

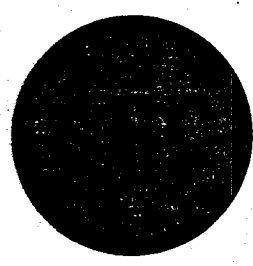
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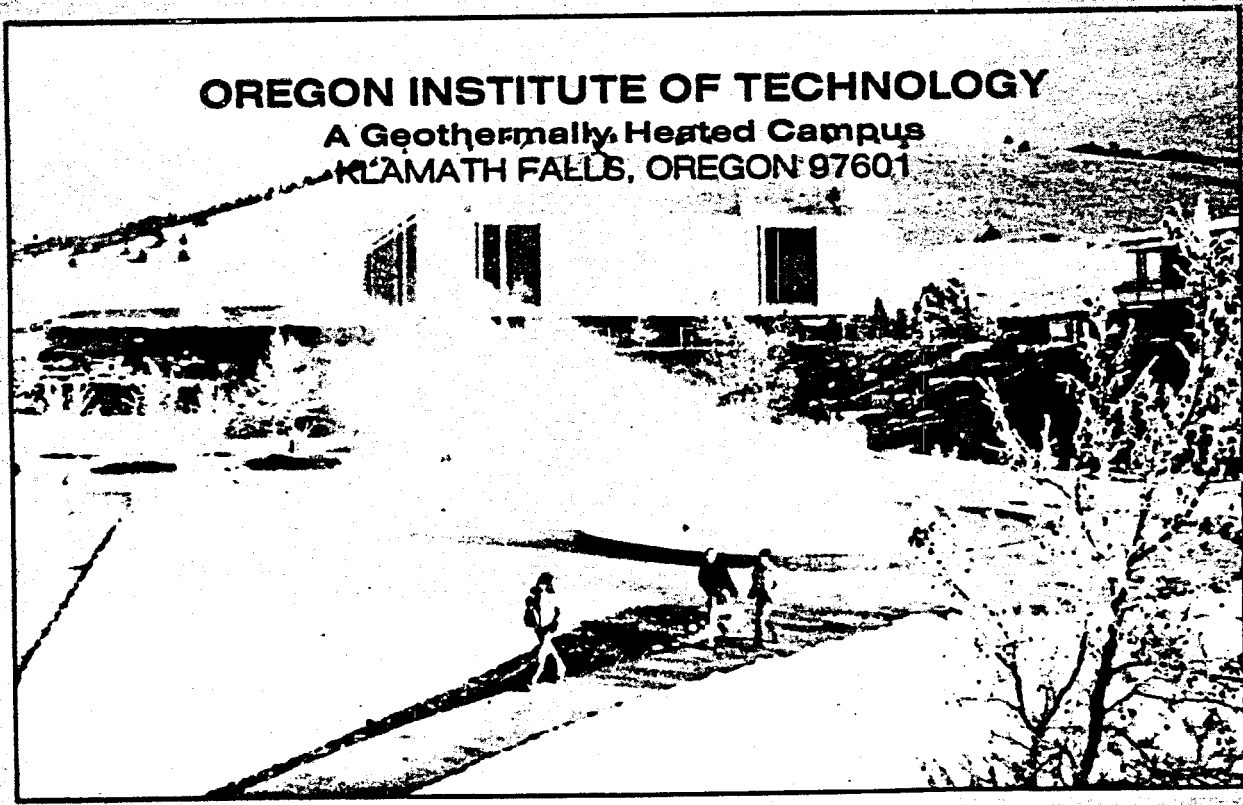
# GEO-HEAT CENTER

COLLEGE INDUSTRIAL PARK  
AN INNOVATIVE APPROACH TO ENERGY CONSERVATION  
THROUGH THE USE OF GEOTHERMAL ENERGY

November 18, 1986

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**OREGON INSTITUTE OF TECHNOLOGY**  
A Geothermally Heated Campus  
KLAMATH FALLS, OREGON 97601



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Prepared for:

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Klamath Falls, Oregon 97601

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AN INNOVATIVE APPROACH TO ENERGY CONSERVATION  
THROUGH THE USE OF GEOTHERMAL ENERGY**

November 18, 1986

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## ABSTRACT

Geothermal effluent from the Oregon Institute of Technology campus and Merle West Medical Center has been discharged to an open drainage ditch adjacent to the City's College Industrial Park since 1964. Over the past few years there has been increasing concern for conservation and preservation of the geothermal aquifers in Klamath Falls, Oregon. An effective way of improving the energy utilization is to cascade the approximately 130°F effluent for heating buildings in the industrial park and disposal of the effluent in an existing injection well. An aquifer stress test was performed using the 1500 foot well in the industrial park. Based on the specific capacity, data indicates that the well is capable of accepting an injection rate of at least 700 gpm of the thermal effluent. A plume of degraded water will develop down-gradient of the well. However, the plume is expected to bypass nearby water supply wells and will have no impact on OIT and MVMC space heating wells.

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## 1. SUMMARY AND CONCLUSIONS

### 1.1 OIT Campus and MWM Effluent.

There has been no recent verification of flows from the three OIT geothermal production wells. Original design information gives a total available flow of 980 gpm at 190°F for the three wells. Based on original building design data, the total peak flow rate is 1216 gpm for the existing ten buildings. The discharge temperature is 131°F indexed to the 190°F supply water temperature. During summer months only, the chiller operates on 600 gpm with a discharge temperature of 170°F. After completion of the new high technology building and the collection system of effluent from five buildings supplying this building, the peak discharge was estimated at 743 gpm and a temperature of 131°F. Allowing for heat removed by the new high tech building, ten existing buildings, and Phase I building additions to Snell, College Union, Physical Plant and Physical Education buildings, the total campus effluent temperature will be approximately 116°F. This is based on a peak available flow of 980 gpm.

Many of the campus buildings are not operating as they were originally designed; therefore, actual discharge temperature and flows are not in agreement with the above calculated values. It is strongly recommended that production rates from each of the three OIT wells be verified prior to final design of any injection system.

MWMC average weekly flows ranged from 90 to 130 gpm during December 1985 and January 1986. It is estimated that brief peak flows can be as high as 300 gpm. Geothermal water is discharged into a vault at 120°F. From the vault, water is circulated to a snow melt system, then disposed of in a drainage ditch that connects with OIT effluent.

### 1.2 OIT and MWM Collection System.

A collection system was constructed during 1986 to collect discharged geothermal water from the Residence Hall, College Union, Snell, Chiller, Physical Education and the Learning Resources Center (Library). This collection system would see a peak flow of 743 gpm at a temperature of approximately 133°F during winter operation and approximately 600 gpm at 170°F during summer operation.

In order to complete the campus collection system, Owens, Semon and South Hall would require short tunnel extensions. A non-tunnel collection system for Cornett and the Physical Plant was used for estimating purposes. Estimated costs to complete the collection system, including tunnel extension, piping and isolation heat exchangers were \$288,500.

Based on existing and future needs for space heating in the CIP, it does not appear that the flow from MWM would be required for space heating

purposes. The added flow would possibly require a positive injection pressure at the injection well not required for disposal of OIT effluent alone. For these reasons, the collection tank, pumps and piping sizes for the transmission line from the pump station to the injection well were sized based only on OIT campus flows. However, three options were considered for pipe routing to deliver MWMC effluent to the pump station. Estimated cost of Option #2, which appears to be the most attractive, is \$56,000.

### 1.3 Pump Station and Transmission Piping.

The pump station will provide energy required to overcome the elevation and pipe friction between the collection tank and the injection well. It will also provide a small positive pressure to assure that the transmission line remains full and provide flow through the CIP buildings. The pump station will be located on OIT property because the City would collect the campus effluent from a gravity discharge point. Two design options were considered for the pump station; a base load variable speed pump or a three pump system with all constant speed pumps. The three pump system is recommended due to the makeup of the required pump head. Each pump would deliver 300 gpm at approximately 60 psi discharge pressure staged according to the water level in a 4000 gallon collection tank.

The pump station would deliver the geothermal effluent through an 8" buried transmission line to the CIP and injection well. A 6" collection line would parallel the main 8" line in the park to collect fluid discharged from the CIP buildings.

Estimated costs for pumping station, transmission and distribution piping (fiberglass) are \$348,500 for only OIT effluent.

If MWMC effluent were added to the disposal system, it is estimated that each of the three pumps would increase from 20 hp to 25 hp and the transmission pipe size would increase from 8" to 10".

### 1.4 CIP Building Energy Analysis.

Quality Components would require a peak flow of 25 gpm based on a 30°F temperature drop. Retrofit of this building would require four hot water unit heaters, finned coil, heat exchanger, circulating pump, and necessary piping amounting to a capital cost of \$15,000.

Precision Data Carriage energy analysis was done assuming R-19 insulation would be installed in the ceiling. Using a system temperature drop of 30°F, peak geothermal flow requirement would be 20 gpm. Retrofit of the building for geothermal would consist of installing hot water unit heaters along side existing gas fired equipment and a finned coil heat exchanger for the office area. Retrofit costs would amount to approximately \$12,000.

Equitorial Communications would require a geothermal flow rate of 33 gpm based on a 30°F temperature drop through the system. Retrofit of the system would consist of a closed loop with a plate heat exchanger, four hot water coils and a hot water unit heater for the receiving area. Retrofit costs would amount to approximately \$22,000.

Total flow of 78 gpm would be required for the three buildings in the CIP, which compares with 743 gpm available from the initial campus collection system and 980 gpm from the completed system.

Peak heating requirement for the remaining tracts was estimated at  $9.76 \times 10^6$  Btu/hour and a geothermal flow rate of 650 gpm, based on a 30°F temperature drop through the system. This estimate was based on existing building energy use for 30.21 acres of developable area on the remaining lots.

### 1.5 Aquifer Stress Test

Testing results indicate that the aquifer in the vicinity of the College Industrial Park is highly transmissive. Calculated values for transmissivity range between  $2.9 \times 10^4$  and  $5.1 \times 10^5$  gpd/foot. In fact, the transmissivity range is so large that the test produced a stress on the aquifer which yielded responses in observation wells that were barely discernible.

Neither classic recharge nor classic discharge hydraulic boundaries were observed during the test. However, the presence of a east-west trending structure located between the OIT geothermal wells and the College Industrial Park is suggested by water level measurements from the observation wells within the study area. This structure serves to retard the flow of ground water southward east of CIP #1, although hydraulic continuity across it is preserved. Test results also indicated a northwesterly trending zone of high transmissivity west of CIP #1. The presence of this structure is also supported by water level data.

Specific capacity data indicate that CIP #1 is capable of accepting an injection rate of at least 700 gpm of thermal effluent. At this rate it is anticipated that the water level in CIP #1 will rise to within approximately 100 feet of land surface.

Because the thermal effluent is of lesser quality than ambient ground water, a plume of degraded water will develop down-gradient of CIP #1. However, the plume is expected to bypass nearby water supply wells and, as a result, will have zero impact on them. It is also anticipated that the plume will not have a thermal impact on OIT space heating wells.

Based on the specific capacity that CIP #1 is capable of accepting, it could receive the estimated 743 gpm from the initial OIT collection system. A positive pressure may be required if the peak flows of 980 gpm from the OIT completed system and 300 gpm from MWMC were disposed of in this well.

## 1.6 Policies Governing Geothermal Effluent Disposal.

Both the State and the City of Klamath Falls require that standards and procedures be met for effluent disposal systems.

Oregon Administrative Rules, Chapter 690, Division 65 describes the standards and procedures for effluent disposal systems. Basic information required by the State Water Resources Department for the use of CIP #1 as an injection well is included in this report.

The City Geothermal Resources Management Act, effective 1 July 1985, requires the elimination of geothermal water surface discharge within five years of the effective date.

## 1.7 Legal Agreement.

Seven factors have been identified that will need to be included in a legal agreement. The legal agreement will be necessary for the coordination of geothermal discharge water usage by the City of Klamath Falls and Oregon Institute of Technology and/or Merle West Medical Center.

Asphalt Construction Company has surface water rights (no. 53353) for 1.5 cfs from a pond fed by OIT/MWMC discharge. Current owners, Klamath Pacific Corporation, use a new well for dust control.

## 1.8 Capital Cost Summary.

Estimates were determined on costs to complete the OIT collection system, pump station, transmission piping to the industrial park and injection well, and retrofit of heating systems for buildings in the industrial park. The fiberglass piping option was used in the following estimates:

a. Completion of OIT collection system	\$288,500
b. Pump station and transmission piping	348,500
c. MWMC collection system	56,000
d. Retrofit of CIP buildings (3)	49,000

## 2. INTRODUCTION

The purpose of this study was to determine the feasibility of utilizing geothermal water discharged from the Oregon Institute of Technology (OIT) campus and Merle West Medical Center (MVMC) heating systems for space heating buildings in the College Industrial Park (CIP), to determine the feasibility of using a College Industrial Park well, CIP #1, for disposal of geothermal effluent and the impact on aquifers in the surrounding area.

Since 1964, geothermal effluent from the campus and hospital buildings, combined with surface run off, has been collected by a storm drainage system and discharged into an open drainage ditch to the west of the campus. The temperature of the water at this discharge point ranges from 130° to 160°F and flows in the drainage ditch just south of the College Industrial Park. It was believed that sufficient flow and temperature would be available to heat buildings in the CIP; therefore, an energy analysis was performed for existing buildings and projections made for future buildings in the park.

Effluent flow and temperature from the OIT campus is dependent on cascaded use from existing campus buildings to heat a new high tech. laboratory/classroom and future buildings on campus. There is an ongoing effort to separate geothermal water from the storm drain system and make it available for reuse, either for these future campus buildings or off campus purpose. Therefore, it was necessary to evaluate the flow and temperature from this revised campus collection system and determine a central location where all the effluent would be delivered. A collection system pump station would be required to deliver all fluids collected from OIT and MVMC to the CIP buildings and injection well.

On 1 July 1985, the City of Klamath Falls enacted a Geothermal Resources Management Act that requires the elimination of geothermal water discharge within five years. Since there are three existing and unused wells in the CIP, two of which belong to the City, an aquifer stress test was performed using well CIP #1 to determine if it could be used as an injection well. In addition, the State Water Resources Department and the Department of Environmental Quality need to be reasonably certain that there will be no substantial thermal, chemical or pressure interference with existing or potential uses of the receiving aquifer. During the pump test of the injection well, ten observation wells, including an OIT production well, were monitored to determine if there was hydraulic communication with other thermal and non-thermal wells in the area.

William E. Nork, Inc. (WEN, Inc.) interpreted and analyzed the data collected during the aquifer stress test on CIP #1. The testing sequence comprised a step-drawdown test August 21, 1986 and a nominally constant-discharge test August 22 through 28, 1986. Test data were collected by personnel under the direction of the Geo-Heat Center. As a result, WEN, Inc. was responsible only for the analysis of the data.

Several factors influenced the analysis of the data and, ultimately, the conclusions reached as a result of this analytical effort. The pumping rate, dictated by the size pump which could fit inside the well casing, in retrospect, was too small relative to the magnitude of the transmissivity of the aquifer. Consequently, WEN, Inc. was forced to discriminate small effects due to pumping from many other effects of similar magnitude. Secondly, the discharge of the pump declined steadily after one day into the "constant-discharge" test. This effect primarily influenced the analysis of the pumped well. The cause of this phenomenon has yet to be determined and probably cannot be ascertained from the available data. As a result, the analysis and the conclusions drawn from it have a lower confidence level than they otherwise would have had.

### 3. OIT CAMPUS EFFLUENT

The current approach to effluent disposal on the OIT campus consists of surface discharge. Effluent from each building heating system is discharged to the storm sewer (typically at roof drain rises) and subsequently enters the campus storm sewer system. This system ultimately discharges into a drainage ditch at the lower end (west side) of the campus.

There is an ongoing effort (subject to available funding) to separate geothermal water from the storm sewer system and make it available for reuse - either for future campus buildings or off campus purposes. A second incentive to the development of a collection system relates to the City's requirement for injection of these fluids. In order to accommodate this request, it is necessary to collect all of the effluent and deliver it to a central location.

During 1986, a major step was taken in this direction. In conjunction with the new classroom building, construction was commenced on a collection system for a number of campus buildings.

Table 1 presents some key data with respect to the OIT geothermal system. These figures were developed by Mr. Roger Wiltrout, OIT Physical Plant Director.

**Table 1  
Key OIT Geothermal System Values**

<u>Building</u>	<u>Peak Load<sup>(1)</sup> Space Heat &amp; DHW MBH</u>	<u>Peak<sup>(1)</sup> Flow Rate GPM</u>	<u>Discharge<sup>(2)</sup> Temperature</u>
Residence	6650	242	135°F
College Union	5078	233	146
Snell	660	24	135
Physical Ed	7010	168	107
Library	1948	76	139
Cornett	7161	198	118
Semon	1675	74	145
South Hall	2599	96	136
Physical Plant	1347	31	103
Owens	1675	74	145
<b>Totals</b>	<b>35,803</b>	<b>1216</b>	<b>131°F</b>
Chiller (summer only)		600 gpm	170°F

(1) Based on original design data

(2) Indexed to 190 supply water temperature

It is important to point out that the above figures for peak load and flow rate are based on the original building designs. Many of the buildings are not operating as they were originally designed. As a result, discharge temperatures are, in most cases, not in agreement with the calculated values shown above. In most cases, the discharge temperatures are significantly above the values indicated.

Table 2 presents original design information for the three OIT geothermal production wells.

**Table 2  
OIT Production Well Information**

<u>Well #</u>	<u>Pump</u>	<u>Make</u>	<u>Design Flow/Head</u>
2	35 stage 7 RM	Aurora	130/637'
5	12 stage 10 RL	Aurora	500/440'
6	26 stage 8 RH	Aurora	350/650'
<b>Total</b>			<b>980 gpm</b>

There has been no recent verification of the flows from these wells. The main campus flow meter (in the heat exchange building) is inoperative. An attempt at measuring flow with an ultrasound meter at each well produced questionable results. It is strongly recommended that production rates

from each of the wells be verified prior to final design of any injection system.

It is apparent from a comparison of the maximum well flows to the peak system requirement that a discrepancy exists. However, due to the great diversity in individual building use, construction, system type and orientation the wells are able to meet heating requirements even when sized for only 81% of peak flow requirements.

#### 4. OIT COLLECTION SYSTEM

##### 4.1 1986 Collection System.

As mentioned earlier, a collection system was constructed during 1986 to collect discharged geothermal water from a number of campus buildings. These buildings included: Residence Hall, College Union, Snell, Chiller, Physical Education and the Learning Resources Center (Library). Major modifications for this collection system included: individual building piping modification, installation of a main collection line in the existing campus tunnel system. Based on the figures shown in Table 1, this collection system would see a peak flow rate of 743 gpm at a temperature of approximately 133°F during winter operation and approximately 600 gpm at 170°F during summer operation. This flow will then be delivered to a 4000 gallon storage tank in the new building. Heating requirements for the new building would be drawn from the tank and tank overflow in addition to the effluent from the new building's heating system would be discharged to the storm sewer system. Allowing for the heat removed in the new building ( $1.2 \times 10^6$  Btu/hr), this flow would be at a temperature of approximately 133°F. The outflow line would join the re-routed storm sewer at a point between Cornett and the new building to the west of both structures.

##### 4.2 Connection of Remaining Campus Buildings to Collection System.

In order to complete the campus collection system the following buildings would have to be added: South Hall, Semon, Owens, Cornett and the Physical Plant.

The method by which some of the buildings would be connected is somewhat in question. The master plan calls for all buildings to be connected to an enlarged campus tunnel system. For Owens, Semon and South Hall, only short tunnel extensions would be required. For the Physical Plant and Cornett, however, the connection to the tunnel system would be integrated with a future building. At present, the location of this building (future Sports Arena) has not been fixed. In addition, there is no guarantee that it will ever be constructed. For purposes of this study, a non-tunnel collection system for Cornett and the Physical Plant was used for estimating purposes. Due to the short time period remaining before injection is required (by

city ordinance) and the uncertainty of future construction, this seemed the most reasonable approach.

The major construction tasks required to complete the collection system, along with their costs are presented in Table 3.

**Table 3**  
**Cost Summary - Collection System Completion Tasks**

<u>Task</u>	<u>Cost</u>
Semon/South	
Tunnel extension	\$90,000
Tunnel piping	12,000
Semon heat exchanger and piping	17,500
South Hall piping	5,000
Cornett piping and heat exchanger	29,000
Physical Plant piping and heat exchanger	6,500
Phys. Plant/Cornett buried piping (collection)	18,000
Owens tunnel extension	12,500
Heat exchanger and piping	19,000
	<u>Subtotal</u>
	\$209,500
	15% contingency
	31,000
	8% engineering
	17,000
	15% O&P
	31,000
	<u>TOTAL</u>
	\$288,500

Estimates are based on the use of fiberglass piping with epoxy joints. For the Physical Plant, Cornett, Semon and Owens, costs have been included for the installation of isolation heat exchangers to eliminate direct use of geothermal fluid in heating equipment. Based on the maintenance difficulties which have been experienced as a result of the direct use of the geothermal fluids and the necessity of modifying the same piping for collection purposes, it would be an economical approach to combine these tasks. The incremental cost for the heat exchanger installation in the four buildings amounts to approximately \$43,000.

Figure 1 presents a diagram of the piping and tunnel additions necessary to accommodate the connection of the five remaining campus buildings to the collection system.

As a result of this construction, all but two of the campus buildings would be tied into the tunnel collection system. This system would discharge into the storage tank in the new classroom building. Allowing for heat removed by this building's heating system ( $1.2 \times 10^6$  Btu/hr), the outflow from this point would amount to 980 gpm @ 131 °F. This flow would then be delivered, through a buried 12" line to a point west of the building where it would join the flow from Cornett and the Physical Plant and terminate at the collection tank of the pumping station as shown in Figure 1.

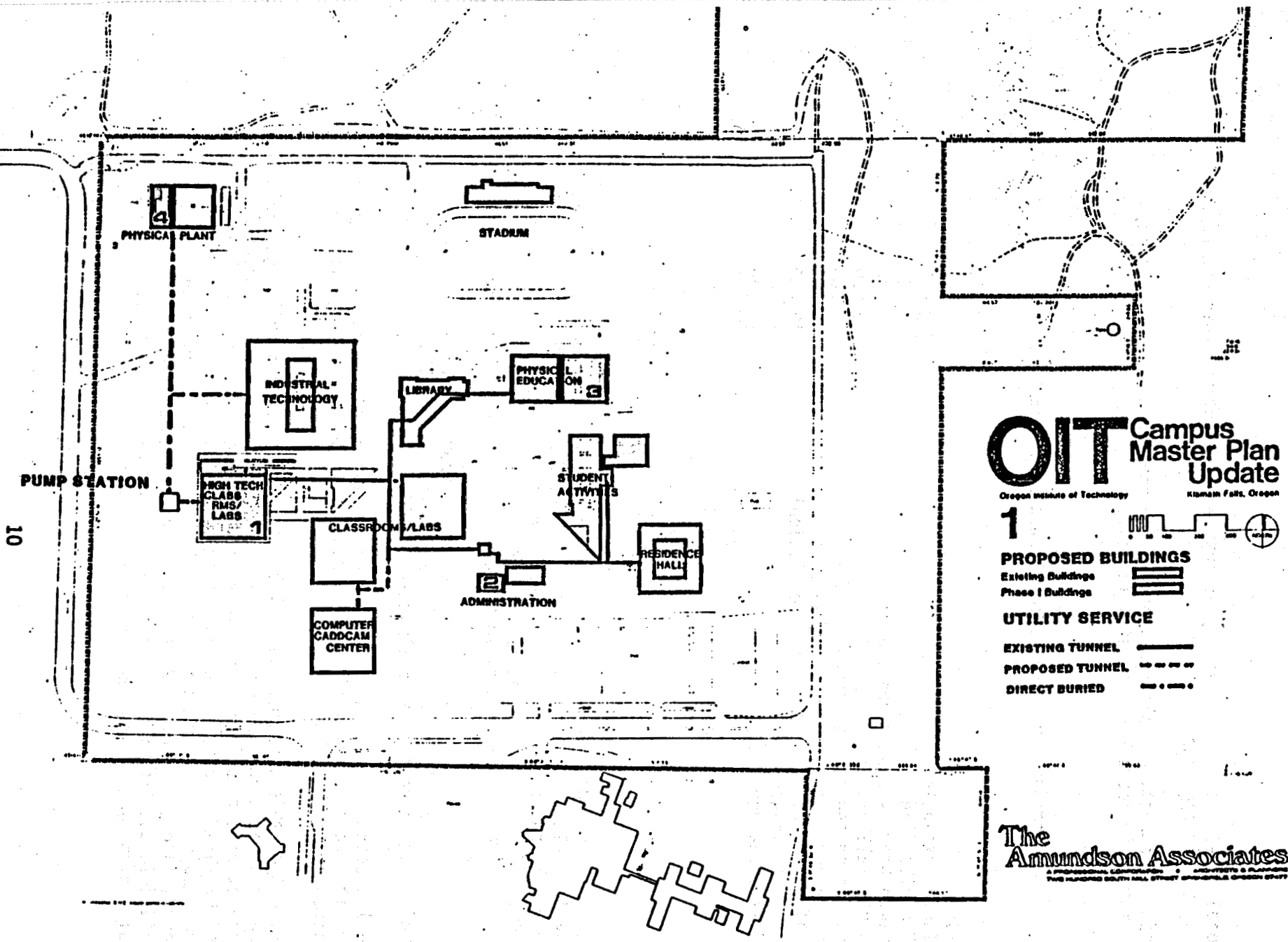


Figure 1. OIT tunnel and piping additions

#### 4.3 Impact of Future Campus Construction.

This flow rate, mentioned above, is based upon the present configuration of the campus, including the new building. The Campus Master Plan outlined in Phase I, addition to four major buildings. Table 4 outlines the key values with respect to heating requirements for each of these additions.

Table 4  
Phase I Building Addition - OIT

<u>Building</u>	<u>Present Area</u>	<u>Proposed Addition</u>	<u>Additional Heating Load</u> <sup>(1)</sup>
Snell	15,370 sq ft	12,000 sq ft	412 MBH
College Union	70,193 sq ft	22,000 sq ft	1193 MBH
Phys. Plant	19,301 sq ft	8,400 sq ft	469 MBH
Physical Ed	60,000 sq ft	44,000 sq ft	4113 MBH
Totals	164,864 sq ft	86,400 sq ft	6187 MBH

(1) Based on 80% of existing building load in Btu/hr x sq ft

The effect of this added heating load upon temperatures at various points around the collection system will be a function of the method of cascading. However, for purposes of off campus use, the primary area of concern is the total campus effluent temperature. Based on the peak flow available and the added peak load, a temperature drop of approximately 13°F would result from the additions set forth in Table 4. This would reduce the temperature of the campus effluent flow to approximately 116°F. This flow would be capable of meeting the full requirements of the industrial park.

#### 5. MERLE WEST MEDICAL CENTER COLLECTION SYSTEM

Subsequent to the signing of the contract for this work, the issue of including the hospital in the collection system surfaced. In order to address this interest, a brief evaluation was made of the cost of piping water from the hospital to the campus collection system. The tank, pump and pipe sizing involved with the pump station and transmission line, as described above, were based only on OIT campus flows.

The current Merle West disposal system involves surface discharge similar to OIT's system. Effluent from the heating system enters a storm drain near the northwest corner of the main parking lot. This storm drain (on OIT property) flows west along the south side of Campus Drive (see Figure 2) to a location just south of South Hall. At this point, it crosses under Campus Drive and travels west along the north side of Campus Drive for approximately 600 feet and then discharges into a drainage ditch along with South Hall effluent.

Peak discharge rate for the hospital amounts to approximately 300 gpm at a temperature of 120°F. Based on both the existing and future needs of the industrial park, it does not appear that this flow would be required for space heating purposes. In addition, the low temperature, in comparison to the OIT effluent, would tend to reduce the overall temperature of the fluid delivered to the industrial park. Finally, the added flow would possibly require a positive injection pressure at the injection not required for disposal of OIT effluent alone. This would depend upon the performance of the well. As a result, the inclusion of Merle West's effluent in the collection/delivery system would complicate and raise the cost of the system, but provide no benefit with respect to space heating in the industrial park.

As shown in Figure 2, three options were considered for pipe routing to deliver the Merle West effluent to the pump station. Each of these options follows the existing storm sewer route to a point on the north side of Campus Drive near South Hall. From here, option 1 would follow Campus Drive west and then north to the area of the present campus disposal ditch. At this point, the line would travel east to the pump station. Option 2 is similar to option 1 with the exception that the line would travel across the lower athletic field in a more direct route to the pump station. Option 3 involved tying into the existing campus collection system at either the tunnel extension for South Hall and Semon (not yet constructed) or at the existing tunnel at the southwest corner of Owens. The existing collection system has not been sized for this flow and would likely require some retrofit. In addition, the option 3 route would involve a major disruption of campus activity since the installation would cross existing parking areas adjacent to South Hall. For these reasons, costs were developed only for options 1 and 2.

In each case, costs were calculated for the installation of buried, uninsulated 6" material. Evaluations for both polybutylene and fiberglass were developed. These figures are summarized in Table 5 below.

**Table 5**  
**Cost Summary - Hospital Piping to OIT Pump Station**

	<u>Polybutylene</u>	<u>Fiberglass</u>
Option #1	\$55,000	\$63,000
Option #2	49,000	56,000

Since the Master Plan suggests no major construction on the lower athletic field in the area of the proposed option #2 and its route does not involve installation on city property (as does option #1), option #2 appears to be the most attractive.

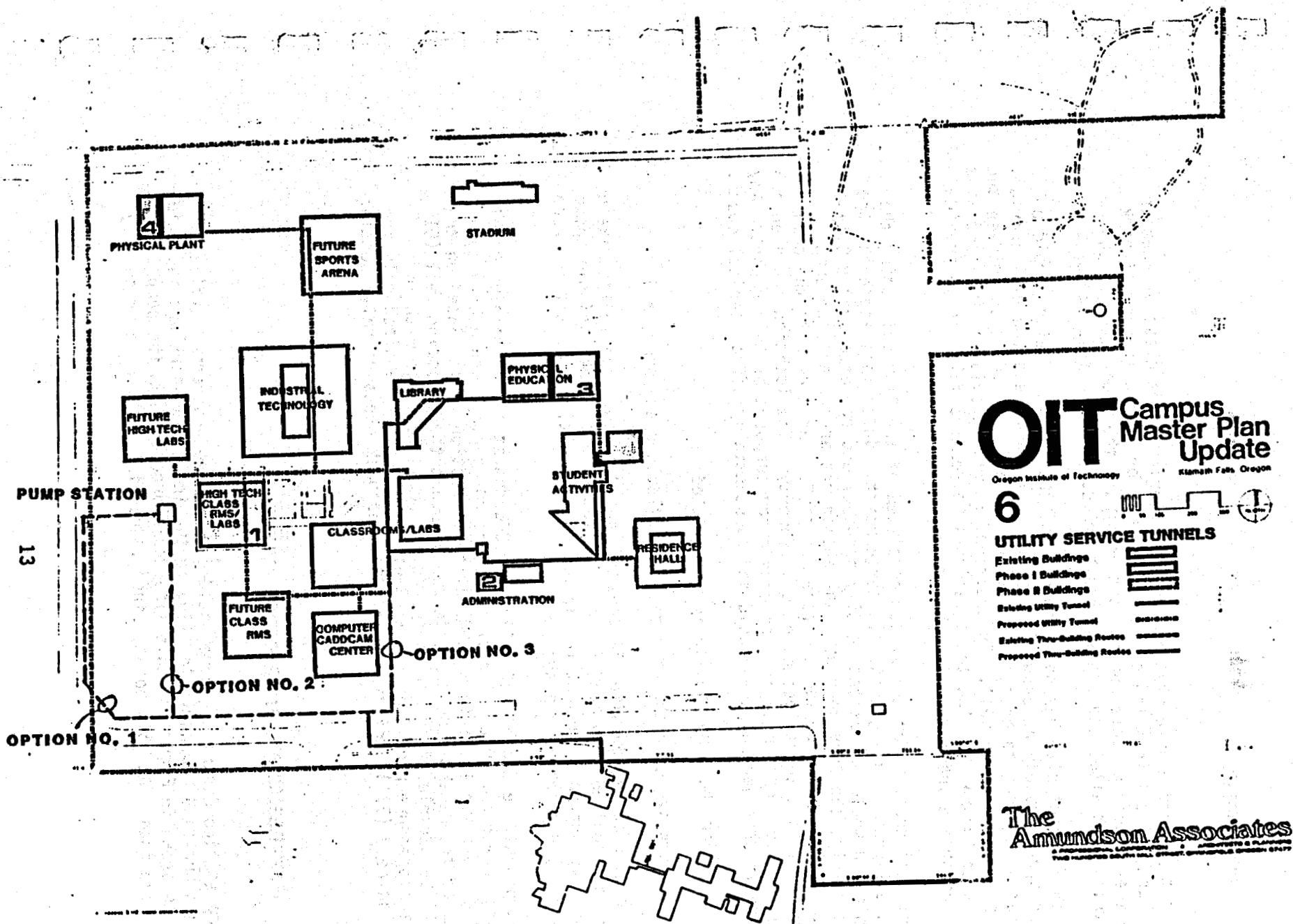


Figure 2. MWM pipe routing options

## 6. PUMP STATION

A collection system pump station would be required regardless of whether the injection well is sited on the OIT campus or in the industrial park. Since the purpose of this report is to evaluate the prospects for the industrial park site, the station has been designed for this purpose.

Although an exact site has not been chosen, the pumping station would be located approximately as shown in Figure 3. The pump station will provide the energy required to overcome the elevation and pipe friction between the tank and the injection well. In addition, it will provide a small positive pressure to assure that the transmission line remains full and to provide flow through the industrial park buildings.

The original concept for the system consisted of the City collecting the campus effluent from a gravity discharge point. Under this approach, the pump station would be located on City property and operated by the City. Unfortunately, the City does not own land in the area where the pump station would have to be located (see Figure 3). As a result, the pump station will have to be located on OIT property. The location shown on the figure is only approximate and was chosen for purposes of this report. Any location in this general area would not substantially affect capital costs.

Two designs for the pump station were evaluated:

- 1) A two pump system with one base load variable speed pump; and
- 2) A three pump system with all constant speed pumps. Due to the make up of the required pump head (primarily static head), the variable speed, two pump approach was not feasible. As a result, cost estimates were based upon a three pump system.

Each pump would be capable of delivering 300 gpm. at approximately 60 psi discharge pressure. Operation of the three 20 hp pumps would be staged according to water level in a 4000 gallon collection tank. Based on the results of the recent pump test, it appears that the injection well (CIP #1) will be capable of accepting the entire peak flow rate with no injection pressure required. As a result, the pumps were sized only for elevation head (77'), friction head (29') and back pressure valve setting (35').

## 7. TRANSMISSION AND DISTRIBUTION PIPING

The pump station described above would deliver the geothermal effluent through a buried 8" transmission line to the industrial park and injection well. As shown in Figure 3, this line would proceed from the pump station west to the road. At this point, since Industrial Park Drive is on City

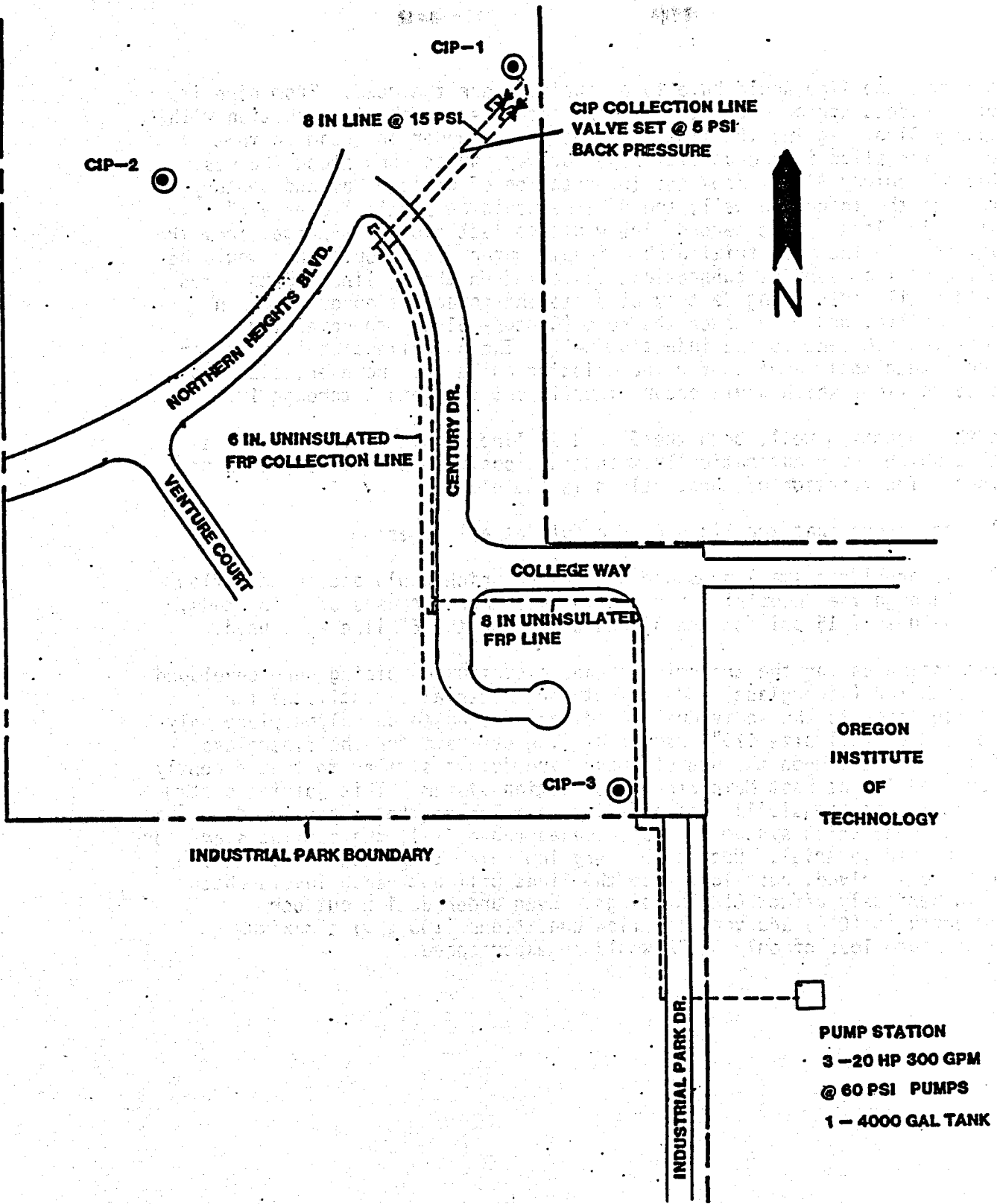


Figure 3. Collection /distribution system layout

property, the line would have to be buried under the road. From here it would proceed north to College Way and then west to the intersection with Century Blvd. Again, the line would be buried under the road so that it can be installed in the utility right of way, which runs along the west side of Century Blvd. From the intersection of College Way and Century Blvd. to the injection well, the 8" line would be paralleled by a 6" collection line. This second line would collect fluid discharged from the buildings in the industrial park. Supply water to the buildings would be delivered from service connections installed on the 8" line. Both lines would trail north along Century Blvd. to the intersection of Northern Heights Blvd. and bore under the road (Century Blvd.) to cover the remaining 650 feet to the injection well. The 6" collection line is far larger than that required for the existing building. However, size was based on flows which would occur at build out for lots 1 through 10.

At the injection well, both the 6" and 8" lines would be fitted with a valve which would automatically maintain a positive back pressure on the lines. The function of these valves is twofold:

- 1) to assure that the lines remain full at all times;
- 2) to provide a small pressure difference which would accommodate flow through the industrial park buildings. For purposes of this study, values of 15 psi for the 8" and 5 psi for the 6" line were used.

Cost estimates for the transmission and distribution piping were developed for both FRP (fiberglass) and polybutylene materials. Estimates for service lines to the individual buildings were based on polybutylene only due to the small size (1½") required. The estimate for the fiberglass piping is based upon the use of epoxy type joints similar to that recently installed in the East Main Street collection system. This joining system eliminates the possibility of a failure similar to that experienced on the main city district system. In both cases (PB & FRP), the estimates are for uninsulated material. Due to the very low water temperature and the short distance involved, heat loss from the lines will not reach levels which would seriously affect CIP buildings. Even under design outdoor temperatures (0°F) and very low flow conditions (100 gpm) a maximum temperature loss of only 5.5°F would be experienced.

**Table 6**  
**Cost Summary - Pump Station, Transmission**  
**and Distribution Piping**

	<u>Polybutylene</u>	<u>Fiberglass</u>
Piping from new OIT bldg. to pump station	10,500 (FRP)	10,500
Pump station	61,000	61,000
Piping from pump station to Industrial Park	118,000	124,000
Industrial Park distribution piping	39,000	44,000
Building service lines	5,000	5,000
Injection well head equipment	9,000	8,000
Subtotal	\$242,500	\$252,500
15% Contingency	36,000	38,000
8% Engineering	19,000	20,000
15% O&P	36,000	38,000
TOTAL	\$333,500	\$348,500

## 8. CIP BUILDING ENERGY ANALYSIS

Three of the existing three buildings in the College Industrial Park were inspected and analyzed for energy requirements. The owner of the fourth building, Swiss Precision Instruments, was contacted and rejected the energy analysis.

### 8.1 Quality Components.

An analysis of the Quality Components Building was done in 1981. At that time it was determined that retrofit of the building would cost approximately \$20,000. Energy use was predicted to be 12,961 kWh/year for the heat pump system and 5359 gallons/year of propane for the shop system. As a result of some changes in system operation, different water temperatures and the existence of actual fuel use data, it is appropriate that these figures be re-evaluated.

The Quality Components Building is a 14,160 ft<sup>2</sup> structure divided into a 12,240 ft<sup>2</sup> shop and a 1920 ft<sup>2</sup> office area.

The shop area contains approximately 300 connected horsepower in machine equipment, most of which is in operation for two shifts daily. In addition, there are 52 four hundred watt lighting fixtures. As a result of the oil vapors produced by the machining processes, a large quantity of ventilation air (approximately 8000 cfm) is employed. This air is admitted to the shop through the air compressor room where it is preheated by compressor waste heat. Building construction in the shop area consists of a slab floor, concrete walls for the first 8' topped by 8' of R-19 insulated frame wall. The flat roof is insulated to R-40. The heating

system is operated only during the two shift occupancy period. The balance of the day and weekends the system is shut off. Peak heating requirements for the shop amount to 111,377 Btu/hour.

For the 1981 study a simple manual calculation was employed for estimating annual energy requirements. Because this approach did not consider the heat gain from machining and lighting, the resulting value for fuel consumption exceeded actual use substantially. In addition, at that time ventilation air was being admitted through one of the gas fired unit heaters. This did not provide for sufficient preheating and the system was discontinued. For this study an annual energy calculation using the Simplified Energy Analysis program developed by Ferreira & Kalasinsky Inc. was performed, Appendix A. The results of this analysis show a predicted fuel use of 1898 gallons/year. This agrees well with the actual 1984 fuel use of 1873 gallons/year.

The office section, consisting of 1920 ft<sup>2</sup>, is a well insulated area with R-19 walls and double glass. In addition, the ceiling adjoins a storage area above. The office area is occupied only during business hours and is heated by a rooftop electric heat pump. Peak heating load amounts to 35,000 Btu/hour. Using the program mentioned above the calculated annual electricity use for this area amounts to 4950 kWh/year based on a C.O.P. of 1.6.

Retrofit of this building would be very similar to that at the Precision Data Carriage location. In the shop area, four new hot water unit heaters would be installed adjacent to the existing gas fired equipment. A new finned coil would be installed in the supply duct to the office. With the necessary distribution piping, heat exchanger, circulating pump and related equipment, retrofit capital cost would amount to approximately \$15,000. This figure is less than that estimated in 1981. Among the reasons for this are: the assumption of a higher available water temperature reduces equipment costs, the availability of off the shelf low temperature unit heaters, the elimination of special units for preheating outside air for ventilation and lower flow requirements reduce piping sizes.

Based on a 30° Δt, peak flow requirements for Quality Components, would amount to 25 gpm. Annual energy requirements would be .136 x 10<sup>9</sup> Btu/year.

## 8.2 Precision Data Carriage.

Precision Data Carriage Inc. was built in 1983 by the Klamath County Economic Development Corporation, the 12,400 ft<sup>2</sup> structure is divided into two general areas: a 9600 ft<sup>2</sup> shop and a 2800 ft<sup>2</sup> general office area.

The basic construction consists of 6" bare concrete "tilt-up" walls, slab on grade, and an uninsulated plywood deck, built up roof with an interior ceiling height of approximately 15'.

In the shop area, space heating is provided by two gas fired, ceiling hung, unit heaters with a combined capacity of approximately 215,000 Btu/hour. In addition, a "clean room" module has been constructed in this area. This module operates as a completely self contained unit within the shop, occupying roughly 25% of the total area. The clean room employs its own heating and cooling system (all electric) using shop air to maintain a positive pressure within the module. Due to the uninsulated construction of the shop, the peak heat loss of 432,715 Btu/hour far exceeds the currently installed heating equipment capacity. As a result, a propane fired portable heater has been rented on a long term basis to provide the additional capacity requirement. As a result of the constant interchange of air between the shop and clean room (through exfiltration), the clean room heating system is also contributing to shop requirements.

The office area was completed with an insulated drop ceiling, frame walls and single glass. Space heating is provided by a gas fired duct heater located in the shop area and connected to the office's air distribution system. Peak load for the office area amounts to 51,733 Btu/hour. An annual energy analysis for this building was performed using the Simplified Energy Analysis Program developed by Ferreira & Kalasinsky Inc., Appendix A. The results of the analysis show a predicted annual natural gas consumption of 15,047 therms per year. Actual fuel consumption for 1985 amounted to 12,219 therms. As mentioned above, however, both propane and electricity contribute to heating requirements. As a result, if all heating were provided by natural gas, the figure would likely be much closer to the predicted value.

Due to the substantial savings potential resulting from the addition of insulation to the shop ceiling, a second analysis was done to evaluate this measure. Assuming the use of R-19 insulation in the shop, peak heating load for this area would be reduced to 162,541 Btu/hour. Two major benefits would arise from this. First, the heat loss would be reduced to the point where the existing unit heaters could meet the load, thus eliminating the need for the propane fuel and heater rental costs. Second, annual energy requirements would be substantially reduced. The calculated natural gas consumption would be reduced 50% to 7595 therms/year. Assuming all present heating costs were provided by natural gas (conservative in view of the higher costs for propane and electricity), the annual savings resulting from the installation of the ceiling would amount to \$3645. As a result, geothermal retrofit and energy requirements have assumed that this insulation retrofit would be accomplished prior to the consideration of a geothermal system.

Retrofitting the building for geothermal use would consist primarily of installing hot water type unit heaters alongside the existing gas fired equipment. A new finned coil heat exchanger would be placed in the office air distribution system. As a result, all present heating equipment could remain as backup. Assuming a 120°F supply water temperature and the use of a single loop distribution system, circulating pump, expansion tank and a minimum of instrumentation, retrofit costs would amount to approximately \$12,000.

Energy purchases based on an efficiency of 75% for the present gas fired equipment would amount to  $.570 \times 10^9$  Btu/year.

Using a system  $\Delta t$  of 30°F, peak geothermal flow requirement would be 20 gpm.

### 8.3 Equitorial Communications.

Equitorial Communications occupies the most recently constructed building in the College Industrial Park on Tracts 9-11, adjacent to Century Drive, the 19,200 ft<sup>2</sup> facility was completed during the fall of 1986. Approximately 5200 ft<sup>2</sup> of office space and a 14,000 ft<sup>2</sup> assembly area with a 16' ceiling are included.

The building is basically of block construction and is very well insulated. The roof system includes R-19 ridged insulation. Walls are 2 x 4 furred and insulated to R-11. The north wall is R-19 insulated and all windows are double glass.

Space heating is supplied from five individual pieces of equipment. In the assembly area, two roof top gas/electric units provide both heating and cooling needs. Both units are of 10 ton capacity. In the southeast corner of the building a 110,000 Btu/hour gas unit heater is located adjacent to the overhead receiving door. The office area is handled by two more gas/electric units; one at 7½ ton and one at 4 ton capacity. The peak heating capacity of this equipment amounts to approximately 716,000 Btu/hour. However, it is important to note that package equipment such as this is generally selected on the basis of its cooling capacity. As a result, the heating side often bores little relationship to the actual requirements of the building.

The peak calculated heating load for the building amounts to approximately 240,000 Btu/hour. Due to the air flow capabilities of the existing equipment, design for this heating load would result in very low supply air temperatures and a "drafty" feeling. As a result, the retrofit coils were selected on the basis of a 100°F supply air temperature. Although this results in a total peak capacity much greater than the actual requirements of the space, it is felt that this system would be more acceptable to the occupants.

An annual energy analysis for this building was performed using the Simplified Energy Analysis software developed by Ferreira & Kalasinsky Inc. The results of this analysis show a predicted annual natural gas consumption for this building of 5876 therms based on a 70% efficiency.

Retrofit of the existing system would consist of the installation of new hot water coils in the supply air duct of the four rooftop units. In addition, a new hot water unit heater would be installed adjacent to the existing gas fired unit in the receiving area. This configuration would allow all existing heating equipment to remain as backup. Assuming a 120°F

supply water temperature and the use of a single loop distribution system, circulating pump, expansion tank and a minimum of instrumentation, retrofit costs would amount to approximately \$22,000.

Energy purchases, based on an efficiency of 70% for the present system, would amount to  $.411 \times 10^9$  Btu/year.

Using a 30°F  $\Delta t$ , peak geothermal flow requirement would be 33 gpm.

## 9. AQUIFER STRESS TESTS

### 9.1 Step-Drawdown Testing.

The step-drawdown pumping test was carried out August 21, 1986. The test comprised three steps. Pumping rates for the steps were 177, 251, and 336 gallons per minute (Marshall Gannett, Oregon DWR, personal communication via Paul Lienau, OIT, 1986). The drawdown data are summarized below in Table 7 and illustrated in Figure 4.

Table 7  
Summary of Step-Drawdown Test Results for CIP #1

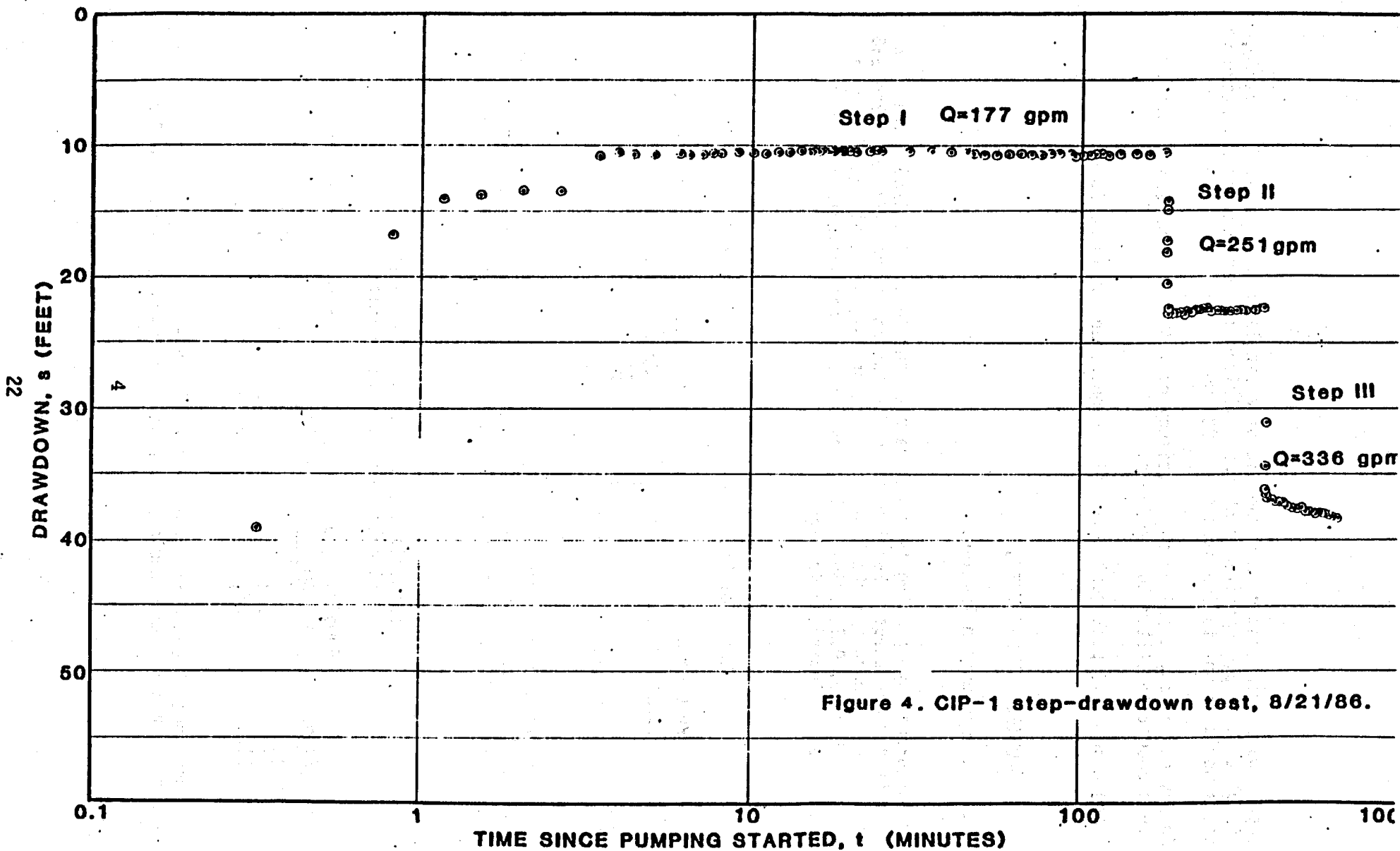
Step	Pumping Rate Q (gpm)	Duration t (mins)	Drawdown s (feet)	Specific Capacity C (gpm/ft)
I	177	180	10.5	16.9
II	251	180	22.5	11.2
III	336	240	37.5	9.0

Specific capacity and drawdown data, plotted in Figure 5, suggest that the well developed during the test. This observation, supported by the well loss analysis depicted in Figure 6, utilizes the data summarized in Table 8.

Table 8  
Specific Drawdown Data from CIP #1  
Step-Drawdown Testing Results

Step	Pumping Rate Q (gpm)	Drawdown s (feet)	Specific Drawdown s/Q (ft/gpm)
I	177	10.5	0.0593
II	251	22.5	0.0896
III	336	37.5	0.0112

The work sheet located in Appendix B shows that these data yield impossible results. That is, they would require a negative loss in the aquifer



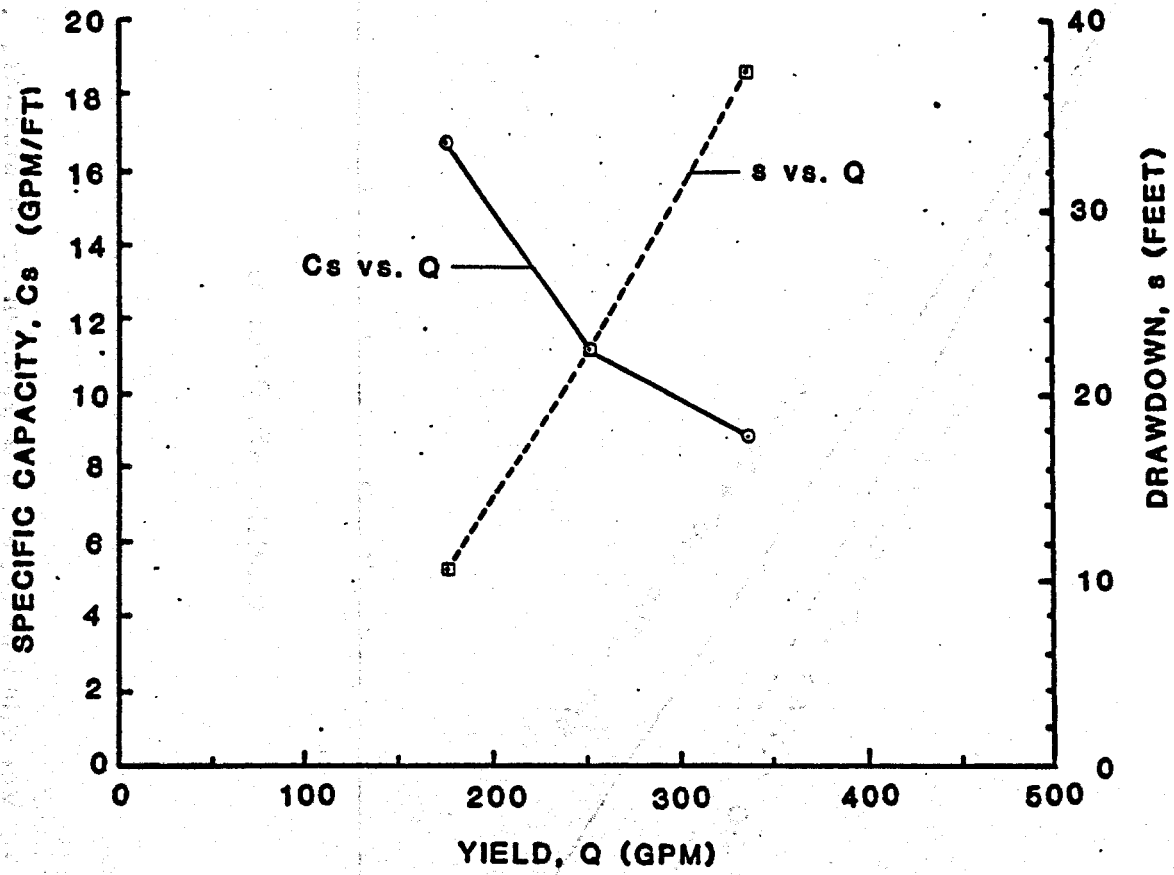


Figure 5. Specific capacity and drawdown versus yield, CIP-1.

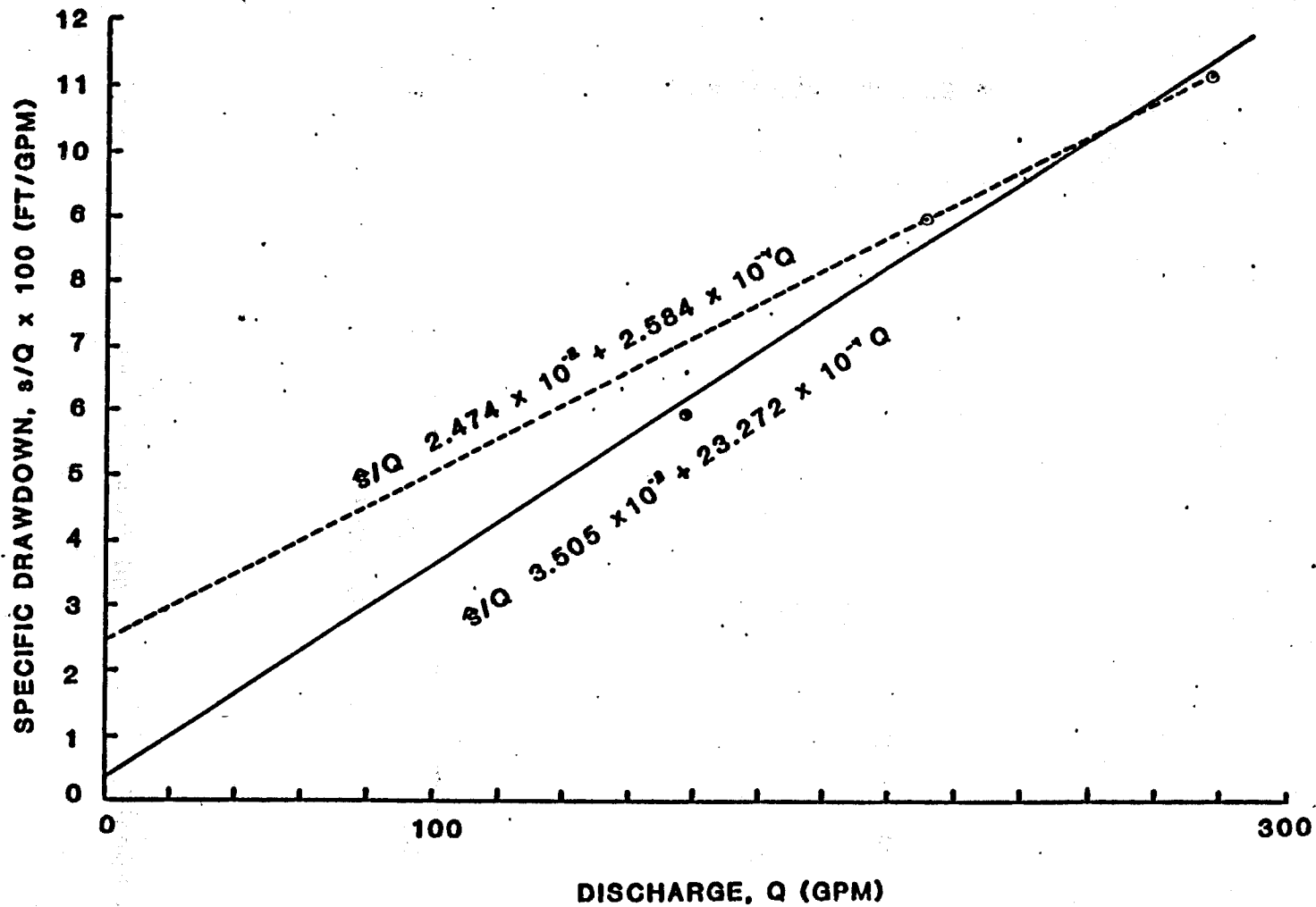


Figure 6. Variation of "specific drawdown" with discharge, CIP-1.

adjacent to the well to fit a straight line to the log-log plot of  $s/Q$  versus  $Q$ , a condition which does not exist in nature. The most rational explanation is that the well was not fully developed prior to the step-drawdown test and became more efficient as the test progressed. This is a reasonable explanation, since the well received physical and chemical treatment prior to testing to remove residual drilling fluids from the well bore and adjacent formation materials in residence in the well since it was completed in May 1978.

The analysis presented in Appendix B portrays the variation in inverse specific capacity of the well with a discharge and the impact of the development of the well in the latter stages of the test. The points should form a curve which becomes steeper with increasing discharge; not flatter, as in the data plot in Appendix B. Consequently, it had to be assumed that the well loss varies with the square of the discharge, and that either all three points are approximately correct or only the last two points are meaningful. Based on these assumptions, the calculated efficiencies equal those presented in Table 9 below.

Table 9  
Calculated Well Efficiencies for CIP #1

Pumping Rate (gpm)	Efficiency %		Specific Capacity (gpm/ft)	
	Three Points	Last Two Points	Three Points	Two Points
100	9.7	48	28	20
300	3.4	24	9.8	9.8
500	2.1	16	6.0	6.5
700	1.5	12	4.3	4.9

(Note - the number of points refers to the number of data points in Figure 7 which were utilized to fit the trend line)

The low efficiencies calculated from the available data are consistent with calculated values of efficiency for other wells completed in consolidated rock aquifers. In the case of CIP #1 the low efficiencies may be exacerbated by well construction deficiencies such as fill in the bottom of the well bore or the offset of perforations from permeable horizons, both of which may retard the influx of ground water into the well.

The effects of well bore storage cannot be evaluated from the data collected. The 8 5/8 inch O.D. casing holds approximately three gallons of water per foot of length. At pumping rates such as those utilized in the testing sequence, the part of the curve dominated by well storage is within the first minute of the test. Manual measurements collected within this initial time period are too infrequent to define this portion of the curve with any confidence.

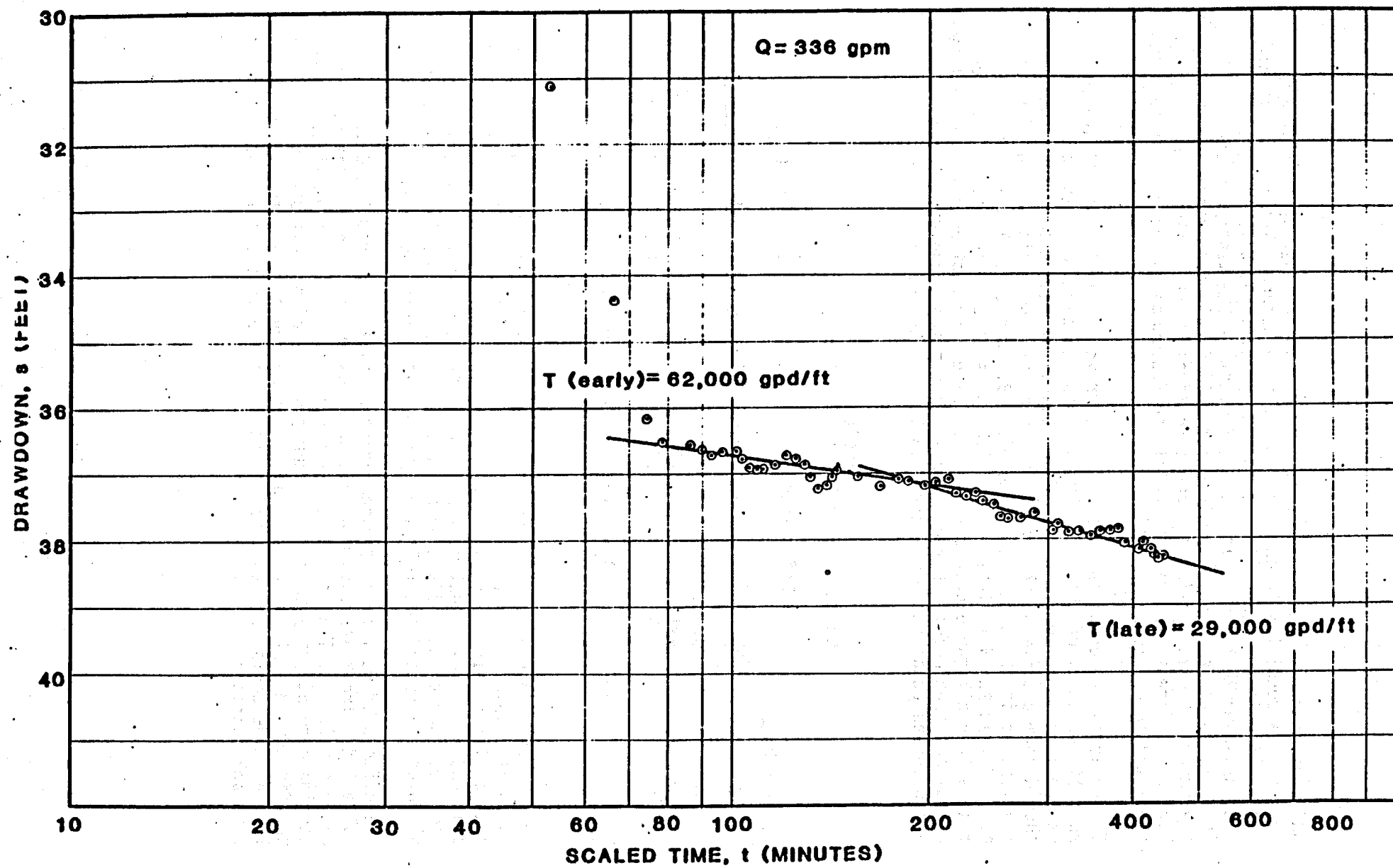


Figure 7. Third step of CIP-1 step-drawdown test. 8/21/86.

The data from the third step were analyzed by the Cooper-Jacob approximation of the Theis Equation. The data, illustrated in Figure 7, were adjusted to removed the influence of the preceding two steps and plotted as if the first two steps did not occur. The break in slope of the data plot after approximately 200 minutes of pumping appears to be caused by a double porosity effect as opposed to an impermeable boundary. Had an impermeable boundary been intercepted, the increase in slope would have been less than that which was observed. The transmissivity of the aquifer, based on the step-drawdown data alone, is calculated as  $2.9 \times 10^4$  gpd/ft. From this value, the (dimensionless) "skin factor",  $2 TBQ$ , where,

T is the aquifer transmissivity,

B is the well loss coefficient (the slope of coefficient from Figure 6), and

Q is the pumping rate,

is calculated.

Table 10  
Skin Factor Calculated from CIP #1  
Step-Drawdown Test Data

Q (gpm)	Skin Factor	
	Three Points	Two Points
100	4.14	3.27
300	12.40	9.81
500	20.70	16.30
700	29.00	22.90

(Note - the number of points refers to the number of points utilized in the well loss analysis as depicted in Figure 7)

If the break in slope was, indeed, the result of an impermeable boundary, the transmissivity becomes  $6.2 \times 10^4$  gpd/ft, and the skin factor is proportionately greater than the values presented above. However, data from the constant-discharge test discussed below do not support this interpretation.

## 9.2 Constant-Discharge Test.

The constant-discharge pumping test was conducted September 22 through 28, 1986. Data were collected from 10 observation wells in addition to the pumped well, CIP #1. These are: CIP #2, CIP #3, OIT #3, OIT #4, OIT #5, the Smith well, La Vista Motel well, North Entrance Motel well, the Klamath-Pacific well, and the JELD-WEN well. Locations of the wells are depicted in Figure 8. Of the 10, the following three yielded to analyzable

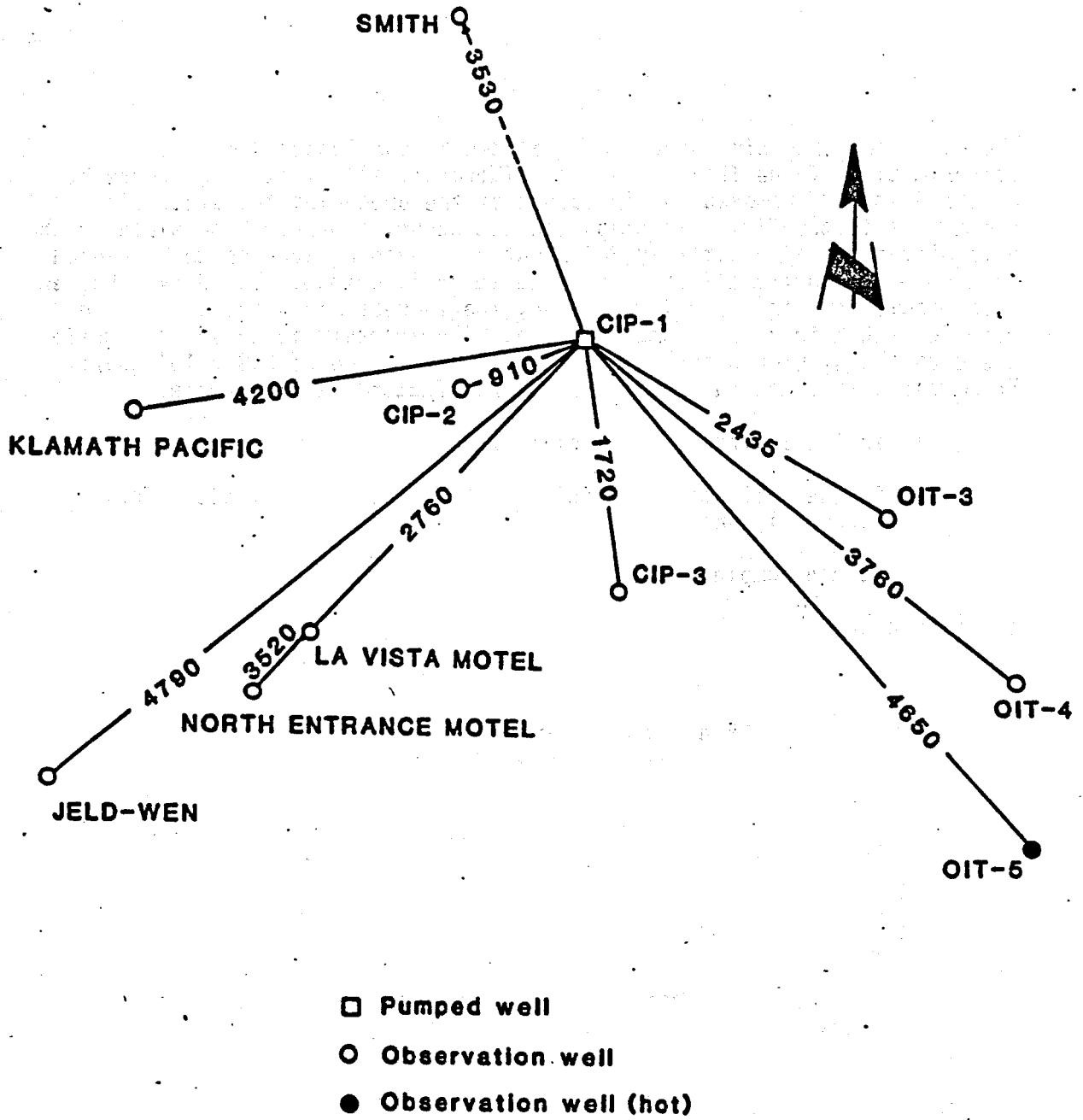


Figure 8. Observation well network, CIP-1 aquifer stress test, 8/22 -28/86.

data: OIT #4, because the pressure gauge on the air line was not sufficiently sensitive; Smith and Klamath-Pacific wells, both because a response due to pumping CIP #1 could not be distinguished from random fluctuations and/or measurement error.

The discharge rate for CIP #1 was not precisely constant for the entire test (refer to Figure 9). In fact, it decreased essentially linearly with time (refer to Figure 10). The cause of the decrease in the discharge of the pump is unknown. It may have resulted from binding of the impellers or plugging of the pump intake by formation materials or a defective gate valve which may have partially closed during the test. However, no information is available to support any of these hypotheses.

The effect of the decreasing discharge was obvious only in the pumped well and manifested itself as a substantial rise in the pumping water level (refer to Figure 11). The drawdowns in the pumped well, CIP #1, were corrected to a constant pumping rate of 348 gallons per minute (gpm) using a simplified version of the theory behind VARFLOW. This equates to the discharge rate for the first day of the test when the discharge rate was essentially constant. The corrected drawdowns, plotted in Figure 11, show that the drawdown still trends upward. This is an impossibility for a constant flow rate. Perhaps in reality, the drawdown data would have stabilized, a hypothesis that is supported by recovery data.

Drawdown data for usable observation wells are plotted in Figures 12 through 18. Data plots for the wells which could not be analyzed are presented in Appendix B. Values of the aquifer coefficients were calculated utilizing the Theis non-equilibrium equation. Late-time data, where available, were preferred because of the potential for double porosity effects to yield erroneous early-time values. Values for transmissivity of the aquifer calculated from the observation well data are summarized in Table 11 below.

Table 11  
Transmissivity Values Calculated from CIP #1  
Aquifer Stress Test, August 22-28, 1986

Well	Radical Distance from CIP #1, r (feet)	Transmissivity T (gpd/ft)	Coefficient of Storage S
CIP #1	--	$3.4 \times 10^5$	--
CIP #2	910	$1.4 \times 10^5$	0.075
CIP #3	1,720	$9.6 \times 10^4$	0.035
OIT #3	2,435	$3.7 \times 10^4$	0.015
OIT #5	4,650	$7.4 \times 10^4$	0.004
La Vista Motel	2,760	$1.9 \times 10^5$	0.039
North Entrance Motel	3,520	$7.4 \times 10^4$	0.013
JELD-WEN	4,790	$5.1 \times 10^5$	0.0210



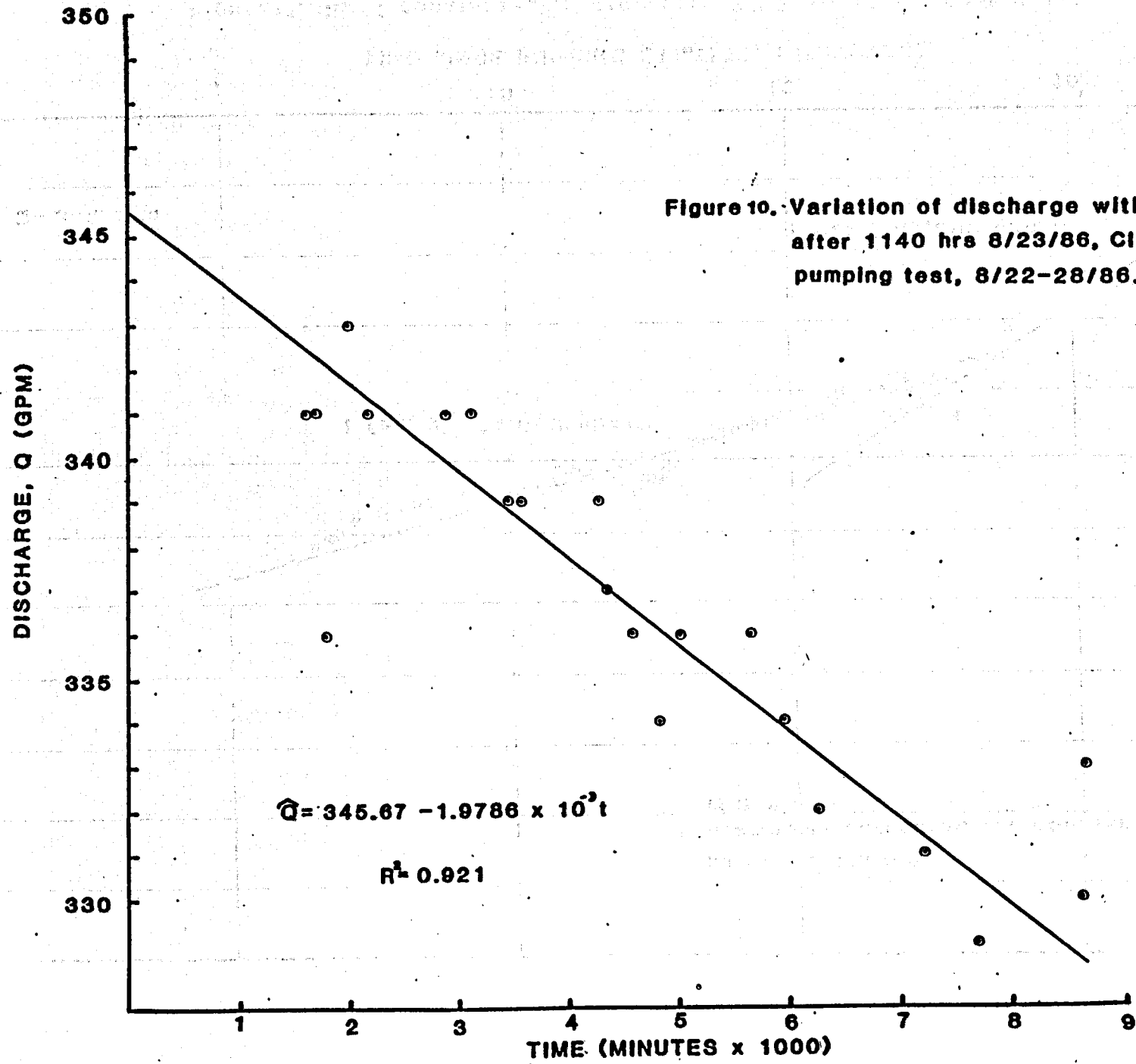


Figure 10. Variation of discharge with time after 1140 hrs 8/23/86, CIP-1 pumping test, 8/22-28/86.

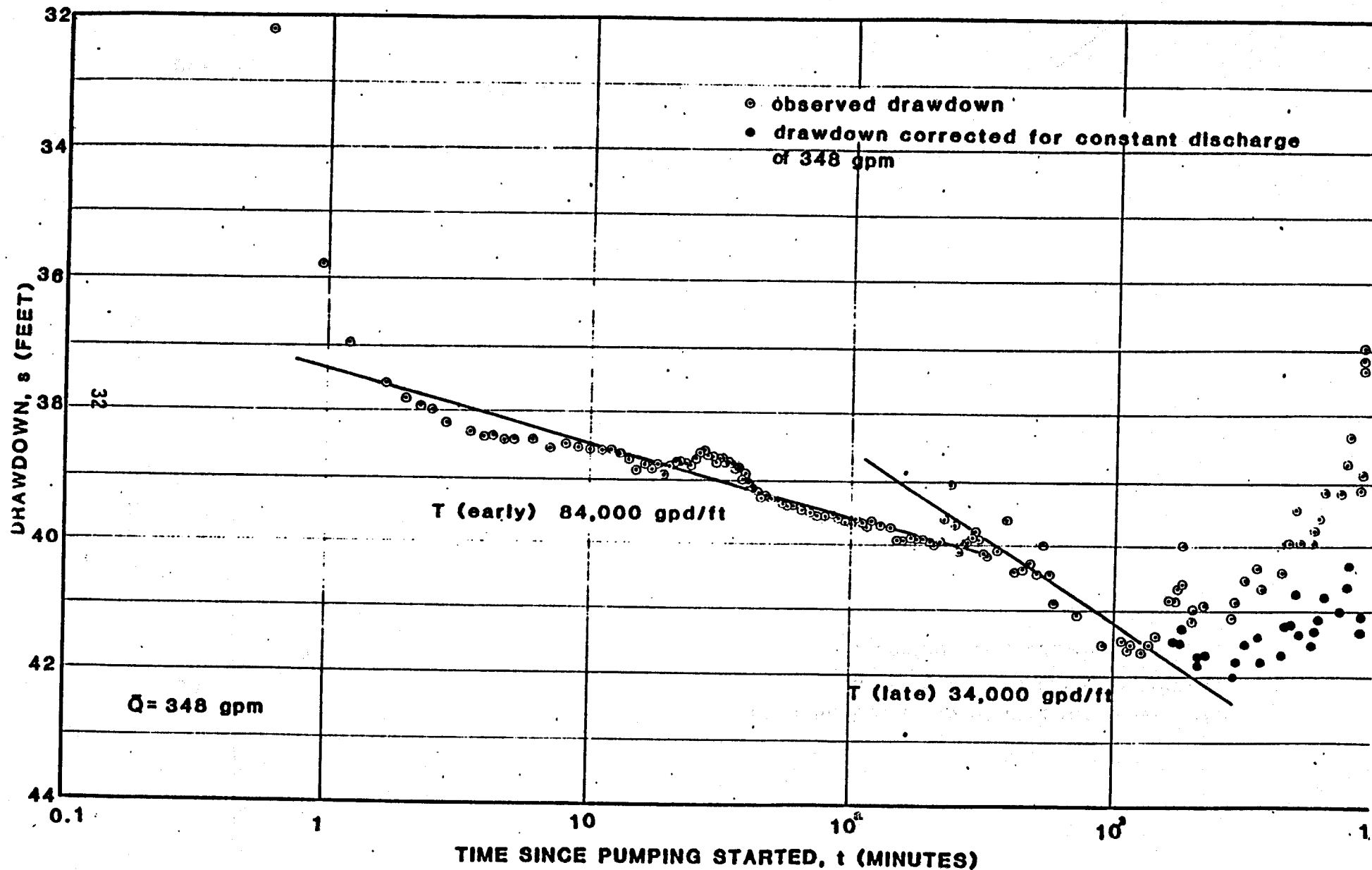


Figure 11. CIP-1 constant-discharge test, 8/22-28/86, data for CIP-1.

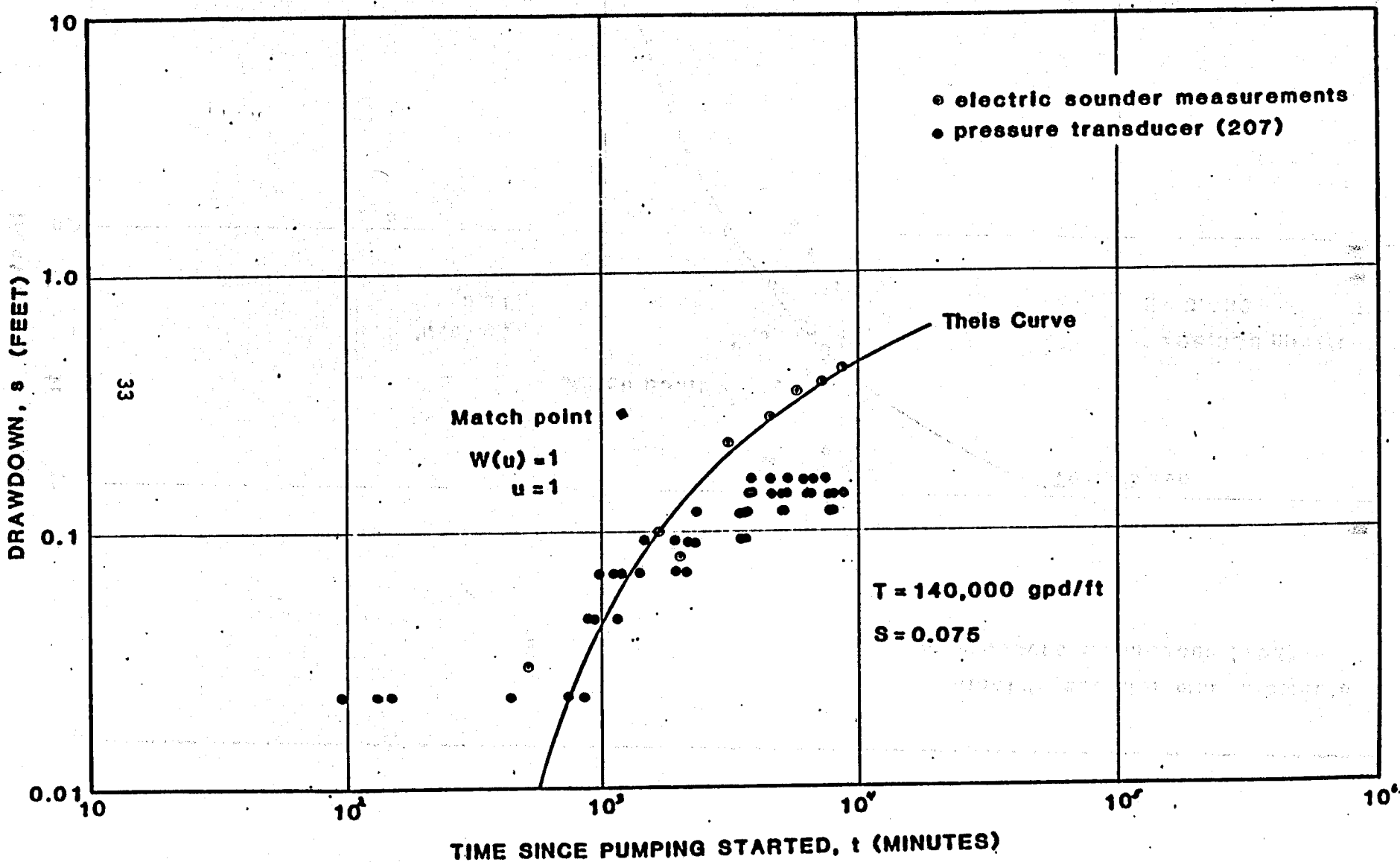


Figure 12. CIP-1 constant-discharge test, 8/22-28/86, data for observation well CIP-2.

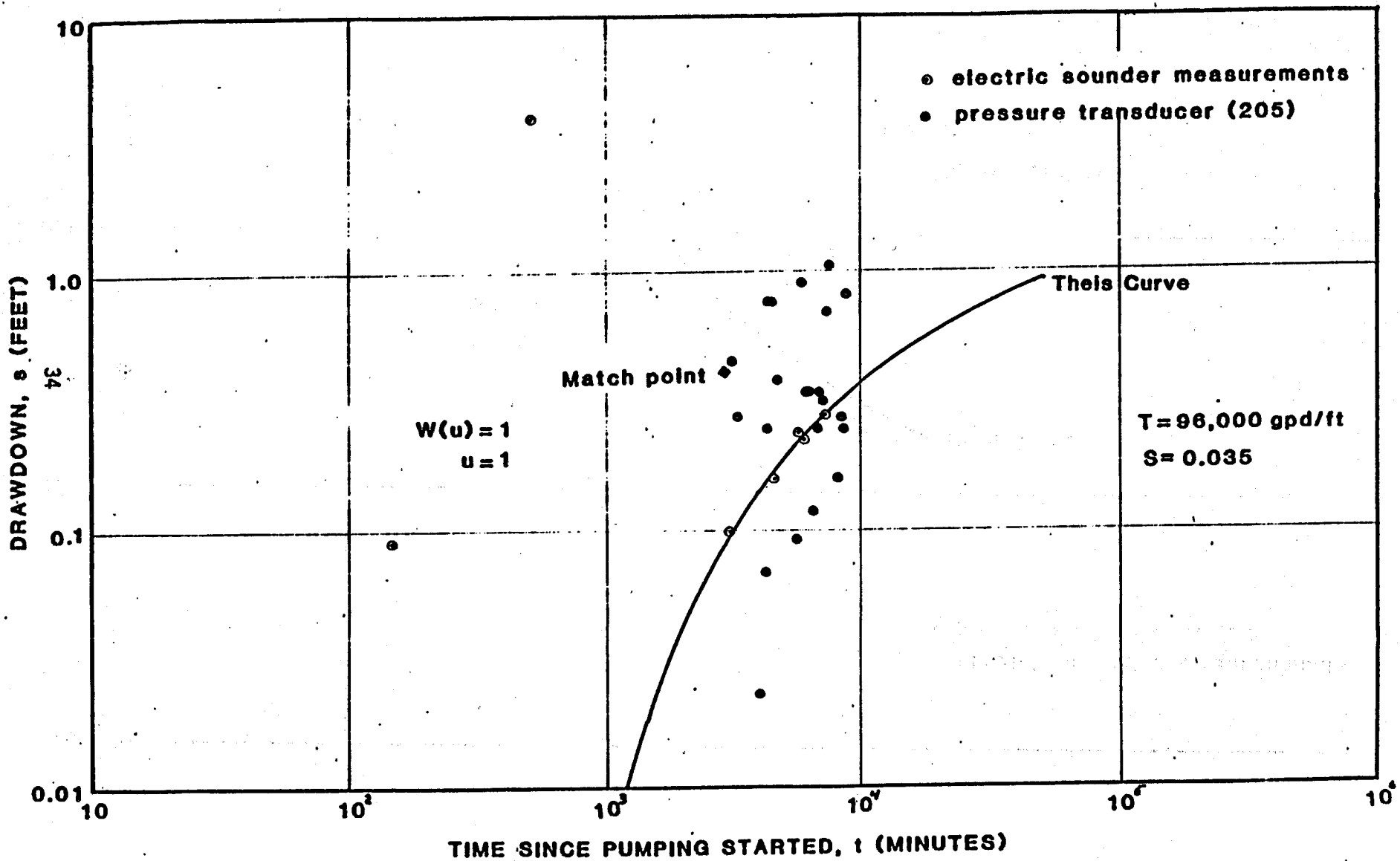


Figure 13. CIP-1 constant-discharge test, 8/22-28/86, data for observation well CIP-3.

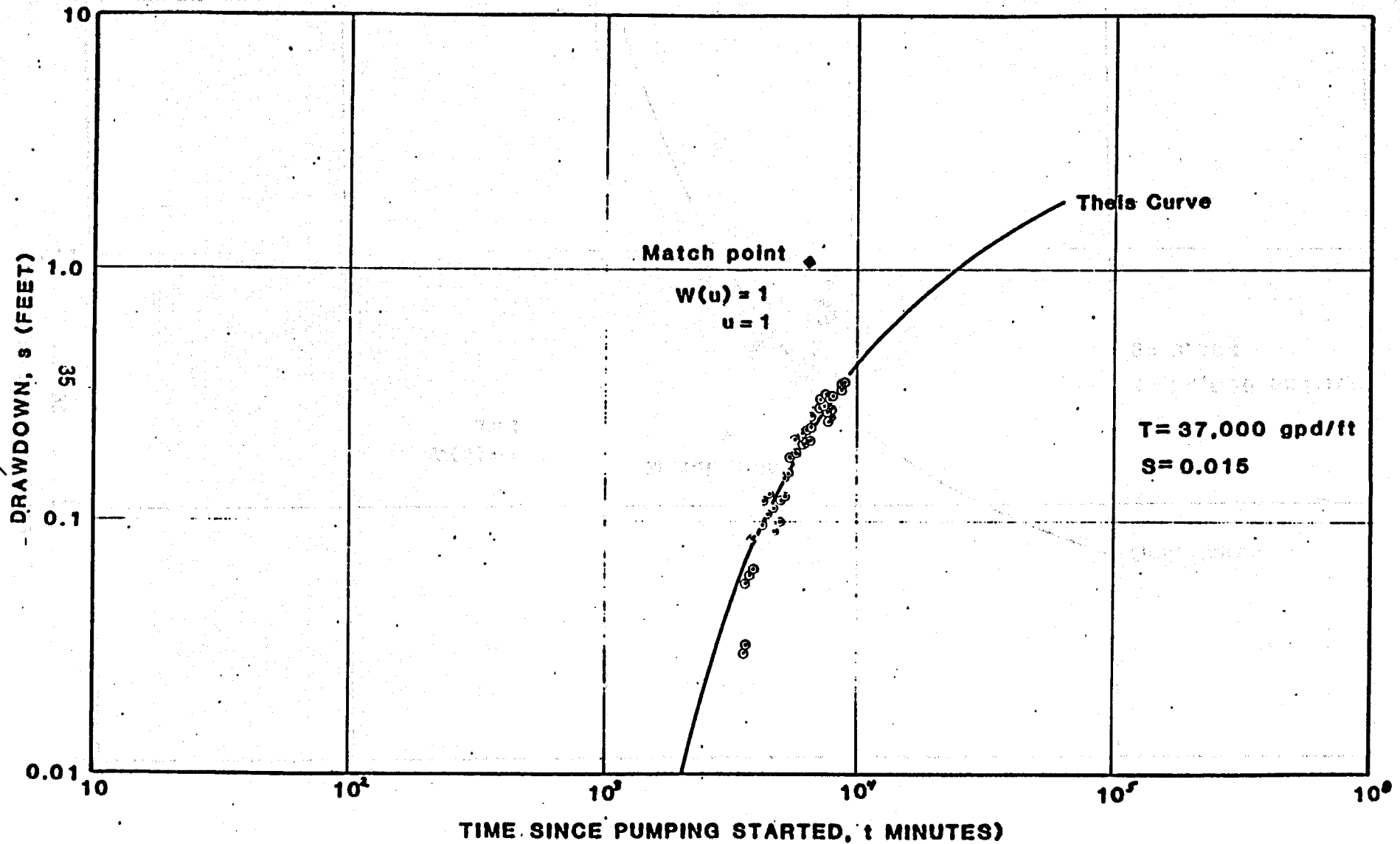


Figure 14. CIP-1 constant-discharge test, 8/22-28/86, data for observation well OIT-3.

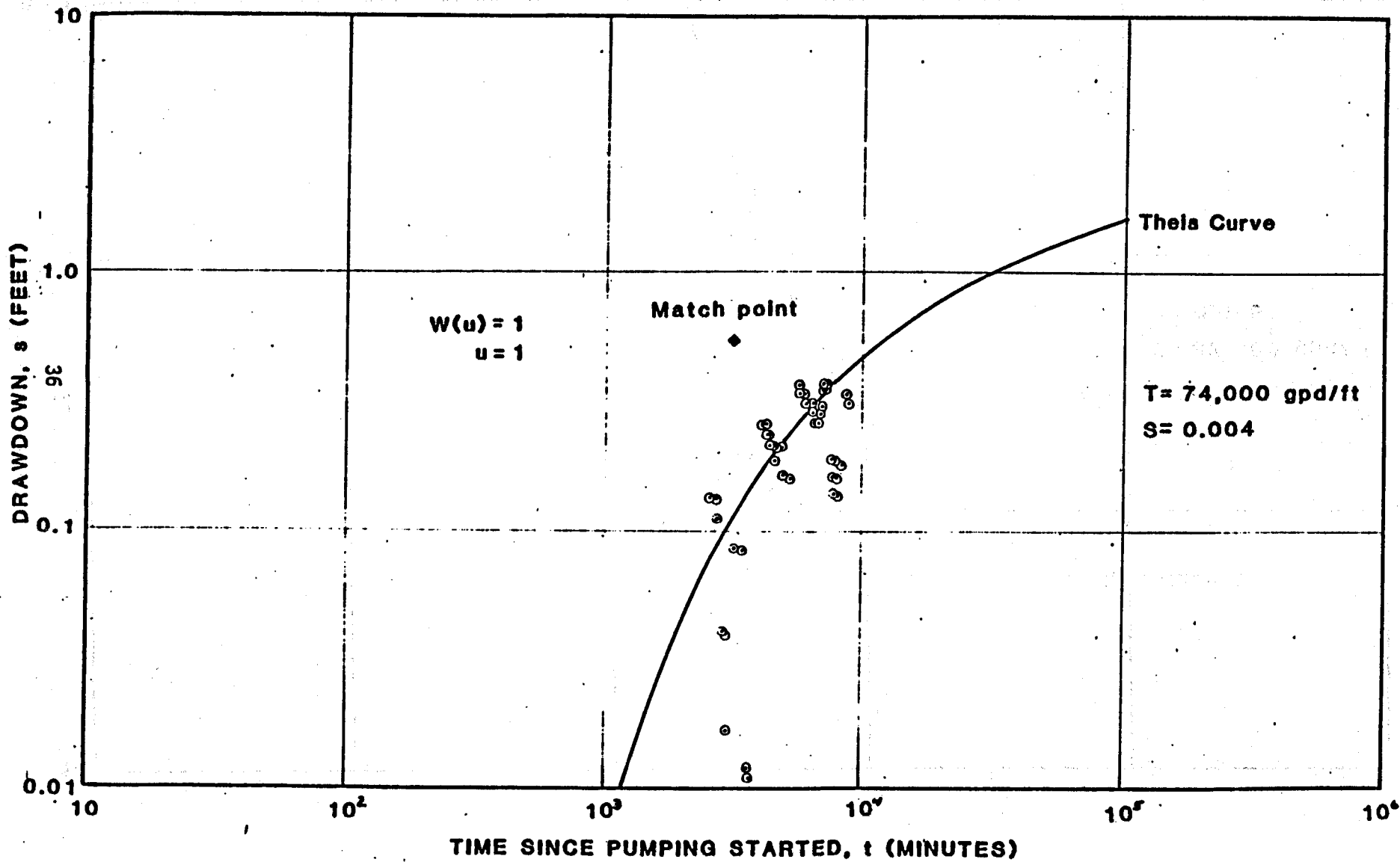


Figure 15. CIP-1 constant-discharge test, 8/22-28/86, data for observation well OIT-5.

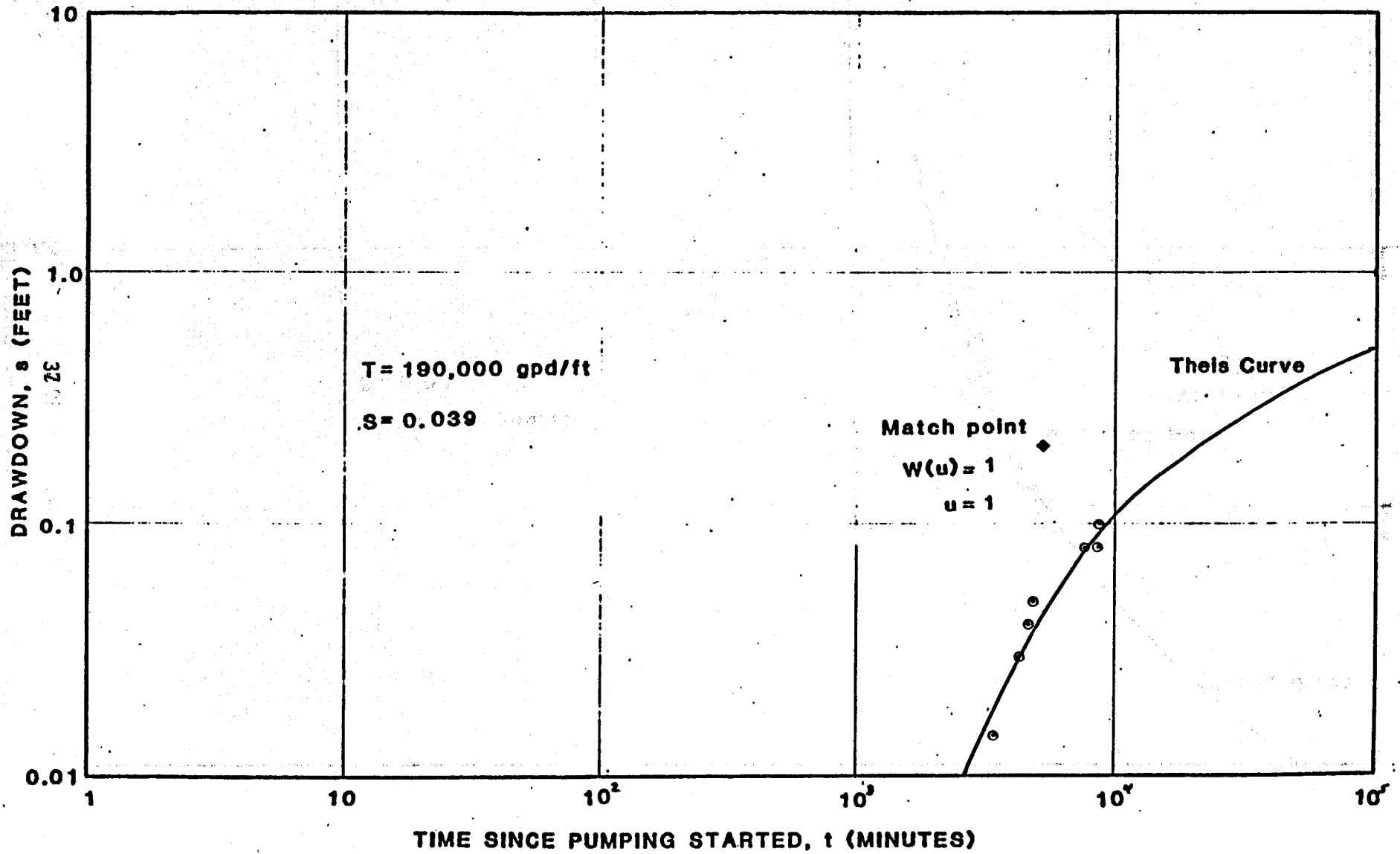


Figure 16. CIP-1 constant-discharge test, 8/22-28/86, data for La Vista Motel observation well.

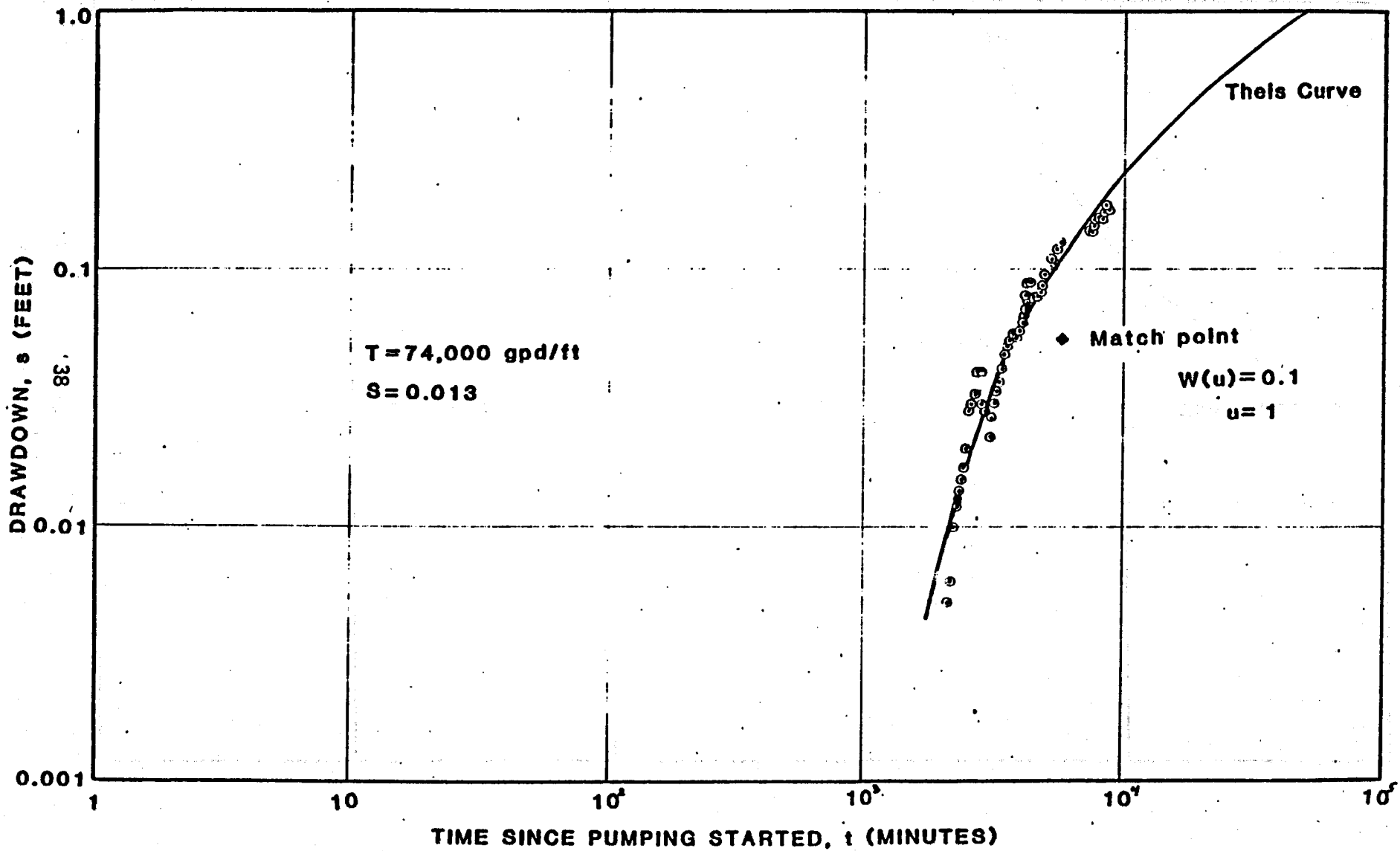


Figure 17. CIP-1 constant-discharge test, 8/22-28/86, data for North Entrance Motel observation well.

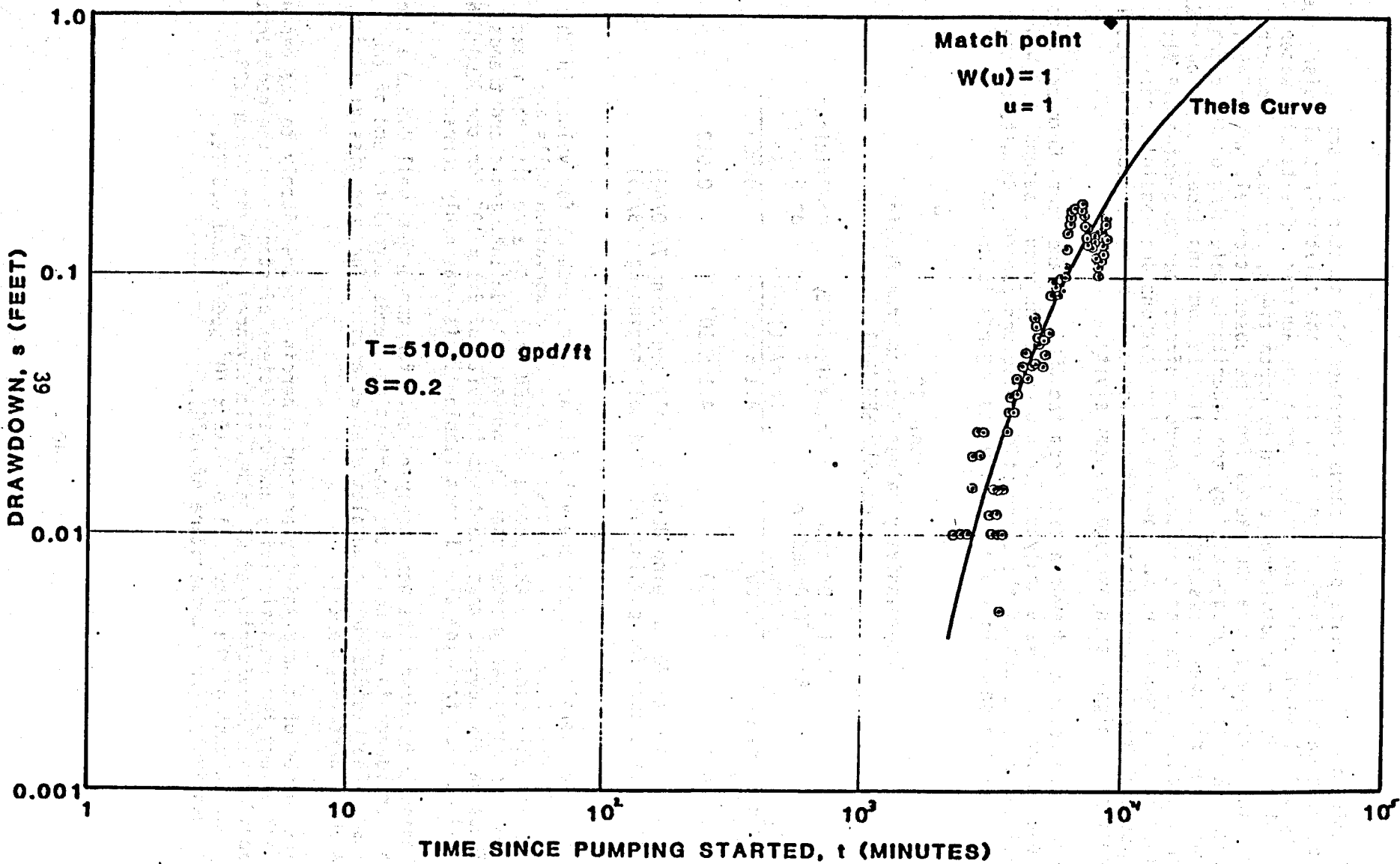


FIGURE 18. CIP-1 constant-discharge test, 8/22-28/86, data for JELD-WEN observation well.

Examination of the data plots shows considerable scatter for some observation wells. This is, in part, due to the relatively small discharge from CIP #1 which could not fully stress the aquifer. Pre-testing water level trends and fluctuations caused by earth tides, barometric, and other outside influences were subtracted from the observation well water-level data. Errors were exaggerated by the logarithmic weighting of the graphs when the drawdowns themselves were small. In some instances, these outside influences were greater than the drawdown effects due to pumping CIP #1. Therefore, large errors in drawdown determinations are possible, particularly for the more distant observation wells or wells located in local regions of higher than average transmissivity.

For these reasons, drawdown data from the JELD-WEN and North Entrance Motel observation wells were analyzed for two occasions when a second JELD-WEN well, located south of the observation well, was pumped. Results of these analyses are shown in Table 12 below and Figures 19 and 20.

Table 12  
Transmissivity Values Derived from Pumping  
the JELD-WEN Well

Well	Radial Distance from JELD-WEN, r (feet)	Transmissivity T (gpd/ft) <sup>s</sup>	Coefficient of Storage S
North Entrance Motel*	2,000	$4.1 \times 10^5$	0.006
JELD-WEN**	1,330	$4.1 \times 10^5$	0.010

(\* denotes data for pumping JELD-WEN well beginning 7/30/86)

(\*\* denotes data for pumping JELD-WEN well beginning 8/8/86)

These serendipitous tests are valuable as a cross-check for values in this area derived from the CIP #1 test because the observation wells are considerably closer to the pumped JELD-WEN well. As a result, the impact due to pumping was greater than the varied outside influences and trends. These values suggest that the transmissivity of  $5.1 \times 10^5$  for the JELD-WEN well derived from the CIP #1 test (Table 11) is reasonably correct.

Residual-drawdown data were analyzed only for the pumped well due to a lack of confidence in extending the pre-testing trends beyond the end of the test. The residual-drawdown plot yielded a value for transmissivity of  $8.4 \times 10^4$  gpd/ft (Figure 21).

The values of transmissivity presented herein compare closely to the benchmark values for transmissivity and coefficient of storage of the geothermal aquifer in the vicinity of Klamath Falls advanced from the Summer 1984 USGS/LBL test of the city well of  $3.15 \times 10^4$  gpd/ft and 0.0022, respectively (for a temperature of 76°F).

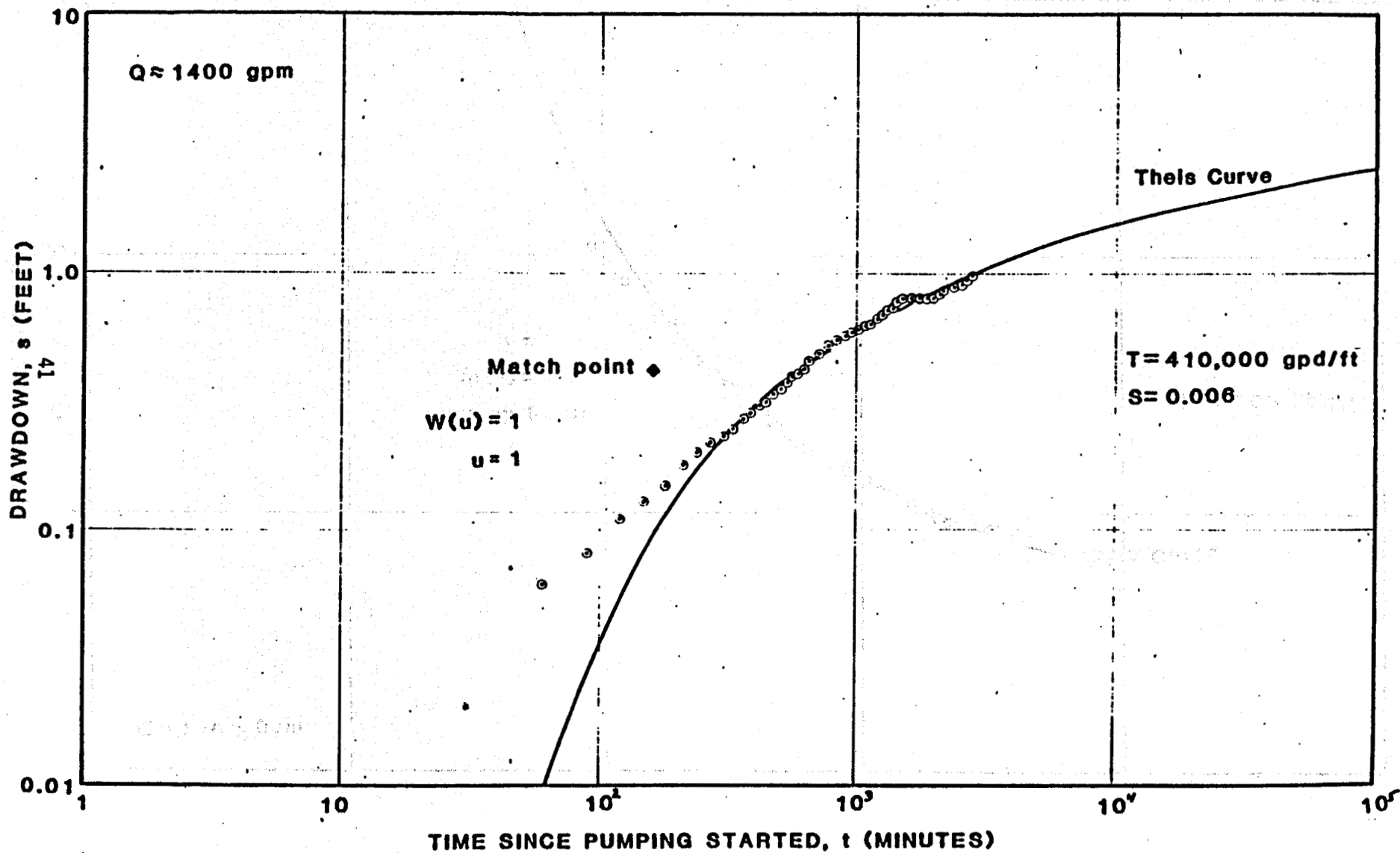


Figure 19. JELD-WEN well pumping starting 8/30/86, data for North Entrance Motel observation well.

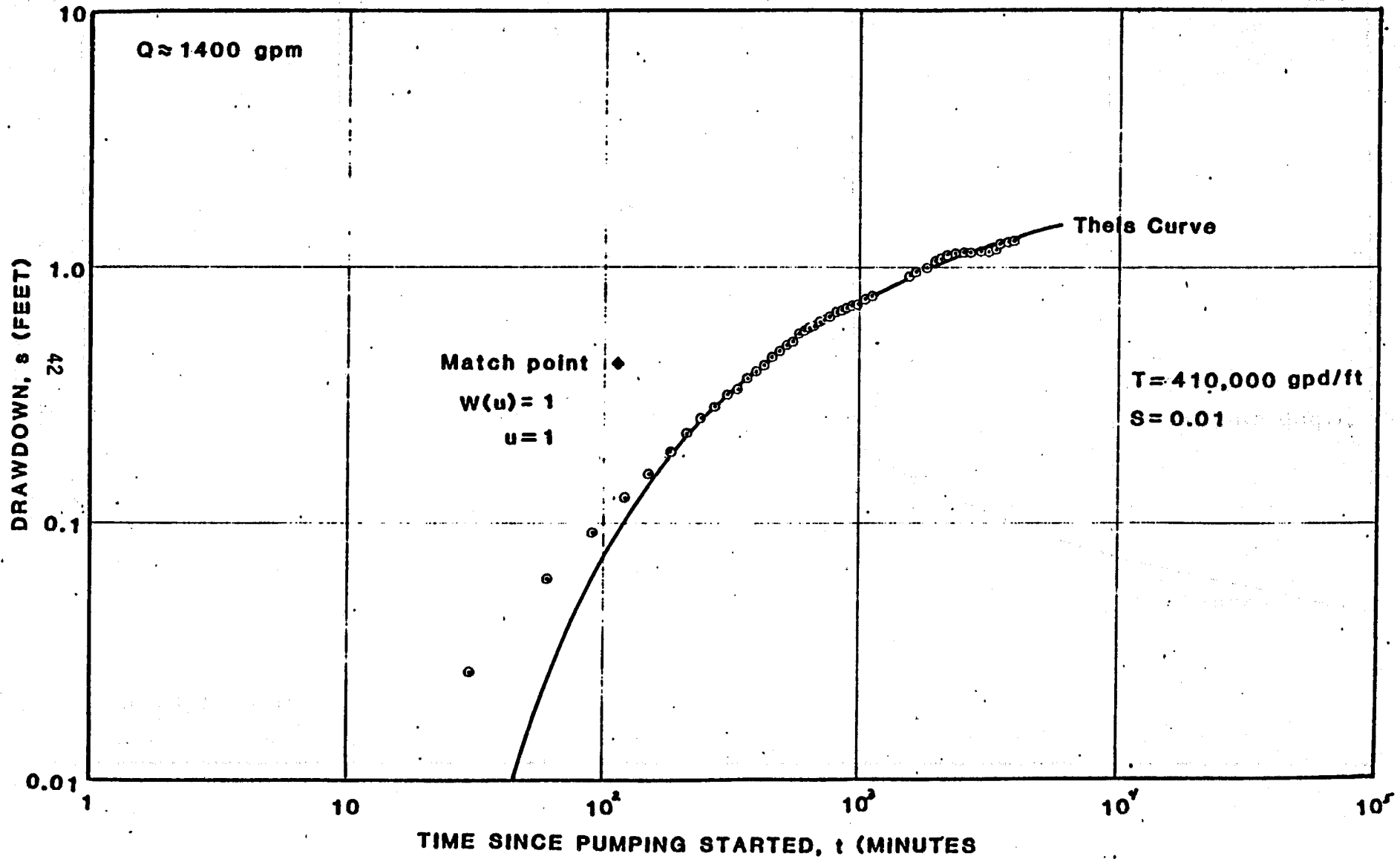


Figure 20. JELD-WEN well pumping starting 8/8/86, data for JED-WEN observation well.

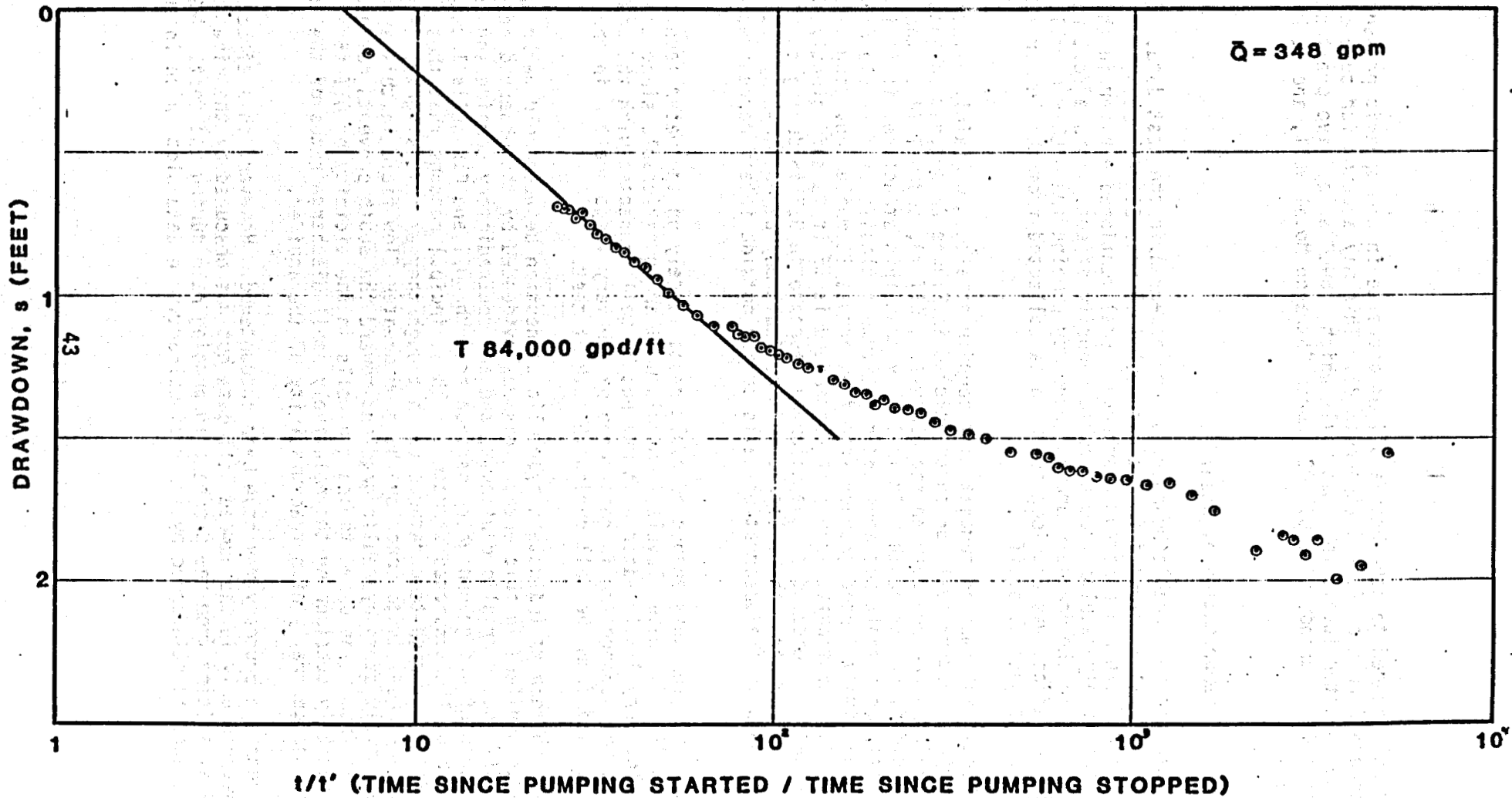


Figure 21. CIP-1 constant-discharge test, 8/22-26/86, residual-drawdown for the pumped well, CIP-1.

There appeared to have been a slight decrease in atmospheric pressure between 8/20 and 8/29/86 (Appendix B). This may account for some of the rise in water levels in those wells which showed a positive pre-testing water level trend, viz. OIT #3, North Entrance Motel well, and JELD-WEN well.

### 9.3 Interpretation of Results.

No classic impermeable boundaries were observed in the test data. The break in slope observed in the data obtained from CIP #1 (both step-drawdown and constant-discharge tests) mentioned earlier in this report likely results from a double porosity effect. This effect was also observed in the drawdown brought about by pumping the JELD-WEN well. A similar effect was ascribed to the data collected from the Summer 1984 test of the geothermal aquifer.

Geologic information suggest that boundaries almost certainly exist in the aquifer. Water-level contours depicted in Figure 22 suggest an east-west trending structure between the CIP wells, OIT #3, and OIT #4 on the north and OIT #5 on the south. It is likely that this structure, if it indeed exists, is a fault but does not constitute a classic impermeable boundary. It may simply impede ground-water flow across it while providing limited hydraulic continuity across it (or around it if it is sufficiently short length) as indicated by the observed response in OIT #5 during the CIP #1 test. Elsewhere, the existence of the fault may be obscured by the small drawdowns in the observation wells or the outside interferences recorded in the data.

The basis for this suspected fault rests largely on the water level data from CIP #3. Water levels from this well are questionable since the well yields very little water. It may not be well connected with the aquifer. This poor connection is reinforced by the very large scatter in the data collected from the well during the CIP #1 test.

Testing results indicate a hydraulic connection between cold water and hot water aquifers in this area. This was evidenced by the observed response in OIT #5. Hydraulic continuity appears contradictory considering the absence of hot ground water derived from the OIT and CIP wells located north of the suspected structure. The principal reasons that these wells do not show high temperatures (even though they exhibit slightly elevated temperatures) is governed by the ground-water flow field in this area. The relatively steep hydraulic gradient toward the south prevents hot ground water from being drawn toward the wells in the vicinity of OIT #5 especially considering the relatively small discharge rates of the wells and the generally high transmissivity of the aquifer.

A recharge boundary may have been encountered. However, this effect was observed only in the pumped well data but was obscured by the rise in water level arising from the decrease in pumping rate during the test. Its presence is reinforced by the residual-drawdown data plot which intercepts

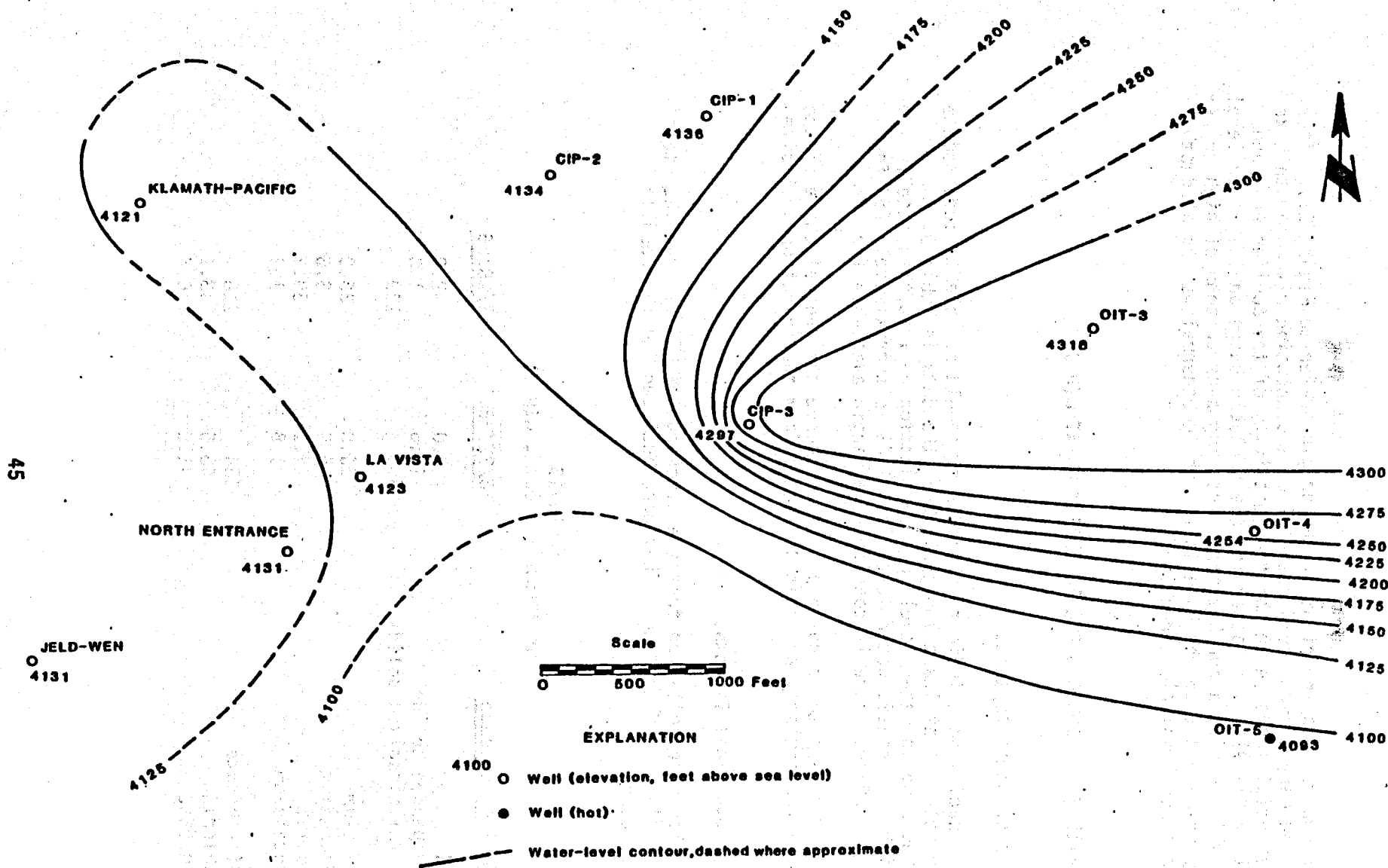


Figure 22. Water-level contour map, College Industrial Park and vicinity.

zero drawdown well to the right of the origin (refer to Figure 21). This recharge boundary effect could result from a connection with Klamath Lake or a substantial increase in the transmissivity of the aquifer west and southwest of CIP #1. The latter is more probable based on the water level contours which depict a northwesterly trending sink or zone of high transmissivity which might be the conduit for the geothermal waters at depth moving in a southeasterly direction.

## 10. ANTICIPATED IMPACTS DUE TO INJECTION AT CIP #1

### 10.1 Hydrodynamic Impacts.

Specific capacity data indicate that CIP #1 should be able to accommodate injection rates of at least 700 gpm. From the data in Table 9, the anticipated rise in water level in the injection well at this rate is 140 feet. Since the static water level in CIP #1 is approximately 250 feet below land surface, there is considerable margin available for error.

Injection at this site is not expected to change the general shape of the flow field, particularly at great distances from the injection well site.

### 10.2 Hydrochemical Impacts.

Water samples were collected from the discharge of CIP #1 near the start and the conclusion of testing. They were analyzed and the results summarized below.

Table 13  
Water Chemistry Data, CIP #1

<u>Collection date</u>	<u>8/22/86</u>	<u>8/28/86</u>
pH	7.0	8.0
Suspended solids	24.6	1.3
Total dissolved solids	383	189
Calcium	25.8	35.9
Magnesium	5.9	3.3
Sodium	44.1	38.3
Potassium	1.8	1.2
Bicarbonate	184	218
Chloride	48.5	13.1
Sulfate	<1.0	10.4

Table 13, Water Chemistry cont'd.

Arsenic	0.05	0.05
Boron	<0.10	<0.10
Iron	10.6	0.71
Manganese	0.27	0.16
Silica	32	37

(Source: Klamath Environmental Services)

(Note - all constituents reported in milligrams per liter with the exception of pH, which is reported in pH units)

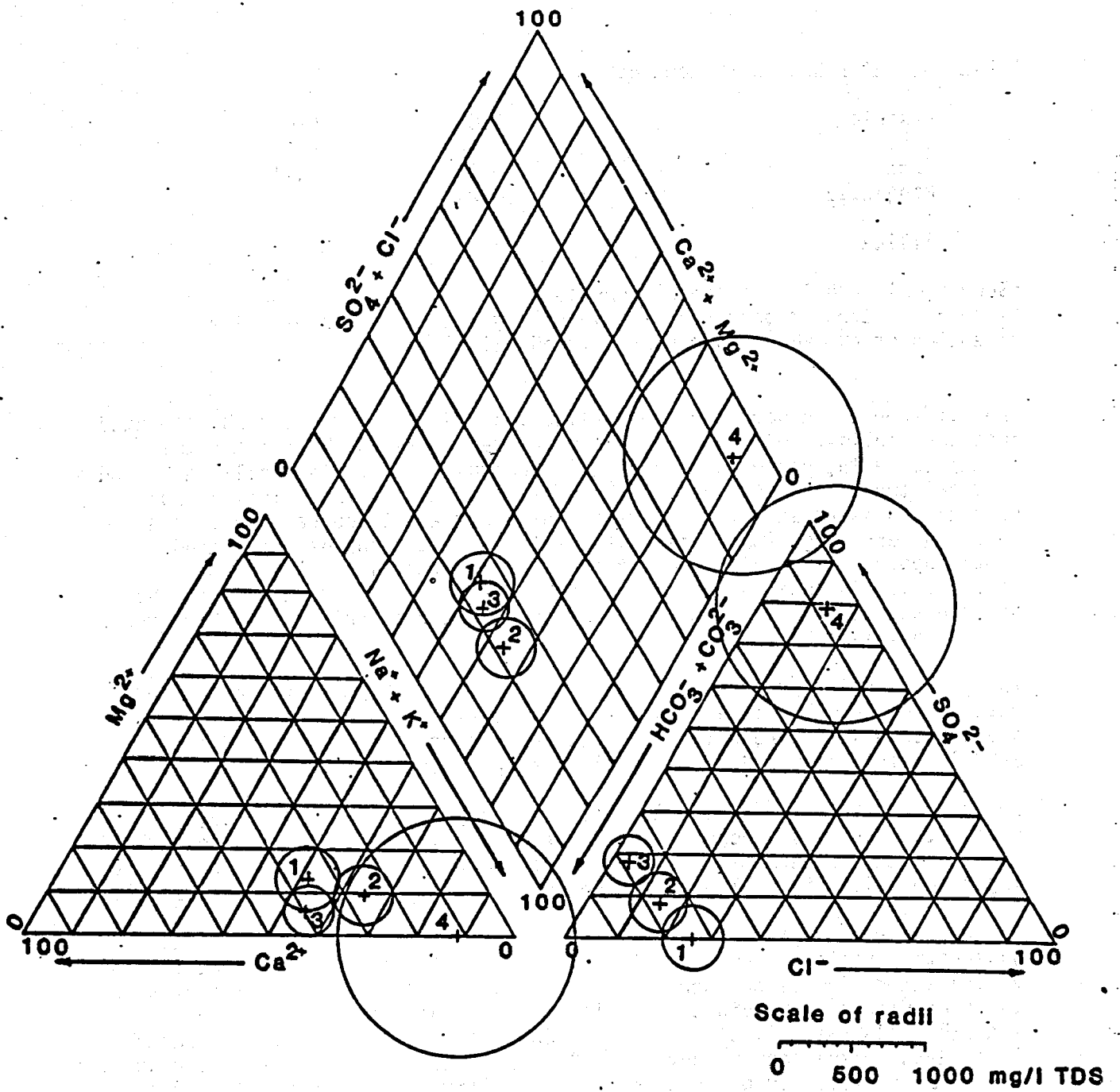
The differences between the two analyses, particularly reduced pH, elevated suspended solids, TDS, chloride, iron and manganese in the first sample appear to be the presence of residual chemicals and metals mobilized by the chemical treatment performed on the well prior to testing. They do not seem to represent an influx of waters of a different source late in the test. A very large imbalance in the anions and cations of the second sample appears to result from difficulties inherent in analyzing waters that are low in total dissolved solids. The error appears to lie in the bicarbonate determination which seems to be uncharacteristically high for waters in this area.

The gross chemistry of water from CIP #1, OIT #5 (thermal effluent), and water from the OIT heat exchangers (presumably from OIT cold water wells) are compared in Figure 23. For plotting purposes, the imbalance in the 8/28/86 sample from CIP #1 was artificially removed.

Because the thermal effluent is hotter and higher in TDS, chloride, sulfate, arsenic, and boron to name a few constituents, it is evident that an impact on the chemical quality of the ground water down-gradient of the injection well site will occur. The question is, is the impact acceptable?

The water level contour map drawn from water levels measured in the 11 observation wells (Figure 22) illustrates that the predominant direction of ground water flow is southerly, away from CIP #1. This is consistent with the conclusions of previous investigations in the main hot well area of Klamath Falls (Sammel, et.al., 1984; WEN, Inc., 1985). Additional water level data in the region south of CIP #3 and the La Vista Motel and east of the JELD-WEN well would serve to more definitively portray the flow field in this area. However, there is a moderately high degree of confidence in the contours drawn in the figure based on the available data. The west to northwesterly flow direction north of CIP #3 appears to be a local phenomenon superposed on a regional pattern.

Thermal effluent disposed of via CIP #1 will migrate down-gradient from the well in response to the established ground water flow field. The width of the thermal effluent plume, neglecting dispersion, can be calculated using Darcy's Law. A first approximation of the plume dimensions is depicted in



- 1 CIP-1, collected 8/22/86
- 2 CIP-1, collected 8/28/86
- 3 OIT cold water supply
- 4 OIT-5

FIGURE 23. Tri-linear diagram

Figure 24. Calculations in support of the plume dimensions are presented in Appendix C. From this analysis it appears that none of the wells immediately down-gradient from CIP #1 will be impacted. That includes the La Vista Motel well, which by virtue of its proximity to the flow path of the effluent, appears to have the greatest potential for impact. Likewise, by virtue of their locations, OIT wells 5 and 6 enjoy immunity from impacts of injection at CIP #1 in spite of the conclusion that the thermal and cold water aquifers appear to be coupled in this area.

Impacts farther down-gradient are more difficult to predict due to gaps in the data. However, in general, it is our understanding that the chemical quality of the ground water decreases between the College Industrial Park and the main hot well area in Klamath Falls. Few, if any, users derive culinary water supplies from the aquifer in this area. A positive impact due to an increase in temperature in some thermal wells that lie along the path of the plume may occur.

## 11. POLICIES GOVERNING GEOTHERMAL EFFLUENT DISPOSAL

### 11.1 State Policy.

The main goal of the State's policy is to prevent degradation of water quality in an aquifer to insure proper management of all ground water resources so maximum beneficial use of the resource will be most effectively attained. The State needs to be reasonably certain that there will be no substantial thermal, chemical or pressure interference with existing uses or potential uses of the receiving aquifer.

Oregon Administrative Rules, Chapter 690, Division 65 (Appendix D) of the Water Resources Department describes the standards and procedures for low temperature geothermal wells and effluent disposal systems. Low temperature geothermal effluent disposal systems are classified as either Standard or Nonstandard.

Basic information included in this report and required by the State Water Resources Department includes lithology and well construction information for CIP #1, aquifer stress test results, hydraulic communication with thermal and non-thermal wells, and water chemistry from both the producing and receiving aquifers. Appendix E contains information on CIP #1 and water chemistry for the OIT geothermal wells.

Since the proposed injection plan includes disposal to a different aquifer as compared to the producing aquifer, DEQ will probably require a Water Pollution Control Facility (WPCF) permit.

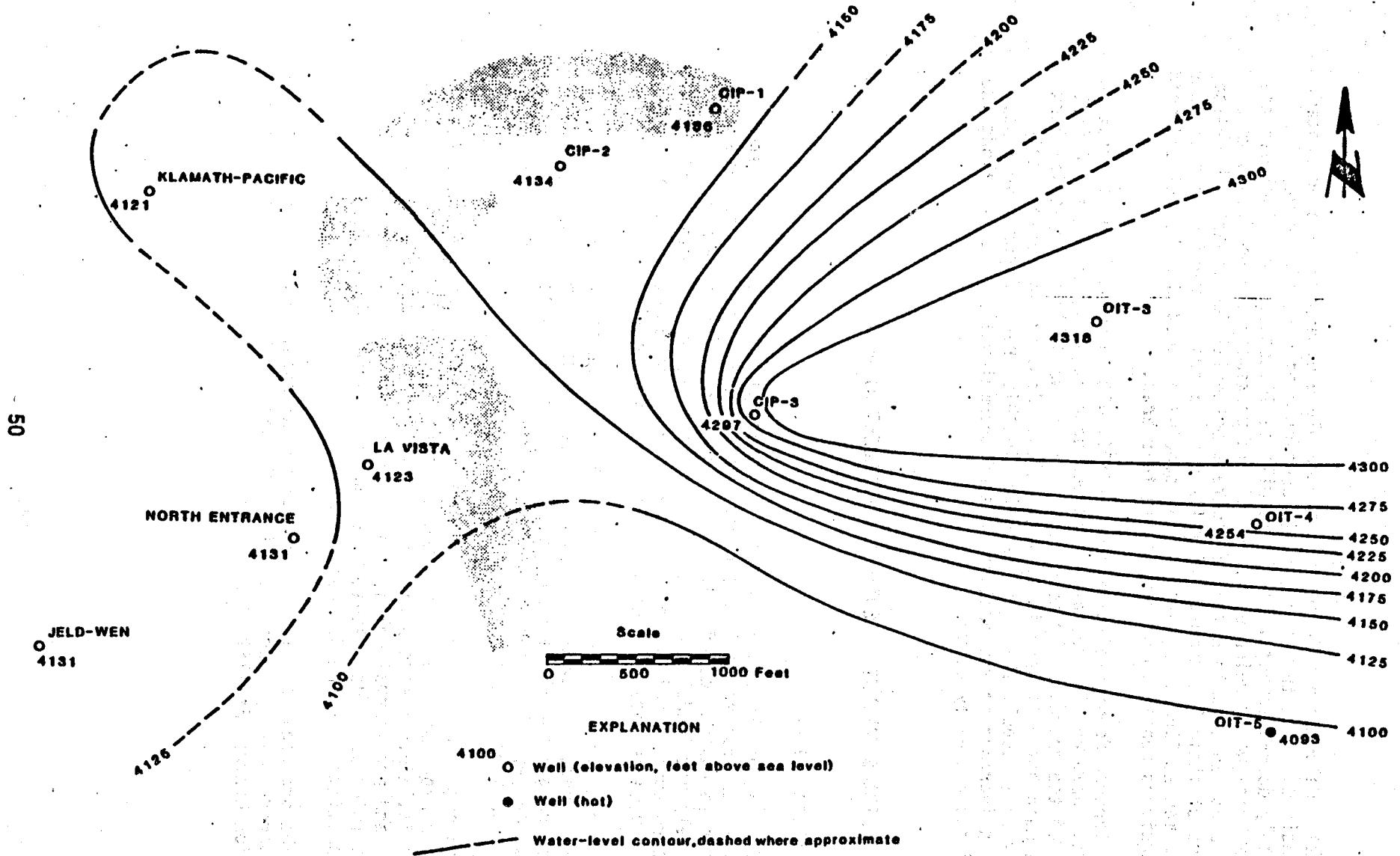


Figure 24. Thermal effluent plume in the vicinity of the College Industrial Park (neglecting dispersion).

## **11.2 City Geothermal Resource Management Act.**

The purpose of the City Geothermal Resources Management Act, Sections 8.250 to 8.298, is comprehensive management of the geothermal resource and thermal ground waters within and adjacent to the City of Klamath Falls, Oregon.

In furtherance of the purpose of this Ordinance, it shall be the policy of the City to conserve and protect the geothermal fluids and ground water within and adjacent to the City. In order to enhance reservoir productivity and benefit; prevent wasteful extraction and disposal of geothermal fluids and thermal ground water; prevent geothermal water temperature degradation; prevent thermal pollution of surface environs and water; and prevent harmful intermixing of geothermal fluids and other ground water.

In order to eliminate wasteful and harmful effects of thermal fluids or water surface discharges, any well or geothermal facility discharging geothermal fluids onto the ground or into any public ditch or drainage facility, shall cease such discharging within five (5) years of the Ordinance effective date, which was July 1, 1985.

The City Manager may, upon recommendation of the Geothermal Advisory Committee, grant exceptions to the five year requirement. The owner must meet certain conditions provided in 8.264 of the Act.

## **12. LEGAL AGREEMENT**

### **12.1 Factors.**

A legal agreement will be necessary for the coordination of geothermal discharge water usage by the City of Klamath Falls and Oregon Institute of Technology.

Based on OIT's present and projected use of discharged geothermal fluid, the City Attorney has identified the following factors that will need to be addresses.

- a. The legal authority of both public bodies to enter into the agreement.
- b. Identification of the point at which the city would "capture" the discharge, assuming such point will be located on the OIT campus, an easement from OIT to the City for purposes of installation, maintenance and repair of the "capture" facilities.
- c. Resolution of what, if any, level of quantity and temperature of geothermal discharge fluid will be available from OIT. OIT plans

to cascade discharged from existing buildings to provide heat to new buildings; therefore, they may be reluctant to guarantee any minimum levels in the discharge.

- d. The responsibility for compliance with the City's Geothermal Resources Management Act and with requirements contained with the Oregon Administrative Rules as both apply to the geothermal injection requirements. It is expected that OIT would request that the City assume this compliance responsibility.
- e. An allocation of the responsibility to address any legal rights of "downstream" users of the current discharge. There are parties currently utilizing the OIT discharge below the College Industrial Park, and though any legal claim to establish water rights would appear questionable, it is expected that OIT would require that the City take the responsibility for addressing any such claims.
- f. The term of the agreement.

## 12.2 Water Rights.

Water rights have been checked in the four section area around the proposed injection well location, namely Sections 17, 18, 19 and 10, Township 38 South, Range 9 East. In addition to OIT, there appears to be three water rights on file. Bruce Froemke has a right for supplemental irrigation of eight acres, JELD-WEN has a water right for flushing the channels of the Harbor View subdivision, and Mountain View Professional Building Association has water rights for water and space heating a building on Mountain View Blvd. In addition to these, there are a number (48) of wells in the area for exempted uses for which not water rights are needed, yet which must be considered.

Asphalt Construction Company has a surface water right on record, No. 53353, of 1.5 cfs from a pond fed by the OIT/MWMC discharge to be used for dust control. In 1985, Klamath Pacific Corporation, current owners of the plant, drilled a new well which is now used for dust control.

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**APPENDICES**

**A P P E N D I X   A**

**Supplemental Calculations for:**  
**OIT effluent flow rate**  
**OIT collection system**  
**MWMC collection system**  
**Pump station**  
**Transmission and distribution piping**  
**CIP energy analysis of buildings**

① AT EFFLUENT FLOW RATE:  
 VARIABLE SPEED DRIVES ON PRODUCTION WELLS  
 ARE USED PRIMARILY FOR "SOFT START" OF  
 PUMPS. PUMPS OPERATE AT FULL SPEED AFTER  
 START UP. FLOW IS REGULATED BY CYCLING  
 WELLS ACCORDING TO PHYSICAL PLANT RECORDS.  
 PROD WELLS HAVE THE FOLLOWING FLOWS:

- #2 - 35 STAGE 7 RM (AUKOLA) 5.000 2  
 120 gpm @ 637'
- #5 - 12 STAGE 10 RL (AUKOLA)  
 500 gpm @ 440'
- #6 - 16 STAGE 8 RH (AUKOLA) 5.000 1  
 250 gpm @ 650'

TOTAL PEAK FLOW RATE  
 920 GPM → EAY 1000 gpm

INDIVIDUAL BUILDING FLOWS  
 FROM CARROLL PEKOVY GULICK + ASSOC REPORT  
 4-25-86 - ACTUAL CALLS BY ROGER WILKOUT.

BUILDING	SPACE HEAT	DHW	TOTAL
RESIDENCE	160	82	242
LEGE UNION	212	21	233
SHEL	24	-	24
HYE. ED.	117	51	168
LIBRARY	76	-	76
LORNETT	139	9	148
EMORI	74	-	74
SOUTH	21	5	26
PHYS. PLNT	31	-	31
OWENS	<u>74</u>	<u>-</u>	<u>74</u>
	1043	168	1216

GPM

RESIDENCE HALL	.....	187	140
STUDENT UNION	KEVIN:	66	= 98 ? EITHER, OR ?
SNELL HALL	THESE QUANTITIES	24	ONLY.
MECH BLDG	ARE FROM A PREVIOUS		
SOUTH HALL	STUDY NOT		
SEMON HALL	DONE BY US, SO	45	.49
OWEN HALL	I REALLY <del>I</del> DON'T	86	74 <sub>gm</sub>
CORNETT HALL	KNOW THE BASIS	86	74
LIBRARY	OF THE QUANTITIES		
PHYSICAL EDUCATION BLDG	D BOMAR	274	198
		60	76
		<u>73</u>	117

835 + 66 = 901

OR

835 + 98 = 933

2

# C.I.H. SYSTEM

(2)

CURRENT PRODUCTION WELLS CAPABLE OF  
 $(980/1216) \cdot 100 = 80.6\%$  OF PEAK

DISCHARGE TEMPERATURE  
 CALCULATED BASED ON CONNECTED BUILDING  
 LOADS.

BUILDING	TOTAL LOAD	FLOW	DECH TEMP
RESIDENCE	6650 MBH	242 gpm	135 °F
COLLEGE UNION	5075	233	146
SHELL	660	24	135
PHYS ED	7010	168	107
LIBRARY	1243	76	139
CORNETT	7161	198	118
SEMON	1675	74	145
SOUTH	2599	96	136
PHYS PLNT	1247	31	103
OWENS	1675	74	145
	<u>35,303</u>	<u>1216</u>	<u>131</u>

DISCHARGE TEMPERATURE INDEXED TO 100 °F

IMMEDIATE COLLECTION SYSTEM FANS:

RESIDENCE, PHYS ED, LIBRARY, AND POSSIBLY SHELL

RESIDENCE	242	135
PHYS ED	168	107
LIBRARY	76	139
SHELL	24	135
COLLEGE UNION	<u>233</u>	<u>146</u>
	743	133

NEW CAMPUS BUILDING REQUIREMENTS:

200 gm 130-113 = 1.2 x 10^6 BTU/H

IMPACT ON COLLEGIUM SYSTEM EFFLUENT TEMP

1.2 x 10^6 / (500 \* 743) = 3.2 deg C

743 gm C (133 - 3.2) = 129.8

IMPACT ON TOTAL CAMPUS EFFLUENT TEMP

1.2 x 10^6 / (500 - 1000) = 2.4 deg F delta

1000 gm C (131 - 2.4) = 128.6 deg F

EFFECT OF PHASE I COMPLETION ON CAMPUS EFFLUENT TEMPERATURES: - ASSUME ADDITIONS WILL REQUIRE ONLY 80% OF THE CAPACITY OF THE EXISTING FLOORS:

GUILD HALL ADDN: EXIST. 15370 GSF ADDN. 12000 GSF

(12/15.4) \* .80 \* 60000 BTU/HK = 412000 BTU/HK

COLLEGE UNION ADDITION:

EXIST: 70193 GSF ADDN: 22000 GSF

(22/70.2) \* .80 \* 475700 BTU/HK = 1,193,000 BTU/HK

PHYS. ED ADDN:

EXIST: 60000 GSF ADDN: 44000 GSF

(44/60) \* .80 \* 700000 = 4,113,000 BTU/HK

PHYS PLANT ADDN:

EXIST 19301 GSF ADDN: 8400 GSF

(8.4/19.3) \* .80 \* 347000 = 469000 BTU/HK

TOTAL ADDED PHASE I LOAD = 6,187,000 BTU/HK

REDUCTION IN TOTAL EFFLUENT TEMPERATURE FROM ABOVE LOAD

6,187,000 / (500 \* 980) = 12.6 deg F

EFFLUENT TEMPERATURE:

128.6 - 12.4 = 116.2 deg F

ENERGY USE ESTIMATE FOR REMINDER OF INDUSTRIAL PARK

PREDICTING HEATING ENERGY USE FOR FUTURE CONSTRUCTION IN THE COLLEGE INDUSTRIAL PARK IS DIFFICULT. DUE TO VARIATION IN CONSTRUCTION TECHNIQUES, AND THE DESIRE OF VENTILATION EMPLOYED, ENERGY USE PER FT<sup>2</sup> CAN VARY SUBSTANTIALLY. AS A RESULT, FOR THE PURPOSE OF THIS STUDY, AN AVERAGE OF THE EXISTING BUILDING'S ENERGY USE WILL BE EMPLOYED.

EXISTING BUILDINGS ENERGY USE:

	BLDG. AREA	HTG. ENERGY	BTU/FT <sup>2</sup> ·YR
PRECISION DATA WORKSHOP	12400	.760 x 10 <sup>9</sup>	61290
QUALITY COMPONENTS	14160	.185 x 10 <sup>9</sup>	13277
INDUSTRIAL COMMUNICATIONS	19240	.585 x 10 <sup>9</sup>	29617
	<u>46400</u>	<u>1.530 x 10<sup>9</sup></u>	

WEIGHTED AVG CONSUMPTION:

33103 BTU/FT<sup>2</sup>·YR.

AVG. BUILDING AREA TO LOT SIZE RATIO

PDC	12400	1.34
QC	14160	1.02
EC	<u>19240</u>	<u>1.75</u>
	46400	4.11 → 11289 FT <sup>2</sup> /ACRE

DEVELOPABLE AREA ON REMAINING LOTS.

1.11 + 1.19 + 1.33 + 1.25 + 2.22 + 1.07 + 1.01 + 1.40 + 1.15 + 1.55 + 1.36 + 1.91 + 2.90 + 1.80 + 1.42 + 4.45 + 1.71 + 0.58 + 0.60 = 30.21 ACRES

@ 11300 FT<sup>2</sup>/ACRE + 33100 BTU/FT<sup>2</sup>·YR

30.21 · 11,300 · 33100 = 11.30 x 10<sup>9</sup> BTU/YR

@ 70% AFUE ENERGY COSTS WOULD AMOUNT TO:

11.3 x 10<sup>9</sup> · .70 = 7.91 x 10<sup>9</sup> BTU/YR

PEAK HEATING REQUIREMENTS FOR REMAINING TRACTS.

FOR THE SAME REASONS MENTIONED UNDER ANNUAL ENERGY USE PREDICTIONS OF PEAK REQUIREMENTS FOR THESE BUILDINGS IS ALSO SUBJECT TO WIDE VARIATION. AGAIN EXISTING BUILDING DATA WILL BE EMPLOYED

EXISTING BUILDING PEAK LOADS:

POG	12400 FT <sup>2</sup>	260000 BTU/HK
QC	14160	350000
EL	19840	716000
	<hr/>	<hr/>
	46400	1326000 (BASED ON INSTALLED CAPACITY - LOADS ARE LESS)

AVG PEAK LOAD PER FT<sup>2</sup>:

$$1326000 \div 46400 = 28.6 \text{ BTU/HK FT}^2$$

USING PROBABLE BUILDING AREAS FROM ABOVE:

$$30.21 \text{ ACRES} \cdot 11300 \text{ FT}^2/\text{ACRE} \cdot 28.6 \text{ BTU/HK FT}^2 \\ = \underline{\underline{9.76 \times 10^6 \text{ BTU/HK}}}$$

@ 1000 gpm (COMBUST EFFLUENT ALT)

$$9.76 \times 10^6 \div (1000 \cdot 500) = \underline{\underline{19.5^\circ \Delta T}}$$

HOSPITAL LINE TO DRAIN TANK:

3 OPTIONS:

1. WEST ALONG CAMPUS DR, UNDER ROAD AT SOUTH HALL WEST ALONG N SIDE CAMPUS DRIVE, NORTH THROUGH WEST SIDE OF LOWER BUS PAD TO TANK
2. SIMILAR TO OPTION #1 ONLY CONTINUE ALONG ROAD TURN NORTH CONTINUE TO EXIST DITCH AND THEN EAST TO TANK.
3. WEST ALONG CAMPUS DR, UNDER ROAD AT SOUTH HALL NORTH UNDER PARKING LOT TO TUNNEL.

CONSIDERATIONS

OPTION 1 HAS THE LEAST IMPACT UPON EXISTING & FUTURE CAMPUS CONSTRUCTION. OPTION 2 COULD POSSIBLY INTERFERE W/ FUTURE DEVELOPMENT ON THE LOWER PAD. OPTION 3 WOULD REQUIRE RESIZING OF COLLECTION SYSTEM NOW BEING INSTALLED. ALSO EXTENSIVE PAVEMENT R.M.K.

CALCULATE COSTS FOR OPTIONS #1 & 2

MATERIALS PB, FFP 2" UNINSULATED, DRY BURIAL

	POLYBUTYLENE	\$/LF	FIREGLASS
PIPE	8.77 (50x13.5)		12.00
WRAPPING	-		2.75
BRICK LINING	2.96		-
TRENCH	4.93		4.93
PIPE	-		0.41
FEED	0.93		0.93
SUBTOTAL	19.50		21.00
15% O&P	2.79		3.27
TOTAL	21.30 \$/LF		25.07 \$/LF

ROAD POLE	-	\$/LF
PAVEMENT REMOVAL	- 2.10	↓
REPAVE (3' WIDE 3" THK)	- 3.33	↓

OPTION #1

2.2400 LF		
2400 LF	PB 51312	FG 60168
ROAD EXE		
PAVEMENT 4x2 180'	1032	1032
	52344	61200

HOSPITAL LINE TO DRAIN TANK:

OPTION # 2. 2100 LF.

2100 LF PS 44998

Eq. 52647

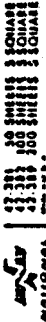
PAVEMENT 24K  $\frac{1032}{45930}$

$\frac{1032}{53679}$

ADD TO EACH:

2-6" TEE 741  
2 6" GATE 1040  
5 2L 1115  
2896

440  
1040  
775  
2255



### OIT COLLECTION SYSTEM

SEMAN/SOUTH TE IN

MASTER PLAN CALLS FOR A TUNNEL EXTENSION TO JOIN THESE TWO BUILDINGS TO THE EXISTING TUNNEL AT THE SW. CORNER OF OWENS.

BASED ON RECENT BID FOR TUNNEL EXTENSION TO RESIDENCE HALL COSTS FOR TUNNEL CONSTRUCTION EXCLUSIVE OF PIPING AMOUNT TO APPROX. 500 \$/LF

APPROX LENGTH OF TUNNEL REQUIRED FOR SEMAN/SOUTH - 100 LF.

COST 100 · 500 = \$50,000

INSTALL SUPPLY & RETURN LINES IN TUNNEL

FLOW RATE (PEAK) SEMAN - 74  
SOUTH - 91

165 gpm

ASSUME 4" LINES INCL FQ. EQ. WAY.  
4" CU HANGERS IN TUNNEL - 27.05

100' (=) (27.05) = 10026

ADD 15% FOR FITS = 1504

11530 EST \$12,000

BID FOR HEAT EXCHANGER INSTALLATION IN SEMAN: (PER ROOM WITHOUT)

\$15770

EST \$16000

RE ROUTE EXISTING DISPOSAL LINE TO S.E. CORNER OF BUILDING:

60' INCL 3" FQ. CU HANGERS.

60 · 23.88 = 1433

EST \$1500

RE ROUTE EXISTING DISPOSAL LINE FROM SW. CORNER OF SOUTH HALL TO NE. CORNER:

210' INCL 3" FQ. CU HANGERS

210 · 23.88 = 5015

EST \$5000

SEMAN/SOUTH SUPPLY

\$124500

# CORJET HALL COLLECTION SYSTEM :

BASIS: TIE-IN TO EXISTING DISPOSAL SYSTEM AT  
 — STORM SEWER CONNECTIONS. INSTALL  
 NEW INSULATED BLACK IRON COLLECTION SYSTEM  
 TO MECHANICAL ROOM.

## NEW RETURN PIPING

11.50 x 6.6 = 5.27 x 3.05	3" BURIED	60' @	24.21	= \$	1453	
	3" OVERHEAD	290' @	21.90	=	5475	(SEE DRAWINGS)
	2" "	200' @	14.31	=	2862	
	1 1/2" "	75' @	11.82	=	887	
			ADD 10% FITTINGS	=	1067	
					<u>11744</u>	

## MECHANICAL ROOM

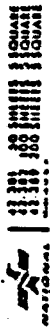
HEAT EXCHANGER	3.2 x 10'	-	6000	
4-4" GATES		-	1280	
2 THERM		-	136	
4 PRESS		-	68	
RE-INSTALL CIRC PUMP		-	320	
CONTROLS		-	2000	
STRAINER + AIR CTRL		-	665	
EXPANSION TANK		-	330	
PIPING 4" INCL		-	800	
FLOW METER		-	130	11725
MAKE-UP LINE		-	75	
			<u>11804</u>	

LINE FROM MECHANICAL RM TO WEST SIDE			
4" BURIED	30' @	25.30	- 760
4" OVERHEAD	150' @	26.70	- 4005
		10% FITTINGS	- 477
			<u>5242</u>

SYSTEM SUBTOTAL	28790
15% CONT	<u>4319</u>

TOTAL \$ 33109

SAY \$ 33000



PHYSICAL PLANT:

HEAT EXCHANGER INST. + DISPOSAL LINE.

EDGIS: INSTALL HEAT EXCHANGER IN PUMP ROOM AT SOUTHEAST CORNER OF BUILDING. RE-ROUTE HWW LINE FROM STORM DRAIN TO PUMP ROOM. INSTALL NEW DRAIN LINE OUT OF BUILDING AND ACROSS ROAD TO THE SOUTH.

NEW 2" RETURN LINE TO PUMP ROOM  
75' INCL STL (10.35 + 5.96) = 1.10 FTL - \$ 1180

EXPANSION TANK	--	240
HT -x 7000 BTU/HK INSTALLED	--	2300
RE INSTALL PUMP	-	150
4 - 2" GATES	-	260
2" STRAINER	-	64
4 TEMP	-	136
4 PRESS	-	60
CONTROLS	-	1500
MAKE UP LINE TO LOOP	-	75
20' INCL 2" STL 0.15 FTL FRIGS	-	470

5231  
6520

DISPOSAL LINE TO EDGE OF LOT

3" FQ UNINS  
CRANE WORK  
0.50'

5' FQ UNINS  
MOTL - 7.58  
INST - 2.61  
TAB - 3.28  
PAVERIA - 4.62  
BED - 0.93  
18.02 @ 60'

1081

LINE TO CONNECTION W/ COLNETT  
3" FQ UNINS @ 14.4 SLF.  
400 LF

5760

LINE FROM COLNETT TO CONNECT. POINT  
4" FQ UNINSULATED 150'

MOTL	-	10.11
INST	-	5.23
TAB	-	3.28
PAVERIA	-	3.62
BED	-	0.93
		<u>21.17</u>

= 3175

CONCRETE PAV ALLOW 500 2675

LINE FROM CORNET / PHYS. PLANT CONNECTION TO TANK  
300 LF 6' FG WINKER.

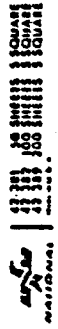
MAT'L - 16.40  
INST - 4.51  
TRB - 3.28  
RED - 93  
25.12 • 300 LF

7536

BIDS FOR HT-x INSTALLATION IN:

OWENS - \$16062  
SEMAN - \$15770

EXTENSION OF TUNNEL TO SEMAN / SOUTH  
BASIS: ASSUME EXTENSION SOUTH FROM EXISTING  
TUNNEL TO COLLOR BETWEEN SEMAN  
& SOUTH. NORTH SOUTH LEG TO CONNECT  
BUILDINGS. BASE COST ON RESIDENCE HALL  
EXTENSION.



# BT COLLECTION SYSTEM

OWENS HALL TIE IN.

MASTER PLAN CALLS FOR A TUNNEL EXTENSION TO COLLECT OWENS TO THE EXISTING SYSTEM. A BID HAS ALREADY BEEN RECEIVED FOR HEAT EXCHANGER RETROFIT FOR THIS BUILDING.

TUNNEL EXTENSION 25'  
 $25 \cdot 500 \text{ \$/LF} =$  \$12500

BID FOR HT-X INSTAL.  
= 16062

EXTENSION OF EXIST. DISPOSAL LINE TO  
TUNNEL EXTENSION  
120' 3" INSUL FRP

$120 \cdot 23.88 =$  \$2866

OWENS SUBTOTAL \$31427 \$49000

LINE FROM NEW BUILDING TO PUMP STATION:  
CONNECT TO "GWW" 12" LINE AT MAN HOLE  
(WEST OF BUILDING). DISTANCE TO PUMP  
STATION - ASSUMED TO BE 150'

150' UNINSULATED FRP 66 \\$/LF \$9900  
12" TEE - 670  
\$10570

## PUMP STATION:

USE 2 BOOSTER PUMPS IN PARALLEL. ONE VARIABLE SPEED (STAGE 1) & CONSTANT SPEED (STAGE 2).

PUMP SIZING: STAGE 1 VS PUMP SHOULD BE SIZED FOR CHILLER FLOW (SUMMER) TO AVOID UNNECESSARY CYCLING OF STAGE 2 PUMP DURING THE COOLING SEASON. IN ADDITION, THIS SIZE WOULD MEET ~ % OF THE PEAK HEATING DEMAND RESULTING IN ~ % OF THE ANNUAL HEATING OPERATING HOURS. STAGE 2 PUMP WOULD ONLY OPERATE DURING PEAK PERIODS.



# AT COLLECTION SYSTEM

## PUMP STATION:

USE 3 PUMPS 300 gpm EA.

OVERSIZE 1<sup>ST</sup> STAGE MOTOR TO ACCOMMODATE RIGHT SHIF. LEVEL CONTROL OF STAGING & THROTTLE VLV.

1<sup>ST</sup> STAGE 300 gpm @ 140' - 20 HP (AC FULL SPEED TO 110' MIN)  
 2<sup>ND</sup> STAGE 300 gpm @ 140' - 20 HP  
 3<sup>RD</sup> STAGE 300 gpm @ 140' - 20 HP  
 5" CB 1510  
 ~66%

## TANK SIZE:

BASE ON MOTOR RECYCLE TIME.

FOR 20 HP MOTOR ~ 15 STARTS HR  
 OR 4 MIN RECYCLE TIME.

ASSUME SQUARE TANK OF 4000 GAL:

VOLUME (WITH HEIGHT = DIA) =  $2\pi r^2 h$

$$4000 \text{ GAL} \div 7.48 = 534 \text{ FT}^3$$

$$534 = 2\pi r^2 h$$

$$4.4 = r$$

$$\therefore h = 8.8'$$

ALLOWING 1' @ TOP FOR OVERFLOW  
 1.5' @ BOTTOM FOR EXHAUST

REMAINING 6.3'

$$\text{STG 1} = 3.6'$$

$$\text{STG 2} = 5.7'$$

$$\text{STG 3} = 7.8'$$

REQUIRED TANK INTERVAL FOR 4 MIN @ 300 GPM

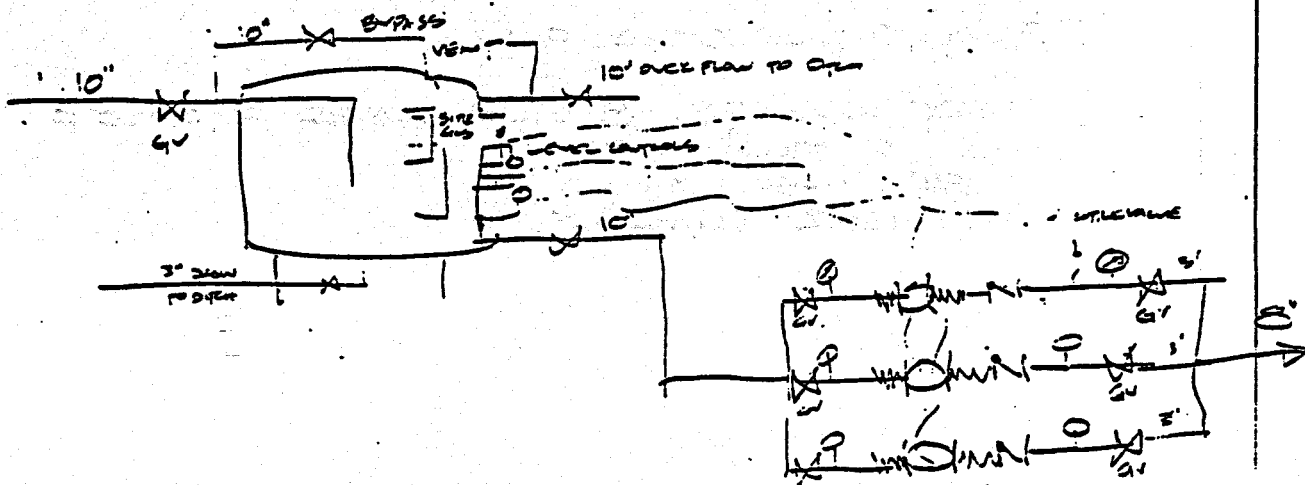
$$4 \cdot 300 = 1200 \text{ GAL} \quad (\text{ASSUMES NO MAKEUP TO PUMP})$$

$$6.3' \cdot \pi \cdot 7.48 = 455 \text{ GAL/FT}$$

$$(4.4)^2 \cdot \pi \cdot 7.48 = 455 \text{ GAL/FT}$$

$$1200 \div 455 = 2.63' \quad (\text{FOR 2<sup>NDRD</sup> STAGE) \quad \checkmark$$

GO WITH 4000 GAL "SQUARE" TANK



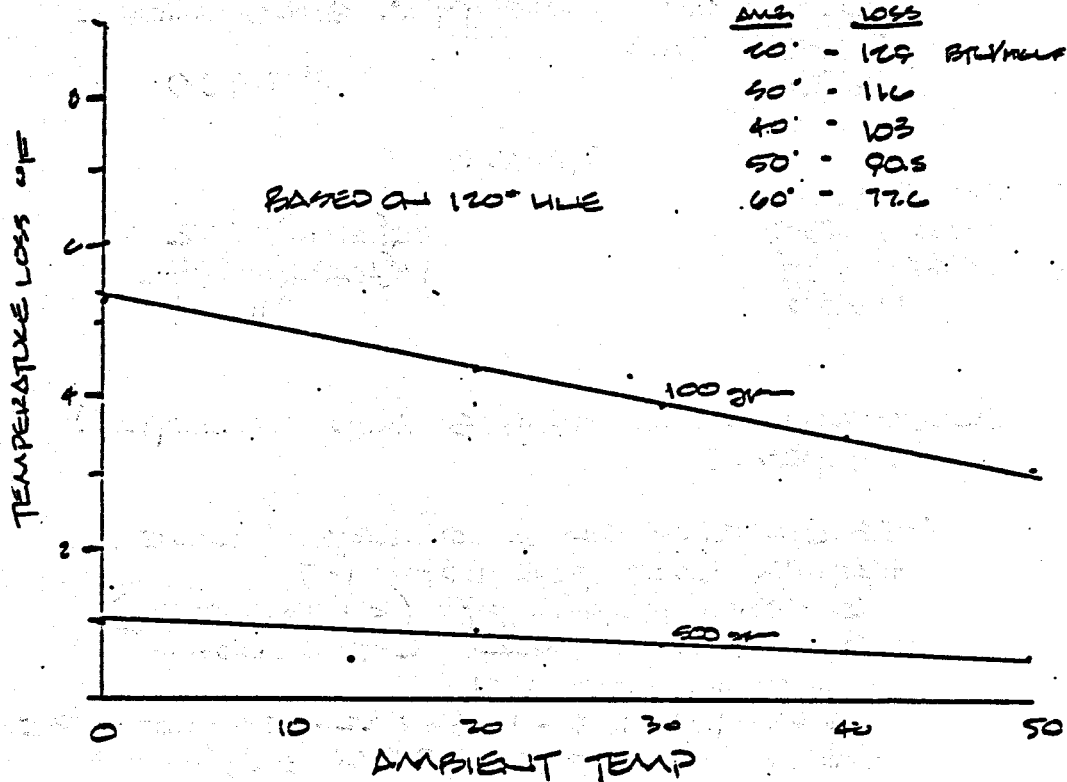


OIT COLLECTION SYSTEM:

DISTANCE FROM TANK TO INTERSECTION OF COLLEGE WAY + CENTURY BLVD - ~ 1700'

HEAT LOSS =  $1700 \cdot 15512 \text{ BTU/HR LF}$   
 = 263840 BTU/HR

TEMP LOSS @ 500 gpm  
 =  $263840 \div (500 \cdot 500)$   
 = 1.05°F



MAX LOSS @ LOW FLOW IS APPROXIMATELY 5.3°F  
 ∴ UNINSULATED MATERIAL SHOULD BE ACCEPTABLE.

TRANSMISSION LINE COSTS:

	FL (2000' LANE)	FB (2000' LANE) (FOR BASE 0")
MATERIAL	19.50	18.08
INSTALL	5.88	4.76
Joint Kit	0.91	4.93
Trench + BK FL	4.93	4.93
BEID	.93	.93
<b>SUBTOTAL</b>	<b>30.15</b>	<b>28.70</b>
15% O&P	4.52	4.31
<b>% TOTAL</b>	<b><u>34.67</u></b>	<b><u>33.01</u></b>

### OIT COLLECTION SYSTEM

8" LINE FROM PUMP STATION TO INJECTION WELL 3300 LF

FIBERGLASS  
= 3300 \* 34.67  
= \$114411

POLYETHYLENE  
= 3300 \* 33.01  
= \$108933

ADD 3 ROAD CROSSINGS

ALL ON CITY PROPERTY ∴ ROKE UNDER E 1000 \$/EA

\$7500

\$7500

### FITTINGS

45 40265 = 1060  
90 20265 = 530  
TEE 10365 = 305  
\$1955

20(20265) = 672  
1(10365) = 545  
\$1221

### INDUSTRIAL PARK TEINS AND COLLECTION SYSTEM:

SIZE CENTURY DRIVE COLLECTION LINE

HEATING LOAD FOR LOTS 1-7

C 20.6 BTU/HK FT<sup>2</sup> (OF BUILDING)

AND 11300 FT<sup>2</sup>/AC (OF BUILDING)

ACRES IN LOTS 1-7

$$1.25 + 1.11 + 1.13 + 1.33 + 2.22 + 1.34 + 1.07 = 9.51 \text{ ac}$$

$$9.51 \text{ ac} \cdot 11300 \text{ FT}^2/\text{ac} \cdot 20.6 \text{ BTU}/\text{HK} \cdot \text{FT}^2$$

$$= 50.7 \times 10^6 \text{ BTU}/\text{HK}$$

ASSUME 20° ΔT

$$Q = 5.07 \times 10^6 \div (500 \cdot 20)$$

$$= 207 \text{ gpm}$$

∴ USE 6" LINE

ASSUMING INSTALLATION IN SAME TRENCH AS SUPPLY LINE REDUCE T&B COSTS 50%

FROM SHEET —

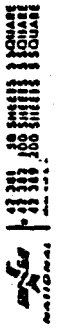
FG SUBTOTAL = 21.80 \$/LF

PS SUBTOTAL = 18.59 \$/LF

### CO-EQUAL COST

$$FG (21.80 - (4.93 \cdot 50)) \cdot 1.15 = 22.23 \text{ $/LF}$$

$$PS (18.59 - (4.93 \cdot 50)) \cdot 1.15 = 18.54 \text{ $/LF}$$



# OIL COLLECTION SYSTEM

## CENTURY DRIVE COLLECTION LINE COST:

$$900 \text{ LF} \cdot 22.23 \text{ \$/LF} = \$20007 \text{ (FG.)}$$

$$900 \text{ LF} \cdot 18.54 \text{ \$/LF} = \$16686 \text{ (PB.)}$$

8' LINE UP TO WELL (FROM CENTURY DRIVE)  
650 LF

$$650 \cdot 34.67 \text{ \$/LF} = \$22535 \text{ (FG.)}$$

$$650 \cdot 33.01 \text{ \$/LF} = \$21457 \text{ (PB.)}$$

## FITTINGS:

2 TEES (1 @ COLLESTE WAY + CENTURY RVD, 1 @  
CENTURY RVD + NORTHEAV HEIGHTS)

$$2 \text{ C 3US} = \$730 \text{ (FG.)}$$

$$2 \text{ C 549} = \$1099 \text{ (PB.)}$$

## INJECTION WELL HEAD:

6 ELBS, 2-6" GATE, 2-8" GATE, 2 PRESS MOUNT VALVE.

	FG.	PB.
6 ELBS	1590	2016
2-6" GATES	1020	1040
2-8" GATES	1520	1520
2- P.M.V. 6"	960	960
REWORK WELL HEAD	500	500
WELL HEAD ENCLOSURE	2500	2500
6 PRESS GATE	102	102
2 THERM	68	68
	<u>8250</u>	<u>8706</u>

## BUILDING SERVICE LINES:

ASSUME 1 1/2" FLUORID P/B LINES (UNINSULATED)

MST'L	- 2(1.94)	= 1.88
INSUL	- 1.5(1.62)	= 2.43
COND T/F	- .83(1.73)	= 1.43
SUBTOTAL		5.74
15% O/P		0.86
		<u>6.60</u>
		7.90 €/LF



BUILDING: Quality Components

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CLIENT CITY OF KLAMATH FALL PROJECT INDUSTRIAL PARK

\*\*\*\*\*

SIMPLIFIED ENERGY ANALYSIS (SEA) VERSION 3

PRODUCT OF FERREIRA & KALASINSKY ASSOCIATES, INC

\*\*\*\*\*

ZONE DESCRIPTION SUMMARY FOR ZONE 2

OFFICE

ZONE AREA	1920	SQ FT
VENT RATE	.067	CFM PER SQ FT
SUMMER OCCUP. TEMP	78	DEGREES FAHR
WINTER OCCUP. TEMP	70	DEGREES FAHR
UNOCCUPIED TEMP.	62	DEGREES FAHR
DESIGN R.H.	50	%

	<u>ROOF</u>	<u>WALL 1 NET</u>	<u>GLASS</u>	<u>WALL 2</u>
AREA	1920	636	324	256
U-FACT	.033	.091	.6	.65
MASS	30	30	N/A	50
COLOR	1	.65	N/A	.65
DIR.	HORIZONTAL	WEST	WEST	NORTH
SHADING	N/A	N/A	.9	N/A
INSIDE				
SHADING	N/A	N/A	1	N/A

SUSPENDED CEILING SPECIFIED

FLOOR DATA  
ON-GRADE PER 132 FEET LOSS 45 BTU/LF

INTERNAL LOADS

	OCCUPIED	UNOCCUPIED	
PEOPLE			
NUMBER	.0015	0	PER SQ FOOT
SENS HEAT	250	250	BTU/ PERSON
LATENT HEAT	245	245	BTU/ PERSON
AVE OCCUP	90	0	PERCENT
LIGHTING			
LOAD	3	.5	WATTS/ SQ FOOT
AVE USAGE	90	100	PERCENT
% TO PLENUM	0	0	PERCENT
EQUIPMENT			
LOAD	.5	0	WATTS/ SQ FOOT
AVE USAGE	90	0	PERCENT
INFILTRATION			
AIR CHANGES	N/A	.5	
ZONE HEIGHT	N/A	8	

PF TEST

INDUSTRIAL PARK

PEAK HEATING LOAD SUMMARY FOR ZONE 2

OFFICE

	SENSIBLE	LATENT	TOTAL
ROOF	-3864.96		-3864.96
WALL	-13680.84		-13680.84
GLASS	-11858.4		-11858.4
FLOOR	-5940		-5940
TOTAL BTU/HR	-35344.2	0	-35344.2
BTU/HR-SF	-18.40844	0	-18.40844

PRESS ANY KEY TO CONTINUE

TEMP	HEATING	HUMID
97	0	0
92	0	0
87	0	0
82	0	0
	0	0
77	0	0
72	0	0
67	0	0
62	0	0
57	0	0
52	0	0
47	0	0
42	0	0
37	0	0
32	0	0
27	3215.056	0
22	7441.762	0
17	11668.5	0
12	15895.23	0
7	20121.96	0
2	24348.69	0
-3	28575.41	0
-8	32802.14	0

ENERGY SUMMARY

	OCCUP	UNOCCUP	TOTAL
FANT MBTU	0		0
PANT MBTU	6923.205	9974.72	16897.93
G & EQ KWH	15474	6000.121	21474.13
UXIL KWH	708.9197	0	708.9197
IACEL (KWH)	0		0
OAL MBTU	62155.52	30453.13	92608.66
BTU / SQ FT	32.35582	15.85275	48.20857

22.23

U L USAGE

U L	CONSUMPTION	
ELECTRICITY	27134.09	KWH
AM	0	CU FT
IL	0	LBS
IAL	0	GALS
I SEL	0	LBS
		GALS

DO YOU WANT TO VIEW SEA GRAPHICS :?

\*\*\*\*\*  
 CLIENT CITY OF KLAMATH FALL PROJECT INDUSTRIAL PARK  
 \*\*\*\*\*  
 SIMPLIFIED ENERGY ANALYSIS (SEA) VERSION 3  
 PRODUCT OF FERREIRA & KALASINSKY ASSOCIATES, INC  
 \*\*\*\*\*  
 ZONE DESCRIPTION SUMMARY FOR ZONE 1 SHOP

ZONE AREA	12240	SQ FT
VENT RATE	.733	CFM PER SQ FT
SUMMER OCCUP. TEMP	78	DEGREES FAHR
WINTER OCCUP. TEMP	68	DEGREES FAHR
UNOCCUPIED TEMP.	50	DEGREES FAHR
DESIGN R.H.	50	%

	<u>ROOF</u>	<u>WALL 1 NET</u>	<u>GLASS</u>	<u>WALL 2</u>
AREA	12240	2048	0	2048
U-FACT	.025	.586	.1	.091
MASS	10	50	N/A	30
COLOR	1	.65	N/A	.65
DIR	HORIZONTAL	EAST	NORTH	EAST
SHADING	N/A	N/A	.27	N/A
INSIDE SHADING	N/A	N/A	1	N/A

SUSPENDED CEILING SPECIFIED

FLOOR DATA  
 ON-GRADE PER 256 FEET LOSS 45 BTU/LF

INTERNAL LOADS

	OCCUPIED	UNOCCUPIED	
PEOPLE			
NUMBER	.002	0	PER SQ FOOT
SENS HEAT	345	345	BTU/ PERSON
LATENT HEAT	435	435	BTU/ PERSON
AVE OCCUP	90	0	PERCENT
LIGHTING			
LOAD	1.69	0	WATTS/ SQ FOOT
AVE USAGE	100	0	PERCENT
% TO PLENUM	0	0	PERCENT
EQUIPMENT			
LOAD	18.3	0	WATTS/ SQ FOOT
AVE USAGE	90	0	PERCENT
INFILTRATION			
AIR CHANGES	N/A	.25	
ZONE HEIGHT	N/A	16	

PROJECT INDUSTRIAL PARK

PEAK HEATING LOAD SUMMARY FOR ZONE 1

SHOP

	SENSIBLE	LATENT	TOTAL
ROOF	-18054		-18054
WALL	-81803.27		-81803.27
GLASS	0		0
FLOOR	-11520		-11520
TOTAL BTU/HR	-111377.3	0	-111377.3
BTU/HR-SF	-9.09945	0	-9.09945

PRESS ANY KEY TO CONTINUE

TEMP	HEATING	HUMID
97	0	0
92	0	0
87	0	0
82	0	0
77	0	0
72	0	0
67	451.7787	0
62	1298.712	0
57	2125.644	0
52	2962.577	0
47	3799.508	0
42	4636.441	0
37	5473.374	0
32	6310.305	0
27	7147.239	0
22	7984.171	0
17	8821.105	0
12	9658.035	0
7	10494.96	0
2	11331.91	0
3	12168.84	0
8	13005.76	0

ENERGY SUMMARY

	OCCUP	UNOCCUP	TOTAL
C' ANT MBTU	0		0
HF PLANT MBTU	25728.56	145133	170861.6
LGT & EQ KWH	1077528	1.976714	1077528
AUXIL KWH	20242.73	0	20242.73
MISCEL (KWH)	0		0
TOTAL MBTU	3772414	145139.8	3917554
MBTU / SQ FT	308.1786	11.85685	320.0354

FUEL USAGE

FUEL CONSUMPTION

FUEL	CONSUMPTION	UNIT
ELECTRICITY	1097771	KWH
GAS	189846.2	CU FT
STEAM	0	LBS
OIL	0	GALS
COAL	0	LBS
DIESEL	0	GALS

DO YOU WANT TO VIEW SEA GRAPHICS :?

# RETROFIT COSTS:

CHANGES SINCE '81 STUDY. CO. ADMITTED THROUGH AIR COMPETITION FOR PRETEXT. CO. NO LONGER INTRODUCED THROUGH NW. UNIT HTK.

## SHOP AREA

2 NEPO 198 @ 62000 @ 177 = 1354  
 2 NEPO 226 @ 95000 @ 941 = 1082

## OFFICE

LOIL AS @ PDL = 500

## INSTALLATION

UNIT HTRS. @ 54 @ = 216  
 LOIL @ = 300

## PUMPING EA. UNIT

SAME AS PDL @ 103 = 812  
 LOIL VALVE @ = 47

## DISTRIBUTION:

ARRANGEMENT AS IN '81 WORK  
 LINE SIZES REDUCED TO 1 1/2 MAX.  
 2 1/2 = 1 1/2, 1 1/4 = 3/4 1/2 SAME

200' 1 1/2" INSUL STL @ 0.65 @ 317 = 2424  
 200' 3/4" INSUL STL @ 0.55 @ 239 = 2220

## MECHANICAL EQUIPMENT SAME AS PDL

= 2607

## SUBS. BLDG ONLY

12702

5% SY  
 15% UNIT

12000

1500

13500

10% O&P

1350

15150

SY

11500

## PRESENT ENERGY USE:

1982 - 1073 GAL/YR - PROPANE  
 ELC. USE NOT AVAILABLE

## ENERGY USE PREDICTED BY SED:

1893 GAL/YR @ 70% EFF (1003 @ 95% @ 100%)  
 - 4950 KWH/YR @ 1.6 COP @

## ENERGY REQUIRED FROM GEO-T

.136 x 10<sup>9</sup>

BUILDING: Precision Data Carriage

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CLIENT CITY OF KLAMATH FALL PROJECT INDUSTRIAL PARK

\*\*\*\*\*

SIMPLIFIED ENERGY ANALYSIS (SEA) VERSION 3

PRODUCT OF FERREIRA & KALASINSKY ASSOCIATES, INC

\*\*\*\*\*

ZONE DESCRIPTION SUMMARY FOR ZONE 1 SHOP

ZONE AREA	9600	SQ FT
VENT RATE	0	CFM PER SQ FT
SUMMER OCCUP. TEMP	78	DEGREES FAHR
WINTER OCCUP. TEMP	68	DEGREES FAHR
UNOCCUPIED TEMP.	68	DEGREES FAHR
DESIGN R.H.	50	%

	ROOF	WALL 1 NET	GLASS	WALL 2
AREA	9600	1120	0	2320
U-FACT	.53	.586	.1	.545
MASS	10	50	N/A	50
COLOR	1	.65	N/A	.65
DIR	HORIZONTAL	SOUTH	NORTH	NORTH
SHADING	N/A	N/A	.27	N/A
INSIDE SHADING	N/A	N/A	1	N/A

SUSPENDED CEILING NOT SPECIFIED

FLOOR DATA  
ON-GRADE PER 320 FEET LOSS 60 BTU/LF

INTERNAL LOADS

	OCCUPIED	UNOCCUPIED	
PEOPLE			
NUMBER	.0021	0	PER SQ FOOT
SENS HEAT	345	345	BTU/ PERSON
LATENT HEAT	435	435	BTU/ PERSON
AVE OCCUP	90	0	PERCENT
LIGHTING			
LOAD	1.391	0	WATTS/ SQ FOOT
AVE USAGE	100	0	PERCENT
% TO PLENUM	0	0	PERCENT
EQUIPMENT			
LOAD	.5	0	WATTS/ SQ FOOT
AVE USAGE	90	0	PERCENT
INFILTRATION			
AIR CHANGES	N/A	.25	
ZONE HEIGHT	N/A	14.5	

\*\*\*\*\*  
 CLIENT CITY OF KLAMATH FALL PROJECT INDUSTRIAL PARK  
 \*\*\*\*\*  
 SIMPLIFIED ENERGY ANALYSIS (SEA) VERSION 3  
 PRODUCT OF FERREIRA & KALASINSKY ASSOCIATES, INC  
 \*\*\*\*\*  
 ZONE DESCRIPTION SUMMARY FOR ZONE 2 OFFICE

ZONE AREA	2700	SQ FT
VENT RATE	0	CFM PER SQ FT
SUMMER OCCUP. TEMP	78	DEGREES FAHR
WINTER OCCUP. TEMP	68	DEGREES FAHR
UNOCCUPIED TEMP.	62	DEGREES FAHR
DESIGN R.H.	50	%

	ROOF	WALL 1 NET	GLASS	WALL 2
AREA	2700	948	538	0
U-FACT	.033	.033	1	.06
MASS	30	30	N/A	30
COLOR	1	.65	N/A	.65
DIR	HORIZONTAL	WEST	WEST	NORTH
SHADING	N/A	N/A	.9	N/A
INSIDE SHADING	N/A	N/A	1	N/A

SUSPENDED CEILING SPECIFIED

FLOOR DATA  
 ON-GRADE PER 147 FEET LOSS 60 BTU/LF

INTERNAL LOADS

	OCCUPIED	UNOCCUPIED	
PEOPLE			
NUMBER	.002	0	PER SQ FOOT
SENS HEAT	250	250	BTU/ PERSON
LATENT HEAT	245	245	BTU/ PERSON
AVE OCCUP	90	0	PERCENT
LIGHTING			
LOAD	2.5	.5	WATTS/ SQ FOOT
AVE USAGE	90	100	PERCENT
% TO PLENUM	10	0	PERCENT
EQUIPMENT			
LOAD	.5	0	WATTS/ SQ FOOT
AVE USAGE	90	0	PERCENT
INFILTRATION			
AIR CHANGES	N/A	.25	
ZONE HEIGHT	N/A	8	

PROJECT INDUSTRIAL PARK

P. K HEATING LOAD SUMMARY FOR ZONE 1

SHOP

	SENSIBLE	LATENT	TOTAL
ROOF	-300192		-300192
WALL	-113322.5		-113322.5
GLASS	0		0
FLOOR	-19200		-19200
TOTAL BTU/HR	-432714.5	0	-432714.5
BTU/HR-SF	-45.07443	0	-45.07443

PRESS ANY KEY TO CONTINUE

PROJECT INDUSTRIAL PARK

P. K HEATING LOAD SUMMARY FOR ZONE 2

OFFICE

	SENSIBLE	LATENT	TOTAL
ROOF	-3258.9		-3258.9
WALL	-2964.396		-2964.396
GLASS	-14692		-14692
FLOOR	-5820		-5820
TOTAL BTU/HR	-31735.29	0	-31735.29
BTU/HR-SF	-19.16048	0	-19.16048

PRESS ANY KEY TO CONTINUE

PROJECT

INDUSTRIAL PARK

T P	HEATING	HUMID
1	0	0
2	0	0
87	0	0
82	0	0
7	0	0
12	0	0
67	0	0
12	0	0
17	0	0
52	749.832	0
17	40855.95	0
12	80962.1	0
37	121068.2	0
32	161174.3	0
17	201280.4	0

PRESS ANY KE: TO CONTINUE

PROJECT

INDUSTRIAL PARK

T P	HEATING	HUMID
23	241366.5	0
7	295097.5	0
10	330587	0
7	376076.0	0
10	421565.7	0
10	467055.1	0
8	512544.5	0

ENERGY SUMMARY

	OCCUP	UNOCCUP	TOTAL
COOLANT MBTU	0		0
WATER COOLANT MBTU	278598.7	1226115	1504714
LGT & EQ KWH	70047.98	8093.245	78141.22
AUXIL KWH	7574.377	0	7574.377
MISCEL (KWH)	0		0
TOTAL MBTU	543523.8	1253737	1797261
MBTU / SQ FT	44.18892	101.9298	146.1188

FUEL USAGE

FUEL	CONSUMPTION	
ELECTRICITY	85715.59	KWH
GAS	1504714	CU FT
STEAM	0	LBS
OIL	0	GALS
COAL	0	LBS
DIESEL	0	GALS

DO YOU WANT TO VIEW SEA GRAPHICS :?

PROJECT

INDUSTRIAL PARK

< HEATING LOAD SUMMARY FOR ZONE 1

SHOP

	SENSIBLE	LATENT	TOTAL
ROOF	-30019.2		-30019.2
WALL	-113322.5		-113322.5
GLASS	0		0
FLOOR	-19200		-19200
TOTAL BTU/HR	-162541.7	0	-162541.7
BTU/HR-SF	-16.93143	0	-16.93143

PRESS ANY KEY TO CONTINUE

TEMP	HEATING	HUMID
7	0	0
92	0	0
87	0	0
82	0	0
77	0	0
72	0	0
67	0	0
62	0	0
57	0	0
52	5947.063	0
47	18439.22	0
42	30931.4	0
37	47423.57	0
32	65915.74	0
27	86407.89	0
22	108900.04	0
17	133392.19	0
12	159884.34	0
7	173882.2	0

ENERGY SUMMARY

	OCCUP	UNOCCUP	TOTAL
C. LANT MBTU	0		0
HPLANT MBTU	94054.64	665455.4	759510.1
LGT & EQ KWH	70047.98	8093.245	78141.22
AUXIL KWH	3353.075	0	3353.075
MISCEL (KWH)	0		0
TOTAL MBTU	344572.4	693077.7	1037650
MBTU / SQ FT.	28.01402	56.34778	84.3618

FUEL USAGE

FUEL	CONSUMPTION	
ELECTRICITY	81494.3	KWH
GAS	759510.1	CU FT
STEAM	0	LBS
OIL	0	GALS
COAL	0	LBS
DIESEL	0	GALS

DO YOU WANT TO VIEW SEA GRAPHICS :?

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RETROFIT COSTS:

PRECISION DATA CARRIAGE:

5. GAS FIRED UNITS ONLY SIZE FOR INSULATED ROOF

SHOP

2 UNIT HTR'S @ 10000 BTU/HR EA.

ASSUME 120° WATER TO UNITS

NEED 1/4" JH - 364 12000 @ 120°F @ 1126

\* 2252

OFFICE

- FINNED COIL IN EXIST DUCT 20000 BTU/H

ASSUME EXIST STAL @ 2000

FOR 500 FPM FACE VEL. NEED 4 FT<sup>2</sup>

24" x 24" COIL USE 30" x 30" FOR

REQUIRED AIR ΔP

ENT AIR 60°  
 WGT AIR 100°  
 ENT WTR 120  
 WGT WTR 90  
 CFM 2000  
 FACE A. 6.25  
 FOS 1  
 CODE 1  
 EL 4000  
 ROW 3  
 FPI 9 (3.56)  
 AIR ΔP .13 in. wg.

COIL  
 INSTALLATION

500  
 500

EA UNIT

1 - AIR VENT - 12  
 2 - 3/4" BALL WV - 38  
 1 - STRAIN - 28  
 2 - 3/8" UNION @ 20 - 40  
 10 - 3/4" PIPE 5551.294 - 55  
123

609

2 WAY COIL W/ COIL - 47

47

UNIT HEATER INSTALLATION 54 EA

108

DISTRIBUTION

240' 1 1/2" INXL STL @ 3.65 + 3.17

2908

EQUIPMENT

CIRC PUMP 1/2 HP 395  
 EXP. TANK 2.5 GAL 81  
 H-X 5000 BTU/HR @ 10000 1500  
 4 1/2" GATES @ 51 204  
 4 PRESS. G.O. @ 17 68  
 4 THERM @ 34 136  
 1/2" CL WV 48  
 FILL & MUX LINE 100  
 ELEV. TO PUMP 75

2607

2607

SUBTOT

9331

SAY	9331.
45% CONT	<u>1400</u>
	10731.
10% O&P	<u>1073</u>
	11804
SAY	<u>12000</u>

PRESENT ENERGY USE:

1984-1985 = 12219 THERMS

DUE TO INSUFFICIENT CAPACITY IN SHOP UNIT HEATERS. A SUPPLEMENTAL PROPANE HEATER IS USED. IN ADDITION W/ SHOP AIR IS INTRODUCED TO THE CLEAN ROOM SYSTEM TO MAINTAIN POSITIVE PRESSURE. AS A RESULT SOME REC HEAT DATA CONTRIBUTES TO SHOP HEATING.

ENERGY USE PREDICTED BY SEA. PROGRAM.

SHOP & OFFICE. 15097 THERM/ML (E 75% EFF)

W/ ADDED R-19 ROOF INSULATION

7595 THERM/ML (E 75% EFF)  
(4250 SHR @ 2.555/SH)

DUE TO VERY GOOD PAYBACK (< 1 YR) OF ROOF INSL. 7595 THERM FLOOR COULD BE USED FOR BEST COMPARISON.

ENERGY REQUIRED FROM GEOTHERMAL:

7595 THERM · 100000 BTU/TH · .75

= 0.570 × 10<sup>9</sup> BTU/ML

\*\*\*\*\*  
 GEO-HEAT CENTER  
 CLIENT CITY OF K.FALLS PROJECT EQUITORIAL  
 \*\*\*\*\*  
 SIMPLIFIED ENERGY ANALYSIS (SEA) VERSION 3  
 PRODUCT OF FERREIRA & KALASINSKY ASSOCIATES, INC  
 \*\*\*\*\*  
 ZONE DESCRIPTION SUMMARY FOR ZONE 1 OFFICE CORE

ZONE AREA	2058	SQ FT
VENT RATE	.1	CFM PER SQ FT
SUMMER OCCUP. TEMP	78	DEGREES FAHR
WINTER OCCUP. TEMP	72	DEGREES FAHR
UNOCCUPIED TEMP.	65	DEGREES FAHR
DESIGN R.H.	50	%

	ROOF	WALL 1 NET	GLASS	WALL 2
AREA	2058	0	0	0
U-FACT	.045	.06	.1	.06
MASS	20	30	N/A	30
COLOR	.5	.65	N/A	.65
DIR	HORIZONTAL	NORTH	NORTH	NORTH
SHADING	N/A	N/A	.27	N/A
INSIDE				
SHADING	N/A	N/A	1	N/A

SUSPENDED CEILING SPECIFIED

INTERNAL LOADS

	OCCUPIED	UNOCCUPIED	
PEOPLE			
NUMBER	.005	0	PER SQ FOOT
SENS HEAT	250	250	BTU/ PERSON
LATENT HEAT	245	245	BTU/ PERSON
AVE OCCUP	30	0	PERCENT
LIGHTING			
LOAD	1.5	.25	WATTS/ SQ FOOT
AVE USAGE	90	100	PERCENT
% TO PLENUM	50	0	PERCENT
EQUIPMENT			
LOAD	.5	0	WATTS/ SQ FOOT
AVE USAGE	90	0	PERCENT
INFILTRATION			
AIR CHANGES	N/A	.25	
ZONE HEIGHT	N/A	9	

PROJECT EQUITORIAL

PEAK HEATING LOAD SUMMARY FOR ZONE 1 OFFICE CORE

	SENSIBLE	LATENT	TOTAL
ROOF	-2917.215		-2917.215
WALL	0		0
GLASS	0		0
FLOOR	0		0
TOTAL BTU/HR	-2917.215	0	-2917.215
BTU/HR-SF	-1.4175	0	-1.4175

PRESS ANY KEY TO CONTINUE

PROJECT EQUITORIAL

PEAK COOLING LOAD SUMMARY FOR ZONE 1 OFFICE CORE

	SENSIBLE	LATENT	TOTAL
SOLAR	0		0
TRANSMISSION	1250.235		1250.235
FLOOR	0		0
INTERNAL	11352.44	2521.05	13873.49
TOTAL BTU/HR	12602.68	2521.05	15123.73
BTU/HR-SF	6.12375	1.225	7.34875
SPACE SEN.	12602.68		
BTU/HR-SF	6.12375		

PRESS ANY KEY TO CONTINUE

ENERGY SUMMARY

	OCCUP	UNOCCUP	TOTAL
CPLANT MBTU	3217.964		3217.964
HPLANT MBTU	5141.5	16946.36	22087.86
LGT & EQ KWH	10394.56	3084.427	13478.99
AUXIL KWH	2057.033	1918.496	3975.53
MISCEL (KWH)	0		0
TOTAL MBTU	50856.77	34021.33	84878.1
MBTU / SQ FT	24.71174	16.53126	41.243

FUEL USAGE

FUEL CONSUMPTION

ELECTRICITY	18397.38	KWH
GAS	22087.86	CU FT
STEAM	0	LBS
OIL	0	GALS
COAL	0	LBS

\*\*\*\*\*

GEO-HEAT CENTER  
 CLIENT CITY OF K.FALLS PROJECT EQUITORIAL

\*\*\*\*\*

SIMPLIFIED ENERGY ANALYSIS (SEA) VERSION 3  
 PRODUCT OF FERREIRA & KALASINSKY ASSOCIATES, INC

\*\*\*\*\*

ZONE DESCRIPTION SUMMARY FOR ZONE 1 OFFICE PERIMETER

ZONE AREA	3158	SQ FT
VENT RATE	.503	CFM PER SQ FT
SUMMER OCCUP. TEMP	78	DEGREES FAHR
WINTER OCCUP. TEMP	72	DEGREES FAHR
UNOCCUPIED TEMP.	65	DEGREES FAHR
DESIGN R.H.	50	%

	ROOF	WALL 1 NET	GLASS	WALL 2
AREA	3158	772	344	486
U-FACT	.045	.085	.58	.13
MASS	20	30	N/A	30
COLOR	.5	.9	N/A	.9
DIR	HORIZONTAL	WEST	WEST	NORTH
SHADING	N/A	N/A	.5	N/A
INSIDE SHADING	N/A	N/A	1	N/A

SUSPENDED CEILING SPECIFIED

FLOOR DATA  
 ON-GRADE PER 204 FEET LOSS 36 BTU/LF

INTERNAL LOADS

	OCCUPIED	UNOCCUPIED	
PEOPLE			
NUMBER	.06	0	PER SQ FOOT
SENS HEAT	250	250	BTU/ PERSON
LATENT HEAT	245	245	BTU/ PERSON
AVE OCCUP	50	0	PERCENT
LIGHTING			
LOAD	1.5	.25	WATTS/ SQ FOOT
AVE USAGE	90	100	PERCENT
% TO PLENUM	50	0	PERCENT
EQUIPMENT			
LOAD	.5	0	WATTS/ SQ FOOT
AVE USAGE	90	0	PERCENT
INFILTRATION			
AIR CHANGES	N/A	.5	
ZONE HEIGHT	N/A	9	

PROJECT EQUITORIAL

PEAK HEATING LOAD SUMMARY FOR ZONE 1

OFFICE PERIMETER

	SENSIBLE	LATENT	TOTAL
ROOF	-4476.465		-4476.465
WALL	-8114.4		-8114.4
GLASS	-12569.76		-12569.76
FLOOR	-7344		-7344
TOTAL BTU/HR	-32504.62	0	-32504.62
BTU/HR-SF	-10.29279	0	-10.29279

PRESS ANY KEY TO CONTINUE

53921  
-2-2-  
1.5=571

PROJECT EQUITORIAL

PEAK COOLING LOAD SUMMARY FOR ZONE 1

OFFICE PERIMETER

	SENSIBLE	LATENT	TOTAL
SOLAR TRANSMISSION	23719.76		23719.76
FLOOR	9394.179		9394.179
INTERNAL	-7344		-7344
	60842.82	46422.6	107265.4
TOTAL BTU/HR	86612.77	46422.6	133035.4
BTU/HR-SF	27.42646	14.7	42.12646
SPACE SEN. BTU/HR-SF	86612.77		
	27.42646		

PRESS ANY KEY TO CONTINUE

ENERGY SUMMARY

	OCCUP	UNOCCUP	TOTAL
CPLANT MBTU	11568.75		11568.75
HPLANT MBTU	73760.78	93042.48	166803.3
LGT & EQ KWH	15950.45	4733.05	20683.5
AUXIL KWH	4279.774	3294.327	7574.101
MISCEL (KWH)	0		0
TOTAL MBTU	154375.3	120439.9	274815.2
MBTU / SQ FT	48.86388	38.13804	87.02192

FUEL USAGE

FUEL CONSUMPTION

FUEL		
ELECTRICITY	31647.22	KWH
GAS	166803.3	CU FT.
STEAM	0	LBS
OIL	0	GALS
COAL	0	LBS
DIESEL	0	GALS

DO YOU WANT TO VIEW SEA GRAPHICS :?

\*\*\*\*\*  
 GEO-HEAT CENTER  
 CLIENT CITY OF K.FALLS PROJECT EQUITORIAL  
 \*\*\*\*\*  
 SIMPLIFIED ENERGY ANALYSIS (SEA) VERSION 3  
 PRODUCT OF FERREIRA & KALASINSKY ASSOCIATES, INC  
 \*\*\*\*\*

ZONE DESCRIPTION SUMMARY FOR ZONE 1 RECIEVING

ZONE AREA	800	SQ FT
VENT RATE	.433	CFM PER SQ FT
SUMMER OCCUP. TEMP	78	DEGREES FAHR
WINTER OCCUP. TEMP	72	DEGREES FAHR
UNOCCUPIED TEMP.	60	DEGREES FAHR
DESIGN R.H.	50	%

	ROOF	WALL 1 NET	GLASS	WALL 2
	----	----	-----	-----
AREA	800	780	0	0
U-FACT	.047	.085	.58	.13
MASS	20	30	N/A	30
COLOR	.5	.9	N/A	.9
DIR	HORIZONTAL	EAST	WEST	NORTH
SHADING	N/A	N/A	.5	N/A
INSIDE SHADING	N/A	N/A	1	N/A

SUSPENDED CEILING NOT SPECIFIED

FLOOR DATA  
 ON-GRADE PER 60 FEET LOSS 36 BTU/LF

INTERNAL LOADS

	OCCUPIED	UNOCCUPIED	
PEOPLE			
NUMBER	.0025	0	PER SQ FOOT
SENS HEAT	250	250	BTU/ PERSON
LATENT HEAT	245	245	BTU/ PERSON
AVE OCCUP	50	0	PERCENT
LIGHTING			
LOAD	1.5	0	WATTS/ SQ FOOT
AVE USAGE	90	0	PERCENT
% TO PLENUM	0	0	PERCENT
EQUIPMENT			
LOAD	.5	0	WATTS/ SQ FOOT
AVE USAGE	90	0	PERCENT
INFILTRATION			
AIR CHANGES	N/A	1	
ZONE HEIGHT	N/A	13	

PROJECT EQUITORIAL

PEAK HEATING LOAD SUMMARY FOR ZONE 1

RECIEVING

	SENSIBLE	LATENT	TOTAL
ROOF	-2368.8		-2368.8
WALL	-4176.9		-4176.9
GLASS	0		0
FLOOR	-2160		-2160
TOTAL BTU/HR	-8705.7	0	-8705.7
BTU/HR-SF	-10.88212	0	-10.88212

PRESS ANY KEY TO CONTINUE

PROJECT EQUITORIAL

PEAK COOLING LOAD SUMMARY FOR ZONE 1

RECIEVING

	SENSIBLE	LATENT	TOTAL
SOLAR	0		0
TRANSMISSION	4015.455		4015.455
FLOOR	-2160		-2160
INTERNAL	5960.8	490	6450.8
TOTAL BTU/HR	7816.255	490	8306.255
BTU/HR-SF	9.770319	.6125	10.38292
SPACE SEN.	7816.255		
BTU/HR-SF	9.770319		

PRESS ANY KEY TO CONTINUE

ENERGY SUMMARY

	OCCUP	UNOCCUP	TOTAL
CPLANT MBTU	0		0
HPLANT MBTU	34351.49	50107	84458.48
LGT & EQ KWH	4040.646	0	4040.646
AUXIL KWH	1211.304	1210.874	2422.178
MISCEL (KWH)	0		0
TOTAL MBTU	52276.39	54239.72	106516.11
MBTU / SQ FT	65.34549	67.79964	133.1451

FUEL USAGE

FUEL CONSUMPTION

ELECTRICITY	6462.824	KWH
GAS	84458.48	CU FT
STEAM	0	LBS
OIL	0	GALS
COAL	0	LBS
DIESEL	0	GALS

DO YOU WANT TO VIEW SEA GRAPHICS :?

\*\*\*\*\*  
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 CLIENT CITY OF K.FALLS PROJECT EQUITORIAL  
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 \*\*\*\*\*

ZONE DESCRIPTION SUMMARY FOR ZONE 1 ASSEMBLY

ZONE AREA	13284	SQ FT
VENT RATE	.108	CFM PER SQ FT
SUMMER OCCUP. TEMP	78	DEGREES FAHR
WINTER OCCUP. TEMP	72	DEGREES FAHR
UNOCCUPIED TEMP.	60	DEGREES FAHR
DESIGN R.H.	50	%

	ROOF	WALL 1 NET	GLASS	WALL 2
	-----	-----	-----	-----
AREA	13284	1160	240	2652
U-FACT	.047	.052	.58	.085
MASS	20	30	N/A	30
COLOR	.5	.9	N/A	.9
DIR	HORIZONTAL	NORTH	NORTH	SOUTHEAST
SHADING	N/A	N/A	1	N/A
INSIDE				
SHADING	N/A	N/A	1	N/A

SUSPENDED CEILING NOT SPECIFIED

FLOOR DATA  
 ON-GRADE PER 347 FEET LOSS 36 BTU/LF

INTERNAL LOADS

	OCCUPIED	UNOCCUPIED	
PEOPLE			
NUMBER	.0023	0	PER SQ FOOT
SENS HEAT	250	250	BTU/ PERSON
LATENT HEAT	245	245	BTU/ PERSON
AVE OCCUP	75	0	PERCENT
LIGHTING			
LOAD	2	.25	WATTS/ SQ FOOT
AVE USAGE	90	100	PERCENT
% TO PLENUM	0	0	PERCENT
EQUIPMENT			
LOAD	1.33	.1	WATTS/ SQ FOOT
AVE USAGE	90	100	PERCENT
INFILTRATION			
AIR CHANGES	N/A	.5	
ZONE HEIGHT	N/A	13	

PROJECT EQUITORIAL

PEAK HEATING LOAD SUMMARY FOR ZONE 1

ASSEMBLY

	SENSIBLE	LATENT	TOTAL
ROOF	-39333.92		-39333.92
WALL	-18001.62		-18001.62
GLASS	-8769.6		-8769.6
FLOOR	-12492		-12492
TOTAL BTU/HR	-78597.14	0	-78597.14
BTU/HR-SF	-5.916678	0	-5.916678

PRESS ANY KEY TO CONTINUE

PROJECT EQUITORIAL

PEAK COOLING LOAD SUMMARY FOR ZONE 1

ASSEMBLY

	SENSIBLE	LATENT	TOTAL
SOLAR	7608.132		7608.132
TRANSMISSION	31546.8		31546.8
FLOOR	-12492		-12492
INTERNAL	158614.8	7485.534	166100.3
TOTAL BTU/HR	185277.8	7485.534	192763.3
BTU/HR-SF	13.94744	.5635	14.51094
SPACE SEN.	185277.8		
BTU/HR-SF	13.94744		

PRESS ANY KEY TO CONTINUE

ENERGY SUMMARY

	OCCUP	UNOCCUP	TOTAL
CPLANT MBTU	28364.16		28364.16
HPLANT MBTU	57541	256700.6	314241.6
LGT & EQ KWH	111713	27873.13	139586.1
AUXIL KWH	10916.75	5755.487	16672.24
MISCEL (KWH)	0		0
TOTAL MBTU	504440.5	371475.1	875915.6
MBTU / SQ FT	37.97354	27.9641	65.93765

FUEL USAGE

FUEL CONSUMPTION

ELECTRICITY	164569	KWH
GAS	314241.6	CU FT
STEAM	0	LBS
OIL	0	GALS
COAL	0	LBS
DIESEL	0	GALS

DO YOU WANT TO VIEW SEA GRAPHICS :?

EXISTING SYSTEM: 4 ROOMS GAS/ELECTRICALS (2-10 TAU, 1-7 1/2 TAU, 1-4 TAU) AND 1 GAS UNIT HEATER.

HEATING CAPACITY: 10 TAU UNITS 2- 18500 BTU/HK (OUT)  
 7 1/2 TAU 1- 16000  
 4 TAU 1- 8600  
 UNIT HEATER 1- 8800

TOTAL INSTALLED CAPACITY  
 716000 BTU/HK

RETROFIT SYSTEM: CLOSED LOOP W/ PUT HT-X, H.W. COILS IN S.D. DUCTS FOR GAS ELEC. H.W. UNIT HTK FOR RECEIVING AREA.

2 10 TAU UNITS  
 LOAD IN ASSEMBLY RM TRANS - 78597 (SEA)  
 H.F.L. - 97916 (1.5 CH/14K)  
 176513 BTU/H

2 UNITS @ 1000 AIR + 4000 L  
 8000 · 1.09 (100-70) · 0.864 = 223749 - JK

COIL	EA	LA	EW	LW	CFM	FD	FDS	CODE	EL	ZEW	FTL	D.P
	70	100	120	100	4000	5.0	4	1	4000	3	10 (9.43)	.23
	70	100	120	100	3000	6.0	4	1	4000	3	10	.23
	70	100	120	100	1600	4.0	4	1	4000	3	13	.24

CALLS 2-10 TAU - 1600  
 1-7 1/2 TAU - 600  
 1-4 TAU - 450  
 INSTALLATION  
 200 EA = 900

PUMPING COMPONENTS EA UNIT

7 1/2 x 10 TAU CALLS 1 1/2"  
 1- AIR VENT - 12  
 2- 1 1/2" GATES C&I - 102  
 1- CLEAN - 23  
 2- 1 1/2" JUNCTION C&I - 44  
 10' 1 1/2" INCH W - 218  
 399  
 + 5  
 798

4 TAU # UNIT HTK - 1"  
 - 12  
 - 62  
 - 23  
 - 36  
 - 154  
 287  
 + 2  
 574

1- 9000 BTU/HK UNIT HTK  
 HEPCO U286 - 800  
 INST - 50

DISTRIBUTION PIPING:

2 1/2" 290  
2" 90  
1 1/2" 50  
1" 50

FLOW = 71.6 TH (ASSUME ENTRANCE @ GAS METER)

2 1/2" 290	15.45 + 3.97	1.1	= 6200	- 274
2" 90	11.15 + 3.70	1.1	= 1470	475
1 1/2" 50	8.65 + 3.47	1.1	= 667	205
1" 50	6.8 + 3.14	1.1	= 547	145
			<u>8944</u>	<u>3122</u>

CENTRAL MECHANICAL EQUIPMENT:

CIRC PUMP 72040	- 780	570
EX TANK 18 GAL	- 93	33
HT-x 750000 PUMP SS	- 6200	2000
4 - 2 1/2" GATES c 12s	- 500	240
4 PRESS GATE	- 68	34
4 THERM	- 136	56
1 - 2 1/2" CKLV	- 115	78
FILL AND LINE	- 100	50
ELEC TO PUMP	- 100	30
4 2 1/2" UNIQ	- 125	87
10' 2 1/2" INSUL SL .15 FT-45	- 539	215
	<u>4809</u>	

CAPITAL COST SUMMARY:

INSTALLED W/UNIQ LABOR

MATERIAL ONLY

CALS	3150
UNIT HTX	850
CAL PUMPS	1400
DIST PIPE	5950
CENT. MECH	6500
	<u>17450</u>
SW	2000
15% W/T	<u>22350</u>

3150	
800	
1050	75
3100	
<u>3550</u>	
1150	
1700	
<u>12850</u>	

CALCULATED ANNUAL ENERGY USE FROM SED ANALYSIS @ 70% AFUE

ZONE	THERMS
OFFICE COKE	221
OFFICE PERIM	1668
RELIEVING	845
ASSEMBLY	<u>3142</u>
TOTAL	5876 TH/yr

**A P P E N D I X B**

**Supplemental data plots for aquifer stress test**

STEP DRAWDOWN TEST - CIP #1

8/21/86

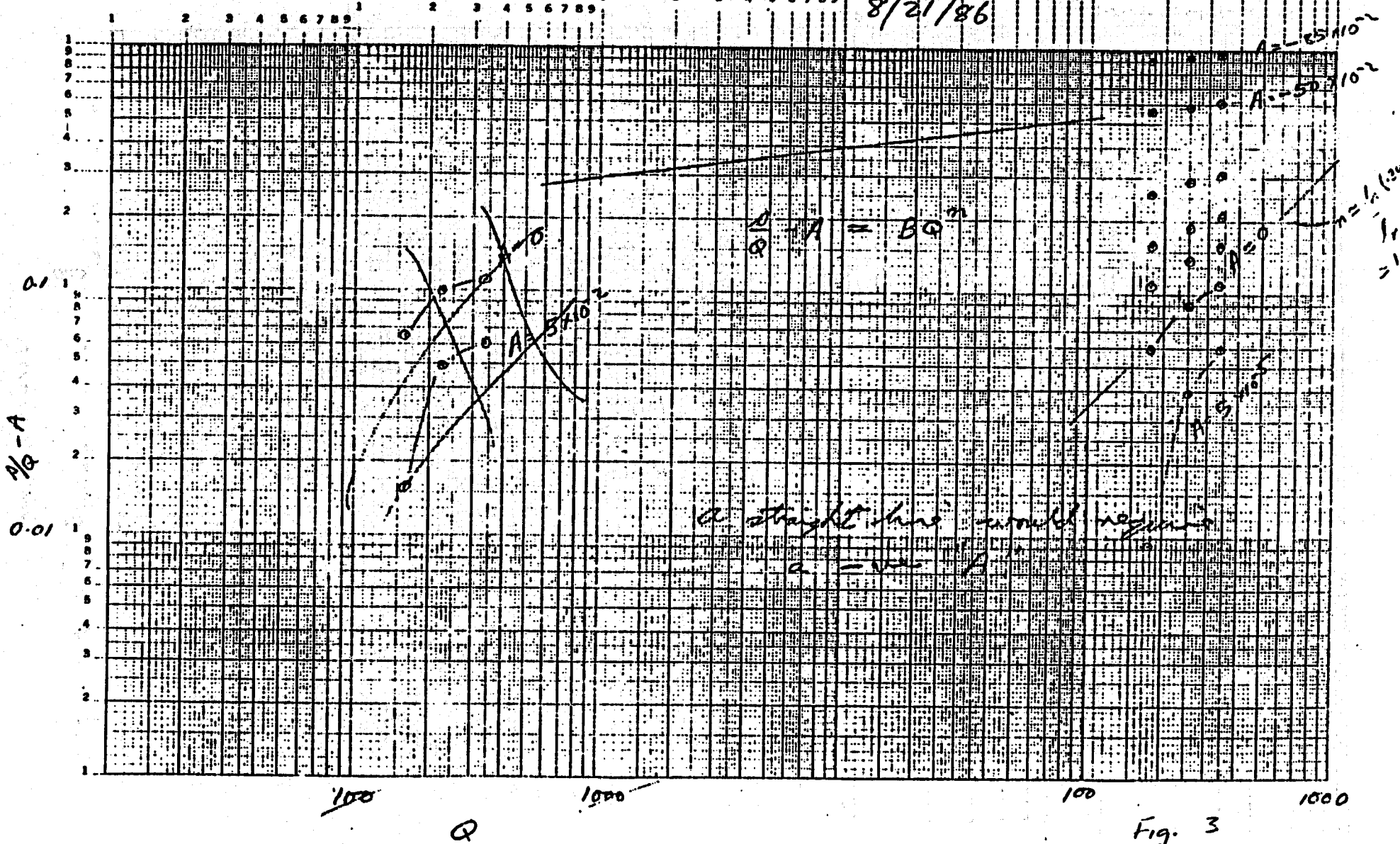
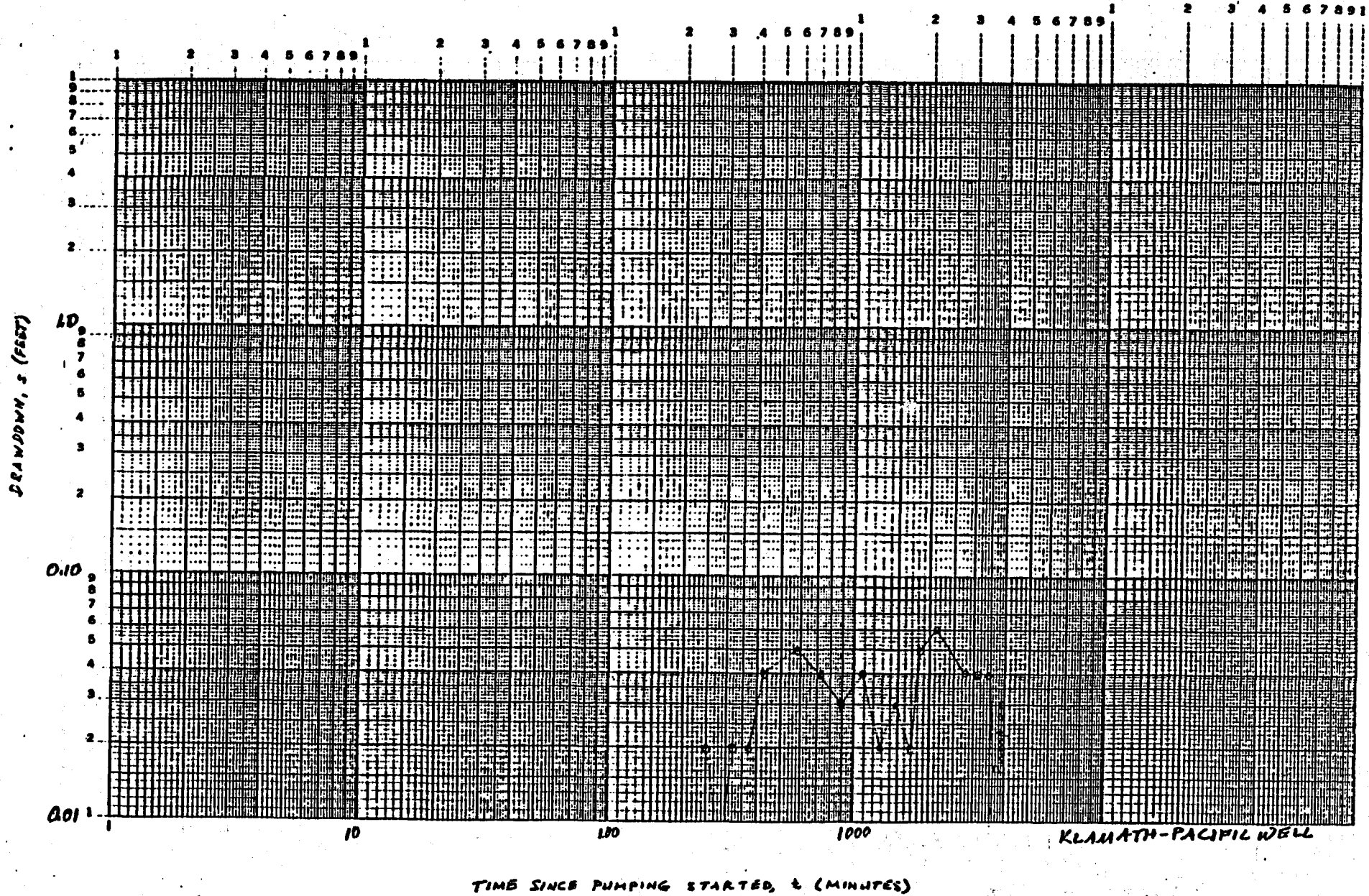


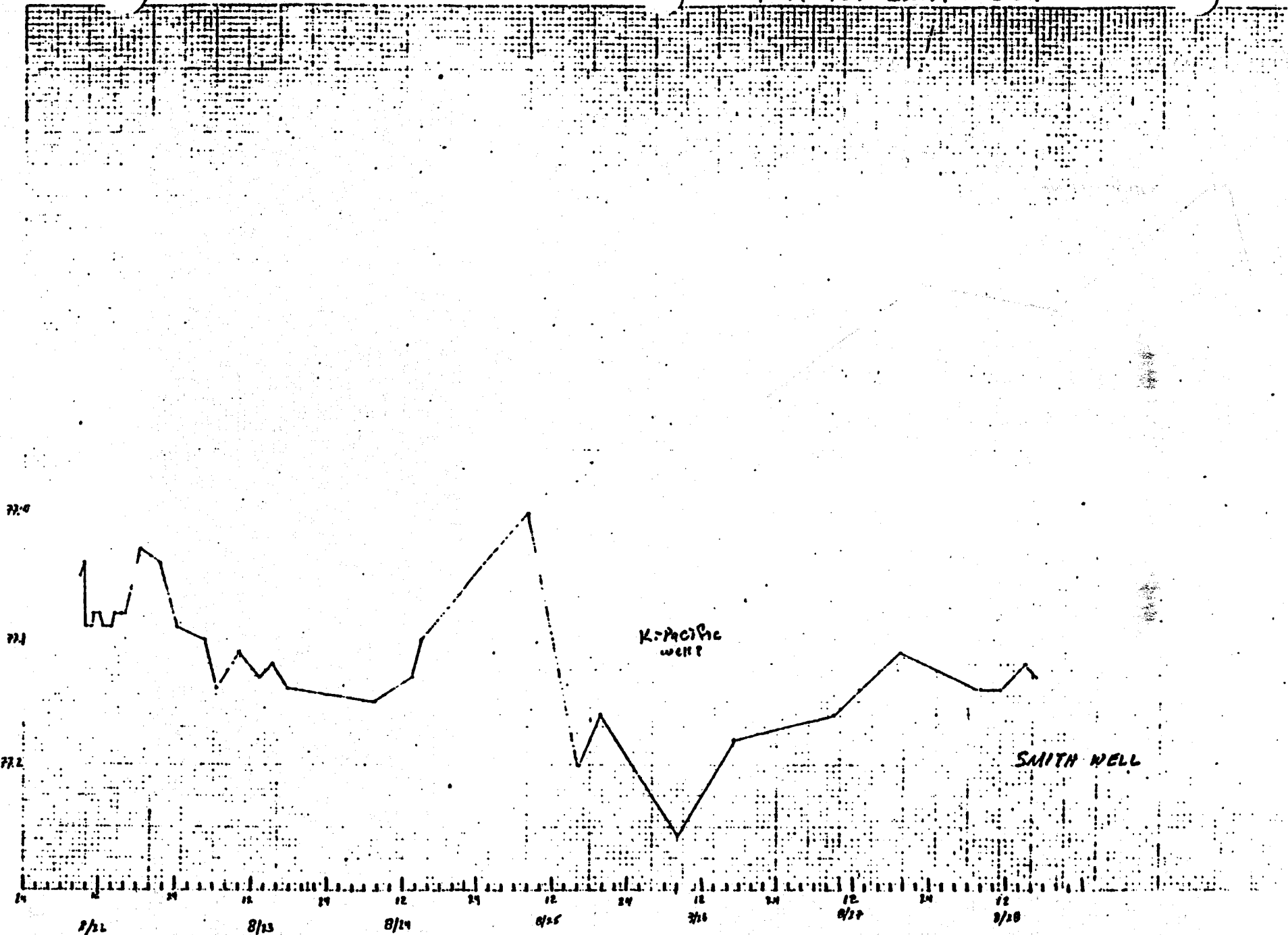
Fig. 3

o Data for Klamath-Pacific Well

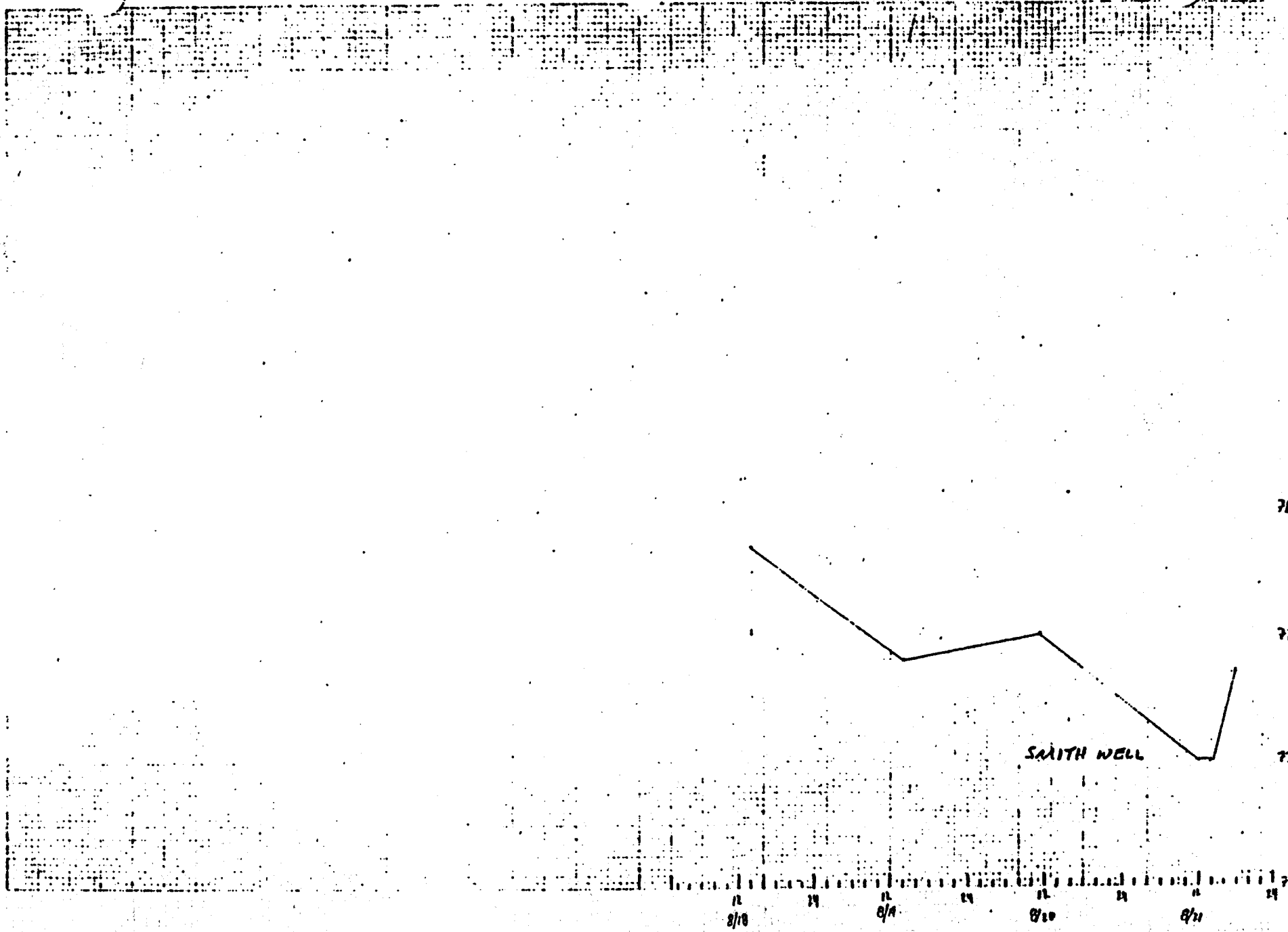


Date for Smith well

Depth to water (feet)



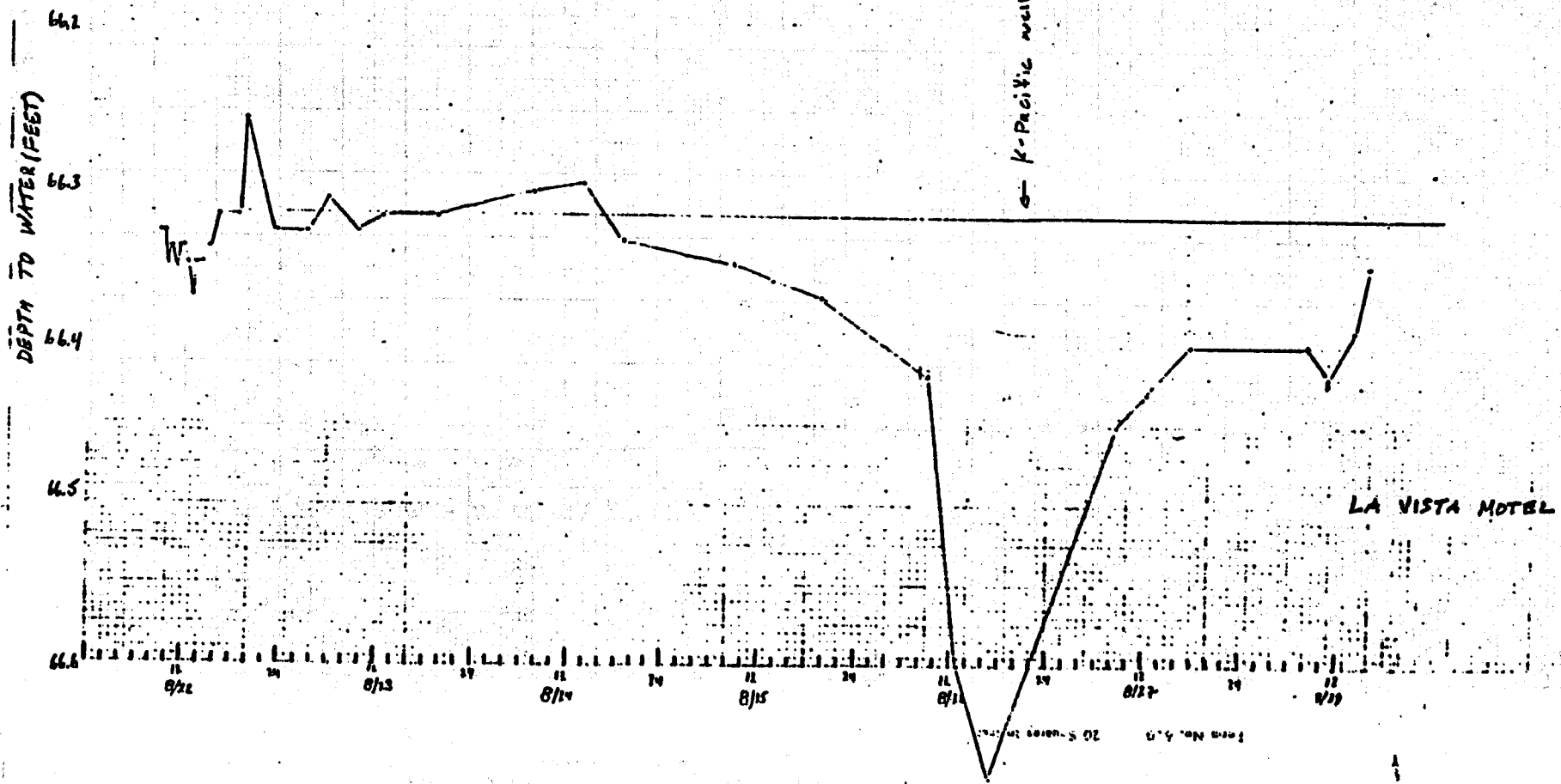
(Pre-tool) Data for Smith well



SMITH WELL

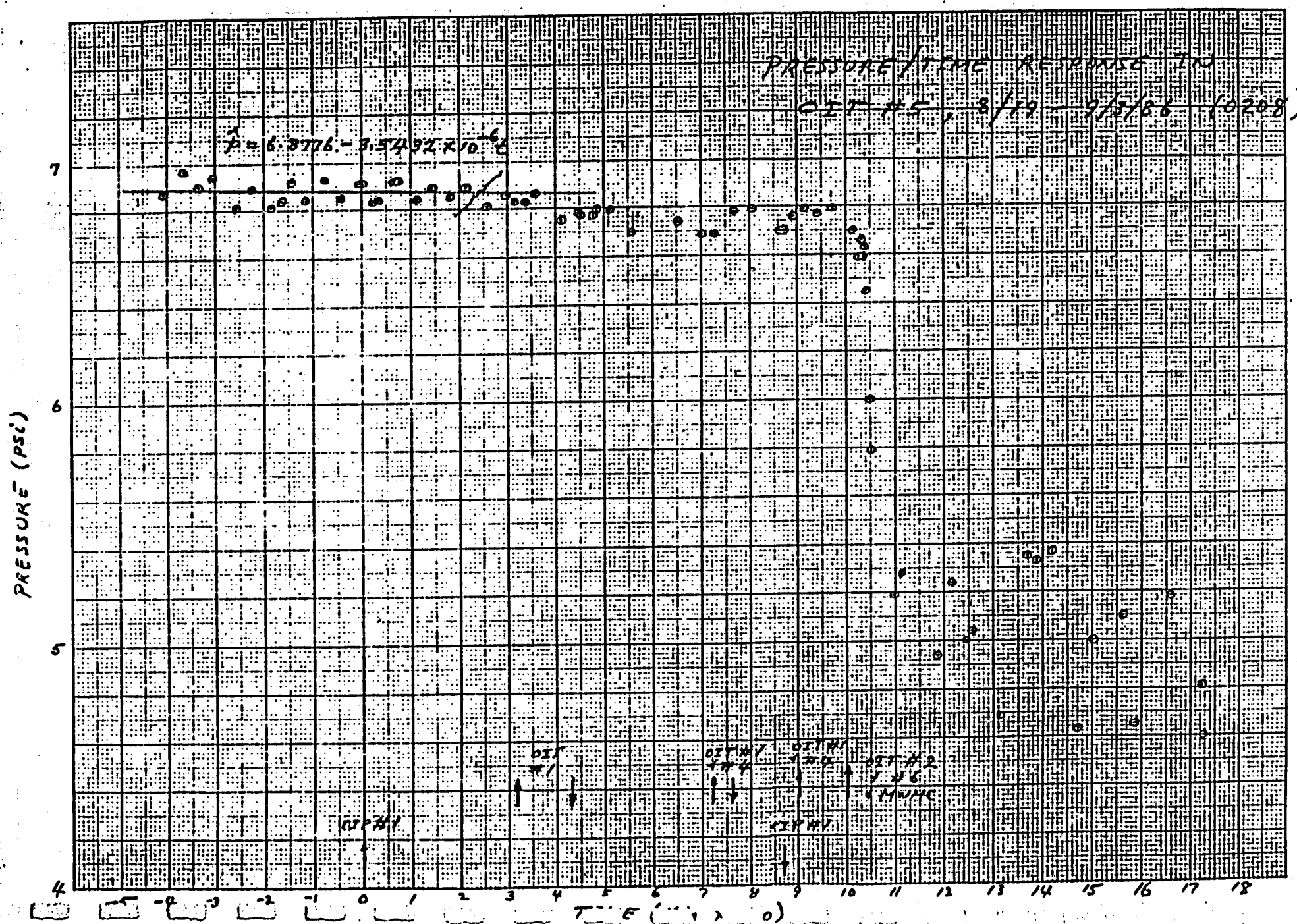
From No. 1120  
Engineering Co.

La Vista Motel



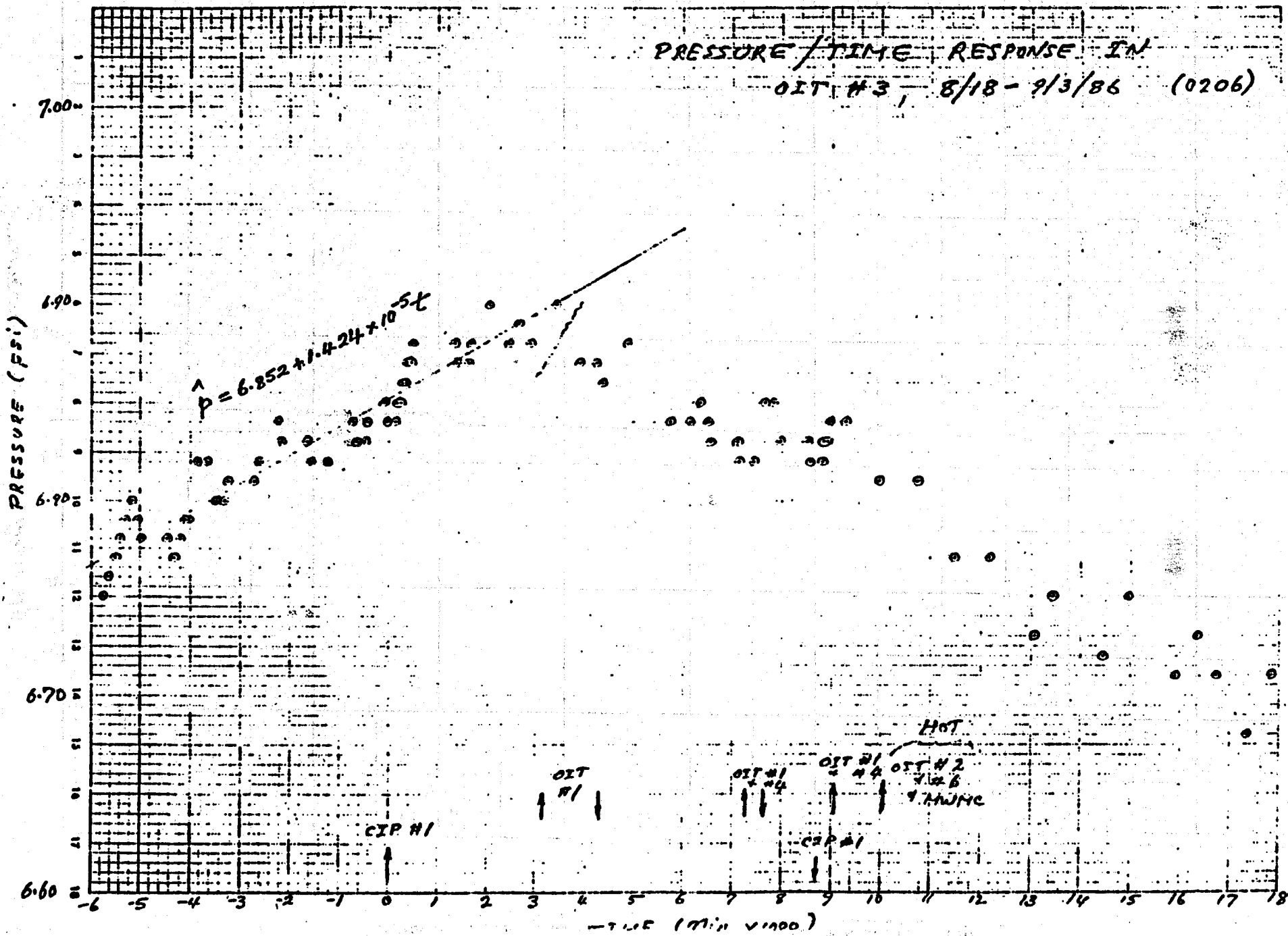
PRESSURE/TIME RESPONSE IN  
 OBT #5, 8/19 - 9/15/86 (0208)

$$P = 6.9776 - 3.5492 \cdot 10^{-6} t$$



PRESSURE / TIME RESPONSE IN

OIT #3, 8/18 - 9/3/86 (0206)





**A P P E N D I X C**

**Calculations in support of plume dimensions**

Darcy 's Law

$$Q = T i w$$

where

- Q is the flow in the aquifer (gallons per day),  
T is the transmissivity of the aquifer (gpd/ft width),  
i is hydraulic gradient (ft/ft), and  
w is the width of the stream tube (feet).

Therefore, the width of the plume, neglecting dispersion,

$$w = Q / T i$$

In the vicinity of CIP-1

$$w = \frac{(700 \text{ gpm} \times 1440 \text{ min/day})}{(3.4 \times 10^4 \text{ gpd/ft}) \times (50 \text{ ft} / 500 \text{ ft})}$$
$$= 296 \text{ feet}$$

In the vicinity of CIP-2

$$w = \frac{(700 \text{ gpm} \times 1440 \text{ min/day})}{(1.4 \times 10^5 \text{ gpd/ft}) \times (25 \text{ ft} / 2300 \text{ ft})}$$
$$= 662 \text{ feet}$$

In the vicinity of La Vista Motel

$$w = \frac{(700 \text{ gpm} \times 1440 \text{ min/day})}{(1.9 \times 10^5 \text{ gpd/ft}) \times (25 \text{ ft} / 3100 \text{ ft})}$$
$$= 657 \text{ feet}$$



WILLIAM E. NORK, Inc.

**A P P E N D I X D**

**Oregon Administrative Rules  
Chapter 690, Division 65**

See page 1

OREGON ADMINISTRATIVE RULES  
CHAPTER 690, DIVISION 65 - WATER RESOURCES DEPARTMENT  
STANDARDS AND PROCEDURES FOR  
LOW TEMPERATURE GEOTHERMAL WELLS AND EFFLUENT DISPOSAL SYSTEMS

**65-005 POLICY AND PURPOSE:**

- (1) All Low Temperature Geothermal Fluids are part of the ground water resources of the State of Oregon and shall be administered by the Water Resources Director (Director) under the provisions of ORS 537.010 to 537.795. The Director recognizes that these fluids are developed primarily because of their thermal characteristics and that special management is necessary. Reservoir assessment of Low Temperature Geothermal Fluids shall be conducted by the Director in the same manner as ground water investigations outlined in ORS 537.665 and ORS 537.685.
- (2) The purpose of the following rules is to provide standards and procedures for the development, use and management of Low Temperature Geothermal Fluids, while insuring proper management of all ground water resources so maximum beneficial use of the resource will be most effectively attained.
- (3) These rules supplement OAR 690-10-005 to 690-10-045, 690-60-005 to 690-63-045, and 690-64-000 to 690-64-010. Rule 690-60-050, paragraph 47 and 690-61-181 are hereby rescinded.

**DELETE:**

[690-60-050 (47) "Thermal Ground Water": means ground water having a temperature greater than 90 degrees Fahrenheit or 32 degrees Celsius. (The statutes of Oregon delegate to the Department of Water Resources the appropriation and supervision of thermal ground water having a temperature of less than 250 degrees Fahrenheit or 121 degrees Celsius, and occurring within 2,000 feet of the land surface.)]

**[690-61-181 CONSTRUCTION OF THERMAL OR HOT WATER WELLS:**

All thermal or hot water wells having a maximum water temperature of less than 250 degrees Fahrenheit (121 degrees Celsius) and constructed to depths of less than 2,000 feet shall be constructed in conformance with rules 690-61-006 through 690-61-176. The bottom-hole temperature shall be measured and recorded on the water well report.]

**65-010 DEFINITIONS:**

- (1) **Bottom Hole Temperature:** The maximum temperature measured in the well or bore hole. It is normally attained directly adjacent to the producing zone, and commonly at or near the bottom of the borehole.
- (2) **Low Temperature Geothermal Effluent:** The outflow, discharge or waste fluid, with its associated dissolved or suspended constituents (being original or introduced), that is produced by a Low Temperature Geothermal Well and its utilization system.
- (3) **Low Temperature Geothermal Fluid:**
  - (a) Any ground water produced from a Low Temperature Geothermal Well which is used for its thermal characteristics; or
  - (b) any other fluids, approved by the Director, that circulate, with or without withdrawal, within a Low Temperature Geothermal Well, where in all cases of (a) and (b) the fluid circulated because of its thermal characteristics, is used for various heating and/or cooling purposes including, but not limited to, residential, commercial, industrial, electrical, agricultural and aquacultural applications.
- (4) **Low Temperature Geothermal Reinjection Well:** Any well as defined under ORS 537.515(7) that is constructed or used for returning Low Temperature Geothermal Effluent to a ground water reservoir.
- (5) **Low Temperature Geothermal Well:** Any well as defined under ORS 537.515(7) with a bottom hole temperature less than 250°F that is constructed or used for the thermal properties of the fluid contained within.
- (6) **Nonstandard Low Temperature Geothermal Effluent Disposal System:** Any Low Temperature Geothermal Effluent Disposal System in which one or more of the following conditions are met:
  - (a) Any portion of the effluent is disposed of in a manner considered non-beneficial by the Director. This includes, but is not limited to, disposal via storm sewer, drainage hole or direct discharge to land surface or a surface water body.

- (b) The effluent contains contaminants, other than heat, that have been added to the Low Temperature Geothermal Fluid.
- (c) The effluent is reinjected to a ground water reservoir that is not considered suitable by the Director. Factors which may render a ground water reservoir unsuitable include, but are not limited to, chemical or physical incompatibility of the fluids involved or adverse hydraulic characteristics of the receiving reservoir.
- (d) There are existing or potential problems or special conditions as determined by the Director. Problems or special conditions resulting from the effluent disposal system which may warrant a nonstandard designation include, but are not limited to, instability of near-surface earth materials, undue alteration of thermal characteristics, unreasonable head changes or downslope subsurface leakage of effluent.

(7) Secondary Use: Consumption of Low Temperature Geothermal Effluent for beneficial use including, but not limited to, domestic, irrigation, stock watering, commercial and industrial uses.

(8) Standard Low Temperature Geothermal Effluent Disposal System: Any Low Temperature Geothermal Effluent Disposal System in which one of the following conditions are met:

- (a) No contaminants except heat have been added to the Low Temperature Geothermal Fluid and the effluent is put to a Secondary Use.
- (b) No contaminants except heat have been added to the Low Temperature Geothermal Fluid and the effluent is returned to the producing or other suitable ground water reservoir and there are no other existing or potential problems or special conditions as determined by the Director including, but not limited to, those factors, problems and conditions listed in 65-010 definition 6, paragraphs c and d.

SUBDIVISION I  
WELL CONSTRUCTION STANDARDS

**65-015 LOW TEMPERATURE GEOTHERMAL WELL AND REINJECTION WELL CONSTRUCTION:** Low Temperature Geothermal Wells and Reinjection Wells shall be constructed in conformance with applicable rules (OAR 690-10-005 to 690-10-040 and 690-60-005 to 690-63-045) with specific additions and modifications as described in OAR 690-65-005 to 690-65-070.

**65-020 LOW TEMPERATURE GEOTHERMAL REINJECTION WELL LOCATION:** For appropriations not exceeding 15,000 gallons per day no Low Temperature Geothermal Reinjection Well shall be located within 75 feet of any existing Low Temperature Geothermal Well utilizing the same ground water reservoir without authorization from the Director, unless both the withdrawal and reinjection wells are on the same parcel of land and are used by the same ground water appropriator. A variance from the 75-foot setback requirement may be issued by the Director, following a written request for special standards (described by 690-60-040) by the water well constructor or landowner, who under the provisions of 537.753, is constructing the well, if hydrologic and thermal conditions permit closer spacing.

For appropriations exceeding 15,000 gallons per day, the appropriator shall submit plans for review to the Director or his authorized representative, indicating separation distances between production and reinjection wells on the parcel of land on which the production well is located, on the parcel of land on which the reinjection well is located, and on all adjoining parcels of land. In addition, the plans shall indicate the anticipated hourly production and reinjection rates, the maximum anticipated daily production, and any planned safeguards against undue thermal and hydrologic interference with existing rights to appropriate ground water and surface water.

**65-025 DESCRIPTION OF PROPOSED USE:** For any Low Temperature Geothermal Well or Low Temperature Geothermal Reinjection Well, the report required under ORS 537.762 prior to commencing construction shall identify the intended use of the well, the appropriator's name and the appropriator's mailing address.

**65-030 IDENTIFICATION OF INTENDED WELL USE:** Any Low Temperature Geothermal Well or Low Temperature Geothermal Reinjection Well shall be clearly identified as such on the water well report filed with the Water Resources Department.

**65-035 WELL-HEAD PROTECTION EQUIPMENT:** Adequate well-head equipment to insure public safety and the protection of the ground water resource shall be immediately installed on any Low Temperature Geothermal Well or Low Temperature Geothermal Reinjection Well when fluid temperatures of 65° C (150°F) or greater are encountered during drilling. Low Temperature Geothermal Fluids produced during drilling or testing of such a well shall be disposed of in such a manner as to minimize health hazards. A variance from the requirement for well-head protection equipment may be granted if a written request demonstrates that the equipment is not necessary to safely complete the well.

**65-040 PUMP TESTING OF LOW TEMPERATURE GEOTHERMAL REINJECTION WELLS:** All Low Temperature Geothermal Reinjection Wells shall be pump tested for a period of at least one hour; results must be recorded on the water well report. This minimum test shall be conducted as follows:

- (1) Prior to testing, the static water level in the well shall be measured and recorded.
- (2) Water shall be pumped into or from the well at a measured and steady rate; the rate shall approximate the maximum anticipated injection rate of the operating well.
- (3) For tests that withdraw water, only bailing or pumping the well is acceptable.
- (4) The water level in the well shall be measured and recorded both at the end of pumping and after one hour of recovery.
- (5) For proposed disposal exceeding 15,000 gallons per day the Director may prescribe a more detailed test that could include, but is not limited to, increased frequency of water level measurement, increased test duration and monitoring of observation wells. Such modifications will be required when possible impacts resulting from the development include, but are not limited to, thermal or hydrologic interference with existing water rights, water quality degradation or failure of well construction.

**65-045 WATER TEMPERATURE MEASUREMENT:** For any Low Temperature Geothermal Well that withdraws ground water, the water well report must include the maximum temperature measured in the borehole and its corresponding depth, and the temperature of the fluid as measured at the discharge point at the beginning and conclusion of a timed production test (i.e. pump or bailer test - air test unacceptable). The maximum temperature measured in the borehole and its corresponding depth is required on the water well report for a Low Temperature Geothermal Well that does not withdraw ground water.

**65-050 ADDITIONAL STANDARDS FOR LOW TEMPERATURE GEOTHERMAL REINJECTION WELLS:** Procedures required to reinject effluent into a Low Temperature Geothermal Reinjection Well must not cause failure of casing and seal material or other components of the well construction.

SUBDIVISION 2

LOW TEMPERATURE GEOTHERMAL EFFLUENT DISPOSAL

**65-055 EFFLUENT DISPOSAL BY REINJECTION / FLUID QUALITY ASSESSMENT:**

Prior to reinjection, users required to file for water rights shall supply the Director fluid quality information concerning the Low Temperature Geothermal Fluid, the Low Temperature Geothermal Effluent, and the ground water in the receiving zone of any Low Temperature Geothermal Reinjection Well for systems that withdraw and reinject ground water in order that the Low Temperature Geothermal Effluent Disposal System be classified as Standard or Nonstandard. The required information shall include a certified chemical analysis for the following parameters: Temperature, pH, Suspended Solids, Specific Conductance, Total Dissolved Solids, Total Coliform Bacteria, Arsenic, Boron, Calcium, Carbonate or Bicarbonate, Chloride, Iron, Magnesium, Manganese, Potassium, Silica, Sodium and Sulfate. If poor water quality or water quality incompatible with the reinjection zone fluids is suspected, the Director may require additional specific data. The Director may waive the requirement for specific portions or all of the chemical analysis if the fluid quality is known to be suitable for the intended withdrawal and reinjection.

SUBDIVISION 3  
WATER RIGHTS PROCEDURE

**65-060 PROCESSING OF APPLICATIONS:** The appropriator shall make application for a water right to appropriate Low Temperature Geothermal Fluid unless an exemption is provided for under ORS 537.545.

**65-065 EXEMPTION FROM WATER RIGHT PERMIT APPLICATION / USE OF LOW TEMPERATURE GEOTHERMAL FLUID:** Low Temperature Geothermal Fluid appropriation for single industrial or commercial use including, but not limited to, electrical, agricultural, aquacultural, heating and/or cooling in an amount not exceeding 5,000 gallons per day shall be exempt from application for a water right as provided for under ORS 537.545. This exemption applies to the use of ground water for any such purpose to the extent that it is beneficial and constitutes a right to appropriate ground water equal to that established by a ground water right certificate.

**65-070 WATER RIGHT LIMITATION FOR NONSTANDARD EFFLUENT DISPOSAL SYSTEMS:** If the Low Temperature Geothermal Effluent is disposed of by way of a Nonstandard Low Temperature Geothermal Effluent Disposal System, the right to appropriate the Low Temperature Geothermal Fluid shall be inferior to all subsequent rights for beneficial consumptive use and/or to the rights of those appropriators who make use of a Standard Low Temperature Geothermal Effluent Disposal System. If a Nonstandard Low Temperature Geothermal Effluent Disposal System is upgraded to a Standard Low Temperature Geothermal Effluent Disposal System the associated water right retains the priority date established upon initial filing.

1699B

**A P P E N D I X E**

**Supplemental data for wells CIP #1 and OIT #5**

are to be filed with the  
**WATER RESOURCES DEPARTMENT.**  
 SALEM, OREGON 97310  
 within 30 days from the date  
 of well completion.

STATE OF OREGON  
 (Please type or print)  
 (Do not write above this line)

State Well No. **38SP9E-17d**  
 State Permit No. \_\_\_\_\_

(1) OWNER: NOTE: Page 1 of 2 pages submitted  
 City of Klamath Falls  
 Address South 5th and Walnut Streets  
 Klamath Falls, Oregon 97601

(10) LOCATION OF WELL:  
 County Klamath Driller's well number  
 NW 1/4 SW 1/4 Section 17 T. 38S R. 9E W.M.

(2) TYPE OF WORK (check):  
 New Well  Deepening  Reconditioning  Abandon   
 If abandonment, describe material and procedure in Item 12.

(3) TYPE OF WELL: (4) PROPOSED USE (check):  
 Rotary  Driven  Domestic  Industrial  Municipal   
 Cable  Jetted  Irrigation  Test Well  Other   
 Dug  Bored

CASING INSTALLED: Threaded  Welded   
 5/8" Diam. from 0 ft. to 776 ft. Gage .250  
 6 5/8" Diam. from 724 ft. to 1500 ft. Gage .250

PERFORATIONS: Perforated?  Yes  No.  
 Type of perforator used Factory  
 Size of perforations 1/8 in. by 3 in.  
 260 perforations from 1020 ft. to 1300 ft.

(7) SCREENS: Well screen installed?  Yes  No  
 Manufacturer's Name \_\_\_\_\_ Model No. \_\_\_\_\_  
 Slot size \_\_\_\_\_ Set from \_\_\_\_\_ ft. to \_\_\_\_\_ ft.  
 Diam. \_\_\_\_\_ Slot size \_\_\_\_\_ Set from \_\_\_\_\_ ft. to \_\_\_\_\_ ft.

(8) WELL TESTS: Drawdown is amount water level is lowered below static level  
 Was a pump test made?  Yes  No If yes, by whom?  
 Yield: gal./min. with ft. drawdown after hrs.  
 \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_  
 Bailer test gal./min. with ft. drawdown after hrs.  
 \_\_\_\_\_  
 \*Tested flow E.P. 71.  
 Temperature of water Depth artesian flow encountered \_\_\_\_\_ ft.

(9) CONSTRUCTION:  
 Well seal—Material used Cement  
 Well sealed from land surface to 21 ft.  
 Diameter of well bore to bottom of seal 16 in.  
 Diameter of well bore below seal 12 in.  
 Number of sacks of cement used in well seal 8 sacks  
 How was cement grout placed? \_\_\_\_\_

Was a drive shoe used?  Yes  No Pile: Size: location \_\_\_\_\_ ft.  
 Do any strata contain unusable water?  Yes  No  
 \_\_\_\_\_ of water? \_\_\_\_\_ depth of strata  
 Method of sealing strata off \_\_\_\_\_  
 Was well gravel packed?  Yes  No Size of gravel: \_\_\_\_\_  
 Gravel placed from \_\_\_\_\_ ft. to \_\_\_\_\_ ft.

(11) WATER LEVEL: Completed well.  
 Depth at which water was first found 295 ft.  
 Static level \_\_\_\_\_ ft. below land surface. Date \_\_\_\_\_  
 Artesian pressure \_\_\_\_\_ lbs. per square inch. Date \_\_\_\_\_

(12) WELL LOG: Diameter of well below casing 0  
 Depth drilled 1500 ft. Depth of completed well 1500 ft.  
 Formation: Describe color, texture, grain size and structure of materials and show thickness and nature of each stratum and aquifer penetrated, with at least one entry for each change of formation. Report each change in position of Static Water Level and indicate principal water-bearing strata.

MATERIAL	From	To	SWL
Light brown chalk	0	66	
Light brown shale	66	123	
Sand rock	123	126	
Hard shale	126	130	
Lava rock	130	142	
Gray clay	142	150	
Rock (gray)	150	162	
Gray clay	162	167	
Gray shale (hard)	167	171	
Broken lava rock -with clay-	191	213	
Brkn. lava with grey & green	213	228	
Black lava-water at 295	228	312	
honeycomb lava	312	376	
Black Lava (hard)	376	410	
Broken Lava	410	435	
Blue Gray Clay	435	539	
Rock (gray)	539	577	
Pinkish Gray Lava	577	585	
Black Lava	585	614	

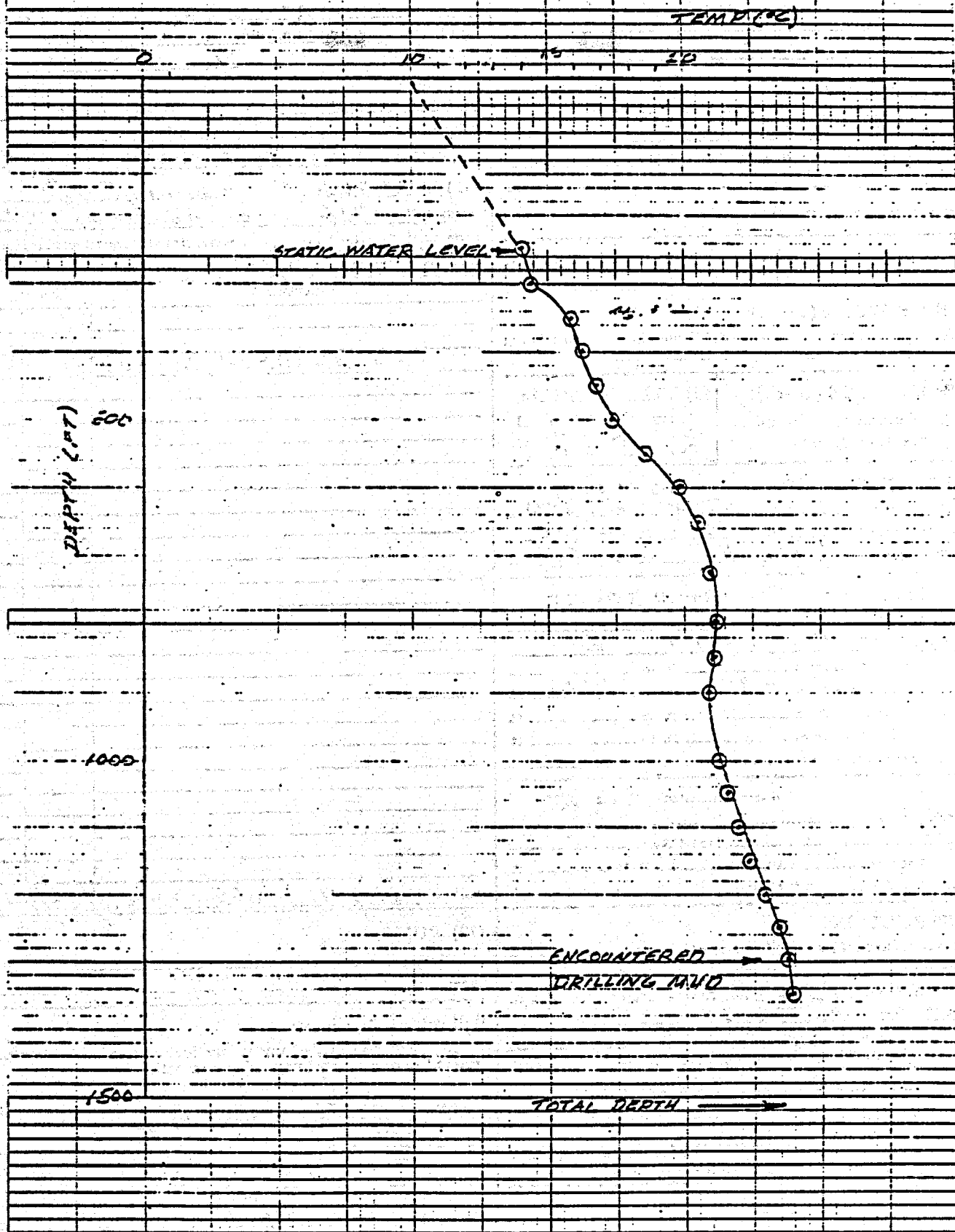
Work started 3/17 18 76 Completed 5/29 19 77  
 Date well drilling machine moved off of well 5/29 19 77

Drilling Machine Operator's Certification:  
 This well was constructed under my direct supervision. Materials used and information reported above are true to my best knowledge and belief.  
 (Signed) Carl D. Embrey Date 6/11 1978  
 (Drilling Machine Operator)  
 Drilling Machine Operator's License No. 342

Water Well Contractor's Certification:  
 This well was drilled under my jurisdiction and this report is true to the best of my knowledge and belief.  
 Name C & C Enloe Well Drilling  
 (Person, firm, or corporation) (Type or print)  
 Address P.O. Box 323 Dorris, CA 95023  
 (Signed) Carl D. Embrey  
 (Water Well Contractor)  
 Contractor's License No. 503 Date 6/11 1978



Figure 5.  
CIP WELL No. 1  
TEMP./DEPTH LOG.







STATE ENGINEER  
Salem, Oregon

State Well No. 13-211-111  
County Clatsop  
Application No. .....

## Well Log

Owner: Oregon Technical Institute Owner's No. # 5

Driller: E. E. Storey Well Drilling Date Drilled 8-23-62

CHARACTER OF MATERIAL	(Feet below surface)		Thickness (feet)
	From	To	
Chalk rock	0	30	30
Brown shale	30	43	13
Yellow clay	43	65	22
Pink lava	65	99	34
Pink shale	99	106	7
Brown lava	106	172	66
Red lava	172	246	74
Gray basalt, W.E.	246	372	126
Blue shale	372	375	3
Gray basalt, W.E.	375	459	84
Brown basalt	459	480	21
Grayish brown basalt	480	495	15
Brown lava	495	515	20
Red tuff rock	515	533	18
Reddish brown rock	533	546	13
Gray basalt	546	559	13
Reddish brown basalt	559	571	12
Gray basalt	571	682	111
Black basalt	682	691	9
Red lava	691	697	6
Gray basalt	697	768	71
Red lava	768	780	12
Gray basalt	780	940	160
Black basalt	940	968	28

PACIFIC NORTHWEST LABORATORY, RICHLAND, WA  
 MATERIALS DEPT., CORROSION RES. AND ENGINEERING

DATE: September 14, 1979

PROJECT: OIT

CONCENTRATION (mg/l) OR OTHER

SAMPLE ID PARAMETER	OIT Heat Exchan- ger #1 9/6/79	OIT Heat Exchan- ger #2 9/6/79	Test Inlet 9/6/79	Test Outlet 9/6/79
pH	8.09	8.08	8.78	8.78
CONDUCTIVITY	246	236	1050	1056
TDS	167	167	798	801
SUSPENDED SOLIDS			<10	<10
TURBIDITY				
HCO <sub>3</sub> <sup>-</sup> (TITRATION)	121	122	37	31
CO <sub>3</sub> <sup>=</sup> (TITRATION)	---			
SO <sub>4</sub> <sup>=</sup> (TURBIDIMETRIC)				
SO <sub>4</sub> <sup>=</sup> (ION CHROM.)	22.5	22.2	378.5	381.9
F <sup>-</sup> (ION CHROM.)			1.4	1.4
Cl <sup>-</sup> (TITRATION)			49.1	49.5
NH <sub>3</sub> (ELECTRODE)	<0.02	<0.02	0.9	1.2
SiO <sub>2</sub> (COLORIMETRIC)			88	88
NO <sub>3</sub> <sup>-</sup> (COLORIMETRIC)				
OTHER: Cl <sup>-</sup> (Ion Chrom.)	3.8	3.2		
PO <sub>4</sub> <sup>=</sup> (Ion Chrom.)	6.2	6.1	0.8	0.3
pH (Field 20°C)			8.9	8.9
O <sub>2</sub> (Field, Added Gas)	>1.0 ppm	>1.0 ppm	0.2 ppm	0.3 ppm
H <sub>2</sub>				
K				

ARGON PLASMA EMISSION SPECTROMETER RESULTS  
 CONCENTRATION (mg/l)

SAMPLE ID PARAMETER	OIT Heat Exchan- ger #1 9/6/79	OIT Heat Exchan- ger #2 9/6/79	Test Inlet 9/6/79	Test Outlet 9/6/79
Al	<0.1	<0.1	0.1	0.1
As	0.05	0.2	0.1	0.15
B	<0.01	<0.01	0.9	1.0
Ba	<0.01	<0.01	<0.01	<0.01
Ca	21.3	21.3	25.0	25.2
Fe	<0.1	<0.1	<0.1	<0.1
K				
Li	<0.05	<0.05	0.15	0.16
g	1.74	1.71	<0.05	<0.05
Na	33	34	224	230
P	<0.1	0.1	<0.1	<0.1
Si	11.8	11.8	38.2	38.7
Sr	0.1	0.1	0.3	0.3

PACIFIC NORTHWEST LABORATORIES, RICHLAND, WA  
 MATERIALS DEPT., CORROSION RES. AND ENGINEERING

DATE: 9/14/79

PLASMA RESULTS

PROJECT: OIT

CONCENTRATION (mg/l)

PARAMETER	SAMPLE ID	OIT Heat Exchanger #1	OIT Heat Exchanger #2	Test Inlet 9/6/79	Test Outlet 9/6/79
Ag		<0.01	<0.01	<0.01	<0.01
Cd		<0.01	<0.01	<0.01	<0.01
Co		<0.01	<0.01	<0.01	<0.01
Cr		<0.01	<0.01	<0.01	<0.01
Cu		<0.01	<0.01	<0.01	<0.01
Mn		<0.01	<0.01	<0.01	<0.01
Mo		<0.5	<0.5	<0.5	<0.5
Ni		<0.01	<0.01	<0.01	<0.01
Pb		<0.1	<0.1	<0.1	<0.1
Sb		<0.05	<0.05	<0.05	<0.05
Se		<0.1	<0.1	<0.1	<0.1
Sn		<0.1	<0.1	<0.1	<0.1
Th		<0.1	<0.1	<0.1	<0.1
Tl		<0.05	<0.05	<0.05	<0.05
Ti		<0.1	<0.1	<0.1	<0.1
U		<0.05	<0.05	<0.05	<0.05
Zn		<0.05	<0.05	<0.05	<0.05
Zr		<0.05	<0.05	<0.05	<0.05

REMARKS: