

1 **Prediction and Demonstration of Periodic Dilation Band Formation in**
2 **Rate-Dependent Porous Cement**

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Abstract

Periodic dilation bands are shown to occur when cylinder-shaped saturated cement specimens are subjected to rapid decompression, with the spacing between the bands scaling as a power law of the decompression time. This behavior is predicted by theory, which is specified to this scenario based on prior work that was more general and applied to compaction. Specifically, the formation of the bands is shown to coincide with periodicity that naturally arises in the effective stress when the material follows a Terzaghi-type consolidation law, although in reverse for dilation and with material deformation described by a rate-dependent viscoplastic law. When strain rate is proportional to effective stress to a power that is greater than one, periodic regions of tensile effective stress arise through the poromechanical fluid-solid coupling. The theoretically-predicted exponent of the resulting power-law relationship between dilation band spacing and unloading time successfully brackets the experimental results, for which it is observed that the exponent is slightly stronger than a square-root relationship (0.28 to 0.67) at testing temperatures of 20 °C and 90 °C. These results demonstrate that rapid-depressurization leads to periodic fracturing that could, on the one hand, be detrimental to the isolation provided by cement used to seal wellbores in the petroleum industry. On the other hand, the dilation bands could also be favorable to production if generated in low-permeability reservoir rocks such as shales that are targeted for petroleum production or granites that are targeted for geothermal energy.

59 1. Introduction

60 Deformation bands that occur in porous geo-materials comprise one important line of
61 evidence for the stress history of the rock. They are mainly categorized into three types of tabular
62 geological structures: shear bands, compaction bands and dilation bands (R. I. Borja, 2004; Du
63 Bernard et al., 2002; Eichhubl et al., 2010; Fossen et al., 2007; Gapais, 1989; Issen & Rudnicki,
64 2000; Wolf et al., 2003). Many of these deformation bands in natural rocks are low-displacement
65 deformation zones of millimeters to centimeters thickness that tend to have different cohesion and
66 permeability compared with ordinary fractures (Fossen et al., 2007). The observable features of
67 these deformation bands can reveal information about the local glacial history, the generation
68 process of soft-sediment deformation or post-burial faulting. They can also significantly alter fluid
69 flow patterns by their impact on the permeability of the geo-materials, in some cases by several
70 orders of magnitude (Antonellini & Aydin, 1994, 1995; Fossen et al., 2007; Jamison & Stearns,
71 1982; Pittman, 1981). As one example, Sigda et al. (1999) found that the deformation bands that
72 occur in the low porosity formations can induce flow channeling which can determine pathways
73 for groundwater flow through the vadose zone. This channelization phenomenon can also be
74 observed during the production of a petroleum reservoir where deformation bands may be forming
75 in low-permeability reservoir rocks during the stimulation phase of reservoir development (Fossen
76 et al., 2007).

77 Given the distinctive characteristics of the deformation bands and information they can
78 provide for geoscientists and engineers, considerable attention has been devoted to understanding
79 the conditions and mechanisms that control their formation. It has been previously concluded that
80 factors such as porosity, mineralogy, and grain size and shape will have some influence on the
81 deformation band formations, but they are mainly controlled by three main factors (Fossen et al.,
82 2007; W. A. Olsson, 1999; Rudnicki & Rice, 1975). These are: 1) geometry (kinematic boundary
83 conditions, layering, etc.), 2) the constitutive response of the material (viscous, elastic, viscoelastic,
84 etc.), and 3) the under-pinning multi-physical coupling and competitions processes.

85 Band width and spacing, which is one of the most readily observable of their features, are
86 a focal point of many studies including recent reviews (Jacquey et al., 2021; Regenauer-Lieb et al.,
87 2013). Based on the classical constitutive modeling approach from elasticity and plasticity, and
88 combining the multi-physical coupling processes with their respective length and time, an

89 overarching feature is the explicit emergence, at various scales, of deformation bands as dissipative
90 patterns from the behavior of the energy equations. The body of previous literature also provides
91 a sound basis to incorporate multiple physical principles such as coupling the laws of
92 thermodynamics, material balances, and constitutive models and, as one might expect, each
93 physical principle brings characteristic length and/or time scales thereby leading to different length
94 and time scales associated with the predicted emergence of deformation bands.

95 One research area focuses on deformation bands associated with fundamentally slow
96 processes, for example examining formations of these patterns in response to tectonic strain rates
97 in the range of 10^{-12} to 10^{-16} s⁻¹ (Jackson & Talbot, 1986; Pfiffner & Ramsay, 1982). However, at
98 the other end of the spectrum in terms of characteristic time scales, deformation bands could also
99 be induced in low-permeability porous geo-materials such as shale and cement from engineering
100 processes which involve rapid pressurization/depressurization. Such rapid processes can come at
101 the beginning of a well's life, for example, from pressure fluctuations induced by reservoir
102 stimulation (i.e. hydraulic fracturing). They can also come at the end of a well's life, for example
103 from the rapid depressurization required as a part of a canonical cement plug test that is often
104 required in order to verify integrity as a part of permanent plugging and abandoning of wells
105 (Khalifeh & Saasen, 2020). Because they involve changes in stresses and/or fluid pressures that
106 are of a similar order to the strength of the rock (order of tens of MPa) over timeframes where pore
107 pressure often cannot drain (seconds to tens of minutes), these engineering processes have
108 potential to induce failure of geomaterials due to coupled poromechanical processes (see e.g.
109 Detournay and Atkinson (2000); Lu and Bungler ; Pearson (1981); Schmitt and Zoback (1989)).

110 With there being a possibility of cement and rock to fail as a result of rapid engineering
111 processes, the tendency of failing geomaterials to form periodic deformation structures is
112 important to understand if such material failure is to be either avoided (i.e. for cement integrity) or
113 pursued (i.e. for reservoir stimulation). Past investigations mainly focus on compaction and shear
114 bands (Baud et al., 2004; Bésuelle, 2001; Okubo & Schultz, 2005; W. Olsson & Holcomb, 2000;
115 W. A. Olsson, 1999; W. A. Olsson et al., 2004; Wong et al., 2001). As for dilation bands, they
116 have been previously observed in highly porous media (Du Bernard et al., 2002). However, little
117 (possibly no) experimental evidence of the formation of dilation bands in low-permeability
118 materials has been observed and reported in the literature. As a consequence, the underlying

119 mechanisms responsible for the formation of dilation bands have not been fully discussed or
120 definitively characterized. The formation of dilation bands may encompass substantial grain
121 rotations and translations, a process bearing resemblance to the formation of compaction and shear
122 bands. Nevertheless, Fossen et al. (2007) have emphasized that tension fractures, stylolites, and/or
123 slip surfaces are more inclined to emerge during the development of dilation bands, particularly in
124 media with very low permeability. Even so, the mechanism(s) of the formation of dilation bands
125 is still a knowledge gap that is addressed here by showing that dilation bands occur in experiments
126 performed in the laboratory on low-permeability, low-porosity cement when it is subjected to rapid
127 reduction in applied stress (decompression). After reviewing a recent theoretical framework for
128 compaction band spacing predictions, this paper extends this theory to the dilation bands family
129 and presents a series of experimental demonstrations. Furthermore, discussions of the implications
130 for practice, especially the methods used for cement plug verification which could damage the
131 integrity they are intended to ensure are also included.

132

133 2. Governing Equations of Periodic Dilation Band Theory

134 A theoretical framework for prediction of deformation bands was recently developed by
135 (E Veveakis & Regenauer-Lieb, 2015) and Jacquy et al. (2021). The prior work is more general
136 at first and then is specified to a form that describes compaction bands under 1D strain conditions.
137 An alternate derivation and summary that is instead targeted to the present interest of dilation
138 bands begin with a simplified one dimensional (1D) problem formulation under the assumption of
139 homogenous deformation where the orientation of the coordinate system (denoted with coordinate
140 z) is selected according to the approach of Issen and Rudnicki (2000) so as to be orthogonal to the
141 anticipated orientation of compaction/dilation bands. Expressing stress equilibrium in the z
142 direction for a material with no body force and under static conditions, as well as stating the
143 definition of Terzaghi's effective stress, gives

$$144 \quad \frac{d\sigma_{zz}}{dz} = 0, \quad (1)$$

$$145 \quad \sigma'_{zz} = \sigma_{zz} - p. \quad (2)$$

146 Here σ_{zz} is the normal total stress acting in the z direction, σ'_{zz} is Terzaghi's effective stress (the
 147 part of the total stress that is bearing on the solid matrix), and p is the pore pressure. Combining
 148 these leads to

$$149 \quad \frac{\partial \sigma'_{zz}}{\partial z} = -\frac{\partial p}{\partial z}, \quad (3)$$

150 Note that compressive stresses are taken to be positive in this sign convention.

151 Next one needs to express mass balances. This begins for a saturated porous medium by
 152 defining the partial densities of solid skeleton phase (ρ_s) and fluid phase (ρ_f). These are defined
 153 as mass of skeleton and mass of fluid over the volume of porous material, respectively. These
 154 relate to the density of each constituent (i.e. the mass of solid over volume of solid and mass of
 155 fluid over volume of fluid) via porosity n , via

$$156 \quad \rho_s = (1-n)\rho_1, \quad (4)$$

$$157 \quad \rho_f = n\rho_2. \quad (5)$$

158 Here we denote the constituent densities as $\rho_i (i=1,2)$, where 1 represents the solid skeleton and 2
 159 represents the fluid. Then in the general case where local mass exchange between the matrix and
 160 the voids (i.e. pores) is allowed, the mass balance equations for the solid and fluid phases are (E
 161 Veveakis & Regenauer-Lieb, 2015)

$$162 \quad \frac{1-n}{\rho_s} \frac{\partial \rho_s}{\partial t} - \frac{\partial n}{\partial t} - \frac{\partial n v_z^s}{\partial x_z} + \frac{\partial v_z^s}{\partial x_z} = 0, \quad (6)$$

$$163 \quad \frac{n}{\rho_f} \frac{\partial \rho_f}{\partial t} + \frac{\partial n}{\partial t} + \frac{\partial n v_z^f}{\partial x_z} = 0, \quad (7)$$

164 In these expressions, v_z^s and v_z^f are the partial velocities of the solid and fluid in the z-direction,
 165 respectively. Furthermore, if the solid and fluid phases are both considered as incompressible, i.e.,
 166 if ρ_s and ρ_f are constant and do not change with time (leading to a time derivative of zero), Eq.6
 167 and Eq. 7 reduce to

168
$$-\frac{\partial n}{\partial t} - \frac{\partial n v_z^s}{\partial z} + \frac{\partial v_z^s}{\partial z} = 0, \quad (8)$$

169
$$\frac{\partial n}{\partial t} + \frac{\partial n v_z^f}{\partial z} = 0. \quad (9)$$

170 By summing Eq.8 and Eq.9, the mass balance equation of the mixture is obtained as

171
$$\frac{\partial n(v_z^s - v_z^f)}{\partial z} + \frac{\partial v_z^s}{\partial z} = 0. \quad (10)$$

172 Next we introduce the classical expression of the Gersevanov filter velocity $n(v_z^f - v_z^s)$ given by
173 (Gersevanov, 1937)

174
$$n(v_z^f - v_z^s) = -\frac{k_\pi}{\mu_f} \frac{\partial p}{\partial z} + \rho g + h.o.t., \quad (11)$$

175 where g is gravitational acceleration, μ_f the viscosity of the fluid ($Pa \cdot s$), and k_π permeability
176 (m^2), which is assumed as constant in this study as commonly done in consolidation theory.
177 Additionally, the higher order terms (h.o.t.) refer to the nonlinear models of the flux. By neglecting
178 the higher order terms as well as the gravitationally-driven part of the flux, the Gersevanov velocity
179 reduces to an expression of Darcy's law, that is

180
$$n(v_z^f - v_z^s) = -\frac{k_\pi}{\mu_f} \frac{\partial p}{\partial z}. \quad (12)$$

181 This expression of Darcy's law will be substituted to the first expression in Eq. (10). To address
182 the second expression in Eq. (10), we recognize that it is giving rate of the normal infinitesimal
183 strain rate in the z -direction ($\dot{\epsilon}_{zz}$), that is

184
$$\dot{\epsilon}_{zz} = -\frac{\partial v_z^s}{\partial z}, \quad (13)$$

185 Here the minus sign is from the compression-positive sign convention. The strain rate can be
186 decomposed into elastic and plastic parts

187
$$\dot{\epsilon}_{zz} = \dot{\epsilon}_{zz}^{(e)} + \dot{\epsilon}_{zz}^{(p)}. \quad (14)$$

188 If some linear relationship between the elastic strain rate and the effective stress is assumed (as in
189 classical consolidation theory), then

190
$$\dot{\varepsilon}_{zz}^{(e)} = a \dot{\sigma}_{zz}' , \quad (15)$$

191 where a is the compressibility. Furthermore, consider the plastic deformation to be described via
 192 a rate-dependent viscoplastic power law of the well-studied form (R. Borja & Kavazanjian, 1985;
 193 Hickman & Gutierrez, 2007; Taylor, 1948)

194
$$\dot{\varepsilon}_{zz}^{(p)} = \text{sgn}(\sigma_{zz}') \eta \left[\frac{|\sigma_{zz}'|}{\sigma_{ref}} \right]^m , \quad (16)$$

195 where η is the creep parameter (in s^{-1}), m is the material sensitivity (R. Borja & Kavazanjian,
 196 1985; Hickman & Gutierrez, 2007), and σ_{ref} a reference stress which depends on the loading
 197 condition used to obtain the creep parameter and material sensitivity. The value selection of these
 198 parameters for this problem will be discussed later. For now, by substitution of Eq. (3), (13), (14),
 199 (15) and (16) into Eq. (12), one can obtain the diffusion equation that considers both elastic and
 200 plastic strain

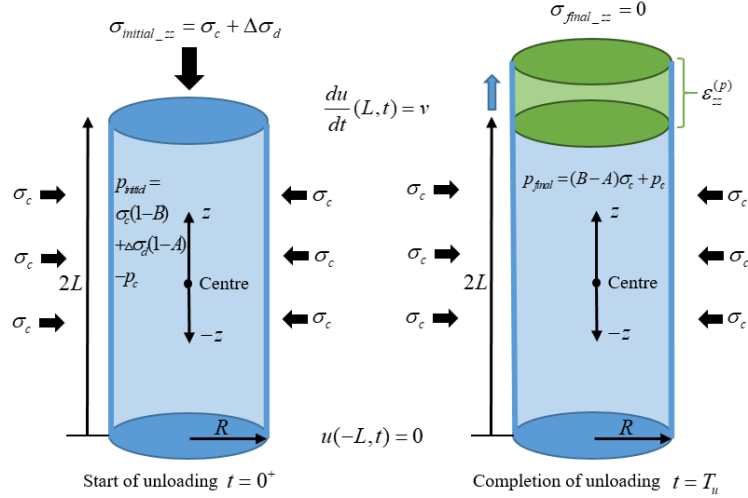
201
$$\frac{\partial^2 S'}{\partial \xi^2} = \frac{a \mu_f L^2}{k_\pi} \frac{\partial S'}{\partial t} + \text{sgn}(S') \frac{\eta \mu_f L^2}{k_\pi \sigma_{ref}} S'^m , \quad (17)$$

202 where $S' = \frac{\sigma_{zz}'}{\sigma_{ref}}$ and $\xi = \frac{z}{L}$. This governing equation for the effective stress can be solved
 203 provided with appropriate initial and boundary conditions along with the necessary inputs of
 204 material properties. Behavior of the solution for cases relevant to the depressurization of a cylinder
 205 is described in the next section.

206

207 4. Behavior of the Model

208 This section presents a range of solution behaviors of the governing equations. Ideally, the
 209 material sensitivity m will be determined from either a creep or relaxation test under the relevant
 210 testing conditions, with each test potentially take up to a week or even more. Unfortunately, due
 211 to constraints and limitations on the available experimental equipment, the material sensitivity m
 212 can not be experimentally determined directly. Thus, some simplifications and specifications will
 213 be made firstly before discussing how the value selections of the material sensitivity m will
 214 influence on the solutions of the model.



215

216 Figure 1. Illustrations of the initial and boundary conditions during dilation in the unloading
217 progress.

218 Figure 1 shows a 1D cylindrical sample that is used for a convenient geometry that
219 illustrates how the unloading can progress to generate the dilation bands. During such axial
220 decompression under the undrained conditions, the plastic part of the strain ($\epsilon_{zz}^{(p)}$) is considered as
221 the primarily contributor to the local fluctuations and pore fluid flow. It is also considered large in
222 magnitude in comparison to the elastic part of the strain ($\epsilon_{zz}^{(e)}$). This perspective is consistent with
223 the consolidation theory which emphasizes the processes that most significantly affect the capacity
224 for fluid storage, namely the changes in porosity. If the majority of porosity alterations are
225 viscoplastic in nature, the system manifests complex behaviors that constitute the focus of this
226 study. So, with this assumption, the immediate response part of the Eq. (17) is dropped here with
227 the idea that the elastic strain will generate very little change in porosity, so that Eq. (17) becomes

228
$$\frac{\partial^2 S'}{\partial \xi^2} = \text{sgn}(S') \frac{\eta \mu_f L^2}{k_\pi \sigma_{ref}} S'^m. \quad (18)$$

229 Now we turn our attention to the initial conditions relevant to the problem of axial
230 decompression. Consider the experimental procedures that include a gradual elevation to the
231 hydrostatic state, then a slow increase in deviatoric stress to the targeted value. Following these
232 steps, the entire system is allowed to stabilize for an additional 20 minutes under these conditions
233 before initiating the decompression process. Hence, before considering the decompression
234 problem, we need to consider the loading stage. In this stage, the sample is loaded by a confining

235 stress σ_c and deviatoric stress $\Delta\sigma_d$. This loading (taken as an increase in the total stress) will be
 236 partitioned between a part that is carried by the pore fluid and a part that is carried by the solid
 237 skeleton, where the former is the pore pressure and the latter is the effective stress (recall Eq. (2)
 238 (Biot, 1941; Terzaghi, 1925). Furthermore, after the immediate response of the pore pressure, there
 239 is a possibility of additional transfer of the total stress from the skeleton to the pore fluid as the
 240 skeleton undergoes some permanent deformation. As reported by Wyrzykowski et al. (2019) and
 241 Tamtsia et al. (2000), a significant portion of creep deformation actually occurs at the very early
 242 stage of creep, almost immediately following the application of deviatoric loading. It is therefor
 243 assumed that a considerable amount of plastic deformation already occurs during the stabilization
 244 time, leading to the induction of creep pore pressure. With these considerations, after applying and
 245 holding the initial loading, the resulting pore pressure is

$$246 \quad p_{load} = B\sigma_c + A\sigma_d + p_c, \quad (19)$$

247 where A is Biot's coefficient of effective stress, B is Skempton's pore pressure coefficient and p_c
 248 is creep-induced pore pressure. The poromechanics-related parameters A and B have been
 249 extensively studied by Detournay and Cheng (1988) and Cheng (1993) and their value ranges have
 250 been reported in the literature (Agofack et al., 2019; Ghabezloo, 2010; Ghabezloo et al., 2008). It
 251 is also worth noting that Detournay and Cheng (1988) also provide an explicit form for Biot's
 252 coefficient of effective stress as a function of Skempton's pore pressure coefficient, with the
 253 Poisson's ratio depending both on drained and undrained conditions. So, after applying the initial
 254 loading, the total stress is

$$255 \quad \sigma_{load_zz} = \sigma_c + \Delta\sigma_d, \quad (20)$$

256 The initial dimensionless effective stress can thus be expressed as

$$257 \quad S'(\xi, 0) = \frac{\sigma_{load_zz} - p_{load}}{\sigma_{ref}} = \frac{\sigma_c(1-B) + \sigma_d(1-A) - p_c}{\sigma_{ref}}. \quad (21)$$

258 During the subsequent unloading (decompression) period, which consists of moving the top platen
 259 at a constant velocity for a travel distance (u) until the entire axial stress is zero after the total
 260 unloading time T_u , the boundary conditions are maintained as

261
$$u(-L, t) = 0, \quad \frac{du}{dt}(L, t) = v_z, \quad (22)$$

262 and the resulting plastic strain rate can be defined as

263
$$\dot{\varepsilon}_{zz}^{(p)} = \frac{\varepsilon_{zz}^{(p)}}{T_u}. \quad (23)$$

264 When the unloading is completed, the total stress in the z -direction returns to zero ($\sigma_{final_zz} = 0$)
 265 and, assuming sufficiently rapid unloading so that undrained conditions apply, the pore pressure
 266 (p_{final}) at this moment is

267
$$p_{unload} = (B - A)\sigma_c + p_c. \quad (24)$$

268 Thus, at the end of unloading, the effective stress is

269
$$\sigma'_{unload_zz} = \sigma_{unload_zz} - p_{unload} = -((B - A)\sigma_c - p_c). \quad (25)$$

270 During this decompression process, the creep parameter η and the reference stress σ_{ref}
 271 can take the value of $\dot{\varepsilon}_{zz}^{(p)}$ and $\sigma_{ref} = (B - A)\sigma_c - p_c$, respectively, as Veveakis et al. (2015) show
 272 that these value selections are valid in the compaction band framework. In the dilation process, the
 273 effective stress boundary condition in the dimensionless form is

274
$$S'(-1) = S'(1) = \frac{\sigma'_{unload_zz}}{\sigma_{ref}} = -1. \quad (26)$$

275 Following sign conventions mentioned above, the negative sign indicates that the effective stress
 276 is tensile which in turn results in $-sgn(S') = -1$. Then, the Eq. (18) becomes

277
$$\frac{\partial^2 S'}{\partial \xi^2} + \zeta S'^m = 0, \quad (27)$$

278 where ζ is a dimensionless unloading rate given by

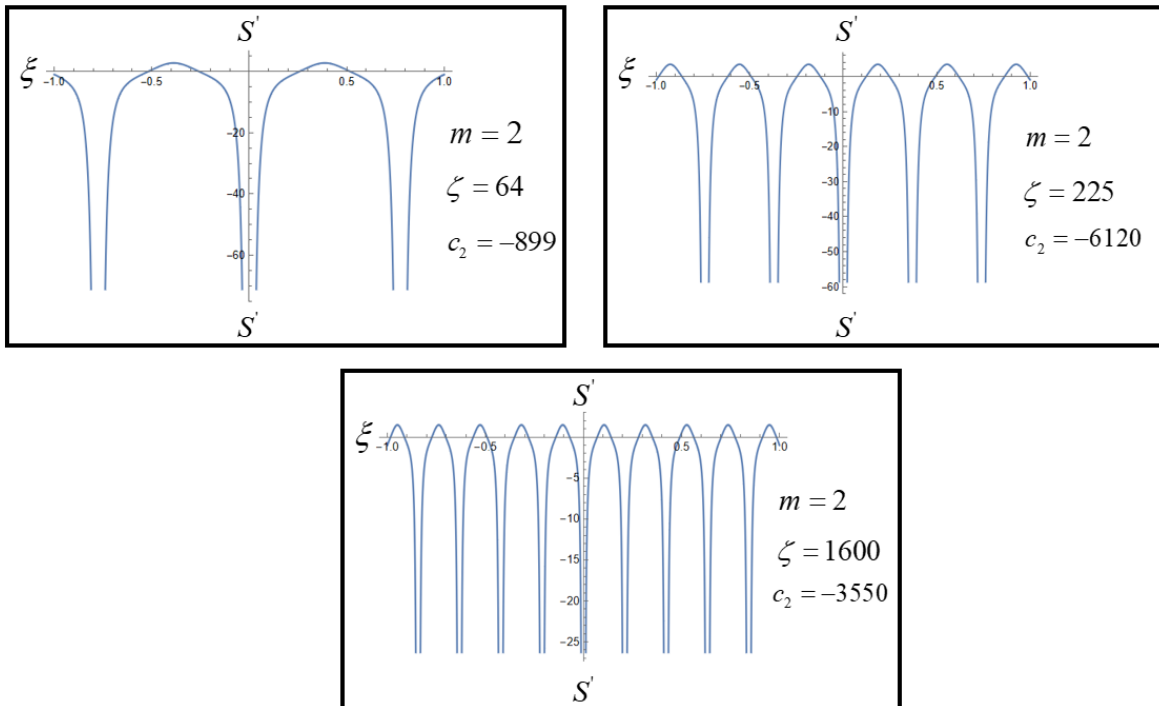
279
$$\zeta = \frac{\varepsilon_{zz}^{(p)}}{T_u} \frac{\mu_f L^2}{k_\pi [(B - A)\sigma_c + p_c]}. \quad (28)$$

280 The solutions of Eq. (27) are dependent on material sensitivity m . For all $m > 1$, the solution
 281 has singularities (Emmanuel Veveakis et al., 2010). The solution, in general, requires numerical

282 methods. However, closed-form analytical solutions exist for $m=2$ and $m=3$ (after Regenauer-Lieb
 283 et al., 2013). The $m=2$ solution has some desirable features (namely, the effective stress is
 284 symmetric rather than antisymmetric about the singularities), and so we focus on the $m=2$ solution
 285 which is expressed as

$$286 \quad S' = \frac{6}{\zeta} \wp(\xi + c_1, 0, c_2), \text{ when } m = 2; \quad (29)$$

287 where $\wp(u, \omega_1, \omega_2)$ is the Weierstrass P function (Abramowitz & Stegun, 1964). Taking the
 288 coefficient $c_1 = 0$ ensures the symmetrical distributions of S' about the center ($\xi = 0$) of the
 289 specimen along the ξ direction. With this symmetry, the boundary conditions from Eq. (26) can
 290 both be satisfied by appropriate selection of c_2 , which is the solution of a transcendental equation
 291 obtained by expression Eq. (29) at $\xi = \pm 1$. The value of c_2 is therefore dependent on the ζ value,
 292 and so must be solved numerically for each given value of ζ . The result is a family of solutions
 293 that depend on ζ , as shown in Figure 2. Clearly the solution is characterized by periodic
 294 singularities, which generate periodically-distributed regions of tensile effective stress and
 295 therefore have the potential to generate periodic tensile fracturing, i.e. “dilation bands”.



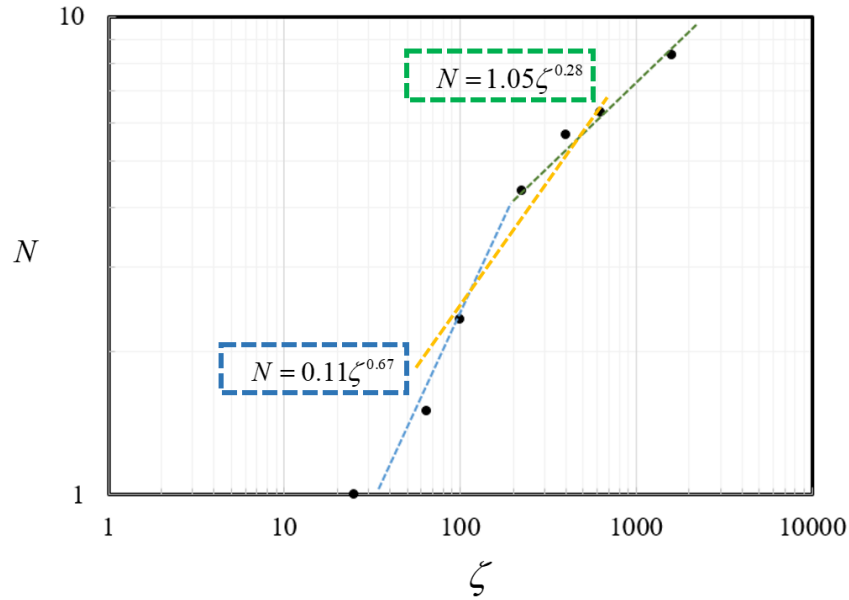
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297 Figure 2. Different solution behaviors of S' when material sensitivity $m = 2$ and ζ is taking
298 different values.

299
300 In the Discussion section that is to follow, we will reflect on the physical nature of the
301 coupling that leads to periodic regions of tensile effective stress. At this point, we will focus on
302 the dependence of the period of the tensile stress regions on ζ with the anticipation that we can
303 vary this quantity in the laboratory by varying the unloading rate and in this sense test the utility
304 of the model for predicting dependence of the dilation band spacing on unloading rate. To this
305 point, when $m = 2$, the number of singularities (N) on the domain $-1 \leq \xi \leq 1$ is scaling as a power
306 law of ζ , with the exponent of the power law taking on different values depending upon the range
307 of ζ (Figure 3). Hence, we describe the number of singularities as

$$308 \quad N = \lambda \zeta^\gamma \quad (30)$$

309 Based on Figure 3, when ζ is in the range from ~ 20 to ~ 200 , the number of singularities (N) is
310 scaling with ζ at an exponent of 0.67. However, when ζ ranges from ~ 200 to ~ 1600 , the number
311 of singularities (N) is scaling with ζ at a smaller exponent of 0.28. Between distinctive power
312 laws lies the region of $100 \leq \zeta \leq 1000$, which is of interest because it is where the number of
313 singularities is between 2 and 7. Hence it is a region where the number could be readily observed
314 in experiments. In this region, the behavior will not strictly follow a power law, however, if one
315 were to fit a power law the exponent would be expected to be around $\frac{1}{2}$, as illustrated by the orange
316 color sketched line with a slope of $N \propto \zeta^{1/2}$.



317

318 Figure 3. When $m = 2$, two different power law relationships occur between the number of
 319 singularities (N) and ζ based on different ranges of ζ . The orange dash line is an illustration
 320 for fitting purpose when the exponent is $1/2$.

321

322 4. Experimental Demonstration

323 4.1 Experimental Setup and Procedures

324 The decompression tests are performed in a temperature-controlled Hoek-type triaxial cell.
 325 This system consists of three main parts: axial loading system, confining stress system, and
 326 temperature-control system (Figure 4). The deviatoric loading is controlled by an INSTRON-
 327 600DX load frame, which can provide up to 600KN. The confining stress is maintained by a high-
 328 pressure syringe pump (ISCO-260D), which is also allowing precise measurement of the volume
 329 change of the specimen associated with a given confining stress up to 70 MPa. The temperature is
 330 provided by wrapping the Hoek cell with the heating tape that can provide a controlled temperature
 331 up to 180°C. The material used in the experiments is Portland cement paste without any aggregate.
 332 The selection of this material was based on its accessibility and the simplicity of casting and its
 333 relevance to the application area of wellbore integrity, owing to the fact that such cement-only
 334 materials types are used to provide the seal between the casing and rock formation in many types
 335 of wells. From the perspective of the mechanical study, it is important to realize that although the
 336 hardened cement paste is usually considered as a quasi-brittle material, it exhibits significant time-

337 dependent behaviors, including creep, which has been reported by Tamtsia and Beaudoin (2000)
338 and Wyrzykowski et al. (2019). According to their research, the material exhibits viscoplasticity,
339 a characteristic derived from the intricate hydro-chemical-thermal coupling processes within the
340 C-S-H of the cement matrix. Furthermore, they found that a considerable part of the viscoplastic
341 deformation takes place at the initial stages of creep, practically instantaneously following the
342 application of deviatoric loading. At the same time, the cement paste is considered to have
343 relatively low tensile strength (2 to 5 MPa) when compared to concrete (Katende et al., 2020; Li
344 & Li, 2015). Therefore, this material aligns well with one of objectives of this study, specifically,
345 to explore formation of dilation bands in decompression laboratory experiments.

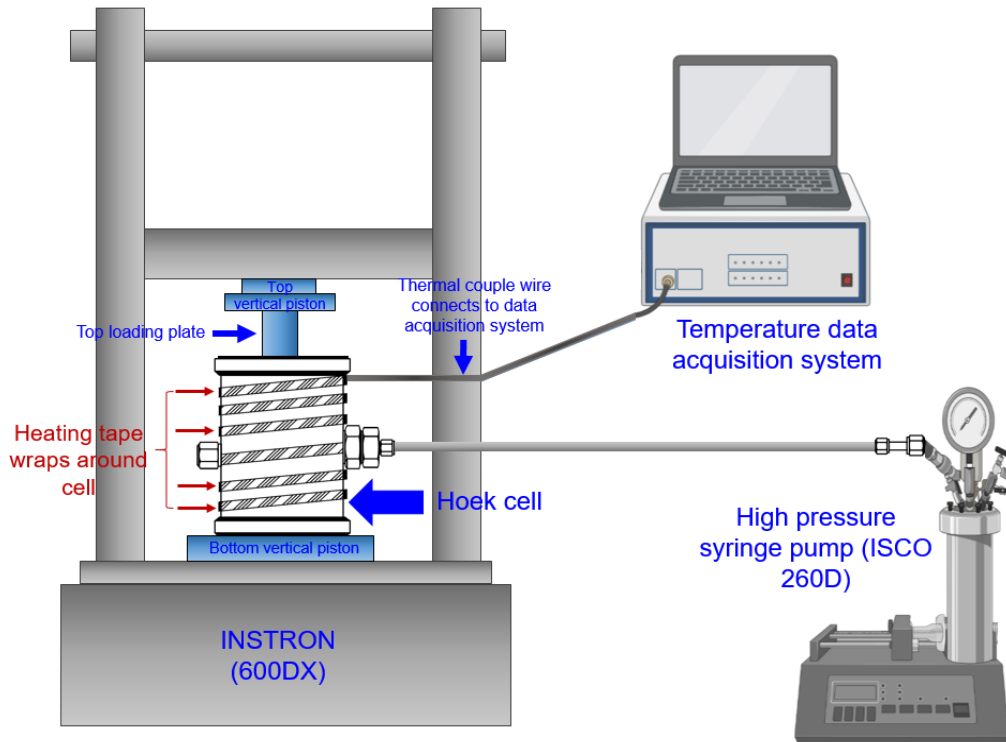
346 In the experimental procedure for performing the decompression tests, the cylindrical
347 specimen (60 mm height; 30 mm diameter) is kept at 100% water moisture content conditions
348 (submerged in water) after demolding. Then it is further subjected vacuum saturation at a pressure
349 of -1 bar for a period of 48 hours before the test. After saturation, the specimen is placed into the
350 membrane, which is specially designed to attach to the cell so that the whole system remains
351 airtight (Figure 5). After the membrane with a specimen inside is placed into the Hoek cell,
352 hydraulic oil is pumped into the space between membrane and cell so the lateral pressure can be
353 provided. The specimen is axially enclosed by two steel loading plates (top-loading plate and
354 bottom spacer). The top vertical piston of the load frame directly contacts the top-loading plate so
355 the deviatoric load can be applied. By using heating tape wrapped around the Hoek cell, the
356 temperature of the cell is slowly increased to the targeted temperature, i.e., 90°C, over two to three
357 hours while maintaining a low hydrostatic (both confining and axial) applied stress of 1.4 MPa.
358 Note that by requiring hours for the heating process, it is possible to drive some evaporation of
359 pore fluid so that the specimen can potentially deviate from fully saturated conditions. However,
360 a slow heating phase is required to avoid the pressurization of the pore fluid due to fast thermal
361 loading. The airtight seal between membrane and specimen and the tight contact between the steel
362 platens and the ends of the specimen is relied upon to prevent evaporation, which is one motivation
363 to perform the heating under a hydrostatic applied stress.

364 After the desired system temperature is achieved and stabilized, the confining pressure and
365 vertical load are increased to the targeted pressure value of 13.7 MPa, so the specimen is initially

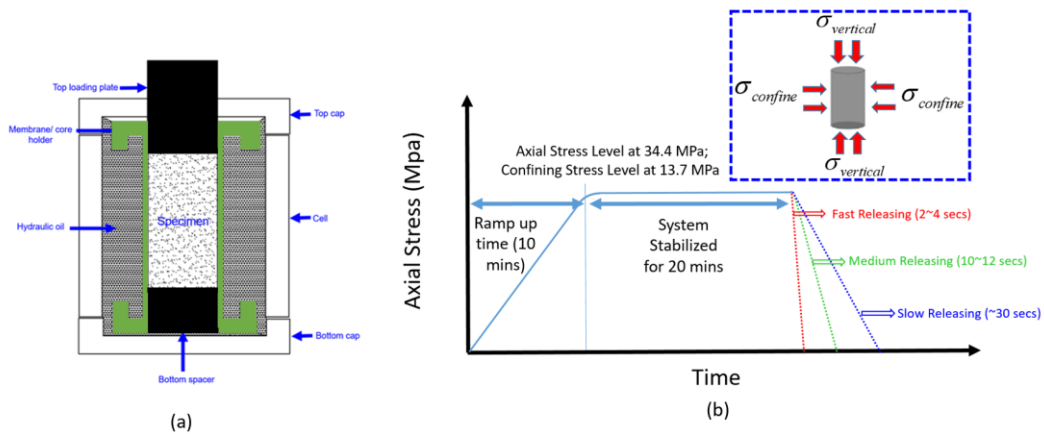
366 loaded isotropically. Next, the deviatoric load is increased to the targeted value of 20.7 MPa and
367 held for a short period of time (around 15 minutes) to make sure the system is stabilized.

368 After this holding period (akin to the initial loading described in the previous section), the
369 the axial stress is released by prescribing a set rate of motion of the actuator that is responsible for
370 applying the axial load. During this time, the axial load is completely released, and the radial load
371 is still maintained at the value of the confining stress. The hydraulic properties of such cement
372 pastes have been extensively studied from the literature (Banthia & Mindess, 1989; Goto & Roy,
373 1981; Nyame & Illston, 1981). Considering the reported permeability range of ($1e-15 - 1e-18 \text{ m}^2$),
374 the diffusion time could span from a few days to several months. In contrast, both the loading time
375 and the slowest unloading time, at around 30 seconds, are very brief in comparison to the diffusion
376 time. Consequently, the transient diffusion process is disregarded in the interpretation of these
377 experiments; the material is effectively undrained.

378 This comprises the experiment as far as the mechanisms of interest are concerned. Hence,
379 the final unloading of the radial stress is done very slowly, after which the specimen is removed
380 for inspection and measurement. These are done both visually and by scanning by a North Star
381 Imaging M-5000 Computed Tomography (CT) scanner with Feinfocus FXE source at 185 kV and
382 200 mA.



383
 384 Figure 4. Experimental setup of the decompression tests, includes the axial loading system,
 385 confining stress system, and temperature-control system.

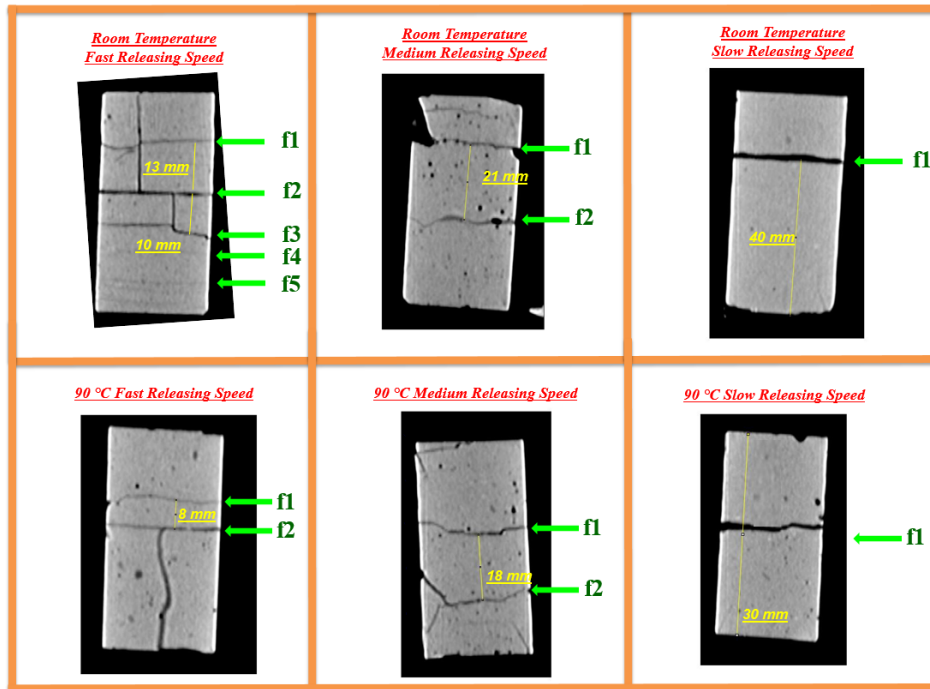


386
 387 Figure 5. (a) The internal structure of the Hoek-cell; (b) Specimen with its boundary conditions
 388 and the loading and unloading paths.

389
 390 4.2 Experimental Results

391 The observed fractures (the locations are indicated as green arrows) and their spacing from
 392 the experiment are shown in Figure 6. Note that there are two types of fractures that are observed.

393 Taking the fast release at room temperature (top left corner) as an example, fractures (f1, f2, and
 394 f3) are penetrating-through fractures which are completely separating the whole cross section of
 395 the sample. These fractures can be observed by visual inspection. However, the internal fractures
 396 (f4 and f5) are not visible by inspection of the outer surfaces of the specimen, but they are visible
 397 in the CT scans. The spacing between these experimental observed fractures and the corresponding
 398 unloading time to generate these fractures are summarized in Table 1.



399
 400 Figure 6. Measurement of tensile bands spacing occurs during decompression test at room
 401 temperature and 90 °C. The location of each observed fractures is indicated and labeled by an
 402 arrow on the right side.

403
 404 Table 1. Unloading time and experimental measured spacing of each specimen in Figure 7

Testing Temperature	Unloading Time (s)	Experimental Measured Spacing (mm)
Room Temperature	3 (fast)	11.5
Room Temperature	11 (medium)	21
Room Temperature	30 (slow)	40
90°C	3 (fast)	8
90°C	11 (medium)	18
90°C	30 (slow)	30

405

406

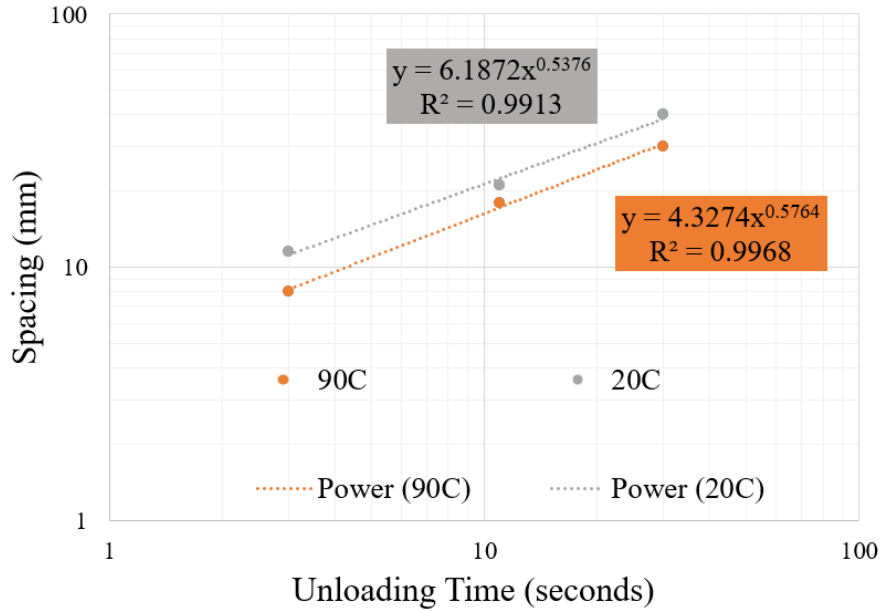
407 4. Discussion

408 Knowing the sample length ($2L$) and the predicted number of singularities, a relationship
409 for the spacing between singularities (fractures) h and the unloading time (T_U) is obtained as

$$410 \quad h = \frac{2L}{N} = \frac{2L}{\lambda} \left[\frac{\varepsilon_{zz}^{(p)}}{T_u} \frac{\mu L^2}{k_\pi [(B-A)\sigma_c + p_c]} \right]^{-\gamma} \propto T_u^\gamma. \quad (31)$$

411 As discussed in the last section, the exponent γ is dependent on the parameters of m and ζ . Its
412 range is from 0.28 to 0.67 when $m=2$. As shown in Figure 7, the power law relationship between
413 the experimental observed spacing and the unloading time is plotted in log-log space, the exponent
414 at high temperature (90°C) is found as 0.5764 with $R^2 = 0.9968$ and the exponent at room
415 temperature (20°C) is found as 0.5376 with $R^2=0.9913$. These reported values are falling into the
416 reasonable ranges of exponents when $m=2$. This is especially true when considering that the total
417 number of observed dilation bands is consistent with ζ being in the range of 100 to 1000. For $m=2$,
418 if one had only three of the model predictions across this limited range, inspection of Figure 2
419 shows that the apparent exponent of a power law would be near 0.5. Hence, these experimental
420 results give support to the scaling relationships between the numbers of tensile effective stress
421 regions and ζ based on the theoretical framework of periodic dilation band formation in rate-
422 dependent porous materials.

423



424

425 Figure 7. Power law relationship between unloading time and spacing from the experimental
 426 observation is summarized in Table 1.

427

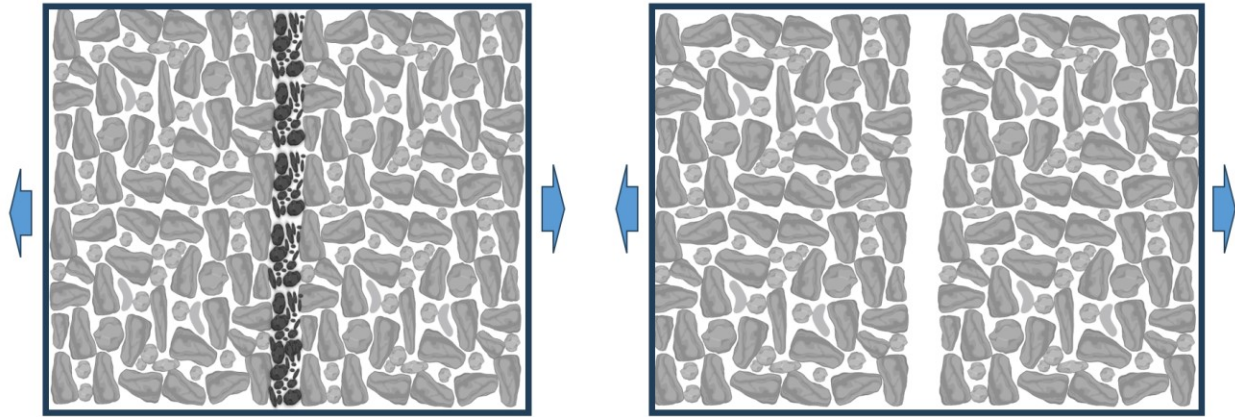
428 Post-experiment inspection of the fracture surfaces shows no evidence of crushing, pore
 429 collapse, or sliding. Hence it is consistent with a tensile mechanism and stands in contrast with
 430 prior work on deformation bands observing different modes that show evidence of grain translation,
 431 shear displacement, or even crushing in poorly consolidated sand and sandstone (Du Bernard et
 432 al., 2002). It is therefore worth reflecting on the coupled mechanisms that lead to the formation of
 433 these dilation bands (DBs).

434 Beginning from the moment the axial load begins to be removed from the specimen, the
 435 internal coupling becomes complex as the movement of the fluid (following diffusion law) and
 436 movement of the solid (viscoplastic law) interplay. Because the starting point is the conservation
 437 of both solid and fluid phases, the movement of these phases must be counter to one another. That
 438 is, as the solid deforms and thus moves in a certain direction (dilation direction), mass balance
 439 requires there to be a counterflow of fluid. So, if we examine what happens in the moments leading
 440 up to the sample being completely unloaded in the axial direction, there is a dilation process
 441 relative to the loaded state. Because of the pore pressure generated during the loading that is then
 442 unable to be dissipated on the rapid timescale of the unloading, the effective stress becomes

443 negative (tensile) everywhere. By the viscoplastic law, the volumetric strain rate has the same sign
444 as the effective stress, which means the sample is volumetrically dilating.

445 If this dilation is initially uniform, consider what would happen if there is a small
446 perturbation locally within the specimen to become even smaller (more tensile). By the
447 viscoplastic law, the dilational strain rate will become even greater at that location. This dilational
448 strain amounts to a movement of the solid phase away from the location of the perturbation. But,
449 throughout this process, the total stress must remain constant in order to satisfy equilibrium, which
450 means that the reduction of the solid phase at that location will transfer more of the load to the
451 fluid phase. This will elevate the pore pressure at the location of the perturbation, thus leading the
452 effective stress to become even more negative. This leads to a further acceleration of the local
453 strain (dilation) and an additional transfer of load to the pore fluid, raising the pore pressure and
454 thus further decreasing effective stress in order to maintain constant total stress. In this way, a local
455 perturbation can grow unstably unless it is somehow mitigated by the rate at which fluid pressure
456 can equilibrate between the local perturbation with high pore pressure and the “far-field” relative
457 to the perturbation with low pore pressure. In this regard, the dilation bands can be seen as a form
458 of material instability existing in a coupled poromechanical system, eventually leading to a
459 separation of the solid phase into a dilation band (as illustrated in Figure 8).

460 With this in mind, the solution to the coupled governing equations is therefore indicating
461 the natural spacing that is predicted to arise as some perturbations grow and others are suppressed,
462 with growth and suppression representing a competition between the rate of solid deformation and
463 the rate at which diffusion can dissipate accumulating pore pressure. This competition is indeed
464 embodied in the form of the dimensionless strain rate, ζ , which is directly proportional to the solid
465 strain rate and inversely proportional to the fluid diffusivity (permeability over fluid viscosity).
466 When this ratio is small (higher diffusivity and/or lower strain rate), the fluid diffusion can take
467 place at a larger scale and so the spacing between tensile regions is larger. When this ratio is higher
468 (smaller diffusivity and/or higher strain rate), fluid diffusion can only occur at a smaller scale and
469 so the spacing between tensile regions resulting from this coupled process becomes smaller.



470
 471 Figure 8. Left: Compaction band surface is characterized by grain translation, displacement, or
 472 even crushing in poorly consolidated sand and sandstone; Right: dilation band surface is
 473 characterized by planar opening-mode fractures with two discrete and neat surfaces in the low
 474 permeable porous materials.
 475

476 The previously-reported compaction bands are proposed to work in the opposite way.
 477 During the compaction process, where compressive total stress is applied and held as constant
 478 throughout, the effective stress is positive (compressive) everywhere in the sample. As the same
 479 viscoplastic law states, the compaction rate will become faster if there is a perturbation that causes
 480 the effective stress to be larger, which will reduce pore pressure in order to keep a constant total
 481 stress. This will in turn lead to an even larger effective stress on the solid phase and will once again
 482 reduce the pore pressure as the results of maintaining the constant total stress. This shedding
 483 process from the pore fluid to the skeleton will make the solid phase take more stress which will
 484 eventually lead to the rearrangements (translation, displacement, or even crushing) of the grains,
 485 which have been reported in the literature to be physical characteristics resulting from the
 486 formation of compaction bands (Du Bernard et al., 2002, Fossen et al., 2007, Olsson et al., 2000).

487 We further note that the effects of temperature are still not very clear from the current
 488 available data. This arises primarily because the impact of temperature on the system is complex,
 489 affecting numerous variables including the material sensitivity parameter m , fluid viscosity u_f ,
 490 permeability k , Biot's coefficient of effective stress A , and Skempton's pore pressure coefficient B .
 491 These latter four parameters are found in both the numerator and denominator of Eq. (28), where

492 they may either cancel out or interact in multifaceted ways. Consequently, the exact influence of
493 temperature on the system remains unclear so far.

494 The discoveries and results from the present work can be applied as a guide to several
495 engineering processes and applications. Taking hydraulic fracturing treatments for example, at the
496 end of each stage there is an abrupt pressure drop associated with the end of pumping. In some
497 cases, there is immediate flowback, while in others there could be another opportunity for rapid
498 decompression when the well is finally flowed back at the end of a multi-stage treatment at the
499 transition to production. Our results show that such rapid depressurization at the end of pumping
500 can induce tensile dilation bands in the well cement sheath and thus can jeopardize the well
501 integrity. On the other hand, if rapid depressurization is translated into the reservoir and occurs in
502 the vicinity of the hydraulic fractures, there could be a generation of secondary fractures as dilation
503 bands with could impact fluid flow and contribute positively to production.

504 Another scenario that could be leading to the occurrence of dilation bands is the process of
505 plug verification during well plug and abandonment. Currently, none of the industrial codes
506 delineates the appropriate loading/unloading rates for the hydraulic pressure tests which are used
507 to verify the zonal isolation functionalities of permanent cement plugs. Because the cement is low
508 permeability and likely rate-sensitive (recall the material used in this study is wellbore cement), it
509 is possible that the load/unload cycle of pressure used for plug verification could, somewhat
510 ironically, be causing damage in the form of induced dilation bands. In fact, the “better” the cement
511 is, in the sense of having lower permeability the more susceptible it will be to dilation band
512 formation and the associated damage to its material properties. This points to a material design
513 tradeoff, guided by Eq. (31), wherein cement plug material should be low enough permeability to
514 ensure zonal isolation, but high enough permeability so that the risk of damage due to dilation
515 band formation is at an acceptable level in light of the anticipated loading/unloading rates to which
516 it will be subjected.

517

518 5. Conclusions

519 A theoretical framework capturing the coupling between fluid diffusion and viscoplastic
520 solid deformation predicts that periodic regions of tensile effective stress could form as a result of
521 rapid depressurization of a low-permeability, rate-dependent porous material. This solution for the

522 effective stress S' is dependent on the material rate-sensitivity exponent m and a dimensionless
523 strain rate ζ , where the latter embodies a comparison between the viscoplastic solid deformation
524 rate and the diffusive dissipation of pore pressure. Mathematical solutions point to power law
525 scaling relationships between the number of tensile zones and the dimensionless strain rate, where
526 a higher dimensionless strain rate (i.e. higher actual strain rate and/or smaller diffusivity of the
527 porous material) results in more closely-spaced regions of tensile stress. More specifically, there
528 are at least two distinct zones with different power laws, one for smaller values of ζ and another
529 for larger values, with the transition occurring when this quantity is in the hundreds.

530 As a proof of concept to the dilation bands predicted by theory, their emergence is observed
531 in experiments on fully-saturated cylindrical cement samples during the decompression tests with
532 different releasing speeds. The experimental observations show that a faster unloading time will
533 cause a higher number of (i.e. a smaller spacing between) the periodic tensile fractures. The
534 exponent of the power law relationship between unloading time and dilation band spacing from
535 the experimental observations is found to fall into ranges consistent with theoretical predictions.

536 Based on this initial level of consistency between theory and experiment, the observed
537 formation of the periodic dilation bands is inferred to occur when the strain rate is following a
538 viscoplastic law and fluid flow is following Darcy's law. To satisfy mass balance, solid
539 deformation must induce a counterflow of fluid. At the same time, whenever fluid pressure
540 increases, it takes more of the load and so to maintain constant total stress, more load must be
541 shifted away from the skeleton, thereby further decreasing the effective stress (making it more
542 tensile). This positive feedback is proposed to be the underlying mechanism of dilation band
543 formation that leads to the observation of planar opening-mode fractures with two discrete and
544 neat surfaces in the low permeable porous materials with strong rate dependency.

545 The mechanism of dilation band formation could be responsible for periodic tensile
546 fracturing in a variety of low-permeability, rate dependent porous materials such as wellbore
547 cement and clay-rich rocks. Focusing on engineering timescales this work shows both the potential
548 pitfalls of dilation band formation that could lead to the reduction of zonal isolation provided by
549 wellbore cement as well as possible benefits if dilation bands form near hydraulic fractures in
550 reservoir rocks and increase the ability of oil and gas to flow to a well. The coupling that leads to
551 these behaviors furthermore comprises a new way to see porous materials, even at an intuitive

552 level, as being prone to localization of dilation in a manner that is spontaneous and regular in its
553 behavior.

554

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562

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