

# Hydrokinetic turbines in yawed conditions: towards synergistic fluvial installations

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## ABSTRACT

Laboratory experiments were performed in critical mobility conditions to study the effects of an in-stream horizontal axis turbine in yawed conditions. The misalignment between the rotor axis and the incoming flow velocity is observed to alter the near and far wake of the turbine, as well as the scour and deposition patterns in the proximity of the monopile support tower. Various hydraulic conditions may lead to such misalignment: as a result of complex fluvial bathymetries distorting the flow, or as a strategy to steer the wake away from downstream units and maximize energy production in a turbine array. The research first investigates the simplest case, with a single turbine deployed along the channel centerline and oriented at different yaw angles, to study the wake deflection and the turbine performance. A second set of experiments is performed moving the turbine relatively close to a non-erodible lateral wall to explore a potential passive yaw control strategy devoted to protect the banks from erosion by steering the

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wake towards the channel center and favor deposition along the bank.

**Keywords:** In-stream Turbines, Yaw, Renewable energy, Streambank protection, River, Erosion, Sediment

## INTRODUCTION

1 In the last decade feasibility studies and resource assessments for the deployment  
2 of Marine and Hydrokinetic (MHK) devices in tidal, estuarine, or fluvial environments  
3 have increased significantly (Haas et al. 2011; Jacobson et al. 2011; Baldwin et al.  
4 2015; Muljadi and Yu 2015; Bane et al. 2017; National Renewable Energy Laboratory  
5 (NREL) ). It is therefore very important to understand the long term effects that these  
6 devices may have on the ecological and morphodynamic equilibrium of the river, as  
7 well as the more immediate effects that changes in fluvial bathymetry may have on  
8 the safety and performance of the device (Neill et al. 2009; Yang and Copping 2017;  
9 Fraser et al. 2017; Nash and Phoenix 2017; Ramírez-Mendoza et al. 2018). Most of  
10 the experiments and numerical studies, so far, have focused mainly on characterizing  
11 the turbulence and the wake structure (Myers and Bahaj 2007; Maganga et al. 2010;  
12 Kang et al. 2012; Churchfield et al. 2013; Lam et al. 2015; Bachant and Wosnik 2015a;  
13 Morandi et al. 2016; Lust et al. 2018), the turbine response to turbulent flow conditions  
14 (Neary et al. 2013; Chamorro et al. 2013; Stallard et al. 2013; Ahmadi 2019), and the  
15 power production capabilities of these devices (Myers and Bahaj 2006; Bahaj et al.  
16 2007; Bachant and Wosnik 2015b; Strom et al. 2017). More recently, the geomorphic  
17 effects of the turbine on the erodible riverbeds have been investigated, characterizing  
18 the scour and deposits forming downstream of the turbine. Hill et al. (2014) quantified  
19 experimentally (later on simulated numerically by Yang et al. (2017)) the interactions  
20 between hydrokinetic turbines and the erodible sediment layer in an open channel flow,  
21 estimating the scour augmentation due to the spinning turbine rotor as opposed to the  
22 typical bridge pier scour occurring at the base of the monopile foundation tower (see

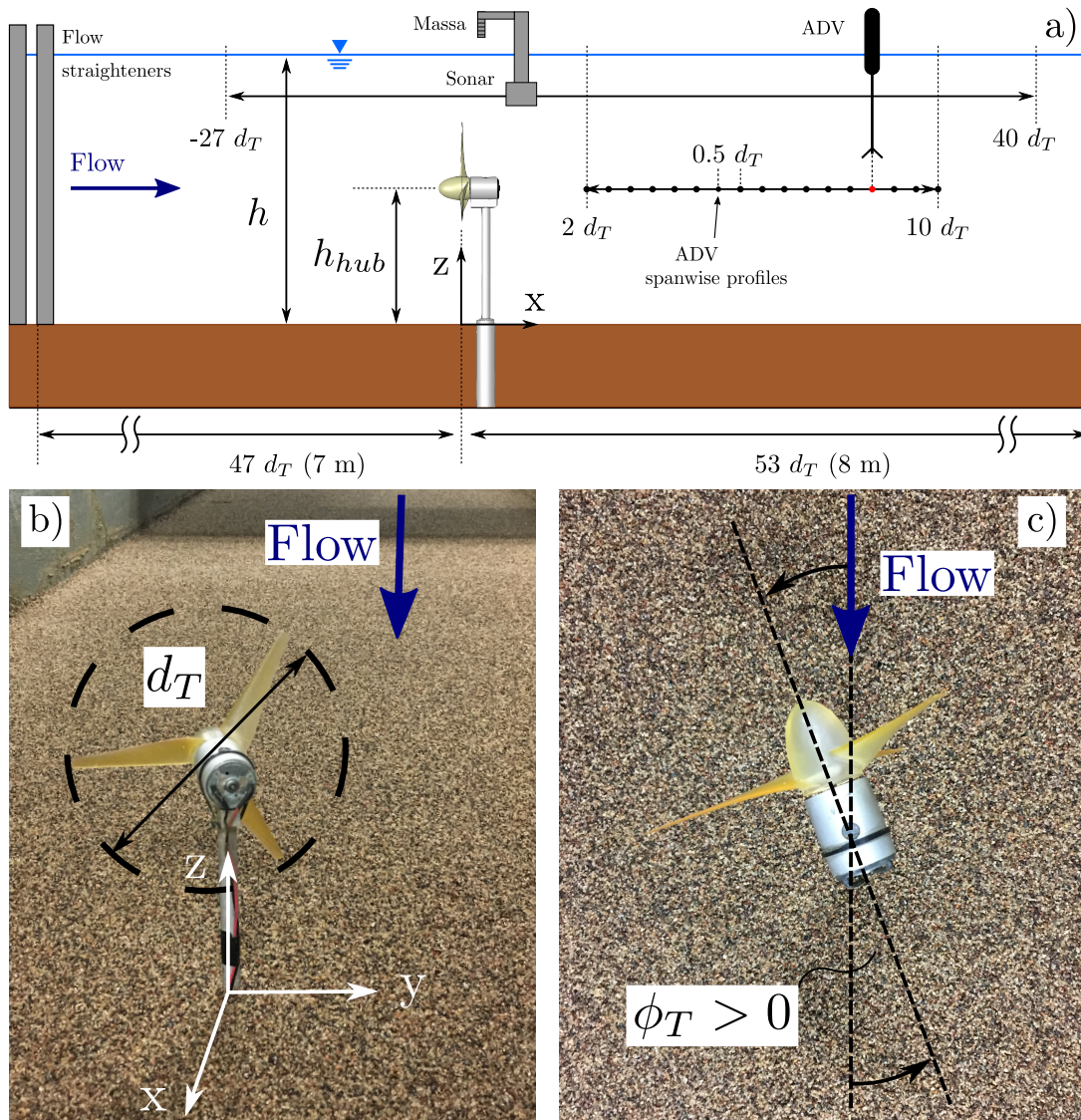
23 also Chen et al. (2017)). Later, the mutual interaction between the turbines and mi-  
24 grating dunes was identified in a straight channel flow (Hill et al. 2016b) and in a  
25 meandering stream (Hill et al. 2016a). The major outcomes are a weak performance  
26 modulation due to the shear layers developing over the dunes and impinging the rotor  
27 (Hill et al. 2016b), and a slight wake distortion affecting the secondary flow in the me-  
28 andering channel and altering the transverse slope (Hill et al. 2016a). The latter work  
29 provided the inspiration to investigate how MHK rotor misalignment with respect to  
30 the incoming flow can be used to steer the wake at minimal performance cost. The  
31 potential goals are very interesting from a streambank erosion protection perspective:  
32 the near wake is delimited by the tip vortex structure and the corresponding annular  
33 shear layer, which represents a highly turbulent and coherent region extending up to  
34 three-five diameters downstream of the rotor ( $3-5 d_T$ ), see for example Kang et al.  
35 (2014). As the wake expands and this flow region interacts with the erodible riverbed  
36 directly, by impinging tip vortices, or indirectly, by accelerating the flow underneath,  
37 augmented localized erosion occurs near the tower foundation (Musa et al. 2018a). The  
38 same erosive mechanism may occur at the stream banks, in case MHK turbines are de-  
39 ployed along the outer meanders thalweg to harvest more kinetic energy (Hill et al.  
40 2016a). An opportunity to mitigate streambank erosion is to employ a yawed rotor  
41 configuration, potentially steering the flow towards the center of the channel (simi-  
42 larly to other traditional protection strategies (Odgaard and Mosconi 1987; Odgaard  
43 and Wang 1991a; Shields Jr et al. 2003; Abad et al. 2008; Khosronejad et al. 2013,  
44 among others)). Controlling the wake of axial flow turbines is a well known strategy  
45 to maximize wind power performance at the wind farm scale by enhancing mixing  
46 and high momentum fluid entertainment in the wakes (Gebraad et al. 2017; Boersma  
47 et al. 2017). More specifically, a minimal yaw angle misalignment allows steering  
48 the upwind turbine wake away from the downwind rotors, providing the incoming  
49 flow direction is steady. For wind energy in the atmospheric surface layer this is a

50 challenging practice, requiring real-time control (Boersma et al. 2017); however in flu-  
51 vial environments, the stream direction is fairly invariant, implying that small angles  
52 can be accurately imposed without continuous yaw adjustments. Several studies have  
53 been focused on the performance of yawed wind turbine and their wake characteris-  
54 tics with particular emphasis on the streamwise location of the velocity minima (Grant  
55 et al. 1997; Medici and Alfredsson 2006; Bastankhah and Porté-Agel 2016; Dou et al.  
56 2019a, among other). Recently, this investigation was extended to hydrokinetic tur-  
57 bines (Galloway et al. 2014; Frost et al. 2015; Piano et al. 2017; Modali et al. 2018),  
58 without however taking into account the erodible riverbed. In an effort to promote the  
59 integration of this technology to fluvial environments, this study explores how the yaw  
60 angle can steer the wake flow, the scour region and the deposit in the turbine wake. This  
61 is investigated experimentally using miniature MHK turbine models deployed both at  
62 the center of the channel and near the bank. A potential configuration of interest for  
63 the protection of the banks at minimal performance cost is presented and discussed as  
64 proof of concept, in broad terms addressing issues of renewable energy production and  
65 streambank erosion.

66 In the next section the reference system and main variables are defined, and the  
67 experimental apparatus is described. Thereafter, results are presented from two sets of  
68 experiments: i) with the turbine installed in the middle of the channel cross-section at  
69 different yaw angles, ii) with the turbine deployed at one rotor diameter from the non  
70 erodible side wall. Discussion and conclusions follow.

## 71 **EXPERIMENTAL APPARATUS AND BASELINE FLOW CONDITIONS**

72 All the experiments were conducted at the St. Anthony Falls Laboratory, University  
73 of Minnesota. A 20 m long, 0.9 m wide tilting flume was used, filled with a 0.2 m thick  
74 layer of fairly uniform sand of median diameter  $d_{50} = 1.13$  mm ( $d_{16} = 0.81$  mm,  $d_{84} =$   
75  $1.44$  mm) and geometric standard deviation of  $\sigma_g = 1.33$  (estimated using Parker ())



**FIG. 1. a) Profile schematic of the experimental set-up, showing inflow boundary conditions, turbine location and part of the measurements range. b) Miniature turbine model (rotor diameter  $d_T = 0.15$  m and hub height  $h_{hub} = 0.13$  m), and c) turbine orientation reference system.**

76 eBook, Chapter 2).

77 Flow conditions were defined in order to keep sediments at the threshold of motion,

78 also defined as critical mobility or clear water, while maintaining the MHK turbine sub-  
79 merged by about half the rotor diameter consistent with Hill et al. (2014). The water  
80 discharge was regulated by a calibrated actuating valve, providing continuous supply  
81 of water drawn directly from the adjacent Mississippi River. In order to break the  
82 large flow structures created by the valve and provide reasonable turbulent intensities  
83 at the inlet, the upstream flow is forced through an array of 18 closely spaced vertical  
84 cylinders (0.05 m diameter and net spacing), resulting in a fairly spanwise homoge-  
85 neous flow before entering the test section. Figure 1a shows a schematic view of the  
86 experimental setup.

### 87 **Turbine model**

88 The miniature turbine model used in all experiments is the three blades, axial-  
89 flow, MHK model tested by Hill et al. (2014) with rotor diameter  $d_T = 0.15$  m, hub  
90 height with respect to the undisturbed bed of  $h_{hub} = 0.13$  m and  $0^\circ$  pitch angle at the  
91 blade tip (see more details in Hill et al. (2014)). The rotor hub and blades were 3D  
92 printed and connected through a shaft to a DC motor (model #RK- 370CA-14420), en-  
93 abling high frequency acquisition of the voltage at 200 Hz (Measurement Computing  
94 MiniLAB 1008 USB data acquisition board) as a measure of the turbine instantaneous  
95 angular velocity under frictional torque (Howard et al. 2015; Hill et al. 2016b). As  
96 demonstrated in Musa et al. (2018b), the voltage signal produced by the three internal  
97 rotating copper coils of the DC motor (used here in reverse), is linearly proportional to  
98 the rotor angular velocity. The miniature turbine employed here is designed to provide  
99 a geometrically scaled representation of a high performance, 0.5 m diameter, turbine  
100 (Chamorro et al. 2013) and a reliable streamwise evolution of the scaled wake velocity  
101 deficit (Musa et al. 2018b). For the first set of experiments, the turbine model was  
102 positioned 7 m downstream of the channel inlet ( $x = 0$  coordinate,  $x$  being the stream-  
103 wise direction) in order to have the test section area sufficiently far from the boundary

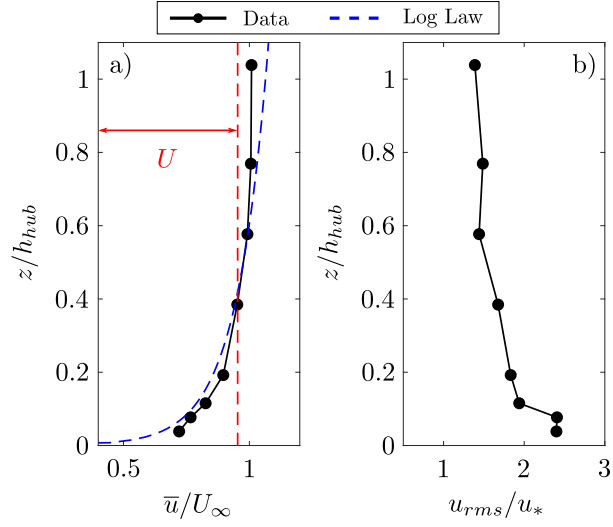
104 conditions and thus characterized by an incoming uniform flow, and centered in the  
105 channel cross-section ( $y = 0$  coordinate,  $y$  being the spanwise direction), as shown in  
106 Figure 1. For the second set of experiments, the  $x$  position was kept invariant, while  
107 the turbine was moved closer to the wall  $y = 0.3$  m, thus keeping a minimum distance  
108 of  $0.5 d_T$  between the rotor tip and the sidewall. The rotor orientation, identified by the  
109 yaw angle  $\phi_T$  was varied from  $-30^\circ$  to  $30^\circ$ , every  $10^\circ$  (see Figure 1c for the reference  
110 system), and it is indicated hereafter with a white line on the topographic scan plots.  
111 In selected cases, the rotor was changed in order to compare the effects of clockwise  
112 or counterclockwise blade rotations (from the flow prospective, looking downstream).

### 113 **Data collection**

114 Flow velocity, turbine generated voltage, bed and water surface elevations were  
115 collected in time, during all the performed experiments. All the devices, except for  
116 the voltage acquisition, were mounted on a computer-controlled data acquisition cart  
117 (DAQ), able to move along the  $x, y$  axes of the flume, and monitor the spatial and  
118 temporal evolution of the erodible bed and water surface elevations. Longitudinal bed  
119 elevation profiles were measured continuously in space and time using an immersible  
120 Olympus Panametrics C305-SU sonar transducer. Measurements were acquired along  
121 the x-axis ( $-27 \leq x/d_T \leq 40$ ), over the turbine spanwise location, coincident with the  
122 channel centerline  $y = 0$ . Measurements were spaced by  $\Delta x = 10$  mm in the stream-  
123 wise direction (spatial resolution) and each line repeated every  $\Delta t = 46$  s (temporal  
124 resolution), for the entire duration of the experiment. The water surface was captured  
125 simultaneously by a Massa M5000 ultrasonic range sensor with the same resolution.  
126 Experiments were conducted for approximately 4-4.5 hours, which was sufficient for  
127 the scour depth and the associated deposit peak to reach the equilibrium. Figure 3b  
128 shows the time evolution of the bed elevation at  $x/d_T = 0.5$  and  $y/d_T = 0$  (the first  
129 measurable location by the sonar, moving downstream from the rotor plane), revealing

130 a plateau after  $t \approx 130$  min. At the end of each experiment the channel was carefully  
131 drained and a high resolution laser topography scan was performed using state-of-the-  
132 art, high-resolution, laser scanning device designed and manufactured at SAFL. The  
133 scanning system is composed by a SICK Ranger E50 camera, an OPTO 1W continuous  
134 laser and a non-cylindrical uniform line lens. The camera captures the spanwise ( $y$ )  
135 bed elevation profile illuminated by the 1 mm thick laser. An ad-hoc designed calibra-  
136 tion procedure converts the outline pixels into the physical coordinates of bed elevation  
137  $z(y)$ . The camera captures these profiles at consecutive streamwise locations ( $x$ ) when  
138 triggered by the moving DAQ carriage, at a user-specified  $\Delta x$ , constructing the final  
139 bathymetry matrix  $z(x, y)$ . The laser measuring system has a vertical accuracy of 0.5  
140 mm, while the spatial resolution was imposed to  $\Delta x = \Delta y = 2$  mm. Figure 3a shows  
141 the final bathymetry for the reference centered case  $\phi_T = 0^\circ$ .

142 Instantaneous flow velocities were measured using a 3 components Acoustic Doppler  
143 Velocimeter (ADV), downlooking probe, sampled at 200 Hz and positioned by the au-  
144 tomated cart system. The velocity profiles were collected i) along the vertical profile  
145  $z$ -axis at different locations to contribute defining the shear velocity and the bottom  
146 shear stress in the undisturbed baseline conditions, 1 m upstream of the turbine, and  
147 ii) along the cross-section coordinate  $y$  at hub height  $z = z_{hub}$  to monitor the wake ex-  
148 pansion, deflection, and the maximum velocity deficit. The wake measurements were  
149 collected along 12 spanwise transects, moving the ADV along the  $y$ -axis from -0.225  
150 m to 0.225 m, at a velocity of  $1 \text{ mm s}^{-1}$  (see Hill et al. (2016a) for details). The lon-  
151 gitudinal spacing of the streamwise wake profiles was  $0.5 d_T$  in the near wake (from  
152  $2 d_T$  to  $4.5 d_T$ ) and  $1 d_T$  in the far wake (from  $5 d_T$  to  $10 d_T$ ). An example of wake  
153 measurements for the centered turbine with  $0^\circ$  yaw angle are shown in Fig. 3c.



**FIG. 2. a) Incoming undisturbed mean velocity profile  $\bar{u}(z)$  normalized by the velocity at hub height  $U_\infty = 0.42 \text{ ms}^{-1}$ .  $U = 0.40 \text{ ms}^{-1}$  represents the depth averaged velocity. b) Streamwise turbulent intensity  $u_{rms}/u_*$ , where  $u_{rms}$  is the root-mean-square of the streamwise velocity fluctuations and  $u_*$  is the shear velocity estimated with the energy method as  $u_* = \sqrt{gR_H S_w} = 0.022 \text{ ms}^{-1}$ .**

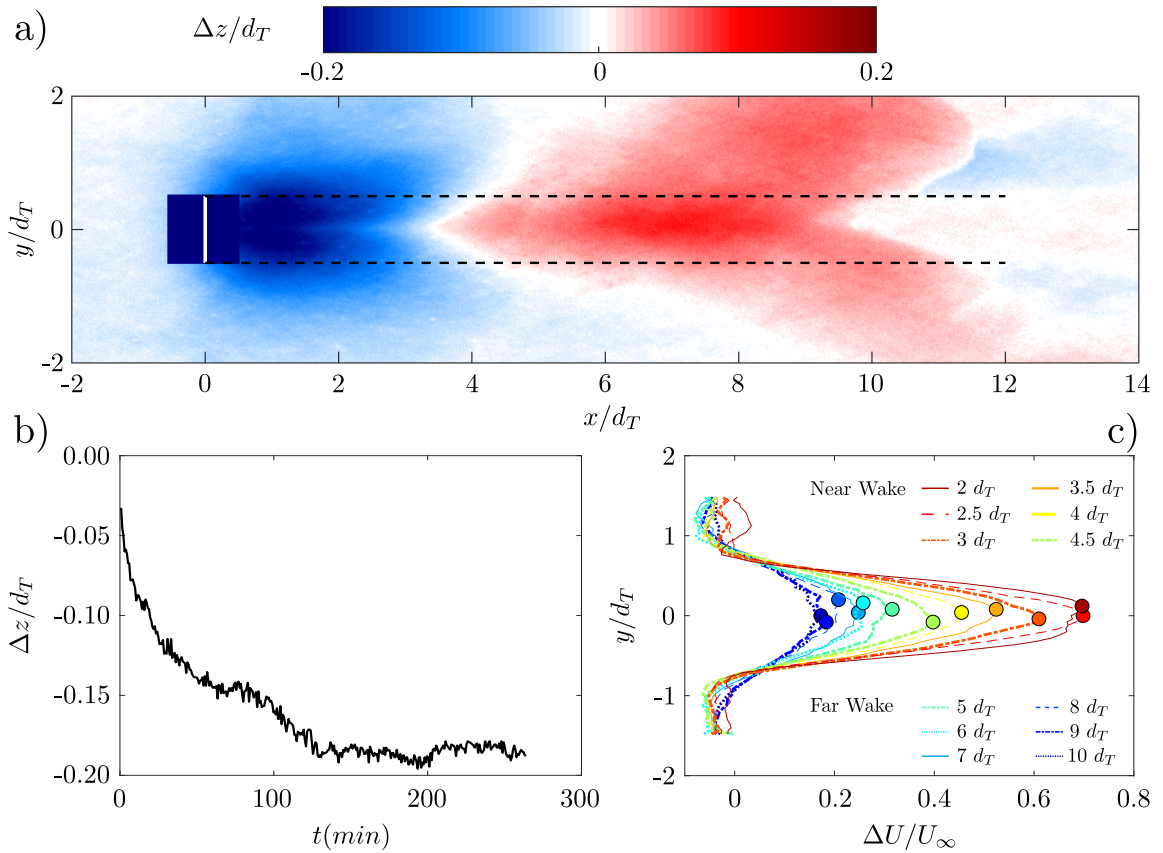
#### 154 **Open channel hydraulic conditions**

155 The flume hydraulic conditions were set to mimic subcritical open channel flows in  
 156 lowland alluvial systems. The water depth  $h = 0.27 \text{ m}$  was controlled by a downstream  
 157 tail gate; the channel slope and flow discharge were adjusted to provide a streamwise  
 158 uniform flow, and keep the shear stress at the bed slightly below the critical mobility  
 159 conditions. Hydraulic parameters and key variables are reported in Table 1.

160 Preliminary experiments were conducted without the MHK turbine to measure the  
 161 mean velocity profile and characterize the undisturbed baseline flow. Measurements at  
 162 9 points along  $z$ , vertical direction, for 5 minutes, sampled at 200 Hz, were collected  
 163 in the center of the channel, 1 m upstream of the turbine selected location. Figure

164 2a shows the vertical profile of the mean streamwise velocity  $\bar{u}(z)$ , where  $U_\infty$  is the  
165 incoming velocity at hub height and  $U$  is the depth averaged velocity. Figure 2b pro-  
166 vides the streamwise turbulent intensity  $u_{rms}/u_*$ , where  $u_{rms}$  is the root-mean-square  
167 of the streamwise velocity fluctuations normalized by the shear velocity  $u_*$ . This latter  
168 was estimated with the Energy-gradient method for uniform flows  $u_* = \sqrt{gR_H S_w}$ ,  
169 where  $R_H$  is the hydraulic radius,  $g$  is the gravitational acceleration ( $9.81 \text{ ms}^{-2}$ ), and  
170  $S_w$  is the water surface slope measured using the Massa sensor. The shear veloc-  
171 ity resulting from the energy gradient was observed to vary between 0.018 and 0.025  
172  $\text{m s}^{-1}$  in all the experiments performed (leading to a mean value  $u_* = 0.022 \text{ m s}^{-1}$ ).  
173 This estimate compared reasonably well with  $u_* = 0.015 \text{ m s}^{-1}$  obtained with the  
174 Clauser method, i.e. by fitting a logarithmic velocity distribution  $\frac{u}{u_*} = \frac{1}{k} \ln\left(\frac{z}{z_0}\right)$  where  
175  $k = 0.41$  is the Von Karman constant, and  $z_0$  is the estimated aerodynamic rough-  
176 ness length (Clauser 1956). However given the limited number of data points in the  
177 logarithmic layer, we decided to impose  $u_* = 0.022 \text{ m s}^{-1}$  calculated from the en-  
178 ergy method, and use the logarithmic law to estimate the roughness length, leading to  
179  $z_0 = 3 \times 10^{-5} \text{ m}$ . The estimated  $z_0$  ultimately suggests a sand equivalent roughness  
180  $k_s \approx 0.9 \times 10^{-3} \text{ m}$  ( $z_0 = 0.033 k_s$ , see Jiménez (2004)) very similar to our median  
181 grain size  $d_{50} = 1.13 \times 10^{-3} \text{ m}$ .

182 Following the procedure proposed by van Rijn (1984) (based on the original work  
183 by Shields (1936)), the Shields diagram provides the critical mobility parameter  $\theta_{cr} =$   
184 0.034 associated with our median grain diameter  $d_{50}$ . With the above estimated  $u_*$ , the  
185 Shields parameter for the grain mobility is calculated as  $\theta = u_*^2 / [(s - 1)gd_{50}] = 0.027$ ,  
186 where  $s$  is the sand to water density ratio  $s = \rho_s / \rho_w = 2.65$ . The clear water conditions  
187 are therefore confirmed as  $\theta < \theta_{cr}$ . We also calculated the critical flow velocity accord-  
188 ing to Melville and Sutherland (1988). Specifically,  $U_c = u_{*c} 5.75 \log(5.53 \frac{h}{d_{50}})$ , where  
189  $h$  is the flow depth and  $u_{*c}$  the critical shear velocity  $u_{*c} = \sqrt{\frac{\tau_{cr}}{\rho_w}} = 0.025 \text{ m s}^{-1}$ . The  
190 resulting critical flow velocity  $U_c = 0.45 \text{ m s}^{-1}$  is higher than our mean flow velocity



**FIG. 3. Experimental procedure: a) bathymetry  $z(x, y)$  at equilibrium conditions, b) temporal evolution of the local scour to assess equilibrium conditions, and c) mean velocity profile in the turbine wake. Data refer to the reference case with the turbine centered in the channel cross-section,  $\phi_T = 0^\circ$  and clockwise blade rotation. Note that the white line in panel a) and in the following figures refers to the rotor location and orientation, while the rectangle surrounding the rotor plane is a region where the laser was sheltered by the turbine nacelle, preventing the measurements.**

191  $U = 0.40 \text{ m s}^{-1}$ , thus confirming the clear-water condition. Furthermore, note that  
192 the value of the critical shear velocity  $u_{*c}$  obtained following Melville (1997) leads to  
193  $u_{*c} = 0.027 \text{ m s}^{-1}$ , similar to the above estimate. Note that clear water conditions  
194 were also monitored during the experiment by visually inspecting the bed. At the be-  
195 ginning of each experiment the bed was flattened and the ADV was positioned for 10  
196 min in the center of the channel 1 m upstream of the turbine to monitor the averaged  
197 velocity at hub height, as a boundary condition. Meanwhile, bed and water surface  
198 elevation streamwise profiles were repeatedly measured to monitor the depth and the  
199 corresponding slopes.

## 200 **RESULTS**

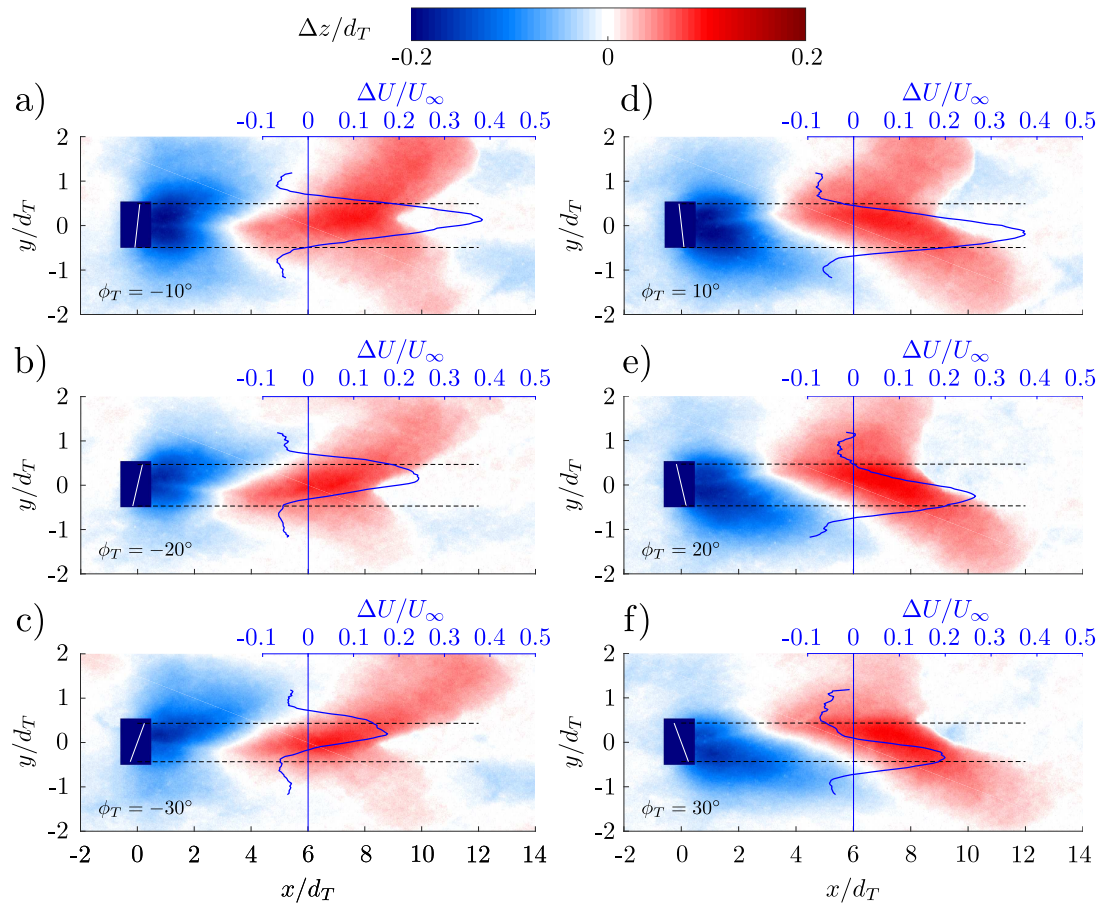
### 201 **Centered turbine**

202 The first set of experiments was performed with the turbine placed in the center of  
203 the channel,  $y = 0$ , focusing on the effect of the yaw angle on the local bathymetry  
204 and on the spatial evolution of the turbine wake (Figure 4). The reference, centered,  
205 turbine experiment is performed at  $\phi_T = 0^\circ$ , and clockwise blade rotation, as tested by  
206 Hill et al. (2014) and shown in Figure 3.

207 In the reference conditions, the scour region extends approximately  $3 d_T$  in the  
208 longitudinal downstream direction and  $2 d_T$  in the spanwise direction, symmetrically  
209 with respect to the turbine location (Figure 3a). This local geomorphic signature is  
210 qualitatively similar to the bridge pier scour, but augmented due to the additional shear  
211 stress induced by the rotor. The phenomenology of the scour mechanism is amply  
212 discussed in the predictive modeling work by Musa et al. (2018a). As a results of the  
213 critical mobility conditions, sediments eroded from the scour region were transported  
214 and deposited downstream in a chevron shaped dune, extending up to  $11 d_T - 12 d_T$ .  
215 The maximum scour and deposit were estimated as  $-0.23$  and  $0.10$  times the rotor di-  
216 ameter  $d_T$  respectively. The temporal convergence of the scour-deposition process is

217 shown in Figure 3b where the bed evolution at  $x/d_T = 0.7$  and  $y/d_T = 0$  is plotted  
218 in time. Both the scour and the deposit, were observed to develop significantly in the  
219 first 60 min and reach a stable value around  $t \approx 130$  min, which is significantly shorter  
220 than the duration of all the experiments. We acknowledge we have not always moni-  
221 tored the scour evolution in time and assessed the equilibrium conditions; first, all the  
222 experiments were performed under the same flow conditions, implying that the tem-  
223 poral evolution of scour-deposition processes should be similar; second, as the scour  
224 depth decreases with increasing yaw angle (no matter which direction), the equilib-  
225 rium time  $t = 130$  min estimated for the maximum scour configuration is regarded as  
226 a conservative estimate.

227 Using the mean velocity component along the  $x$ -axis,  $U(x)$ , the corresponding  
228 deficit value  $\Delta U(x) = U_\infty - U(x)$  was calculated, where  $U_\infty$  is the, undisturbed,  
229 spanwise averaged velocity at hub height upstream of the turbine, also defined as hub  
230 velocity. In Figure 3c, the dimensionless deficit profiles  $\Delta U/U_\infty$ , are plotted as a  
231 function of  $x, y$ , with the location of the maximum velocity deficit (or minimum ve-  
232 locity) marked as a symbol and consistently located in the proximity of the centerline.  
233 Note that this is not significantly affected by the direction of rotation of the rotor (not  
234 shown). To provide a visual of the yaw angle effects on both the local scour-deposition  
235 features and the wake, the spanwise profiles of  $\Delta U/U_\infty$  at  $6 d_T$  downstream of the  
236 turbine are plotted in Figure 4 and superimposed on the final bathymetry for all the  
237 yaw angles investigated. The wake orientation is steered in the opposite direction of  
238 the rotor yaw. Specifically, as the turbine is negatively yawed (panels a,b,c), the wake  
239 is observed to steer in the opposite direction with the velocity minima shifting towards  
240 the left side of the channel (top side of the plot), and vice versa (panels d,e,f). This find-  
241 ing agrees well with all the existing wake models for yawed wind turbines (Jiménez  
242 et al. 2010; Bastankhah and Porté-Agel 2016; Dou et al. 2019a). The wake distortion  
243 is attributed to the coexistence of a drag force acting on the flow in the  $x$ -direction and



**FIG. 4. Span wise profile of the mean velocity deficit (blue line and blue top axis) at  $6 d_T$  downstream of the turbine, superimposed on the final bathymetry, for each yaw angle; from top to down panels  $\phi_T = -10^\circ, -20^\circ, -30^\circ$  (a,b,c) and  $10^\circ, 20^\circ, 30^\circ$  (d,e,f).**

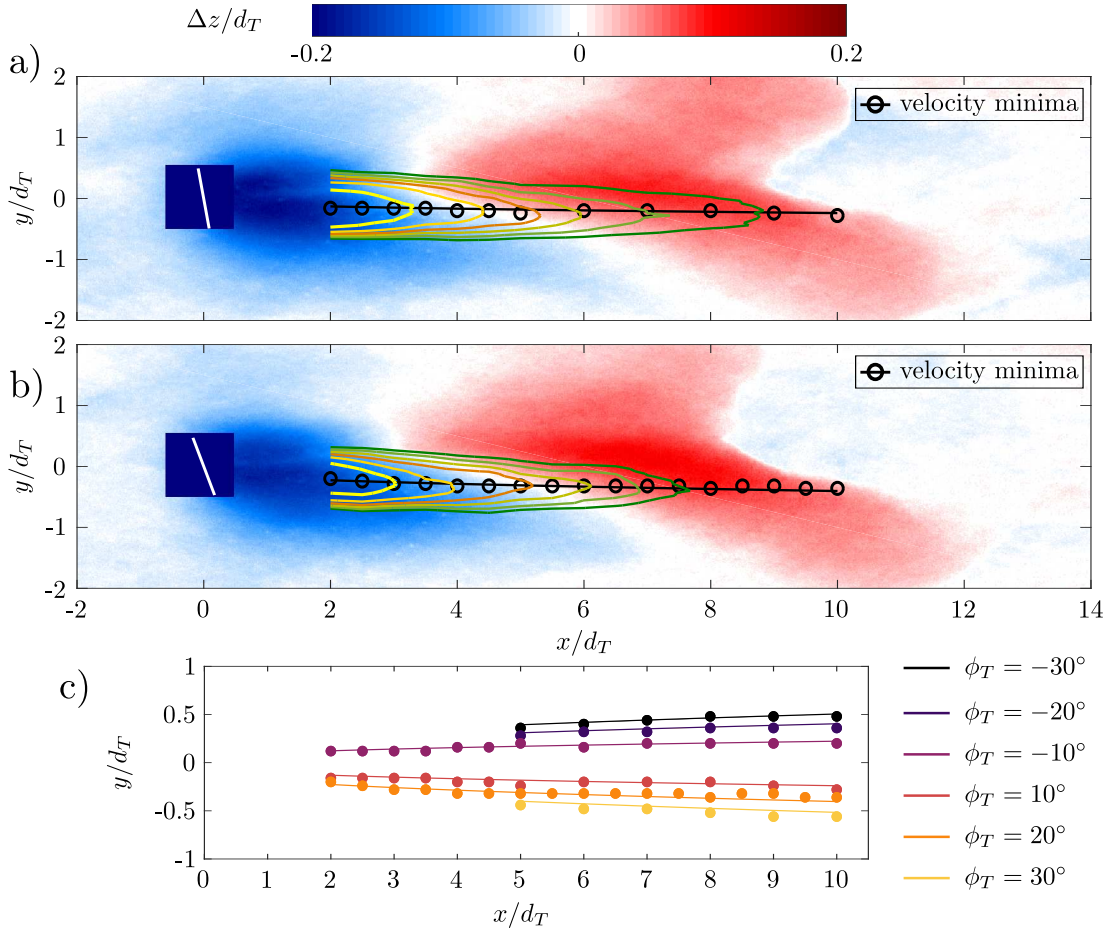
244 a thrust component applied in the skew direction of the rotor plane, which acts on the  
 245 flow passing through the rotor and distort the wake. However, it is quite surprising that  
 246 the scour seems to follow closely the rotor orientation with the crest edge, along which  
 247 sediments are eroded and transported downstream, perpendicular to the yawed rotor  
 248 plane. This suggests that the wake flow at hub height, is counteracting the near-wake  
 249 hydrodynamics close to the bed surface and the support tower (or vice versa). For the

250 negatively yawed cases for instance, the sediments that are ejected towards the right  
 251 hand side of the channel (the lower boundary in Figure 4) start forming a deposit, at  
 252 about  $4d_T$ , that is initially skewed with respect to the tower, but then tilts progressively  
 253 towards the wake minima, as the velocity slows down and sediments stop moving.

254 The spatial evolution of the wake is further highlighted in Figure 5a-b where the  
 255 velocity minima clearly indicate a significant distortion of the wake. In Figure 5c the  
 256 spatial evolution of those points is plotted against the theoretical prediction of Dou  
 257 et al. (2019a), suggesting that axial flow MHK turbines behave in the same way of  
 258 axial flow wind turbines. According to the simplified model by Dou et al. (2019a),  
 259 tested on wind turbine models, the general offset model for yaw angles lower than  $30^\circ$   
 260 is expressed as, (1):

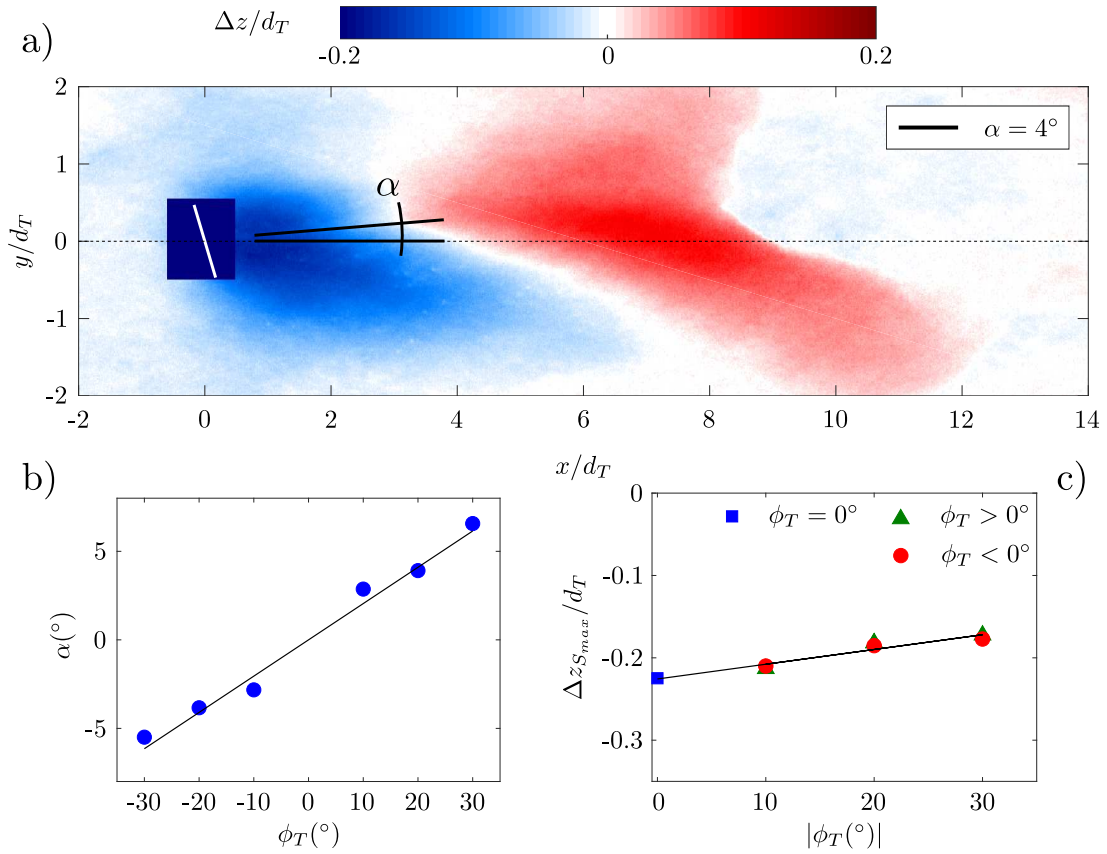
$$\frac{Y_o}{d_T} = \delta [C_T(\phi_T = 0^\circ) \sin \phi_T]^\zeta \cos^2 \phi_T \sqrt{\frac{x}{d_T}} + \frac{d_{rt}}{d_T} \sin \phi_T \quad (1)$$

261 where  $Y_o$  is the spanwise distance, i.e. the offset, between the position of the max-  
 262 imum velocity deficit and the centerline ( $y/d_T = 0$ ),  $\delta$  and  $\zeta$  are two coefficients of  
 263 the model, equal to 0.74 and 0.83 respectively (in the original formulation),  $C_T$  is the  
 264 thrust coefficient for a turbine with yaw angle equal to  $0^\circ$  and  $d_{rt}$  is the distance be-  
 265 tween the rotational center of the tower and the hub center in the  $y$ -direction. For our  
 266 MHK turbine model,  $d_{rt} = 36$  mm and  $C_T$ , for yaw angle equal to  $0^\circ$  is 0.74 (see drag  
 267 measurement performed in Musa et al. (2018b)). The parameter  $\zeta$  was kept equal to  
 268 0.83 as suggested by Dou et al. (2019a), while  $\delta$  was calibrated to 0.35. In Figure 5c the  
 269 colored lines represent the model predictions, while the symbols mark the estimated  
 270 locations of the maximum velocity deficit obtained interpolating the experimental data  
 271 with a Gaussian curve in the close proximity of the peak to improve the estimate of  
 272 the wake center. The percentage error between the experimental data and the model  
 273 prediction is based on the spanwise location of the velocity minima at hub height, de-



**FIG. 5. Mean velocity deficit isolines with minimum velocity marked and plotted over the final bathymetry for yaw angles a)  $\phi_T = 10^\circ$  and b)  $\phi_T = 20^\circ$ , respectively. c) Comparison between the location of the velocity minima at increasing  $x/d_T$  (dots) and the model by Dou et al. (2019a) (lines), described in equation 1. The vertical order of the lines in the legend is the same as the lines in the plot.**

274 fined as  $Y_o$ , and are calculated as  $e_\phi[\%] = \langle |(Y_o^d - Y_o^m)/Y_o^m| \rangle * 100$ .  $Y_o^d$  is evaluated  
 275 on the experimental data,  $Y_o^m$  is the model prediction, the vertical lines indicate the  
 276 absolute value and the angle brackets the average. The percentage errors are:  $e_{-30^\circ} =$



**FIG. 6. Summary of the bathymetric effects due to yaw angle orientation: a) final topography for  $\phi_T = 20^\circ$ , b) scour crest angle  $\alpha$  and c) maximum scour depth  $\Delta z_{S_{max}}$  for the tested yaw angles.  $\alpha$  is defined as the angle between the characteristic scour crest and the turbine  $y$ -location (in this case  $y = 0$ ).**

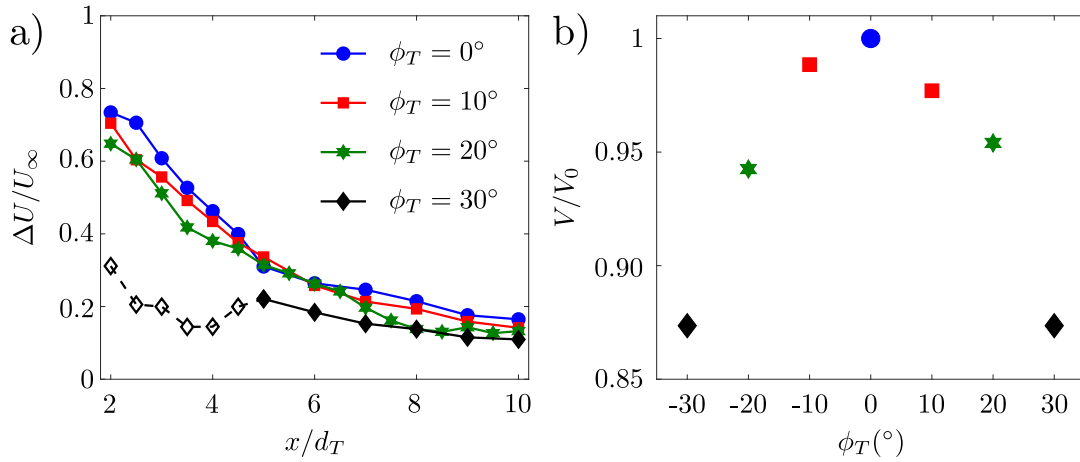
277 4%,  $e_{-20^\circ} = 7\%$ ,  $e_{-10^\circ} = 9\%$ ,  $e_{10^\circ} = 12\%$ ,  $e_{20^\circ} = 8\%$ ,  $e_{30^\circ} = 10\%$ . Please note that  
 278 the formulation by Dou et al. (2019a) has been recently updated providing a more thor-  
 279 ough evaluation of the parameters, and the extension to non-Gaussian, skewed, wake  
 280 profiles for both wind and MHK turbines (Dou et al. 2019b).

281 The effect of the yaw angle  $\phi_T$  on the riverbed configuration is described in Figure

282 6 by measuring: i) the deviation of the monopile scour crest from the  $\phi_T = 0^\circ$  sym-  
 283 metric case, defined by the angle  $\alpha$  (Figure 6a,b), and ii) the maximum scour depth  
 284  $\Delta z_{S_{max}}$  (Figure 6a,c). In particular, the scour crest, which delimits the two lobes of  
 285 the local scour region and coincides with  $y/d_T = 0$  for  $\phi_T = 0^\circ$ , was identified as  
 286 the local maxima of the detrended bed elevation in the scour region. The angle  $\alpha$   
 287 was calculated as the angle between linearly interpolated maxima and the center of the  
 288 channel  $y/d_T = 0$  (i.e. the turbine tower y-location, see Figure 6a). Results plotted in  
 289 Figures 6b,c denote robust statistical trends regardless of the blade rotational direction  
 290 (confirming the minimal changes in key bathymetric variables summarized in Table 2).

291 The effect of the yaw angle on the the flow field and the turbine performance was  
 292 quantified by measuring the wake deficit longitudinal profile  $\Delta U$  (Figure 7a) and the  
 293 mean voltage  $V$  (Figure 7b), respectively. With increasing yaw angle the turbine per-  
 294 formance, as well as the thrust coefficient estimated in a towing tank experiment (not  
 295 shown here) are reduced, resulting in a weaker velocity deficit. Figure 7a shows a re-  
 296 duction in the velocity deficit as the yaw angle increases, both in the near and the far  
 297 wake. Note that for  $\phi_T = 30^\circ$ , points measured at  $x/d_T < 5$  do not follow the ex-  
 298 pected trend and were distinguished by white markers. We infer that as the yaw angle  
 299 increases, the wake is periodically disrupted: perhaps stalling at specific blade cross  
 300 section and resulting in lift, circulation and tip vortex drops, followed by influx of high  
 301 momentum fluid in the wake. This could contribute to increase the unsteadiness of the  
 302 near wake, affecting the convergence and/or the reliability of our measurements close  
 303 to the rotor plane.

304 Consistent with the velocity deficit reduction, Figure 7b shows the mean voltage  
 305  $V$ , normalized by the corresponding measurement from the zero-yaw turbine  $V_0$ . Note  
 306 that the voltage output is linearly proportional to the rotor angular velocity, as demon-  
 307 strated in Musa et al. (2018b) and verified here in selected cases using underwater  
 308 video recording. Therefore, the mean voltage ratio, resulting from continuously mea-



**FIG. 7. Summary of the performance changes due to yaw angle orientation: a) velocity deficit  $\Delta U$  normalized by the incoming flow at hub height  $U_\infty$  and b) rotor mean voltage  $V$  normalized by the mean voltage from the zero-yaw turbine  $V_0$ . White markers in panel a) represent points that do not follow the expected wake trend, potentially due to wake periodical disruptions caused by the yaw angle.**

309 sured voltage time series for the whole duration of the experiments, represents here the  
 310 ratio of averaged angular velocity and a simplified, though robust, metric for turbine  
 311 comparative performance.

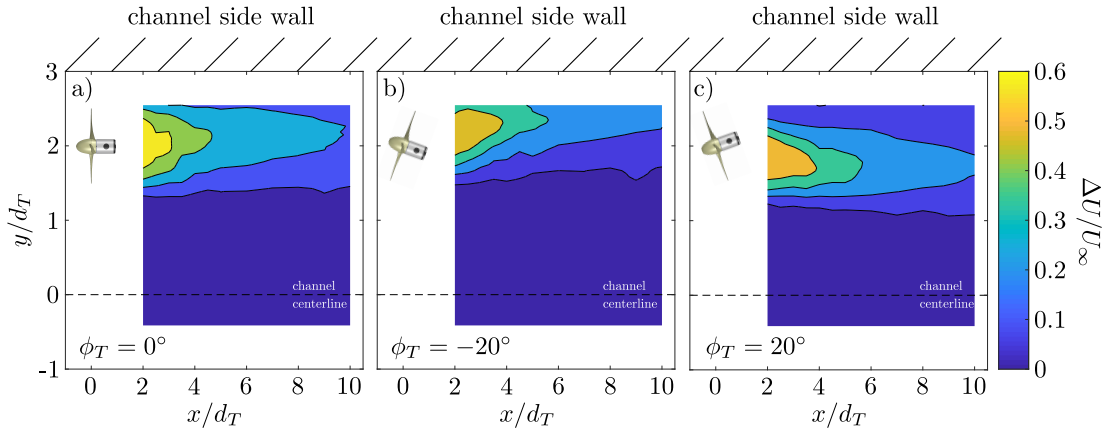
312 A smaller thrust coefficient and a reduced wake deficit imply that, with increas-  
 313 ing yaw angles, the shear stress augmentation downstream of the foundation tower is  
 314 reduced, and thus the scour. The crest orientation in the scour region reveals a clear  
 315 dependence on the yaw angle, exhibiting a direction fairly normal to the rotor and op-  
 316 posite to the orientation of the velocity minima line. The next step is to verify that  
 317 the above findings hold when the turbine is located closer to a side wall, which might  
 318 present opportunities for potential passive control strategies to enhance or mitigate

319 stream bank erosion.

### 320 **Near-bank turbine**

321 In this second set of experiments the MHK turbine was located in the proximity  
322 of the left hand side channel wall as described in the Experimental Apparatus section.  
323 The main goal is to investigate the opportunity to steer the wake away from the side  
324 wall and favor sediment deposition towards the stream bank. Both processes would  
325 contribute to protect the river from lateral erosion. Two yaw angles ( $-20^\circ$  and  $20^\circ$ )  
326 and two rotational directions cases are investigated, in addition to the un-yawed, near  
327 bank, reference case. First, the mean velocity contours in the wake up to  $10 d_T$  is  
328 included in Figure 8. While the aligned rotor generates a fairly symmetrical wake, the  
329 two yawed configurations reproduced a distorted and skewed wake, consistent with  
330 the previous observations. In particular, note that with the  $\phi_T = 20^\circ$  configuration,  
331 the wake is steered towards the center of the channel, thus likely preventing the rotor  
332 annular shear layer to impinge on the side wall (Figure 8c). The same configuration is  
333 observed to promote the transport of eroded sediments from the scour occurring near  
334 the support tower towards the side wall while keeping the maximum scour away from  
335 it (Figure 9c).

336 In order to make a final assessment on the efficiency of the depositional mechanism,  
337 the centroid of the whole deposit  $\Delta y_C$  is estimated with respect to the side wall line and  
338 is identified in a subregion  $1 < x/d_T < 11$  and  $0 < y/d_T < 3$ , weighted on the actual  
339 bed elevation (above a threshold  $\Delta z/d_T > 0.01$ ). In figure 9 the deposit centroid  
340 is found to be located closer to the side bank for the  $\phi_T = 20^\circ$ , with only second  
341 order effects from the blade rotational direction. The final configuration of the deposit  
342 skewed shape for different yaw angles qualitatively resembles the centered turbine  
343 cases and the maximum scours are slightly reduced in comparison to the reference  
344 centered turbine at  $\phi_T = 0^\circ$  (see Table 2). However, the implications of partially

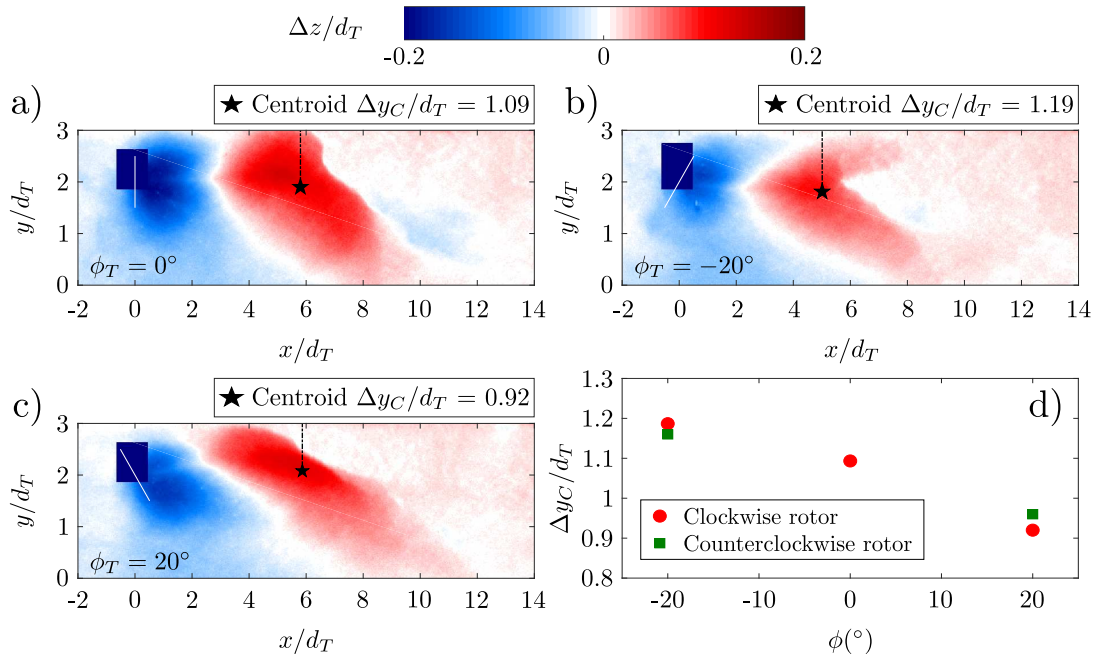


**FIG. 8. Mean velocity deficit contour  $\Delta U/U_\infty$  in the wake of a turbine installed close to the side wall, for different yaw angles: a)  $\phi_T = 0^\circ$ , b)  $\phi_T = -20^\circ$ , c)  $\phi_T = 20^\circ$ .**

345 steering depositional patterns by prescribed MHK turbine yaw angle misalignment are  
 346 far more interesting.

## 347 DISCUSSION

348 Musa et al. (2018b) demonstrated that axial flow turbines can be safely deployed  
 349 in large scale rivers with intense sediment transport and large migrating bedforms. A  
 350 specific turbine array siting was designed to minimize large-scale non-local morphody-  
 351 namic effects while keeping navigability on one side of the channel. In the follow-up  
 352 work, Musa et al. (2019) tested a vane-like turbine array configuration to deflect the  
 353 core of the flow to promote morphodynamic effects and meandering onset. Those ef-  
 354 forts were based exclusively on turbine siting. Here, results show that by yawing a  
 355 single MHK turbine it is possible to govern the wake, steering it away from the chan-  
 356 nel side banks and favoring deposition towards the wall. The same strategy can be  
 357 applied to steer the wake away from downstream turbines and maximize the power  
 358 plant performance, as suggested in the wind energy community (Boersma et al. 2017),



**FIG. 9. Final topography scan for the side-wall turbine case at different yaw angles: a)  $\phi_T = 0^\circ$ , b)  $\phi_T = -20^\circ$  and c)  $\phi_T = 20^\circ$ . d) Normalized deposit centroid distance from the wall  $\Delta y_C$  as function of the yaw angle and blade rotational direction for all the tested cases.**

359 yet the implications on large scale fluvial evolution are far more intriguing. This and  
 360 the above benchmark experiments are devoted to provide a set of guidelines to ex-  
 361 pand renewable energy production in rivers with minimal environmental impacts. The  
 362 current results, in particular, may provide the first example of positive environmen-  
 363 tal impact, consisting in the protection from bank erosion by opportunely sited and  
 364 yawed hydrokinetic turbines, while producing renewable energy. While we acknowl-  
 365 edge that a MHK turbine near the bank still induces a local scour, we have shown  
 366 that the non-local, or far-wake, effects are both contributing to mitigate streambank  
 367 erosion: steering the wake away from the lateral wall, is expected to reduce the mean  
 368 shear and the Reynolds stresses on the sidewall boundary layer; steering the deposit

369 towards the back is expected to supply erodible material and limit near bank scour.  
370 Further investigation should be devoted to test a streamwise array of yawed turbines  
371 near the bank to confirm the current results and assess if the collective behavior of  
372 multiple yawed rotor could provide an effective shelter along a river meander. River  
373 management in urbanized environments is a challenging task that needs to take into  
374 account human (and human structures) safety along with the improvement of physi-  
375 cal and ecological conditions of often degraded urban streams (Bernhardt and Palmer  
376 2011; Wohl et al. 2015). A complete restoration of the natural forms and processes is  
377 often not achievable, because of conflicting river ecosystem services. However, efforts  
378 can be made to diversify streambank protection strategies. This study suggests that it is  
379 possible to positively couple renewable energy production and bank protection, taking  
380 advantage of the turbine downstream deposit to integrate and/or substitute more estab-  
381 lished techniques based on roughness, vegetation patches, vanes, or other more or less  
382 invasive hydraulic structures (Henderson 1986; Odgaard and Mosconi 1987; Odgaard  
383 and Wang 1991b; Maynard 1995; Froehlich and Benson 1996; Allen and Leech 1997;  
384 Li and Eddleman 2002).

## 385 **CONCLUSION**

386 An axial flow Hydrokinetic Turbine has been tested in yawed conditions to explore  
387 potential passive control strategies to produce renewable energy from river flows, while  
388 protecting the banks from erosion. Experiments were carried out in critical mobility  
389 conditions to explore the parameter space through relatively short experiments. Results  
390 with the turbine deployed along the channel centerline show that yaw angle variations  
391 induce a deflection of the wake and consequently affect the near-wall, near-wake di-  
392 rection of the scour and sediment transport, the maximum scour depth, and the size  
393 and orientation of the deposit. In terms of performance, the angular velocity of the  
394 turbine is slightly reduced, while the turbine blade rotational direction (clockwise or

395 counterclockwise) was shown to have negligible effects on the above quantities and  
396 trends. The yawed centered turbine is used here as a reference set of experiments with  
397 potential implications on power performance at the MHK array scale, by preventing  
398 the wakes to impinge of downstream turbines. When the turbine is located close to  
399 the side walls, the qualitative description of the wake and bathymetric evolution is not  
400 significantly changed, paving the road to a different set of passive control strategies.  
401 In particular, with the turbine rotor positively yawed, the upstream rotor facing the  
402 center of the channel, the wake is shown to deflect away from the stream bank while  
403 the deposit is observed to move towards the bank. In addition, the local scour is re-  
404 duced, when compared to the center un-yawed turbine, as a combined effect of lower  
405 incoming velocity and yawed rotor. All these mechanisms suggest that opportunely  
406 chosen yaw angles could passively reduce streambank erosion with a minimal loss in  
407 power production. The limited bedload transport conditions and the absence of migrat-  
408 ing bedforms certainly represent the biggest limitations of this work, which however  
409 provides the proof of concept needed to move this investigation forward and address  
410 the need to better integrate renewable energy devices within the natural environment,  
411 as discussed above and in Musa et al. (2019).

## 412 **DATA AVAILABILITY**

413 Some or all data, models, or code generated or used during the study are available  
414 from the corresponding author by request.

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Variable	Name	Value	Units
$S$	Bed slope	0.04	%
$h$	Water depth	0.27	m
$b$	Channel width	0.90	m
$U$	Depth average velocity	0.40	$\text{m s}^{-1}$
$Q$	Flow discharge	0.10	$\text{m}^3 \text{s}^{-1}$
$S_w$	Water slope	0.03	%
$Fr$	Froude number	0.26	
$Re$	Reynolds number	$1.22 \times 10^5$	
$u_*$	Shear velocity	0.02	$\text{m s}^{-1}$
$d_{50}$	Median grain size	1.13	mm
$U_\infty$	Incoming velocity at hub height	0.42	$\text{m s}^{-1}$
$d_T$	Turbine rotor diameter	0.15	m
$h_{hub}$	Turbine hub height	0.13	m

**TABLE 1. Turbine, flow, channel and sediment parameters used during the experiments. Note that  $U$  is the depth average velocity calculated with the incoming velocity vertical profile,  $Q = U(bh)$ ,  $Fr = U/(gh)^{1/2}$  and  $Re = Uh/\nu$ , where  $\nu = 10^{-6} \text{ m}^2 \text{ s}^{-1}$  is the kinematic viscosity of water.**

Yaw angle $\phi_T$	Location	Blade rotation	$\Delta z_{Smax}/d_T$	$\Delta z_{Dmax}/d_T$	$\Delta y_C/d_T$
0°	Center	Clockwise	-0.230	0.100	-
0°	Center	Counterclockwise	-0.188	0.093	-
10°	Center	Clockwise	-0.231	0.097	-
-10°	Center	Clockwise	-0.210	0.100	-
20°	Center	Clockwise	-0.182	0.120	-
-20°	Center	Clockwise	-0.185	0.101	-
30°	Center	Clockwise	-0.173	0.102	-
-30°	Center	Clockwise	-0.177	0.101	-
0°	Near Wall	Clockwise	-0.200	0.140	1.91
20°	Near Wall	Clockwise	-0.160	0.130	2.08
20°	Near Wall	Counterclockwise	-0.150	0.110	2.04
-20°	Near Wall	Clockwise	-0.150	0.100	1.81
-20°	Near Wall	Counterclockwise	-0.140	0.110	1.84

**TABLE 2. Summary of experiments characteristics and main bathymetric effect for the performed experiments, specifically: maximum scour  $\Delta z_{Dmax}/d_T$ , maximum deposit  $\Delta z_{Smax}/d_T$  and deposit centroid distance from lateral wall  $\Delta y_C/d_T$  (for Near Wall cases only).**