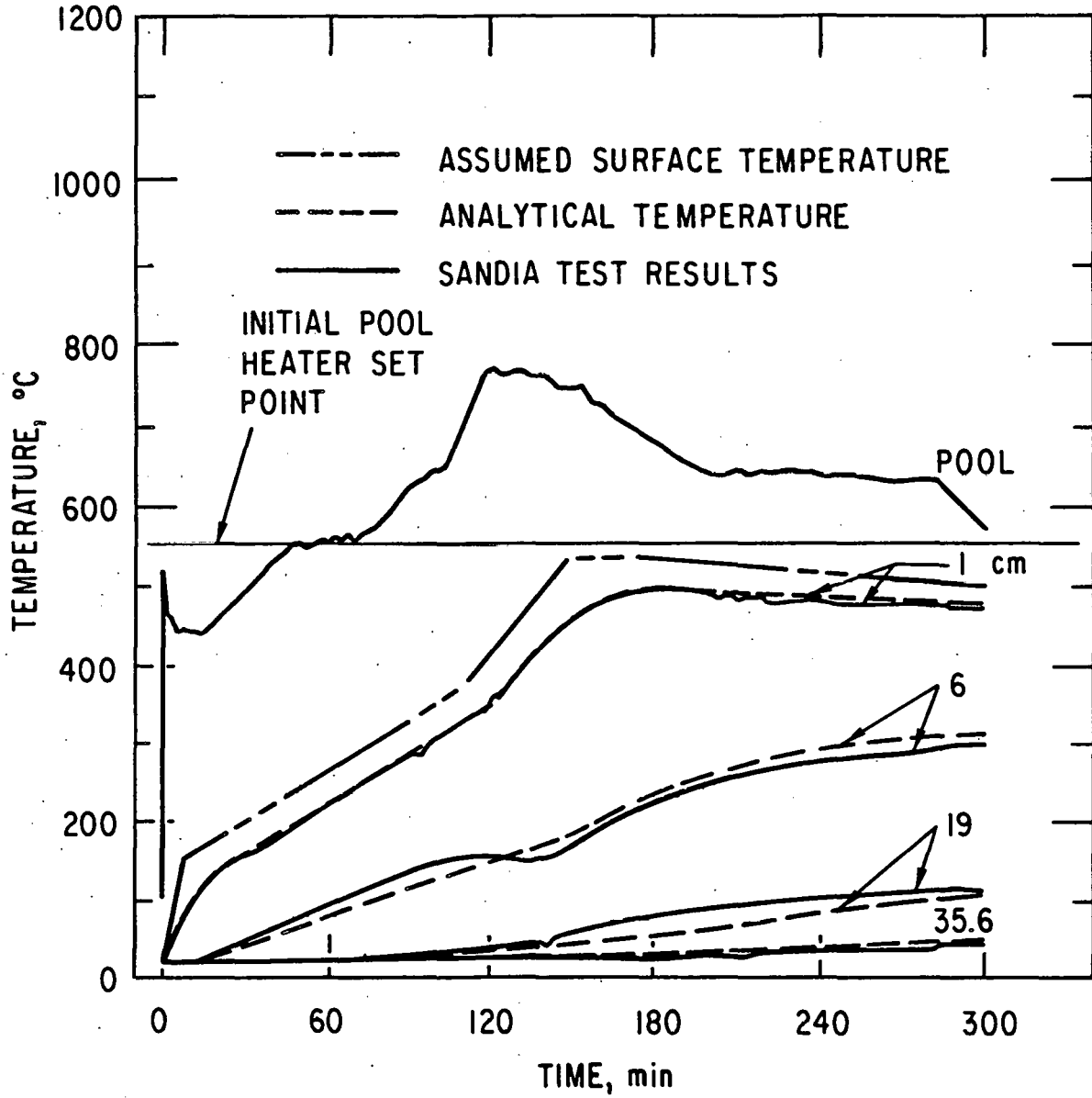
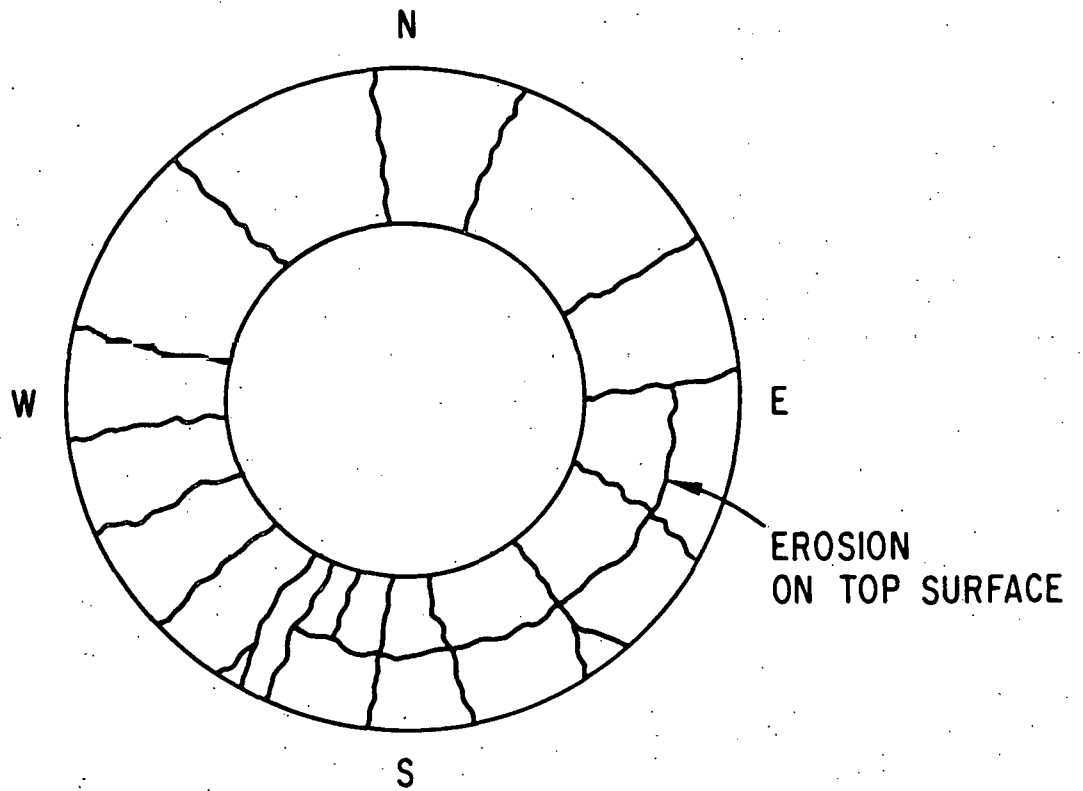


Figure 4.9 Experimental and Analytical Crucible Temperature Histories

A. TOP VIEW



B. SIDE VIEW

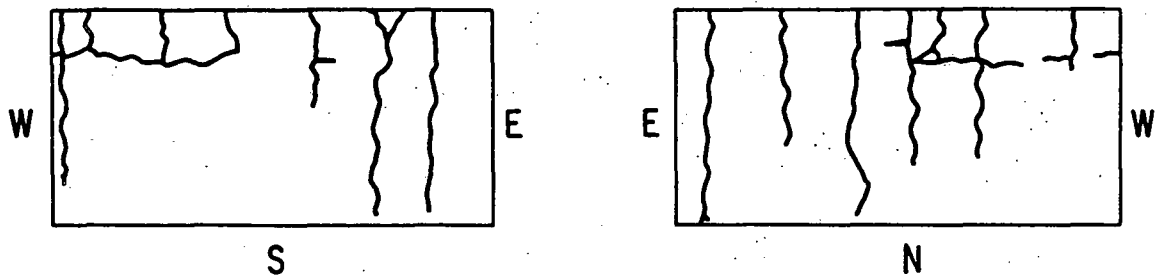


Figure 4.10 Crucible Test Cracking Pattern

/ CIRCUMFERENTIAL CRACKS

* RADIAL CRACKS

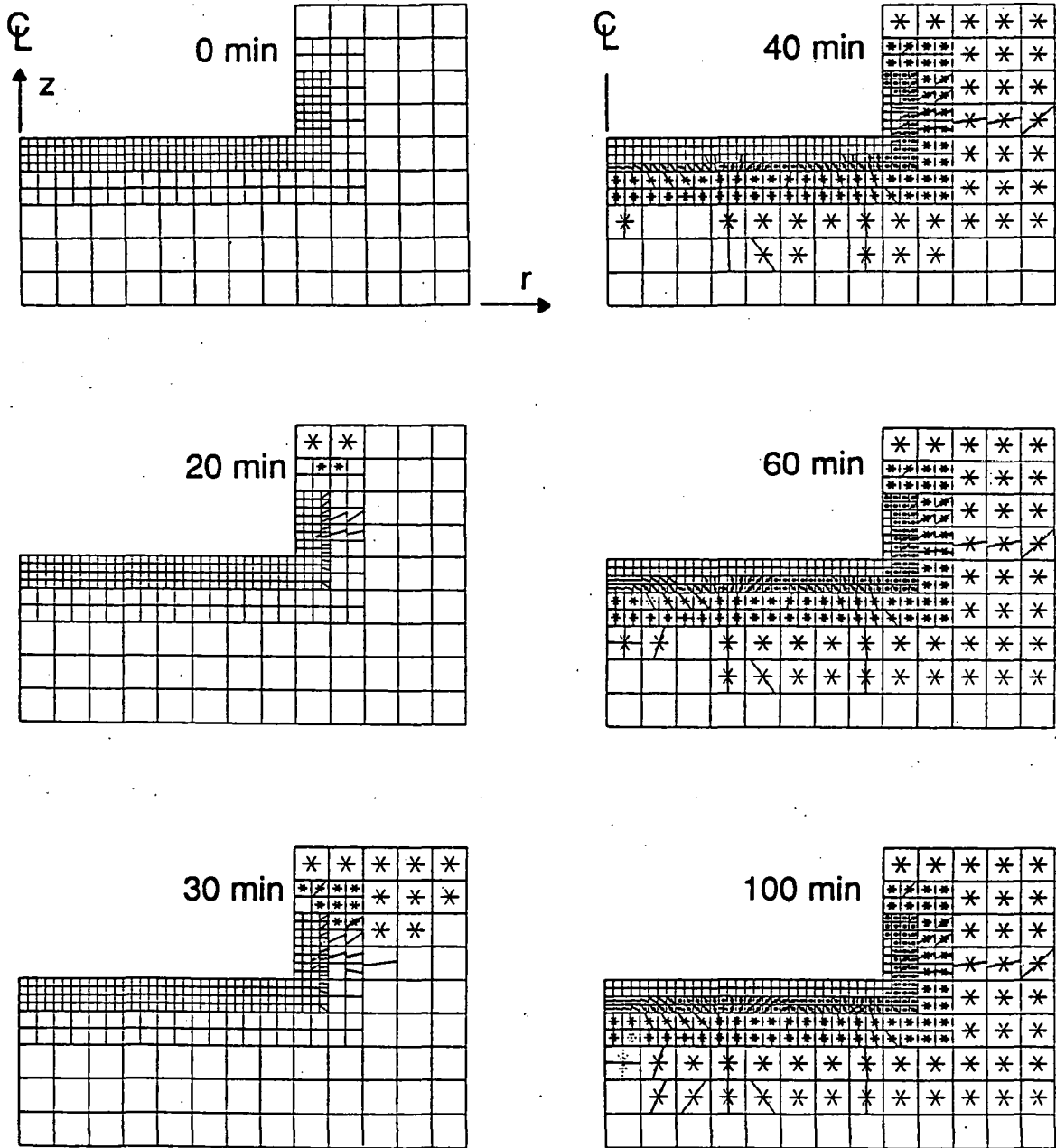


Figure 4.11 Analytical Model and Cracking Pattern of Crucible

Table 4.8 Input Cards for Sample Problem 4

	1		2		3		4		5		6		7		8		
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	
1	SANDIA CRUCIBLE EXPERIMENT - WITH THERMAL CALCULATIONS																Card I
2	402	295	1	10	100	2	60.0	1	2								Card II
3	2	0			90						2000	100	100	1			Card III
4	1	13															Card IVa-13
5	2.326		0.15		9.2E-6			0.0185		0.949		5.0		5.0			Card IVb-13
6			0.0		0.0		1.0	0.0		13.0		0.0		1.0			Card IVc-13
7	21.0	25.00E10	0.00070		1.75E8	0.00125		2.45E8		0.00165		2.70E8					Card IVd-13
8	0.00200	2.80E8	2.00000		2.80E8												Card IVd-13
9	93.0	16.92E10	0.00065		1.10E8	0.00120		1.70E8		0.00175		2.00E8					Card IVd-13
10	0.00220	2.10E8	2.00000		2.10E8												Card IVd-13
11	371.0	8.42E10	0.00095		0.80E8	0.00160		1.20E8		0.00240		1.45E8					Card IVd-13
12	0.00310	1.55E8	2.00000		1.55E8												Card IVd-13
13	538.0	2.81E10	0.00160		0.45E8	0.00310		0.70E8		0.00450		0.80E8					Card IVd-13
14	0.00600	0.85E8	2.00000		0.85E8												Card IVd-13
15	700.0	0.92E10	0.00240		0.22E8	0.00460		0.35E8		0.00680		0.40E8					Card IVd-13
16	0.00900	0.42E8	2.00000		0.42E8												Card IVd-13
17	1		0.0		0.0												Card V
18	9		61.0		0.0												Card V
19	14		99.0		0.0												Card V
20	19		99.0		38.0												Card V
21	23		99.0		68.0												Card V
22	24		0.0		7.6												Card V
23	32		61.0		7.6												Card V
24	36		91.4		7.6												Card V
25	40		91.4		38.0												Card V
26	44		91.4		68.0												Card V
27	45		0.0		15.2												Card V
28	53		61.0		15.2												Card V
29	56		83.8		15.2												Card V
30	59		83.8		38.0												Card V
31	63		83.8		68.0												Card V
32	64		0.0		22.8												Card V
33	72		61.0		22.8												Card V
34	74		76.2		22.8												Card V
35	76		76.2		38.0												Card V

Table 4.8 Input Cards for Sample Problem 4 (cont.)

	1			2			3			4			5			6			7			8			
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890			
71	314				0.0			36.1															Card V		
72	346				61.0			36.1															Card V		
73	347				62.9			36.1															Card V		
74	348				62.9			38.0															Card V		
75	356				62.9			53.0															Card V		
76	357				0.0			38.0															Card V		
77	389				61.0			38.0															Card V		
78	397				61.0			53.0															Card V		
79	398				61.0			53.0															Card V		
80	400				61.0			60.5															Card V		
81	401				61.0			60.5															Card V		
82	402				61.0			68.0															Card V		
83	1	1	2	25	24					1	13	1											Card VI		
84	13	13	14	15	36					1	13	1											Card VI		
85	14	36	15	16	37					1	13	1											Card VI		
86	22	24	25	46	45					1	13	1											Card VI		
87	33	35	36	37	56					1	13	1											Card VI		
88	34	56	37	38	57					1	13	1											Card VI		
89	41	45	46	65	64					1	13	1											Card VI		
90	51	55	56	57	74					1	13	1											Card VI		
91	52	74	57	58	75					1	13	1											Card VI		
92	58	81	82	113	112					1	13	1											Card VI		
93	77	100	101	102	131					1	13	1											Card VI		
94	78	131	102	103	132					1	13	1											Card VI		
95	87	112	113	142	141					1	13	1											Card VI		
96	105	130	131	132	159					1	13	1											Card VI		
97	106	159	132	133	160					1	13	1											Card VI		
98	114	168	79	80	169					1	13	1											Card VI		
99	115	170	171	220	219					1	13	1											Card VI		
100	150	205	206	207	254					1	13	1											Card VI		
101	151	254	207	208	255					1	13	1											Card VI		
102	162	219	220	267	266					1	13	1											Card VI		
103	196	253	254	255	300					1	13	1											Card VI		
104	197	300	255	256	301					1	13	1											Card VI		
105	207	311	165	166	312					1	13	1											Card VI		

Table 4.8 Input Cards for Sample Problem 4 (cont.)

	1			2			3			4			5			6			7			8				
	12345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901		
106	209	266	267	315	314									1	13	1										Card VI
107	242	299	300	301	347									1	13	1										Card VI
108	243	347	301	302	348									1	13	1										Card VI
109	252	314	315	358	357									1	13	1										Card VI
110	284	346	347	348	389									1	13	1										Card VI
111	285	389	348	349	390									1	13	1										Card VI
112	293	398	311	312	399									1	13	1										Card VI
113	295	401	168	169	402									1	13	1										Card VI
114		10100																								Card VII
115		240100																								Card VII
116		450100																								Card VII
117		640100																								Card VII
118		1120100																								Card VII
119		1410100																								Card VII
120		2190100																								Card VII
121		2660100																								Card VII
122		3140100																								Card VII
123		3570100																								Card VII
124		1 39	7	1	1	6																				Card VIIIa
125		0.0		21.0	0.06E+4		160.0		0.66E+4		370.0		0.90E+4		530.0											Card VIIIb-1
126		1.08E+4		530.0	1.8001E+4		500.0																			Card VIIIb-1
127		357																								Card VIIIc
128		1 18	35	0	0	1																				Card IXa
129		64 65	66	67	68	69	70	71	72	73	74	75	76	77	78	79										Card IXb
130		168 401																								Card IXb
131		81 82	83	84	85	86	87	88	89	90	91	92	93	94	95	96										Card IXc
132		97 98	99	100	101	102	103	104	105	106	107	108	109	110	111	140										Card IXc
133		167 313	400																							Card IXc
134		2 27	53	0	0	1																				Card IXa
135		141 142	143	144	145	146	147	148	149	150	151	152	153	154	155	156										Card IXb
136		157 158	159	160	161	162	163	164	165	311	398															Card IXb
137		170 171	172	173	174	175	176	177	178	179	180	181	182	183	184	185										Card IXc
138		186 187	188	189	190	191	192	193	194	195	196	197	198	199	200	201										Card IXc
139		202 203	204	205	206	207	208	209	210	211	212	213	214	215	216	217										Card IXc
140		218 265	310	356	397																					Card IXc

Table 4.8 Input Cards for Sample Problem 4 (cont.)

	1	2	3	4	5	6	7	8	
	1234567890123456789012345678901234567890123456789012345678901234567890								
141	1	3	8	1					Card Xa
142	3570204Z-DISP	NODE	357						Card Xb
143	230104R-DISP	NODE	23						Card Xb
144	230204Z-DISP	NODE	23						Card Xb
145	2520064STZ	AT	STA 1	ELE	252				Card Xc
146	2090064STZ	AT	STA 1	ELE	209				Card Xc
147	1620064STZ	AT	STA 1	ELE	162				Card Xc
148	870064STZ	AT	STA 1	ELE	87				Card Xc
149	2520154TEM	AT	STA 1	ELE	252				Card Xc
150	1620154TEM	AT	STA 1	ELE	162				Card Xc
151	410154TEM	AT	STA 1	ELE	41				Card Xc
152	2970014	APPLIED	TEMPERATURE						Card Xc
153	100	4							Card Xd

4.5 Sample Problem 5: Reinforced Concrete Beam

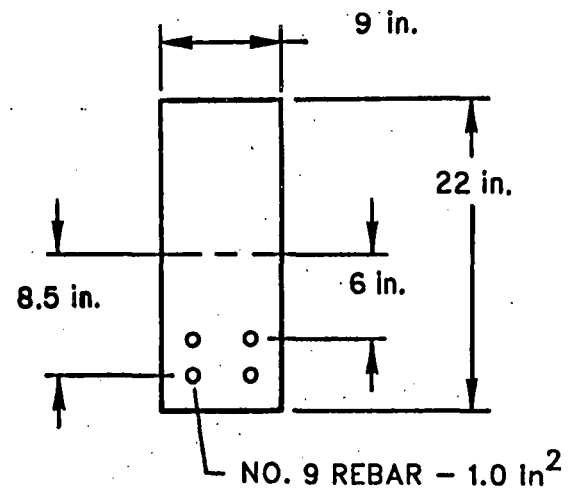
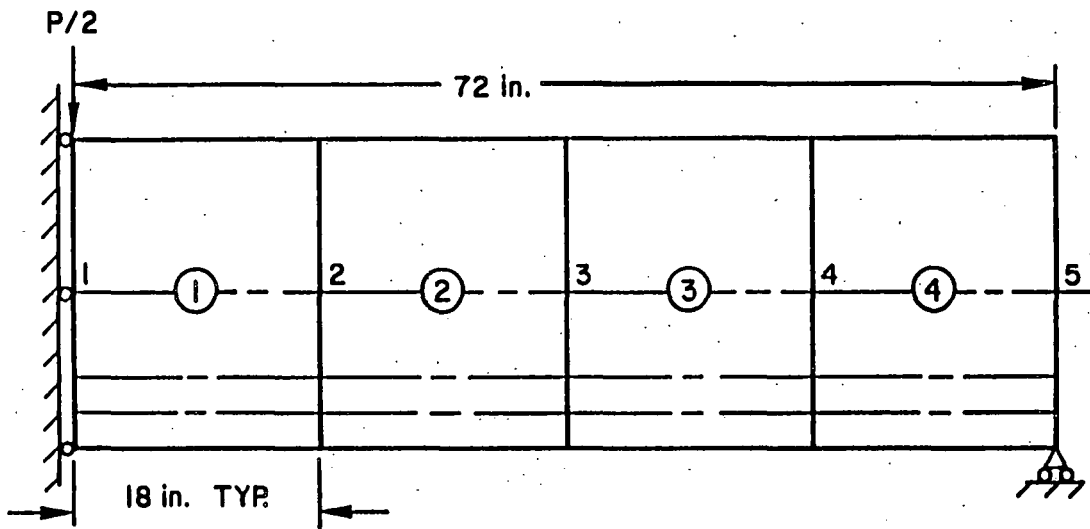
The reinforced beam element and reinforced continuum element are investigated in comparison with test results given by Bresler and Scordelis (1964) of a reinforced concrete slender beam. The test data pertains to a simply supported concrete beam reinforced by four steel bars of 1.0 in² area each, as shown in Fig. 4.12. The beam is subjected to a gradually increasing mid-span load.

The analytical models take advantage of the symmetry of the test specimen and model one-half of the beam. It represents the beam from the center to the support. The material properties are given in Table 4.9. In the first model four reinforced elements (2 layers of reinforcement) of equal size are utilized. The input cards for this mesh are given in Table 4.10. The second model consists of 40 continuum elements with smeared reinforcement in the bottom two layers. The input cards for this mesh are given in Table 4.11.

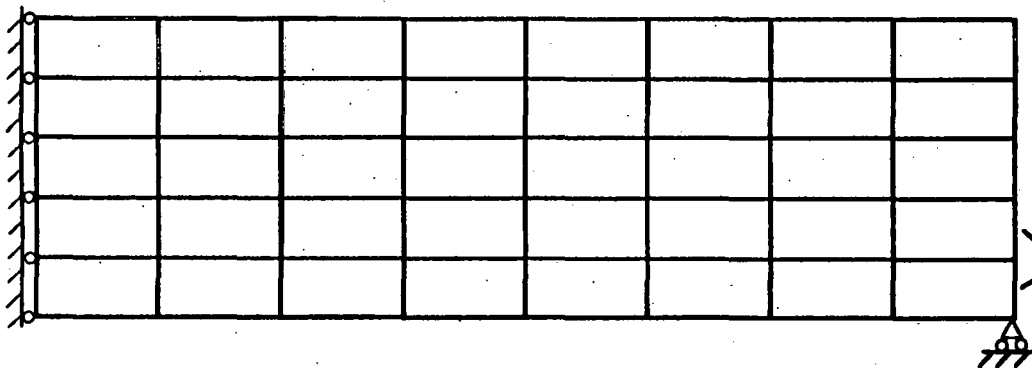
A comparison of test load-deflection and the analytical predictions is shown in Fig. 4.13. The deviations of the analytical results from the test data, as loading increases, are attributed to shear cracking and debonding limitations in the analytical models. The agreement of test data and analytical results is quite good over the range for which the formulation was intended.

Table 4.9 Material Properties for Concrete Beam (Sample Problem 5)

Concrete Young's modulus	$E = 3.1 \times 10^6$ psi
Concrete Poisson's ratio	$\nu = 0.2$
Concrete compression strength	$f'_c = 3500$ psi
Concrete tensile strength	$f'_t = 333$ psi
Concrete tensile failure strain	$\epsilon_{ut} = 0.0007$ in/in
Rebar Young's modulus	$E = 27.8 \times 10^6$ psi
Rebar yield stress	$\sigma_y = 6.0 \times 10^4$ psi
Rebar ultimate stress	$\sigma_u = 1.44 \times 10^5$ psi



(a)



(b)

Figure 4.12 Simply Supported Reinforced Concrete Beam Models

Table 4.10 Input Cards for Sample Problem 5 - Beam Element Model

	1			2			3			4			5			6			7			8			
	12345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901		
1	REINFORCED CONCRETE BEAM - BEAM ELEMENT MODEL																							Card I	
2	10	4	1	3	25	2	1.0	1															Card II		
3	2	0	90						1000						100	100	100	1						Card III	
4	1	4																					Card IVa-4		
5	2.25E-4	0.2														1.0	4.0						Card IVb-4		
6	9.0	22.0	0.0			1.0	0.0	11.0	3.0	5.0													Card IVc-4		
7	70.0	3.25E6	0.000667			2.2E3	0.00152	3.4E3	0.002	3.5E3													Card IVd-4		
8	1.0	3.5E3																					Card IVd-4		
9	1.0	0.11428	1.15			0.8	4.2	-0.0007	-0.006														Card IVe-4		
10	7.4E-4	1.0														Card IVg-4									
11	70.0	2.778E7	0.00216			6.0E4	0.0056	9.6E4	0.02	13.3E4													Card IVh-4		
12	0.06	14.4E4																					Card IVh-4		
13	2.0																					Card IVi-4			
14	2.0	-8.5	0.0																		Card IVj-4				
15	2.0	-6.0	0.0																		Card IVj-4				
16	1	0.0	0.0																		Card V				
17	5	72.0	0.0																		Card V				
18	6	0.0	0.0																		Card V				
19	10	0.0	0.0																		Card V				
20	1	1	2	7	6							1	4	1								Card VI			
21	4	4	5	10	9							1	4	1								Card VI			
22	10100																						Card VII		
23	50010																						Card VII		
24	60110																						Card VII		
25	1	1	4	0	0	1																	Card VIIa		
26	30.0	-40000.0																				Card VIIb-1			
27	1																						Card VIIc		
28	1	3	1	1																			Card Xa		
29	10204Y-DISP NODE 1																							Card Xb	
30	10234Y-FORCE NODE 1																							Card Xb	
31	50244Y-FORCE NODE 5																							Card Xb	
32	90024INTERNAL ENERGY																							Card Xc	
33	25	4																					Card Xd		

Table 4.11 Input Cards for Sample Problem 5 - Continuum Element Model

	1			2			3			4			5			6			7			8			
	12345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901		
1	REINFORCED CONCRETE BEAM - CONTINUUM ELEMENT MODEL																							Card I	
2	54	40	2	7	25	2	1.0	1															Card II		
3	2	0	90																	Card III					
4	1	3															Card IVa-3								
5	2.25E-4	0.2																	Card IVb-3						
6	9.0	0.0	0.0			1.0	0.0	11.0	3.0	1.0													Card IVc-3		
7	70.0	3.25E6	0.000667			2.2E3	0.00152	3.4E3	0.002	3.5E3													Card IVd-3		
8	1.0	3.5E3																	Card IVd-3						
9	1.0	0.11428			1.15	0.8	4.2	-0.0007	-0.006														Card IVe-3		
10	7.4E-4	1.0			4.0															Card IVg-3					
11	70.0	2.778E7	0.00216			6.0E4	0.0056	9.6E4	0.02	13.3E4													Card IVh-3		
12	0.06	14.4E4																	Card IVh-3						
13	2	3															Card IVa-3								
14	2.25E-4	0.2																	Card IVb-3						
15	9.0	0.0	0.0			1.0	0.0	11.0	1.0	1.0													Card IVc-3		
16	70.0	3.25E6	0.000667			2.2E3	0.00152	3.4E3	0.002	3.5E3													Card IVd-3		
17	1.0	3.5E3																	Card IVd-3						
18	1.0	0.11428			1.15	0.8	4.2	-0.0007	-0.006														Card IVe-3		
19	1	0.0			0.0															Card V					
20	9	72.0			0.0															Card V					
21	10	0.0			4.4															Card V					
22	18	72.0			4.4															Card V					
23	19	0.0			8.8															Card V					
24	27	72.0			8.8															Card V					
25	28	0.0			13.2															Card V					
26	36	72.0			13.2															Card V					
27	37	0.0			17.6															Card V					
28	45	72.0			17.6															Card V					
29	46	0.0			22.0															Card V					
30	54	72.0			22.0															Card V					
31	1	1	2	11	10	505	1	3	1														Card VI		
32	9	10	11	20	19	505	1	3	1														Card VI		
33	17	19	20	29	28		2	3	1														Card VI		
34	25	28	29	38	37		2	3	1														Card VI		
35	33	37	38	47	46		2	3	1														Card VI		

Table 4.11 Input Cards for Sample Problem 5 - Continuum Element Model (cont.)

	1			2			3			4			5			6			7			8			
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890			
36	40	44	45	54	53									2	3	1							Card VI		
37		10100																					Card VII		
38		100100																					Card VII		
39		190100																					Card VII		
40		280100																					Card VII		
41		370100																					Card VII		
42		460100																					Card VII		
43		90010																					Card VII		
44		1	1	4	0	0	1																Card VIIIa		
45	30.0			-40000.0																			Card VIIIb-1		
46	46																						Card VIIIc		
47		1		3			1			1													Card Xa		
48		460204Y-DISP NODE 46																				Card Xb			
49		460234Y-FORCE NODE 46																				Card Xb			
50		90244Y-FORCE NODE 9																				Card Xb			
51		450024INTERNAL ENERGY																				Card Xc			
52		25		4																			Card Xd		

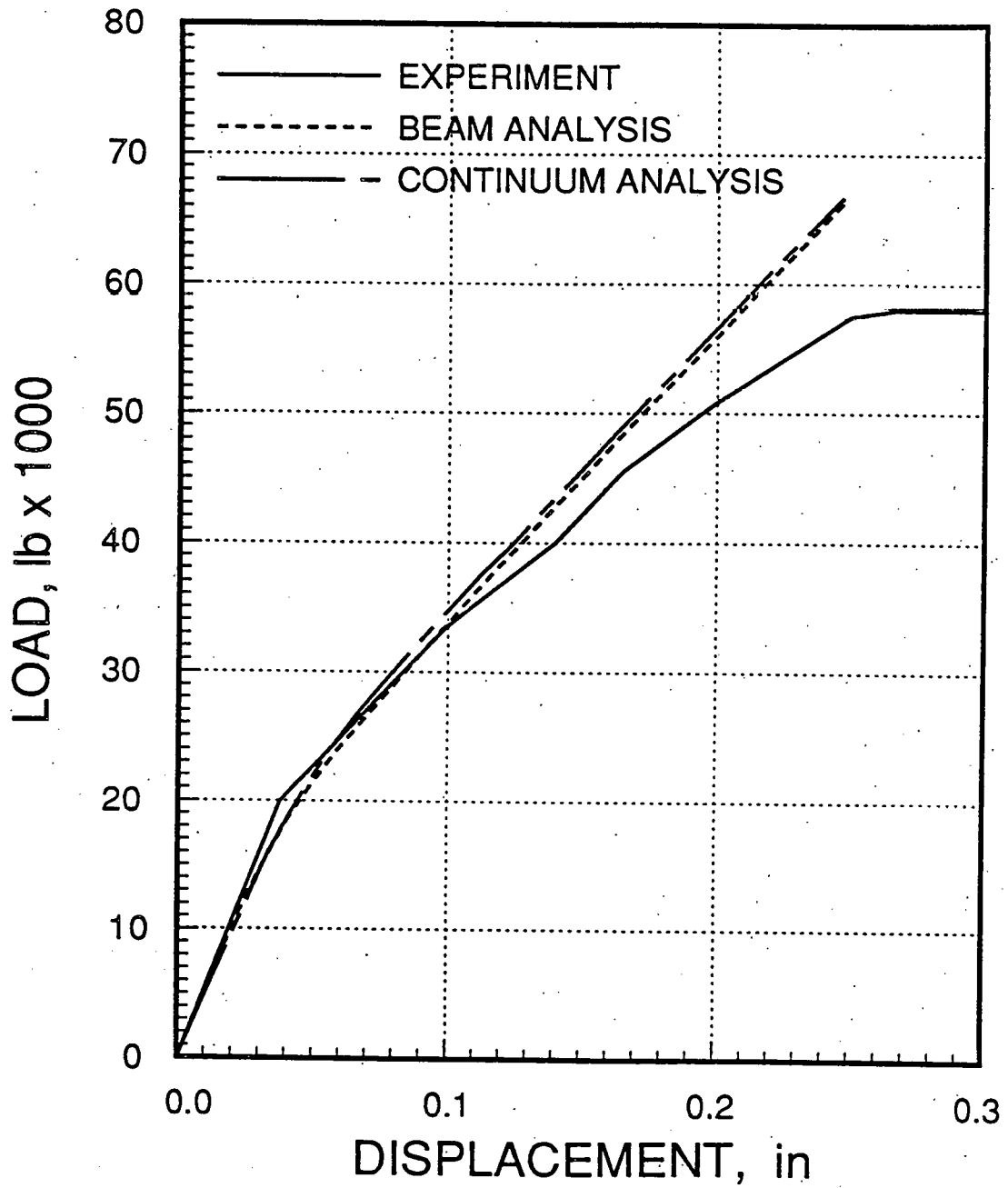


Figure 4.13 Load Versus Deflection for Concrete Beam

4.6 Sample Problem 6: Reinforced Concrete Containment

A scaled reinforced concrete containment model is shown in Fig. 4.14. The analysis of this model is considered for the following three different loading conditions: (1) static internal pressurization to failure with the complete structure at 70°F, (2) static internal pressurization to failure with the structure at 400°F internally and 70°F externally (linear variation thru the thickness) with a steady state thermal field, (3) dynamic threshold failure pressure subjected internally.

The finite element model shown in Fig. 4.14 consists only of the axisymmetric cylindrical vessel and a spherical dome. The size of the containment is given in Table 4.12. This model consists of 31 reinforced concrete shell elements: 13 of them representing the 8 inch thick spherical dome, and 18 elements representing the 8 inch thick cylinder. A liner on the inside surface of the vessel is made up of steel elements. The liner elements, which are offset from the concrete elements through a rigid connection, have a thickness of 0.10 inches in the dome and in the cylinder. The number of reinforcement layers in each concrete shell element is four. Two hoop rebar of 1.0 in² and two meridional rebar of 12.0 in² in area are used. The material properties for the concrete, rebar and liner are given in Tables 4.13 to 4.15, respectively for the temperature range discussed above. The input cards for the three loadings are given in Tables 4.16 to 4.18.

Table 4.12 Dimensions of Containment (Sample Problem 6)

Cylinder

Concrete shell radius $r_c = 136$ in

Steel shell radius $r_s = 132$ in

Concrete shell height $h_c = 297$ in

Dome

Concrete shell radius $r_c = 136$ in

Steel shell radius $r_s = 132$ in

Note: All radii measured to the shell centerline.

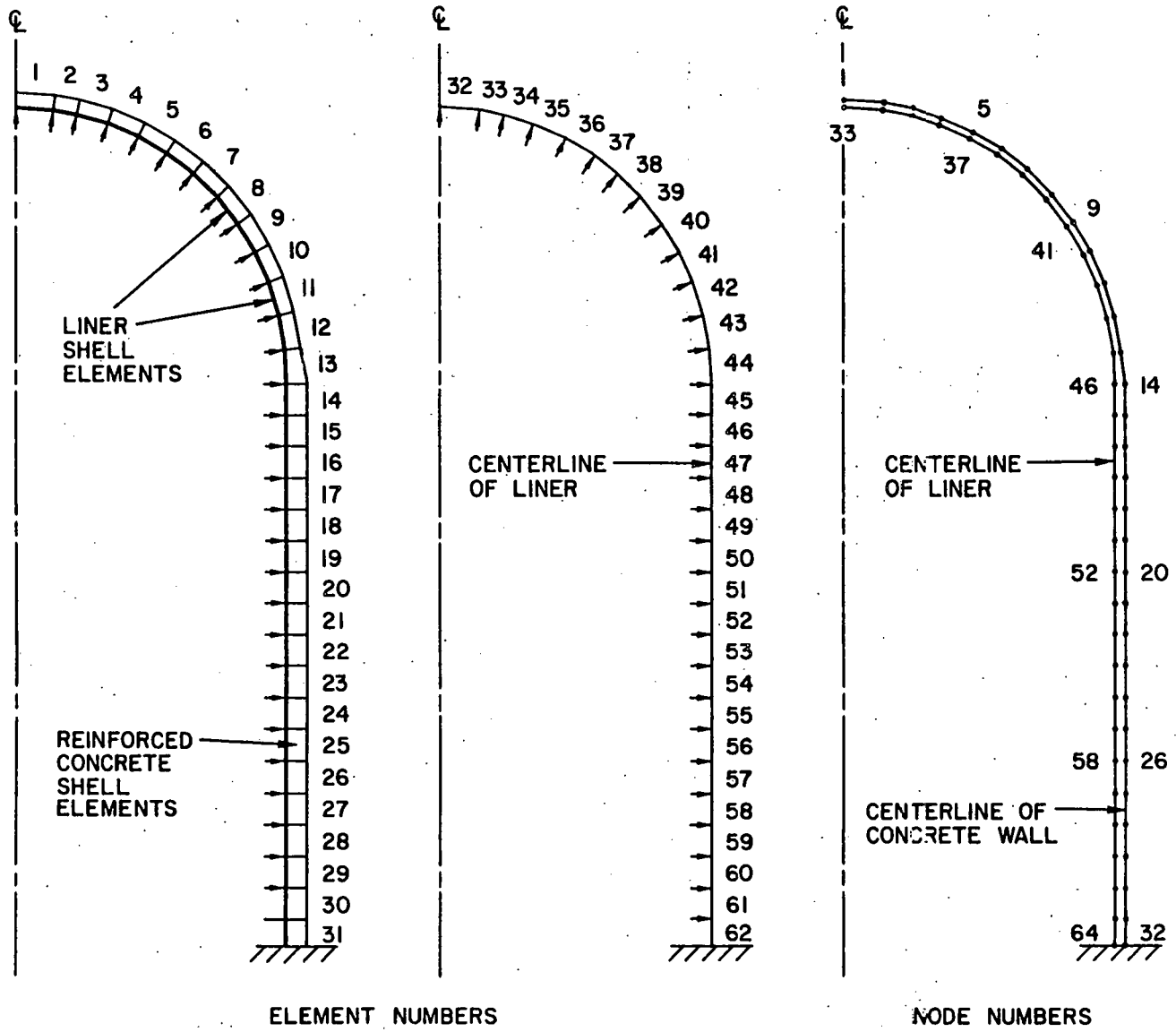


Figure 4.14 Scaled Reinforced Concrete Containment Model

Table 4.13 Concrete Material Properties at Various Temperatures
(Sample Problem 6)

Material Property	Temperature (°F)		
	70	250	700
Young's modulus (ksi)	4800	2300	1200
Poisson's ratio	0.2	0.2	0.2
Tensile strength (psi)	500	353	250
Coefficient of thermal expansion (in/in/°F)	5.5×10^{-6}	5.5×10^{-6}	5.5×10^{-6}
Unconfined compressive stress-strain curve (engineering values)			
	70	250	700
Yield stress (psi)	3400	2400	1700
Ultimate stress (psi)	6800	4800	3400
Strain at ultimate stress (in/in)	2.3×10^{-3}	2.97×10^{-3}	3.72×10^{-3}
Ultimate strain at zero strength (in/in)	0.006	0.006	0.006

Table 4.14 Rebar Material Properties at Various Temperatures
(Sample Problem 6)

Material Property	Temperature (°F)		
	70	400	800
Young's modulus (ksi)	31000	28500	25700
Coefficient of thermal expansion (in/in/°F)	6.2×10^{-6}	6.2×10^{-6}	6.2×10^{-6}
Bilinear engineering stress-strain curve	70	400	800
Yield stress (psi)	66.6	56.6	51.3
Ultimate stress (psi)	82.3	73.1	61.2
Ultimate strain (in/in)	0.02	0.02	0.02

Table 4.15 Liner Material Properties at Various Temperatures
(Sample Problem 6)

Material Property	Temperature (°F)		
	70	400	800
Young's modulus (ksi)	30000	27900	25200
Poisson's ratio	0.3	0.3	0.3
Coefficient of thermal expansion (in/in/°F)	6.5×10^{-6}	6.5×10^{-6}	6.5×10^{-6}
Bilinear engineering stress-strain curve	70	400	800
Yield stress (psi)	50.2	45.2	40.2
Ultimate stress (psi)	56.0	51.0	44.9
Ultimate strain (in/in)	0.03	0.03	0.03

Table 4.16 Input Cards for Sample Problem 6 - Static Loading

	1		2		3		4		5		6		7		8		
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	
1	REINFORCED CONCRETE SHELL VESSEL - STATIC LOADING																Card I
2	128	62	2	4	10	2	1.0	1	1								Card II
3	2	0			90						10000	200	10	1			Card III
4	1	14															Card IVa-14
5	2.25E-4		0.2										1.0		3.0		Card IVb-14
6			8.0	0.0		1.0		13.0		3.0					5.0		Card IVc-14
7	70.0	4.8E6	7.0833E-4	3.4E3	0.0023	6.8E3	1.0	6.8E3	1.0	6.8E3							Card IVd-14
8	1.0	0.07353	1.15	0.8	4.2	-0.0007	-0.006										Card IVe-14
9	7.5E-4		1.0	2.0													Card IVg-14
10	70.0	3.1E7	0.002148	6.66E4	0.02	8.23E4											Card IVh-14
11	4.0																Card IVi-14
12	12.0	-2.5															Card IVj-14
13	1.0	-3.0	90.0														Card IVj-14
14	12.0	2.5															Card IVj-14
15	1.0	3.0	90.0														Card IVj-14
16	2	11															Card IVa-11
17	7.5E-4		0.3									1.0		2.0			Card IVb-11
18		0.1	0.0	0.0				3.0		2.0				5.0			Card IVc-11
19	70.0	3.0E7	0.001673	5.02E4	0.03	5.6E4											Card IVd-11
20	1	1	136.0	90.0	0.0	297.0											Card V
21	14	1	136.0	0.0	0.0	297.0											Card V
22	15		136.0	280.5													Card V
23	32		136.0	0.0													Card V
24	33	1	132.0	90.0	0.0	297.0											Card V
25	46	1	132.0	0.0	0.0	297.0											Card V
26	47		132.0	280.5													Card V
27	64		132.0	0.0													Card V
28	65		0.0	0.0													Card V
29	128		0.0	0.0													Card V
30	1	1	2	66	65			1	14	1							Card VI
31	32	33	34	98	97			2	11	1							Card VI
32	62	63	64	128	127			2	11	1							Card VI
33	10	100															Card VII
34	32	0110															Card VII
35	65	0110															Card VII

Table 4.16 Input Cards for Sample Problem 6 - Static Loading (cont.)

	1		2		3		4		5		6		7		8		
	12345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	
36	960110																Card VII
37	1 32	1	3	1	2												Card VIIa
38	0.0	0.0	100.0	-500.0													Card VIIb-1
39	33																Card VIIc
40	1 32	32	1	1	3												Card IXa
41	1																Card IXb
42	33																Card IXc
43	1	5	0	1													Card Xa
44	10204Z-DISP	NODE 1															Card Xb
45	140104R-DISP	NODE 14															Card Xb
46	140204Z-DISP	NODE 14															Card Xb
47	230104R-DISP	NODE 23															Card Xb
48	230204Z-DISP	NODE 23															Card Xb
49	10	4															Card Xd

Table 4.17 Input Cards for Sample Problem 6 - Thermomechanical Loading

	1			2			3			4			5			6			7			8			
	12345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901		
1	REINFORCED CONCRETE SHELL VESSEL - THERMOMECHANICAL LOADING																							Card I	
2	128	62	2	4	34	2	1.0	6	1															Card II	
3	2	0			90									10000	200	10	1							Card III	
4	1	14																						Card IVa-14	
5	2.25E-4		0.2		5.5E-6												3.0			3.0				Card IVb-14	
6			8.0		0.0		1.0							13.0			3.0			5.0				Card IVc-14	
7	70.0		4.8E6	7.0833E-4			3.4E3	0.0023						6.8E3			1.0		6.8E3					Card IVd-14	
8	250.0		2.3E6	1.0435E-3			2.4E3	0.0029703						4.8E3			1.0		4.8E3					Card IVd-14	
9	700.0		1.2E6	1.4167E-3			1.7E3	0.0037167						3.4E3			1.0		3.4E3					Card IVd-14	
10	1.0		0.07353		1.15		0.8		4.2					-0.0007			-0.006							Card IVe-14	
11	7.5E-4		6.2E-6		3.0		2.0																	Card IVg-14	
12	70.0		3.10E7	0.002148			6.66E4	0.02						8.23E4										Card IVh-14	
13	400.0		2.85E7	0.001986			5.66E4	0.02						7.31E4										Card IVh-14	
14	800.0		2.57E7	0.001996			5.13E4	0.02						6.12E4										Card IVh-14	
15	4.0																							Card IVi-14	
16	12.0		-2.5																					Card IVj-14	
17	1.0		-3.0		90.0																			Card IVj-14	
18	12.0		2.5																					Card IVj-14	
19	1.0		3.0		90.0																			Card IVj-14	
20	2	11																						Card IVa-11	
21	7.5E-4				0.3		6.5E-6										3.0			2.0				Card IVb-11	
22			0.1		0.0		0.0							3.0			2.0			5.0				Card IVc-11	
23	70.0		3.00E7	0.001673			5.02E4	0.03						5.60E4										Card IVd-11	
24	400.0		2.79E7	0.001620			4.52E4	0.03						5.10E4										Card IVd-11	
25	800.0		2.52E7	0.001595			4.02E4	0.03						4.49E4										Card IVd-11	
26	1	1	136.0		90.0		0.0		297.0															Card V	
27	14	1	136.0		0.0		0.0		297.0															Card V	
28	15		136.0		280.5																			Card V	
29	32		136.0		0.0																			Card V	
30	33	1	132.0		90.0		0.0		297.0															Card V	
31	46	1	132.0		0.0		0.0		297.0															Card V	
32	47		132.0		280.5																			Card V	
33	64		132.0		0.0																			Card V	
34	65		0.0		0.0																			Card V	
35	128		0.0		0.0																			Card V	

Table 4.17 Input Cards for Sample Problem 6 - Thermomechanical Loading (cont.)

	1			2			3			4			5			6			7			8				
	12345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901			
36	1	1	2	66	65									1	14	1									Card VI	
37	32	33	34	98	97									2	11	1									Card VI	
38	62	63	64	128	127									2	11	1									Card VI	
39	10	100																							Card VII	
40	32	0	110																						Card VII	
41	65	0	110																						Card VII	
42	96	0	110																						Card VII	
43	1	32	1	3	1	2																			Card VIIa	
44		10.0		0.0		110.0																			Card VIIb-1	
45	33																								Card VIIc	
46	2	32	7	3	5	4																			Card VIIa	
47		0.0		70.0		10.0					400.0			200.0		400.0					300.0		400.0		Card VIIb-1	
48	1																								Card VIIc	
49	3	32	7	3	5	4																			Card VIIa	
50		0.0		70.0		10.0					317.5			200.0		317.5					300.0		317.5		Card VIIb-1	
51	2																								Card VIIc	
52	4	32	7	3	5	4																			Card VIIa	
53		0.0		70.0		10.0					235.0			200.0		235.0					300.0		235.0		Card VIIb-1	
54	3																								Card VIIc	
55	5	32	7	3	5	4																			Card VIIa	
56		0.0		70.0		10.0					152.5			200.0		152.5					300.0		152.5		Card VIIb-1	
57	4																								Card VIIc	
58	6	160	7	3	1	4																			Card VIIa	
59		0.0		70.0		10.0					400.0			200.0		400.0					300.0		400.0		Card VIIb-1	
60	161																								Card VIIc	
61	1	32	32	1	1	3																			Card IXa	
62	1																									Card IXb
63	33																									Card IXc
64		1		5		0					1														Card Xa	
65		10204Z-DISP		NODE	1																					Card Xb
66		140104R-DISP		NODE	14																					Card Xb
67		140204Z-DISP		NODE	14																					Card Xb
68		230104R-DISP		NODE	23																					Card Xb
69		230204Z-DISP		NODE	23																					Card Xb
70		33		4																						Card Xd

Table 4.18 Input Cards for Sample Problem 6 - Dynamic Loading

	1			2			3			4			5			6			7			8			
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890		
1	REINFORCED CONCRETE SHELL VESSEL - DYNAMIC LOADING																							Card I	
2	128	62	2	4	1200	2	6.0E-5	1	1															Card II	
3	1	0																						Card III	
4	1	14																						Card IVa-14	
5	2.25E-4		0.2														1.0				3.0			Card IVb-14	
6			8.0		0.0		1.0									13.0		3.0			5.0			Card IVc-14	
7	70.0		4.8E6	7.0833E-4		3.4E3	0.0023								6.8E3		1.0				6.8E3			Card IVd-14	
8	1.0		0.07353		1.15		0.8		4.2						-0.0007						-0.006			Card IVe-14	
9	7.5E-4				1.0		2.0																	Card IVg-14	
10	70.0		3.1E7	0.002148		6.66E4	0.02								8.23E4									Card IVh-14	
11	4.0																							Card IVi-14	
12	12.0		-2.5																					Card IVj-14	
13	1.0		-3.0		90.0																			Card IVj-14	
14	12.0		2.5																					Card IVj-14	
15	1.0		3.0		90.0																			Card IVj-14	
16	2	11																						Card IVa-11	
17	7.5E-4				0.3													1.0			2.0			Card IVb-11	
18			0.1		0.0		0.0								3.0		2.0				5.0			Card IVc-11	
19	70.0		3.0E7	0.001673		5.02E4	0.03							5.6E4										Card IVd-11	
20	1	1	136.0		90.0		0.0		297.0															Card V	
21	14	1	136.0		0.0		0.0		297.0															Card V	
22	15		136.0		280.5																			Card V	
23	32		136.0		0.0																			Card V	
24	33	1	132.0		90.0		0.0		297.0															Card V	
25	46	1	132.0		0.0		0.0		297.0															Card V	
26	47		132.0		280.5																			Card V	
27	64		132.0		0.0																			Card V	
28	65		0.0		0.0																			Card V	
29	128		0.0		0.0																			Card V	
30	1	1	2	66	65				1	14	1													Card VI	
31	32	33	34	98	97				2	11	1													Card VI	
32	62	63	64	128	127				2	11	1													Card VI	
33	10100																							Card VII	
34	320110																							Card VII	
35	650110																							Card VII	

Table 4.18 Input Cards for Sample Problem 6 - Dynamic Loading (cont.)

	1		2		3		4		5		6		7		8		
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	
36	960110																Card VII
37	1 32	1 3	1 2														Card VIIa
38	0.0	-106.0	1.0	-106.0													Card VIIb-1
39	33																Card VIIc
40	1 32	32 1	1 3														Card IXa
41	1																Card IXb
42	33																Card IXc
43	10	5	0	1													Card Xa
44	10204Z-DISP	NODE 1															Card Xb
45	140104R-DISP	NODE 14															Card Xb
46	140204Z-DISP	NODE 14															Card Xb
47	230104R-DISP	NODE 23															Card Xb
48	230204Z-DISP	NODE 23															Card Xb
49	1200	4															Card Xd

4.6.1 Static Internal Pressurization

For the static internal loading condition with uniform wall temperature, pressurization of the shell model is imposed by monotonically increasing the internal loading in 5 psi increments. It is observed that up to 20 psi the behavior of the vessel is entirely elastic. At internal pressures of 25-30 psi meridional cracking at the base of the vessel begins. The onset of hoop and meridional cracking in the cylinder above 30 psi is followed by hoop and meridional cracking in the dome. Hoop cracking and meridional cracking is practically completed at about 100 psi in the cylinder and dome.

Yielding of the liner is observed to start at about 100 psi at mid-cylinder height, propagates along the cylinder and finally reaches the base at about 110 psi. The yielding of hoop reinforcement at the midheight of the concrete vessel begins at about 110-115 psi. Failure of hoop reinforcement is governed by its strength; this stress limit of 82.3 ksi is reached at an internal pressure of about 125-130 psi. Failure of the hoop reinforcement is found to cause sudden failure of the liner and rupture. The deformed configuration of the vessel, before impending failure, is shown in Fig. 4.15.

Deformation of the vessel is obtained for internal pressures ranging from 5 psi up to 125 psi. Maximum vertical displacement at the dome apex before impending failure of the hoop rebar is 0.12 inches, the corresponding radial and vertical displacements at the vessel springline are 0.64 inches and 0.27 inches. The maximum radial and vertical displacements at cylinder midheight at this load are 2.62 inches and 0.15 inches, respectively, while the respective hoop strain in the liner is 0.02 in/in. The displacements over the complete pressure loading at the cylinder midheight, vessel springline and dome are given in Figs. 4.16 to 4.20.

4.6.2 Thermal Loading and Pressurization

When the structure is subjected to elevated temperature as well as internal pressure, the loadings are performed sequentially. First, the thermal load is applied in 33°F increments on the inside surface of the vessel up to the maximum of 400°F; the external surface is held at 70°F with a linear temperature variation maintained through the thickness. The pressure then is applied in 5 psi increments until failure is reached. Meridional and hoop cracking of the dome and cylinder is observed on the outer surface of the

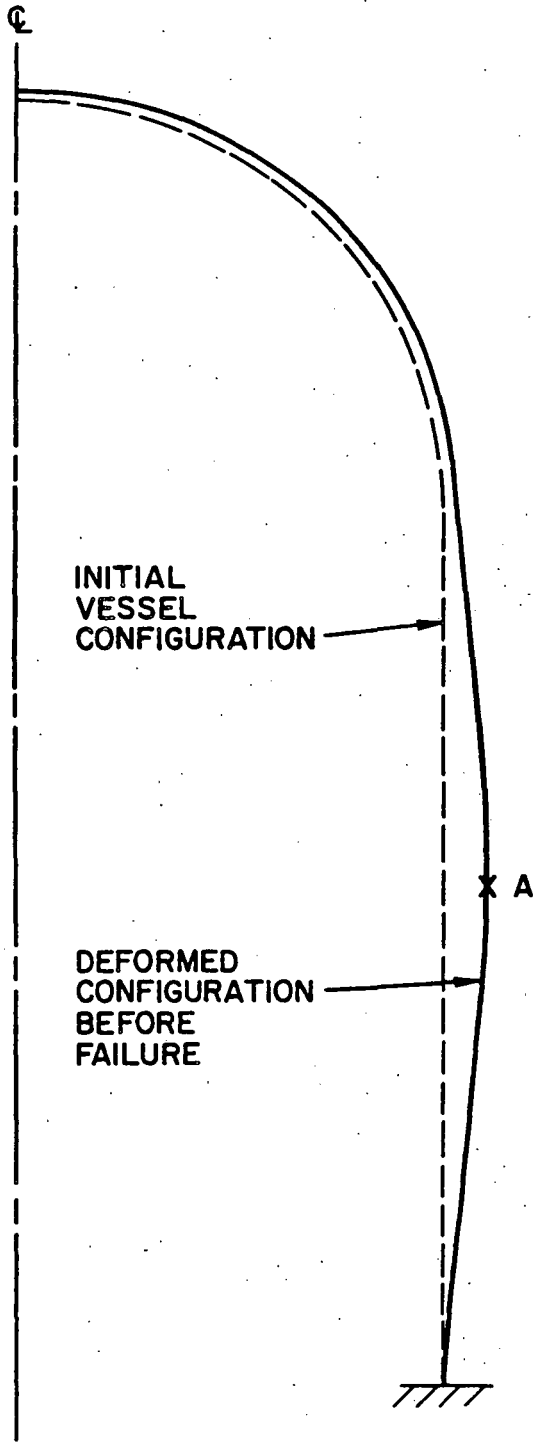


Figure 4.15 Deformation of Vessel at Impending Failure

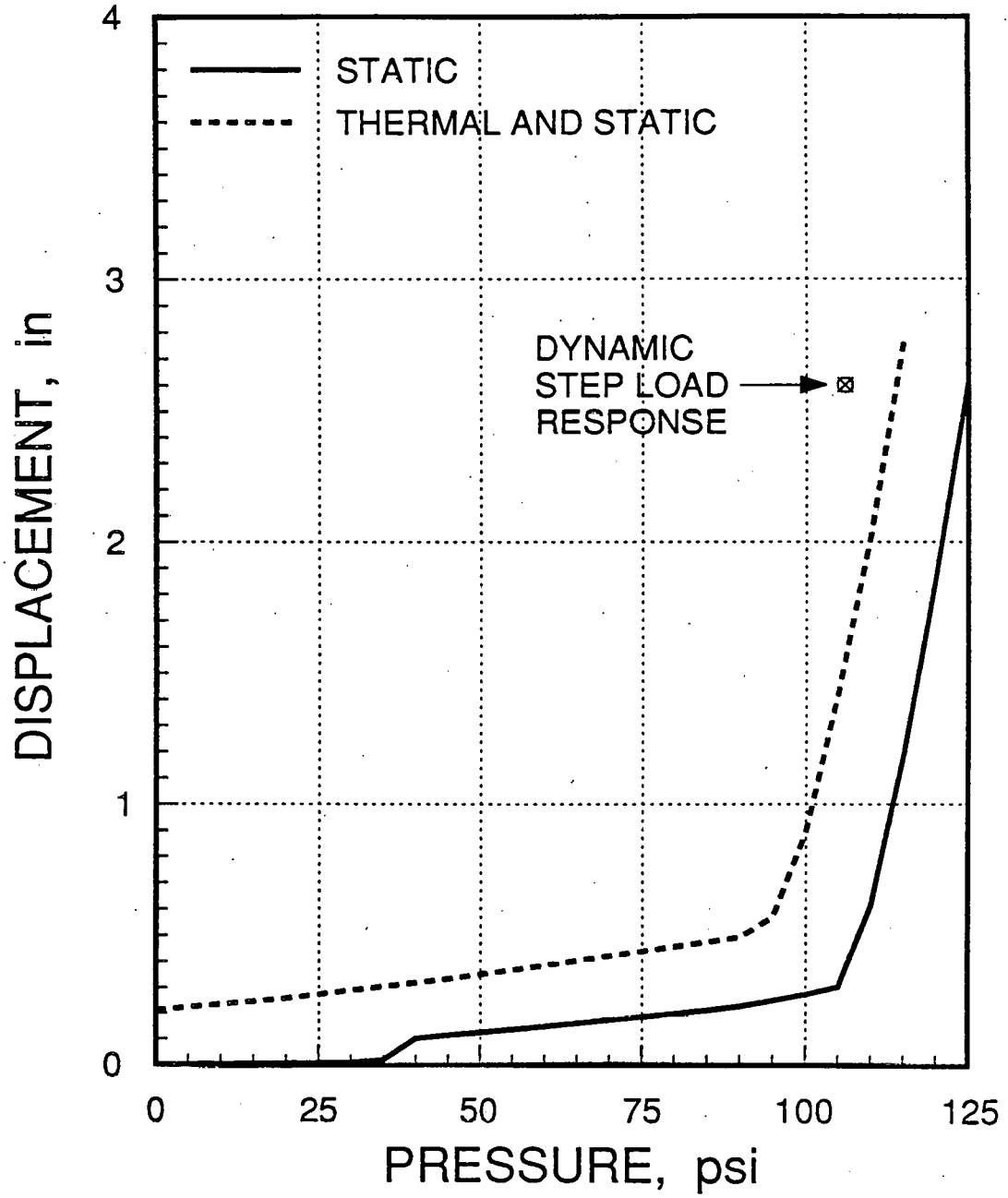


Figure 4.16 Radial Displacement of Liner at Cylinder Midheight

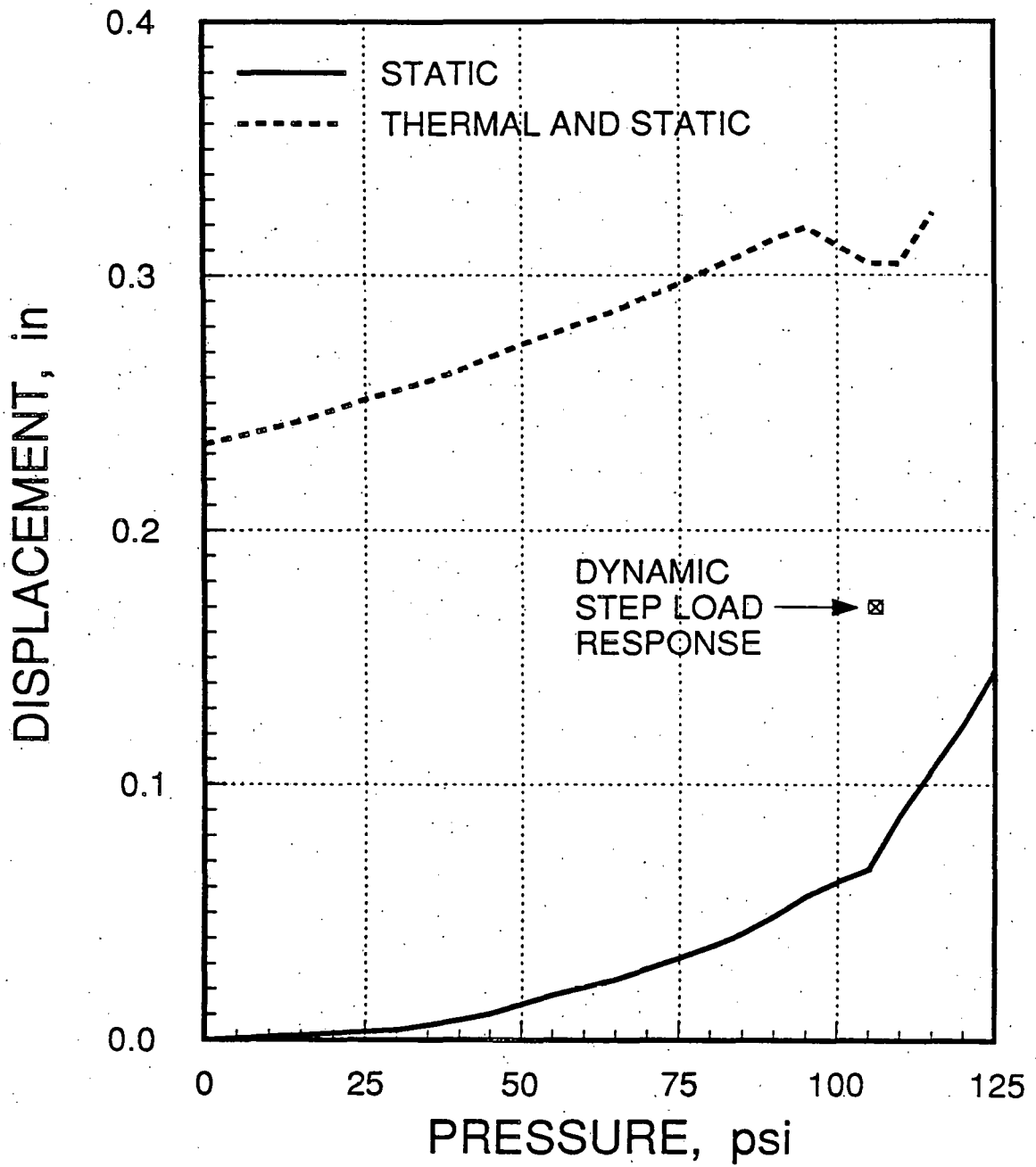


Figure 4.17 Vertical Displacement of Liner at Cylinder Midheight

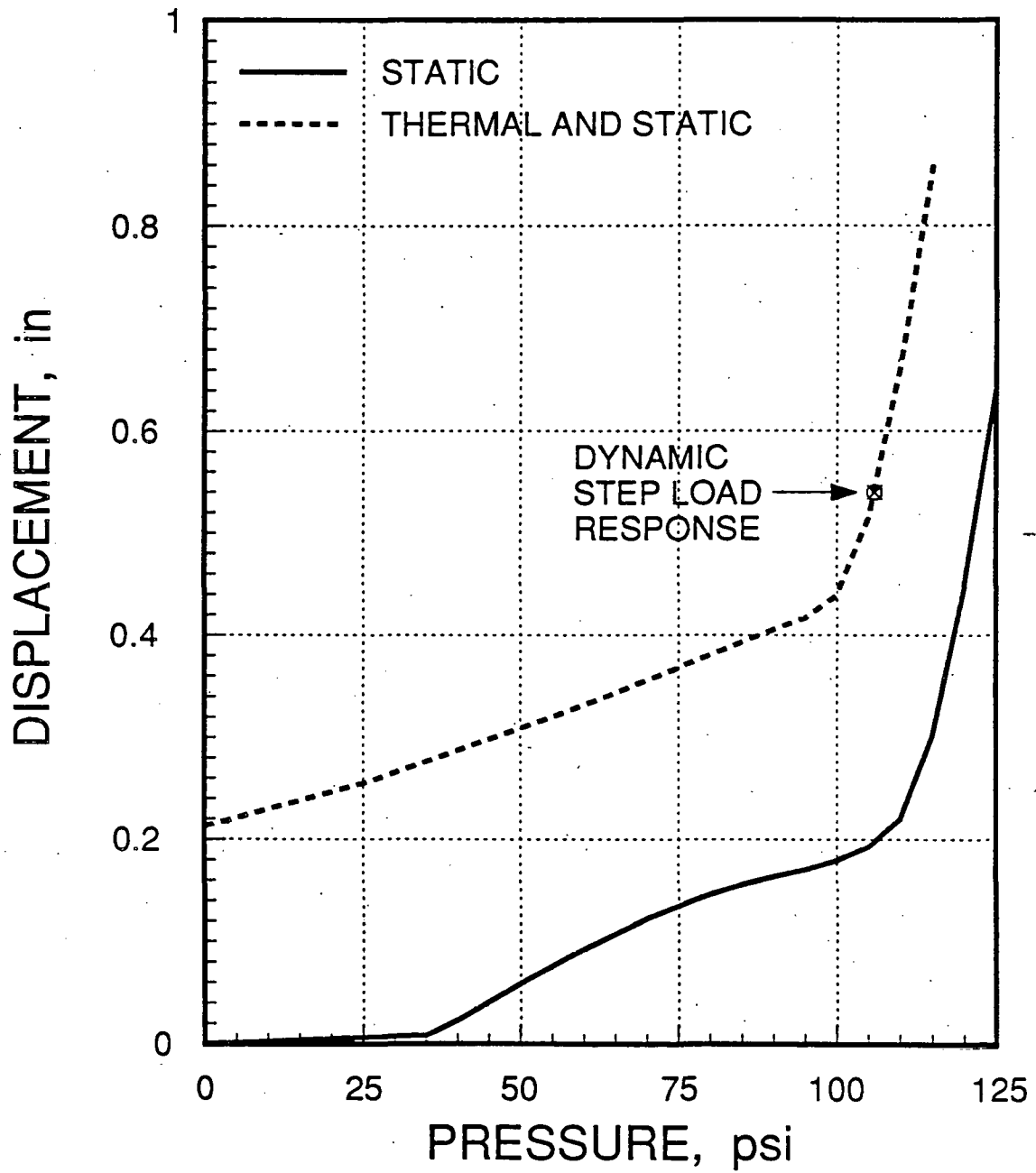


Figure 4.18 Radial Displacement of Liner at Vessel Springline

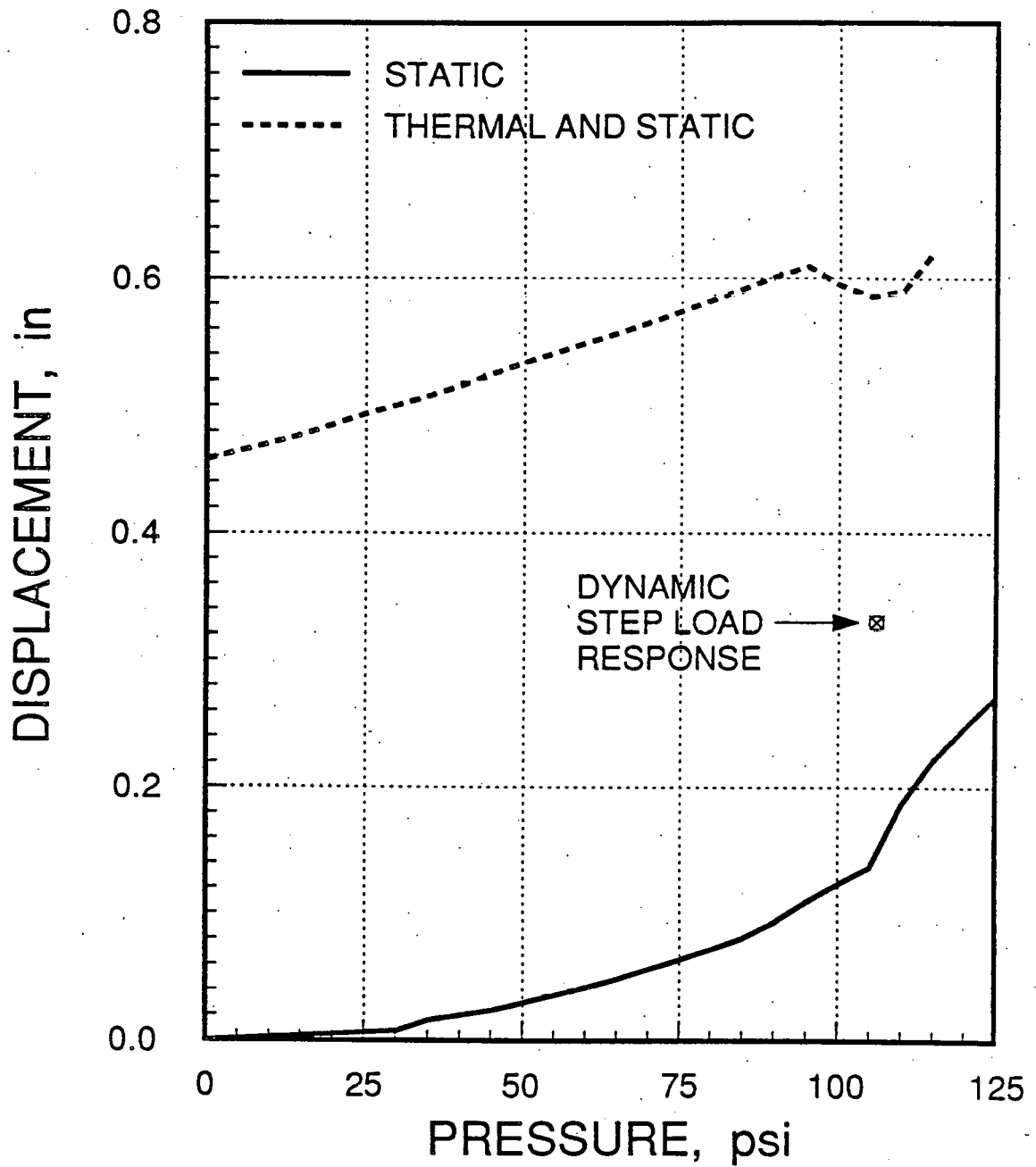


Figure 4.19 Vertical Displacement of Liner at Vessel Springline

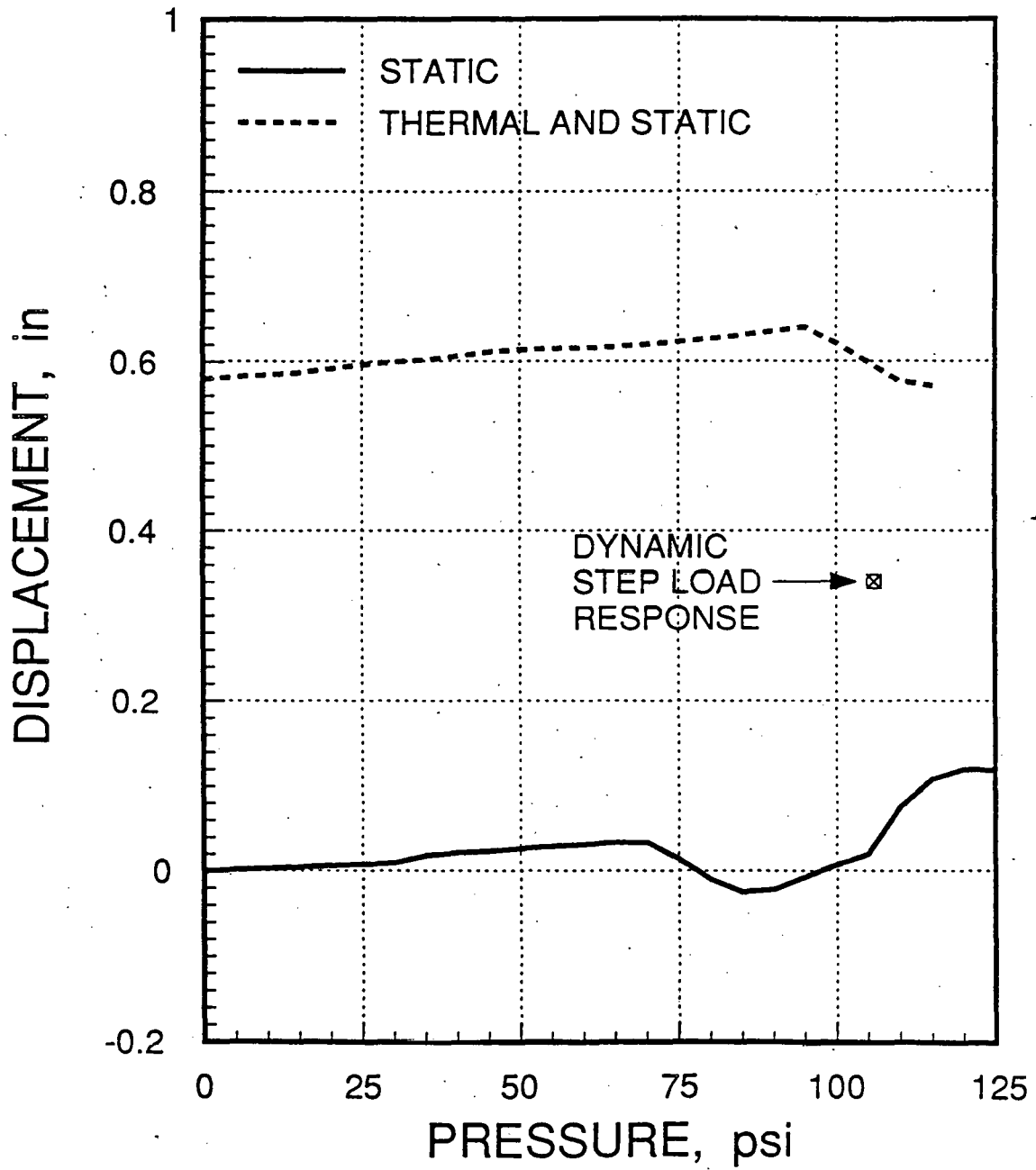


Figure 4.20 Vertical Displacement of Liner at Dome Apex

structure in the first thermal load step. As the thermal load is increased to 400°F, meridional and hoop cracking of the dome and cylinder spreads from the outer surface to about two-thirds of the way through the wall. As the pressure is applied from 5-80 psi, cracking continues in the dome and cylinder. At 100 psi, all the concrete has fully cracked except at the basemat juncture; the concrete in this region is in compression. Yielding of the hoop reinforcement in the cylinder starts at 50 psi. The ultimate strength of the rebar located near the cylinder midheight is exceeded at a pressure of 115-120 psi. This leads to subsequent failure of the liner.

The deformations of the structure are similar to the non-thermal case; there is an increase in magnitude because of thermal expansion and thermal softening effect on the material properties. The maximum vertical displacement of the dome at impending failure is 0.64 inches. The maximum springline radial and vertical displacements are 0.86 inches and 0.62 inches, respectively. At the cylinder midheight, 2.77 inches of radial displacement and 0.32 inches of vertical displacement were calculated. The maximum (hoop) liner strain at the midheight was 0.02 in/in. The displacements over the complete pressure range are given in Figs. 4.16 to 4.20.

4.6.3 Dynamic Loading

The dynamic threshold failure pressure is found by internally loading the structure over a range of step load pressures. The maximum step pressure load that this vessel could withstand before impending failure was 106 psi. A time step of 60 μ sec was employed to ensure stability for this loading. The failure for this loading was of the same mode and in the same general location as that given by the static pressure loading, hoop reinforcement failure followed by liner rupture near the cylinder midheight. In general, the displacement field is similar to the static response except at the springline. The maximum vertical displacement of the dome apex at this threshold failure pressure is 0.34 inches which took 13.2 msec to achieve. At the springline, the maximum radial displacement is 0.54 inches at 26.4 msec while the maximum vertical displacement is 0.33 inches in 13.8 msec. The corresponding radial and vertical displacements at the cylinder midheight are 2.6 inches and 0.17 inches which were reached at 27.0 msec and 15.6 msec, respectively. The maximum values for the dynamic step load are indicated in Figs. 4.16 to 4.20.

4.7 Sample Problem 7: Prestressed Concrete Vessel

This example pertains to an internally pressurized prestressed cylindrical container tested by Sozen and Paul (1968). The container simulates the containment of a nuclear reactor vessel. Figure 4.21 identifies the components of the test structure. The analytical model used in the comparison to the experimental data is given in Fig. 4.22. The axisymmetric model consists of 335 concrete continuum elements, 40 prestressed longitudinal bar elements and 41 prestressed circumferential ring elements. A sliding interface with no friction provides for the interaction of the longitudinal prestress bars and the concrete. The material properties of the concrete and steel are given in Table 4.19. The input cards for the mesh are given in Table 4.20.

Prior to internal pressurization, each layer of tendons is prestressed. Each of the longitudinal tendons are prestressed 25200 lbs, each of the circumferential tendons 3487 lbs. The input cards indicate different loads because the tendons are lumped (4.775 bars longitudinally for 1 radian sector and 3 bars circumferentially) at the nodes. The prestress force is applied initially and then allowed to change during the pressurization. Pressurization of the model is imposed by a monotonically increasing internal loading of 50 psi increments to 750 psi. The resulting central deflection of the top slab (point A in Fig. 4.22) and the increase in longitudinal prestress bar force versus internal pressure are given in Figs. 4.23 and 4.24, respectively. These analytical results compare favorably with the experimental data.

Figure 4.25 shows the extent of cracking for the pressure levels of 450, 550, 650 and 750 psi. Cracking initiates at 350 psi and spreads through the upper slab as the pressure increases. With the pressure increase, the corner joint of the wall and slab begins to crack, allowing the upper slab to rotate, causing significant increase in the central deflection of the upper slab.

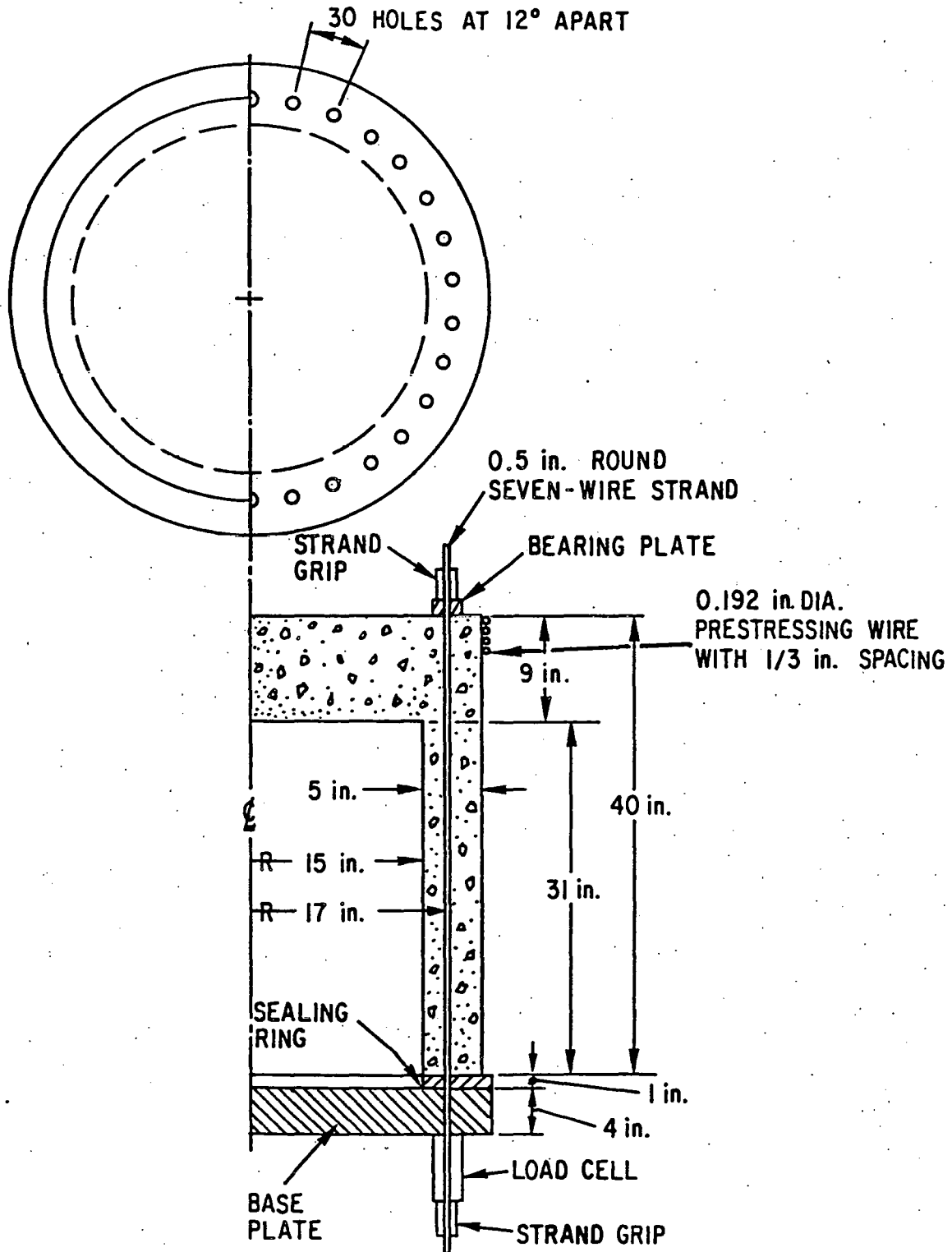


Figure 4.21 Prestressed Concrete Vessel Test Components

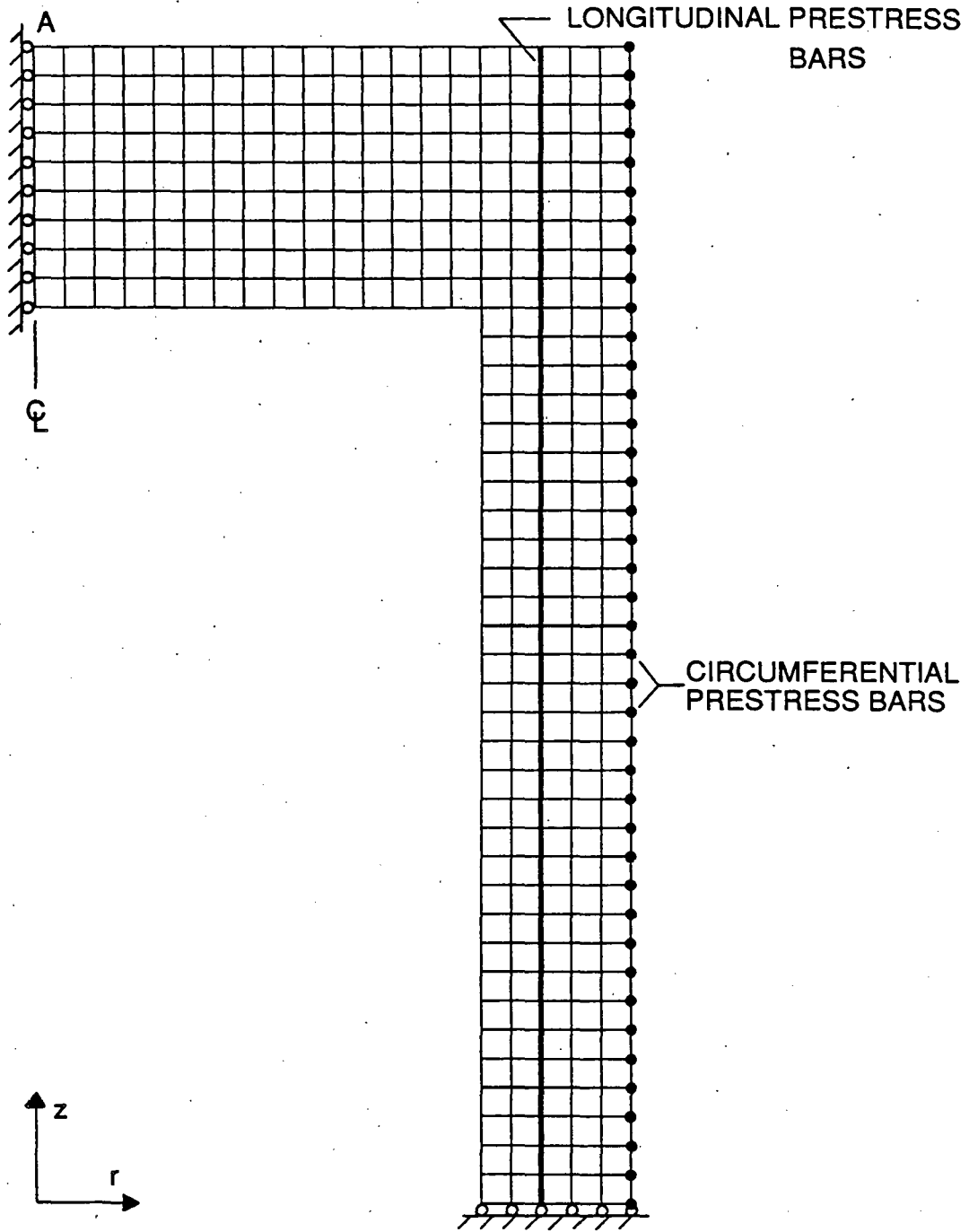


Figure 4.22 Analytical Model of Prestressed Concrete Vessel

Table 4.19 Material Properties for the Prestressed Concrete Vessel (Sample Problem 7)

Concrete Young's modulus	$E = 4.3 \times 10^6$ psi
Concrete Poisson's ratio	$\nu = 0.15$
Concrete compressive strength	$f'_c = 7140$ psi
Concrete tensile strength	$f'_t = 446$ psi
Steel Young's modulus	$E = 2.8 \times 10^7$ psi
Steel yield stress	$\sigma_y = 2.125 \times 10^5$ psi
Steel ultimate stress	$\sigma_u = 2.5 \times 10^5$ psi
Steel ultimate strain	$\epsilon_u = 0.06$ in/in
Longitudinal prestress force	$f_p = 7.56 \times 10^5$ lbs
Equivalent pressure due to circumferential prestressing	$P_e = 510$ psi

Table 4.20 Input Cards for Sample Problem 7

	1			2			3			4			5			6			7			8			
	12345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	
1	PRESTRESSED CONCRETE CONTAINMENT VESSEL																							Card I	
2	435	416	4	16	16	2	1.0	1	1							3									Card II
3	2	0				90									15000	10	10	1							Card III
4	1	13																							Card IVa-13
5	2.25E-4		0.15															1.0		3.0					Card IVb-13
6			0.0	0.0			1.0	0.0		13.0								1.0		1.0					Card IVc-13
7		70.0	4.3E6	0.0008023			3450.0	0.003		7140.0								1.0		7140.0					Card IVd-13
8		1.0	0.062465		1.15		0.8	4.2		-0.0003								-0.006							Card IVe-13
9	2	11																							Card IVa-11
10	7.40E-4																	1.0		2.0					Card IVb-11
11			0.0445							3.0								1.0		1.0					Card IVc-11
12		70.0	2.8E7	0.0075893			2.125E5	0.06		2.5E5															Card IVd-11
13	3	11																							Card IVa-11
14	7.40E-4																	1.0		2.0					Card IVb-11
15			0.0891							3.0								1.0		1.0					Card IVc-11
16		70.0	2.8E7	0.0075893			2.125E5	0.06		2.5E5															Card IVd-11
17	4	1																							Card IVa-1
18	7.40E-4																	1.0		2.0					Card IVb-1
19			0.6875							1.0								2.0		1.0					Card IVc-1
20		70.0	2.8E7	0.0075893			2.125E5	0.06		2.5E5															Card IVd-1
21	1		15.0		0.0																				Card V
22	41		15.0		40.0																				Card V
23	42		16.0		0.0																				Card V
24	82		16.0		40.0																				Card V
25	83		17.0		0.0																				Card V
26	123		17.0		40.0																				Card V
27	124		18.0		0.0																				Card V
28	164		18.0		40.0																				Card V
29	165		19.0		0.0																				Card V
30	205		19.0		40.0																				Card V
31	206		20.0		0.0																				Card V
32	246		20.0		40.0																				Card V
33	247		0.0		31.0																				Card V
34	261		14.0		31.0																				Card V
35	262		0.0		32.0																				Card V

Table 4.20 Input Cards for Sample Problem 7 (cont.)

	1			2			3			4			5			6			7			8			
	12345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901	2345678901		
36	276			14.0			32.0																	Card V	
37	277			0.0			33.0																	Card V	
38	291			14.0			33.0																	Card V	
39	292			0.0			34.0																	Card V	
40	306			14.0			34.0																	Card V	
41	307			0.0			35.0																	Card V	
42	321			14.0			35.0																	Card V	
43	322			0.0			36.0																	Card V	
44	336			14.0			36.0																	Card V	
45	337			0.0			37.0																	Card V	
46	351			14.0			37.0																	Card V	
47	352			0.0			38.0																	Card V	
48	366			14.0			38.0																	Card V	
49	367			0.0			39.0																	Card V	
50	381			14.0			39.0																	Card V	
51	382			0.0			40.0																	Card V	
52	396			14.0			40.0																	Card V	
53	397			17.0			1.0																	Card V	
54	435			17.0			39.0																	Card V	
55	1	1	42	43	2					1	13	1												Card VI	
56	41	42	83	84	43					1	13	1												Card VI	
57	81	83	124	125	84					1	13	1												Card VI	
58	121	124	165	166	125					1	13	1												Card VI	
59	161	165	206	207	166					1	13	1												Card VI	
60	201	247	248	263	262					1	13	1												Card VI	
61	215	261	32	33	276					1	13	1												Card VI	
62	216	262	263	278	277					1	13	1												Card VI	
63	230	276	33	34	291					1	13	1												Card VI	
64	231	277	278	293	292					1	13	1												Card VI	
65	245	291	34	35	306					1	13	1												Card VI	
66	246	292	293	308	307					1	13	1												Card VI	
67	260	306	35	36	321					1	13	1												Card VI	
68	261	307	308	323	322					1	13	1												Card VI	
69	275	321	36	37	336					1	13	1												Card VI	
70	276	322	323	338	337					1	13	1												Card VI	

Table 4.20 Input Cards for Sample Problem 7 (cont.)

	1			2			3			4			5			6			7			8			
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890		
71	290	336	37	38	351									1	13	1									Card VI
72	291	337	338	353	352									1	13	1									Card VI
73	305	351	38	39	366									1	13	1									Card VI
74	306	352	353	368	367									1	13	1									Card VI
75	320	366	39	40	381									1	13	1									Card VI
76	321	367	368	383	382									1	13	1									Card VI
77	335	381	40	41	396									1	13	1									Card VI
78	336	206	206						2	2	11	1													Card VI
79	337	207	207						3	3	11	1													Card VI
80	376	246	246						2	2	11	1													Card VI
81	377	83	397						4	4	1	1													Card VI
82	378	397	398						4	4	1	1													Card VI
83	416	435	123						4	4	1	1													Card VI
84	10010																								Card VII
85	420010																								Card VII
86	830010																								Card VII
87	1240010																								Card VII
88	1650010																								Card VII
89	2060010																								Card VII
90	2470100																								Card VII
91	2620100																								Card VII
92	2770100																								Card VII
93	2920100																								Card VII
94	3070100																								Card VII
95	3220100																								Card VII
96	3370100																								Card VII
97	3520100																								Card VII
98	3670100																								Card VII
99	3820100																								Card VII
100	1	47	1	2	0	2																			Card VIIa
101	1.0		0.0		101.0		5000.0																		Card VIIb-1
102	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16									Card VIIc
103	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32									Card VIIc
104	261	260	259	258	257	256	255	254	253	252	251	250	249	248	247										Card VIIc
105	2	1	11	2	0	3																			Card VIIa

Table 4.20 Input Cards for Sample Problem 7 (cont.)

	1		2		3		4		5		6		7		8		
	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	1234567890	
106		1.0	5230.8		2.0		0.0		100.0		0.0						Card VIIIb-1
107	336																Card VIIIc
108	3	1	11	2	0	3											Card VIIIa
109		1.0	10461.5		2.0		0.0		100.0		0.0						Card VIIIb-1
110	337																Card VIIIc
111	4	1	11	2	0	3											Card VIIIa
112		1.0	120321.0		2.0		0.0		100.0		0.0						Card VIIIb-1
113	377																Card VIIIc
114	1	39	39	1	1	0											Card IXa
115	84																Card IXb
116	397																Card IXc
117		1		1		2		0									Card Xa
118		3820204Y-DISP NODE 382															Card Xb
119		4160034PRESTRESS IN ELE 416															Card Xc
120		4210024INTERNAL ENERGY															Card Xc

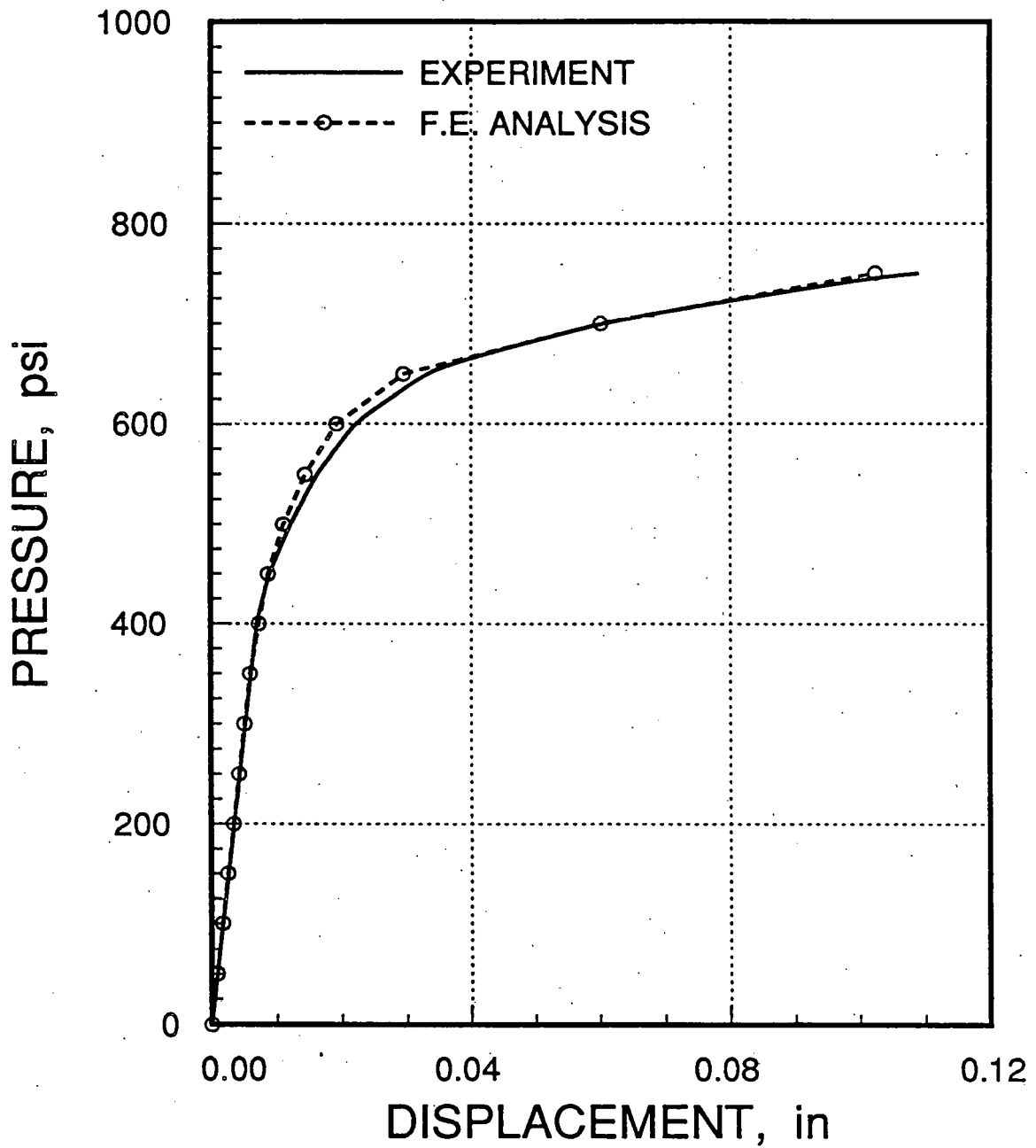


Figure 4.23 Central Deflection of the Top Slab

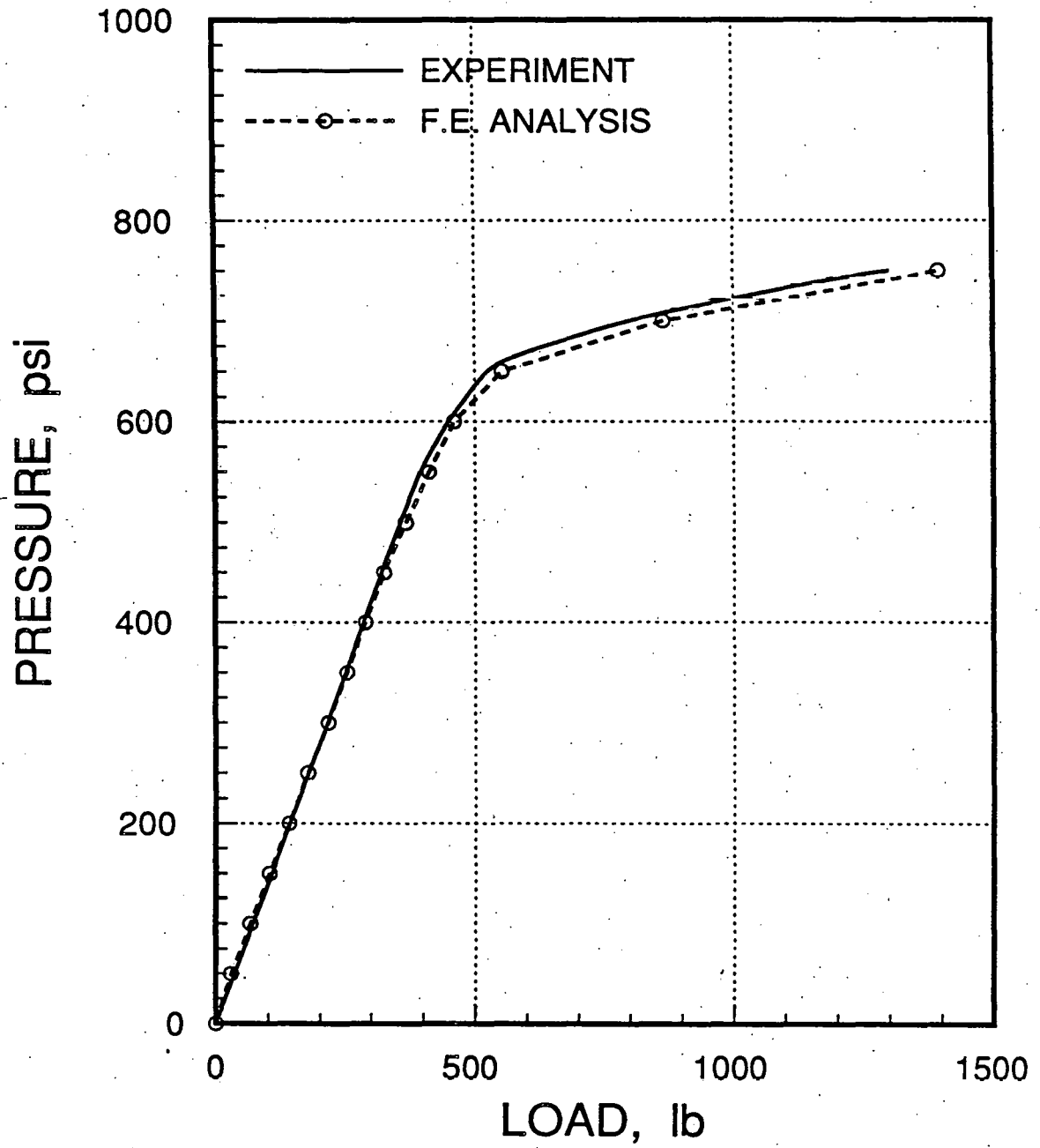
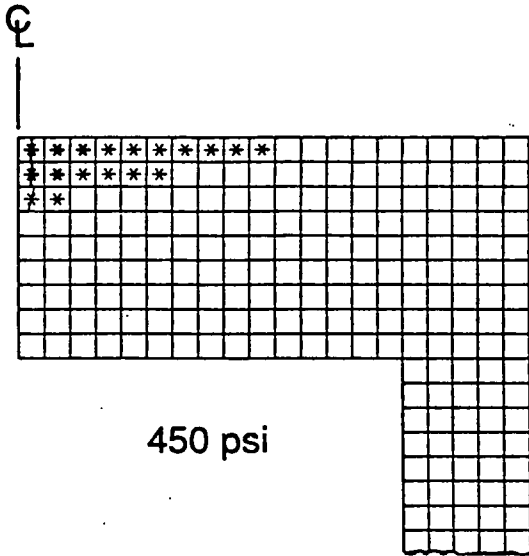


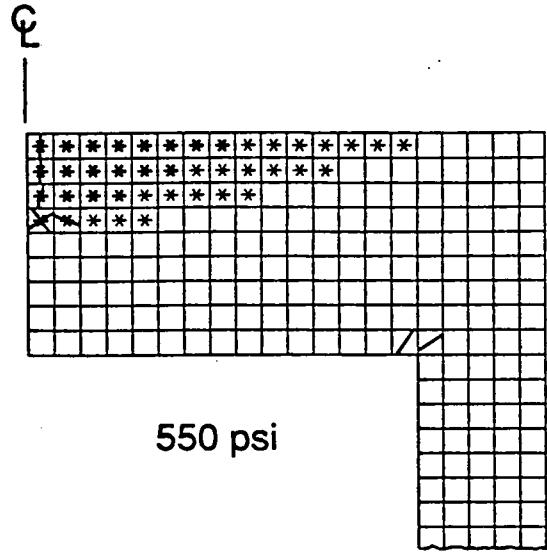
Figure 4.24 Increase in Longitudinal Prestress Bar Force

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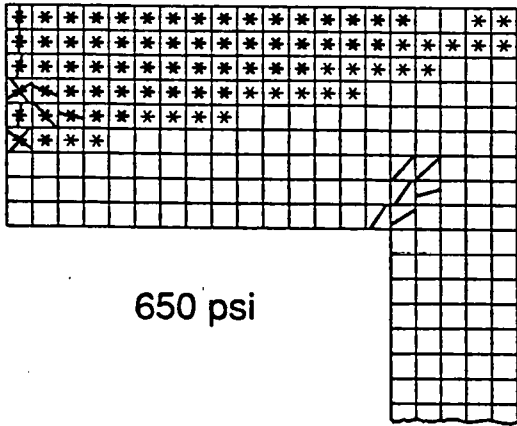
* RADIAL CRACKS



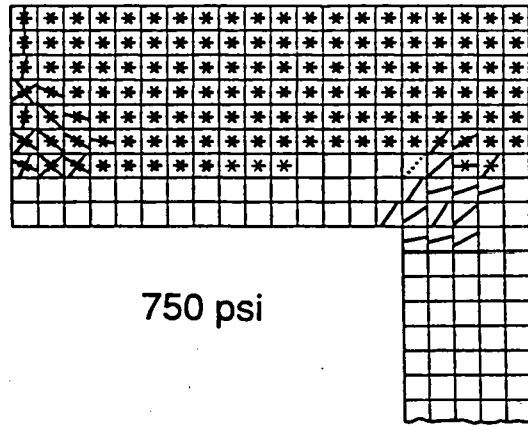
450 psi



550 psi



650 psi



750 psi

Figure 4.25 Extent of Cracking of Prestressed Concrete Vessel Model

ACKNOWLEDGEMENT

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