

SPE Phase I Material Properties

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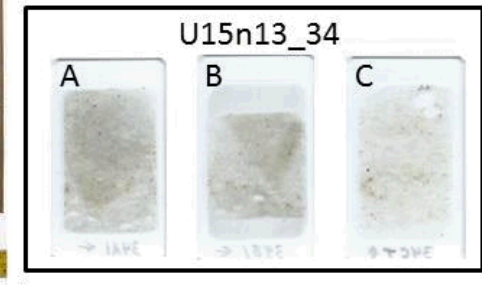
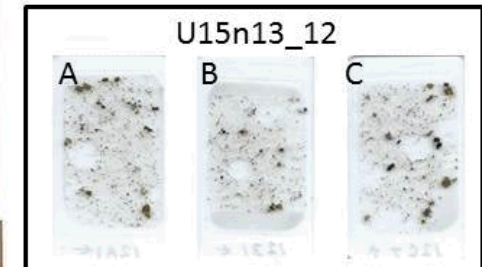
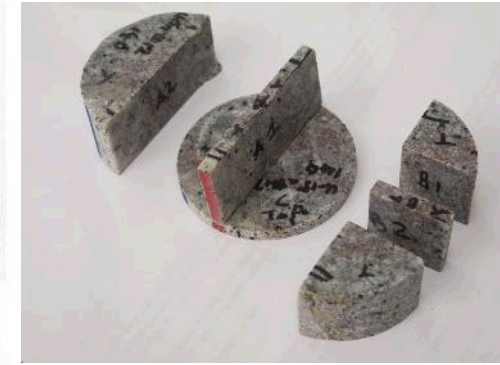
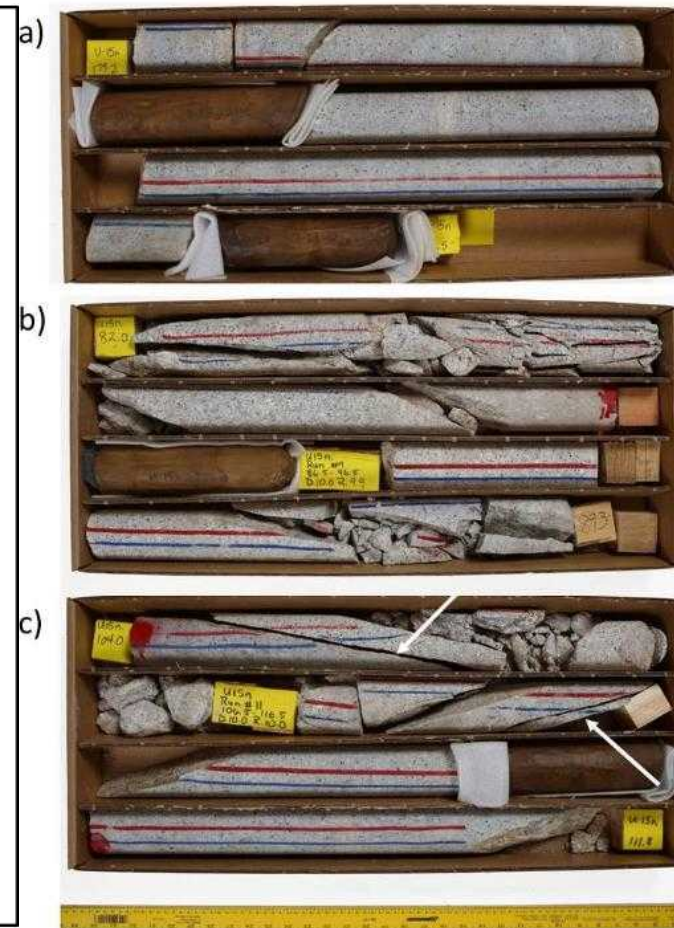
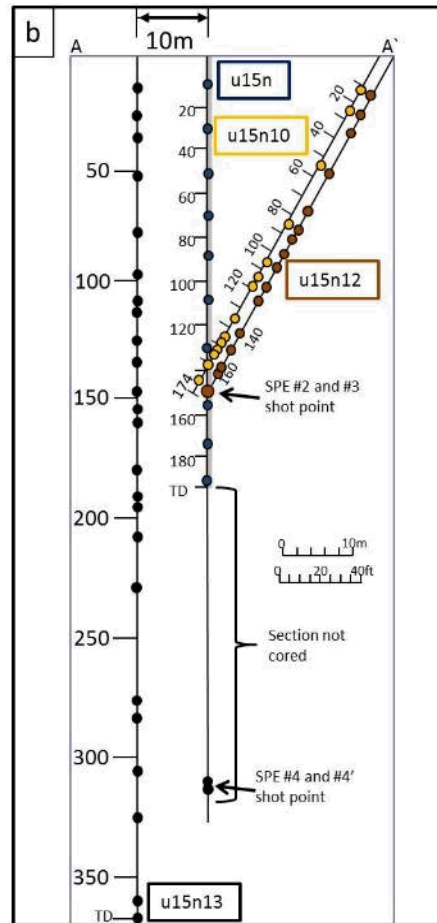
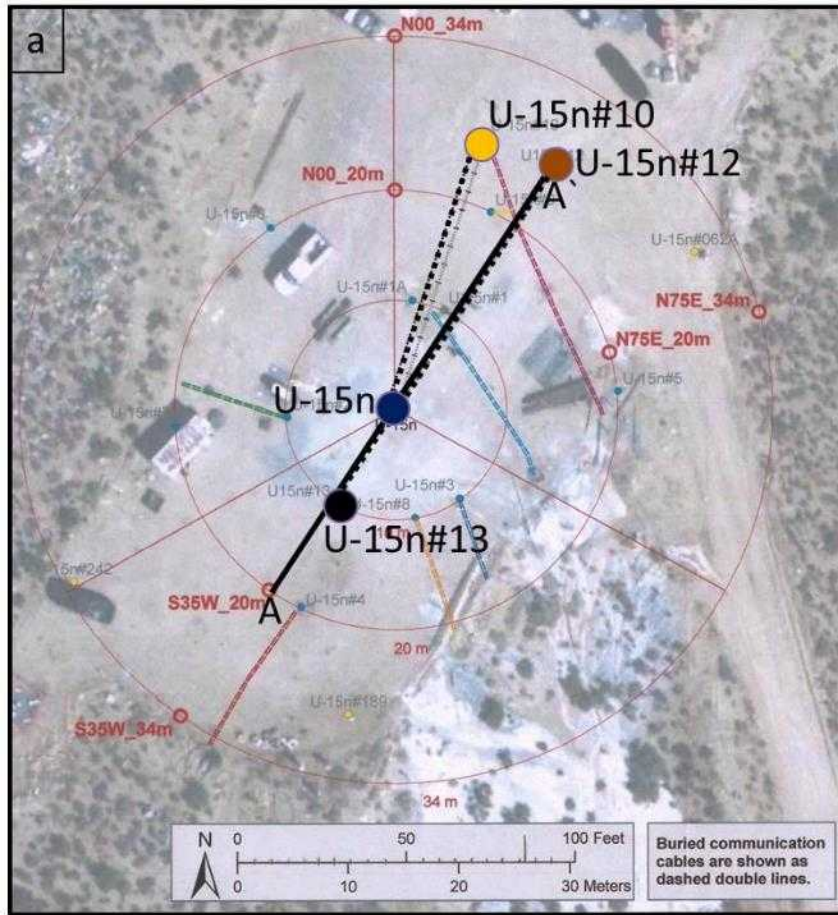
Material and Fracture Property Analyses

Linked multi-scale analyses on pre-SPE-1 and post SPE-2, 3 and 5 granite

- **Optical Microscopy**
 - defines extent of damage and controls on surface effects
 - provides damage history (tectonic vs explosion)
 - constrains anisotropy
- **Core-scale Material Property Testing**
 - Unconfined and Triaxial compression (Hydrocode model development)
 - Dynamic and Quasi-static Brazilian Disc Tension (Hydrocode model development)
 - Triaxial compression to investigate permeability and damage relationship
- **Core-scale Fracture Property Testing**
 - Direct Shear tests on natural fractures
 - Triaxial shear on natural fractures (incorporating pore pressure and confining pressure on natural fractures)
 - P&S velocity as a function of fracture aperture with water as coupling medium

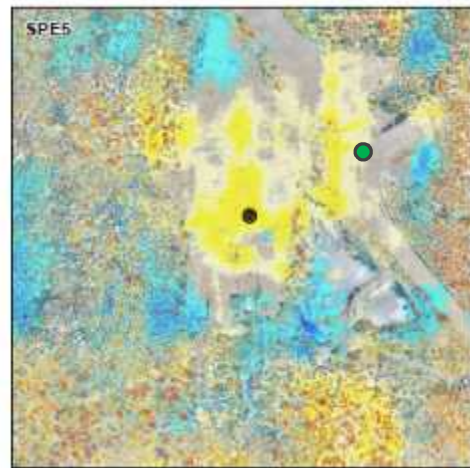
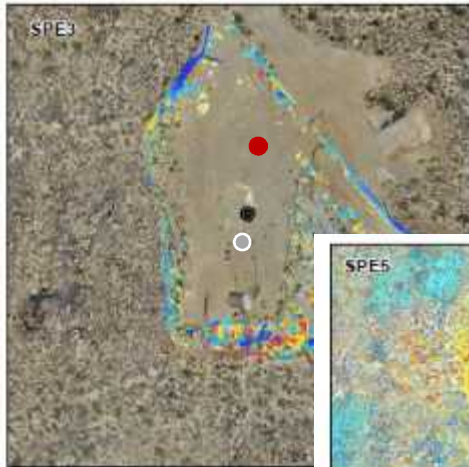
Material and Fracture Property Analyses

Linked multi-scale analyses on pre-SPE-1 and post SPE-2, 3 and 5

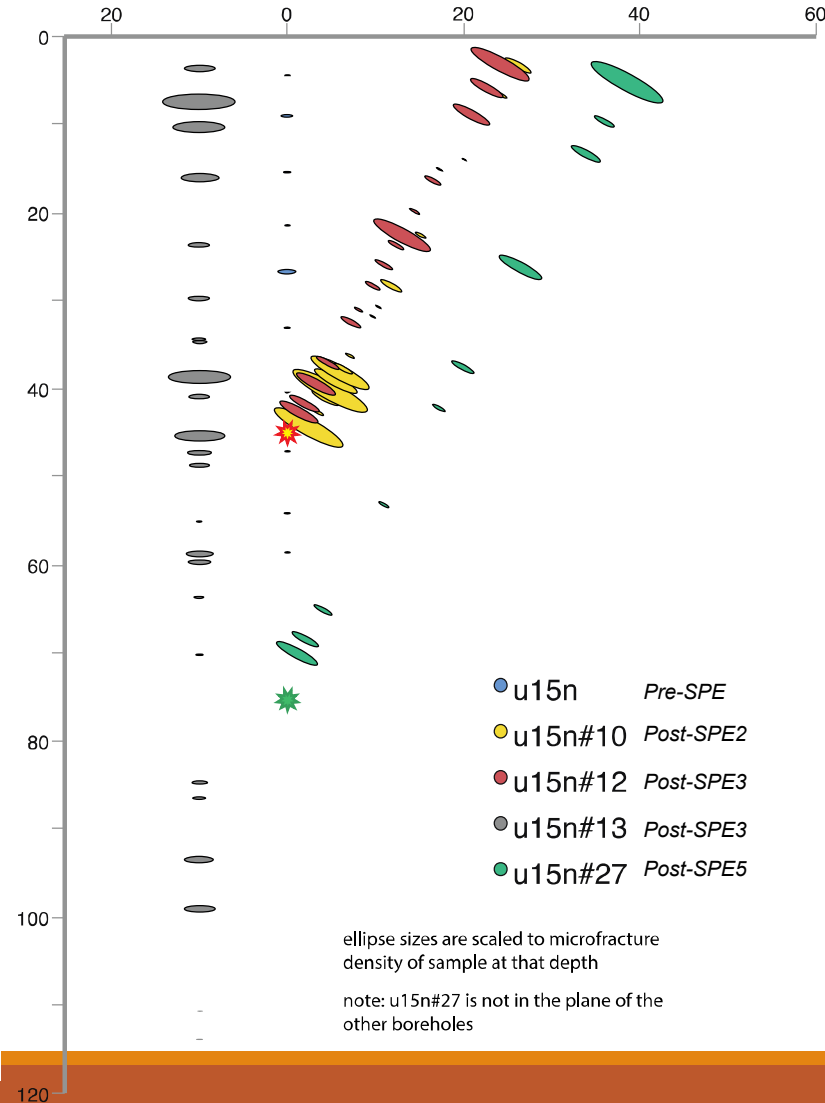


Microscale Material and Fracture Property Analyses

Linked multi-scale analyses on pre-SPE-1 and post SPE-2, 3 and 5 granite demonstrate rock behavior that controls surface effects



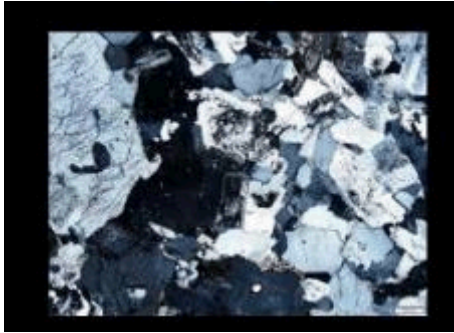
*borehole locations are approximate



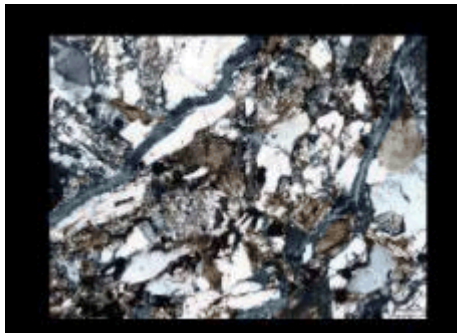
Microscale Material and Fracture Property Analyses

Optical Microscopy: anisotropy and damage history

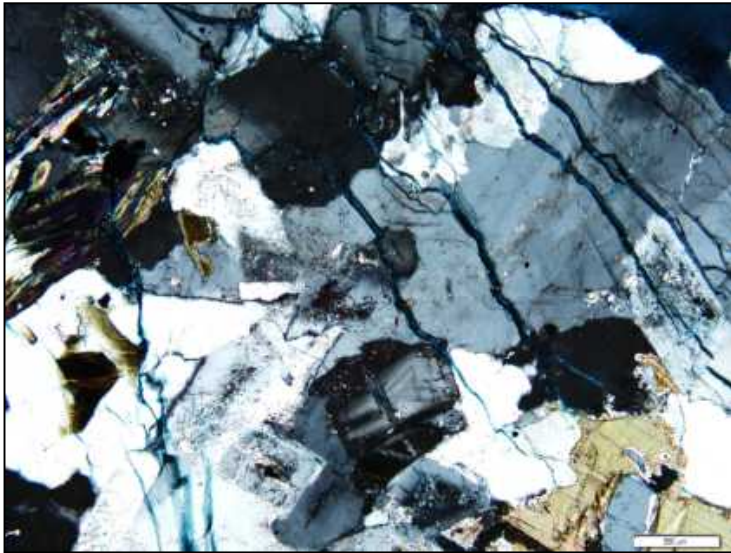
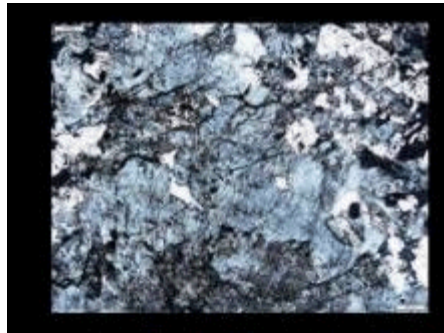
U15n13
78ft depth
Intact granite



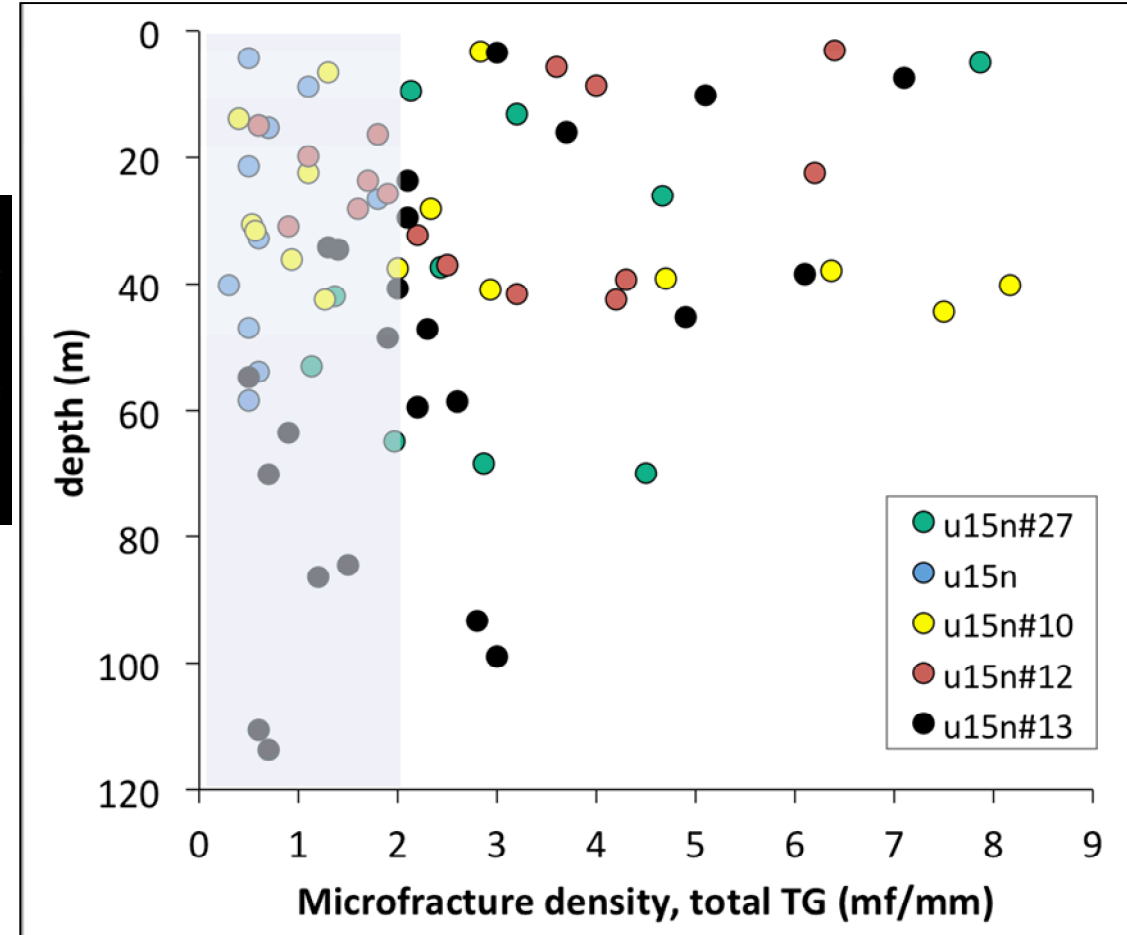
U15n13
148ft depth
Faulted granite



U15n13
34 ft depth
“Weathered” granite



Postshot

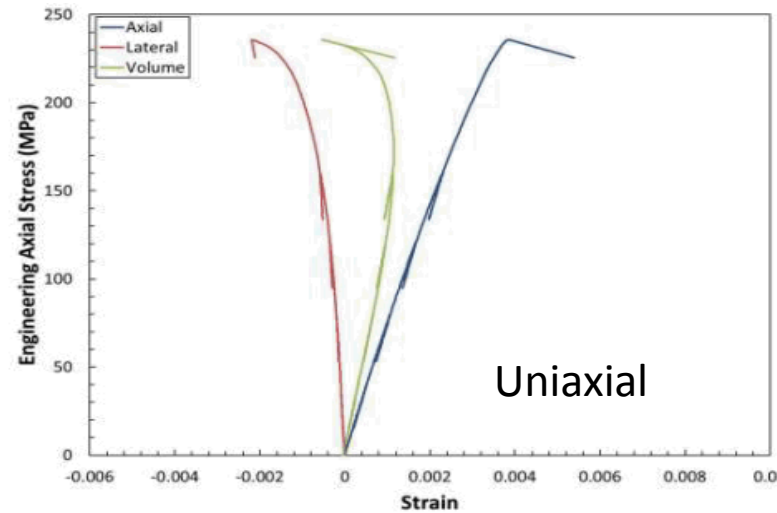


Core Scale Material Property Analyses

Uniaxial and Triaxial compression

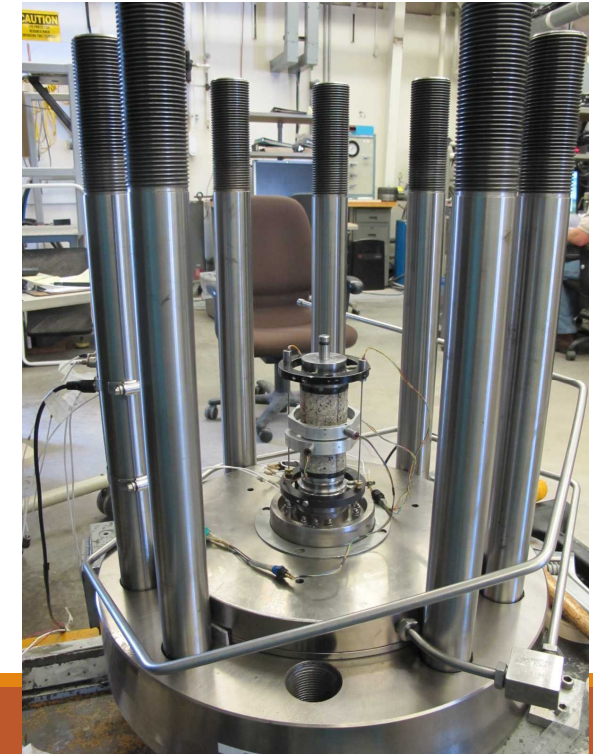
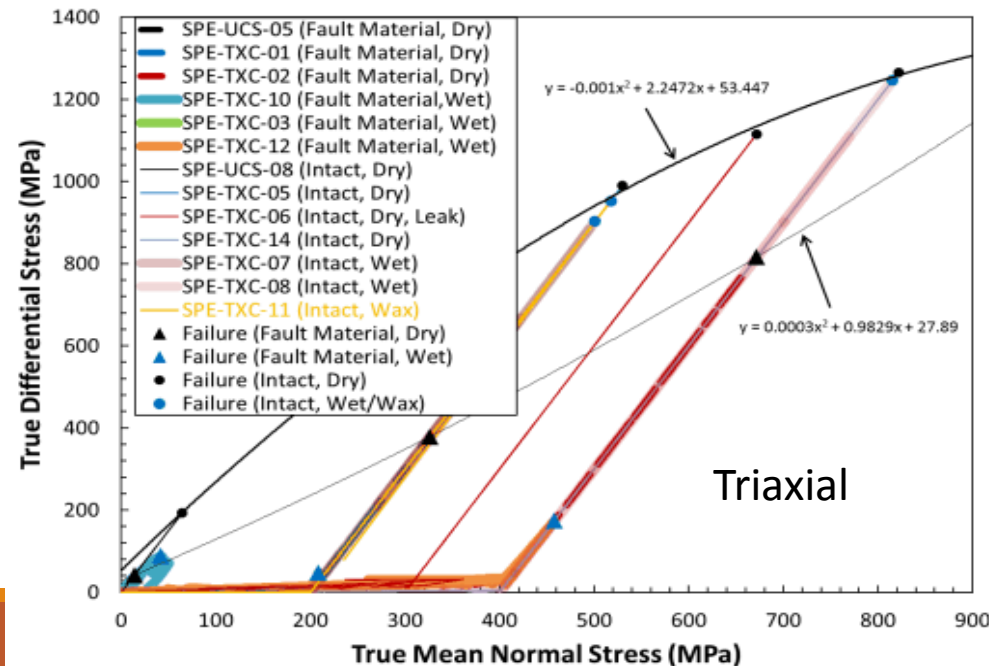
Uniaxial

- Performed from source hole U-15n (pre SPE), U-15n#10 (post SPE2), U-15n#12 (post SPE3), U-15n#13 (post SPE3), and U-15n#27 (post SPE5)
- Used to compare to previous work on Climax Stock and understand rock strength variation between SPE events
- Hydrocode model development



Triaxial

- Performed from source hole U-15n (pre SPE), U-15n#10 (post SPE2), and U-15n#27 (post SPE5)
- Used to establish failure envelope for intact and weathered (both wet and dry) materials
- Hydrocode model development

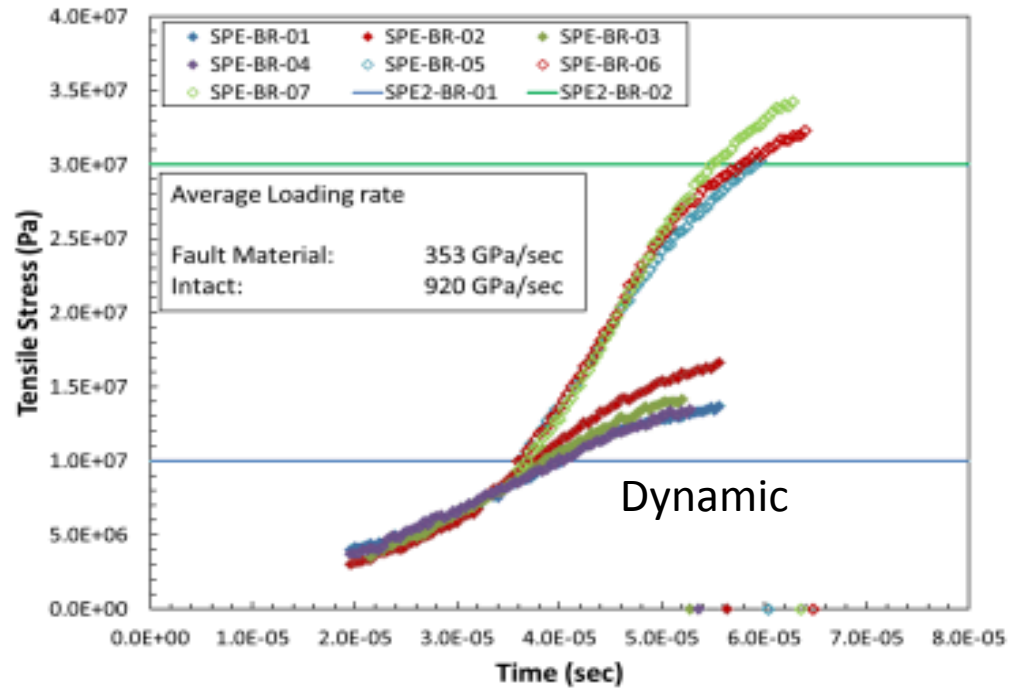


Core Scale Material Property Analyses

Dynamic/Quasi-static Disc Tension

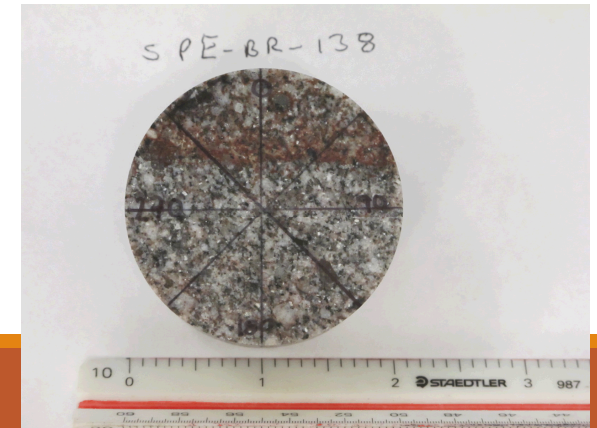
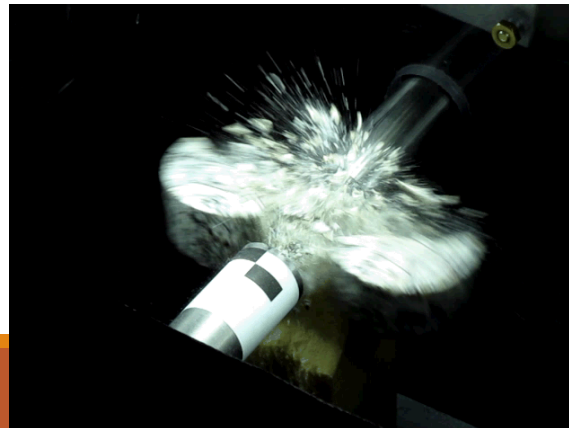
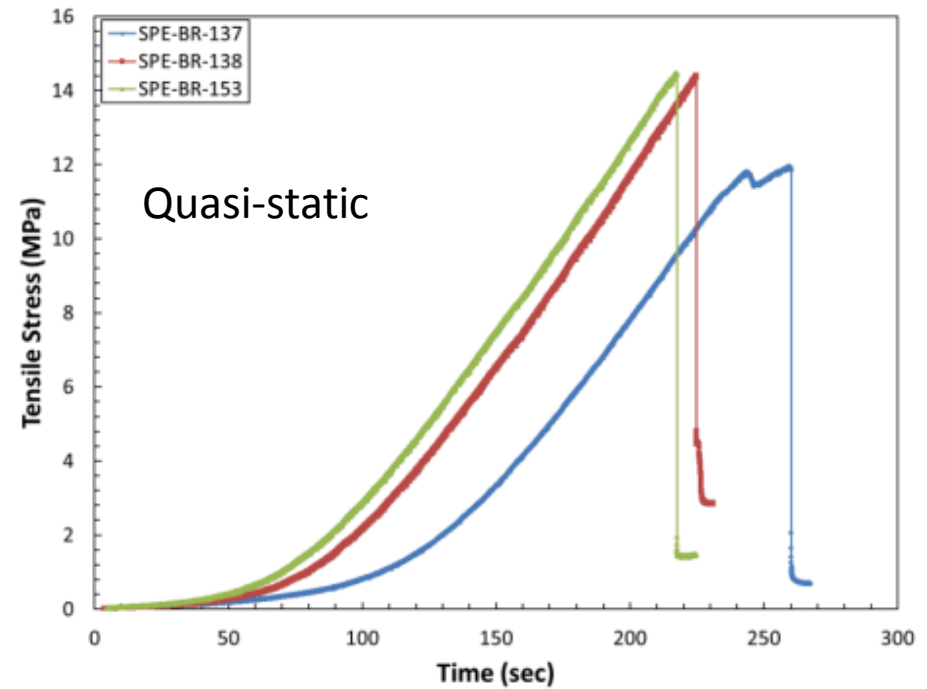
Dynamic

- Performed from source hole U-15n (pre SPE) and U-15n#10 (post SPE2)
- Strain rate $\sim 200/\text{second}$
- FDEM development



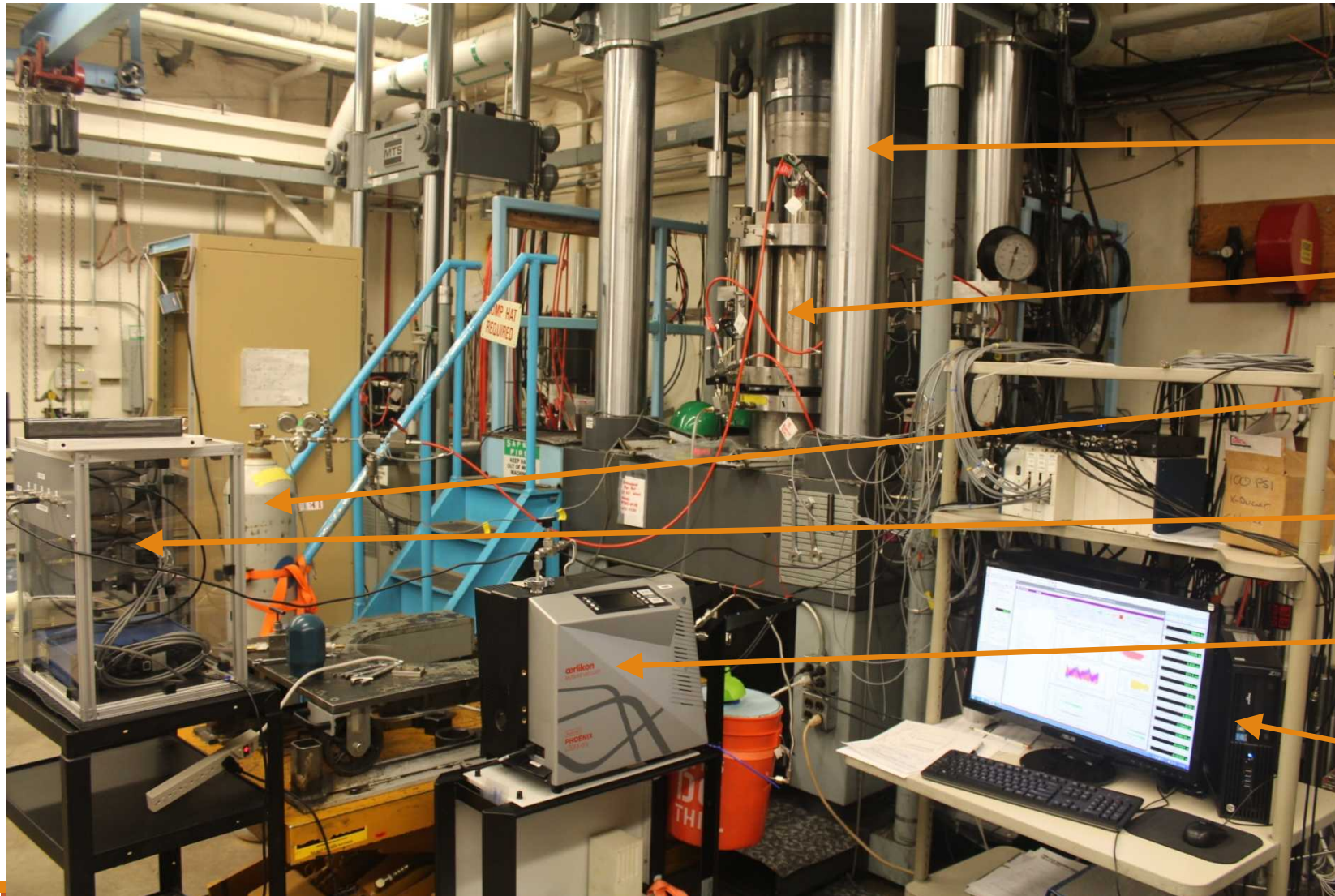
Quasi-static

- Performed from source hole U-15n (pre SPE)
- Strain rate $\sim 1\text{E-}4/\text{second}$
- FDEM development



Core Scale Material Property Analyses

Triaxial compression to investigate permeability and damage relationship



Load frame

Pressure vessel

Upstream helium source

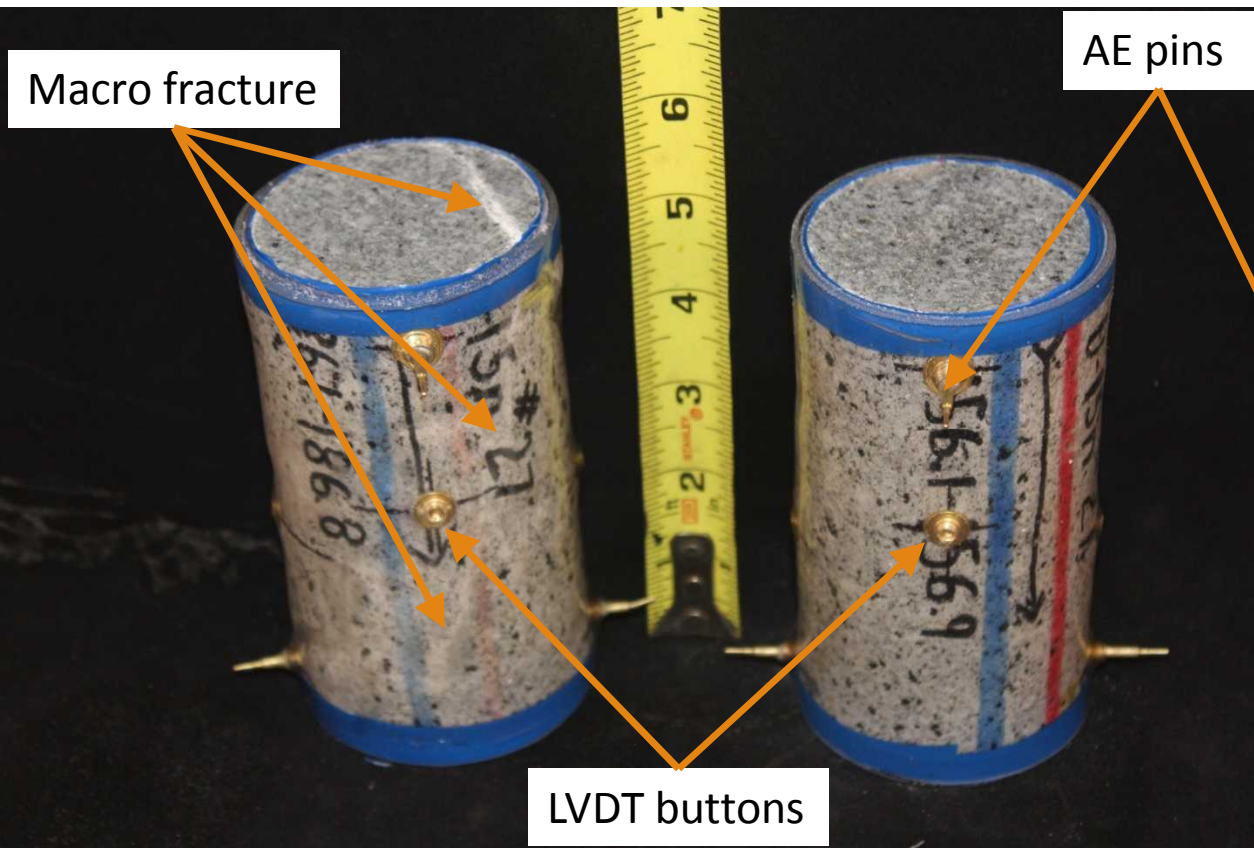
Flow bench

Helium leak detector

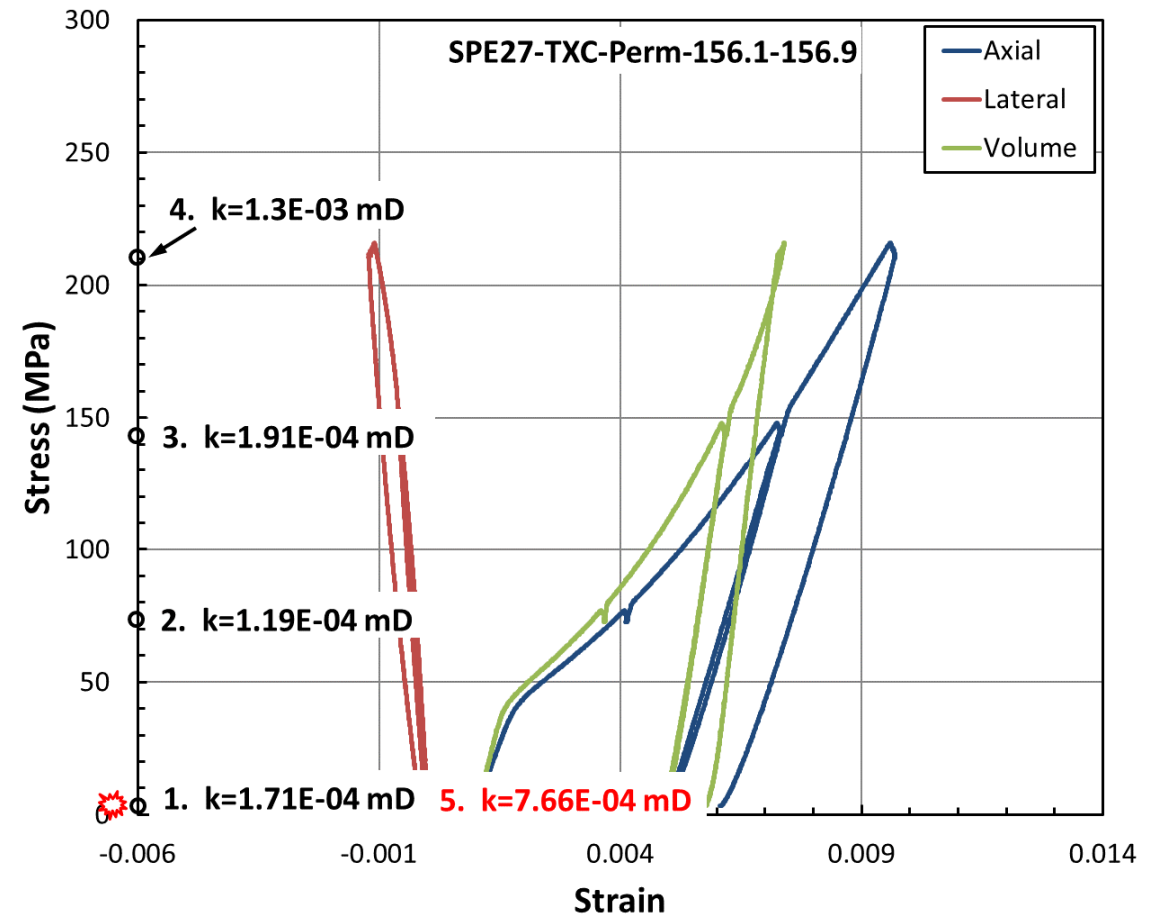
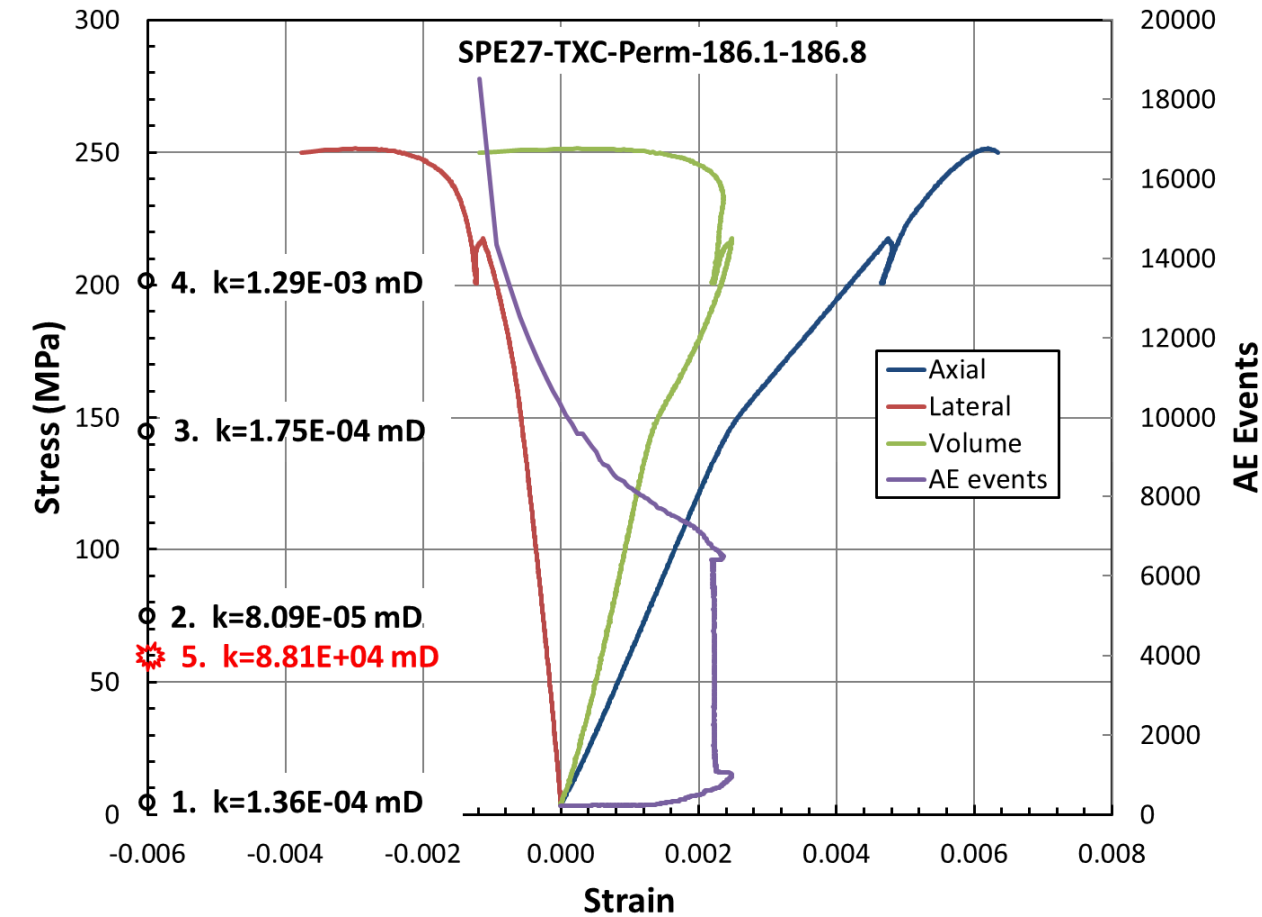
Data Acquisition System

Core Scale Material Property Analyses

Triaxial compression to investigate permeability and damage relationship



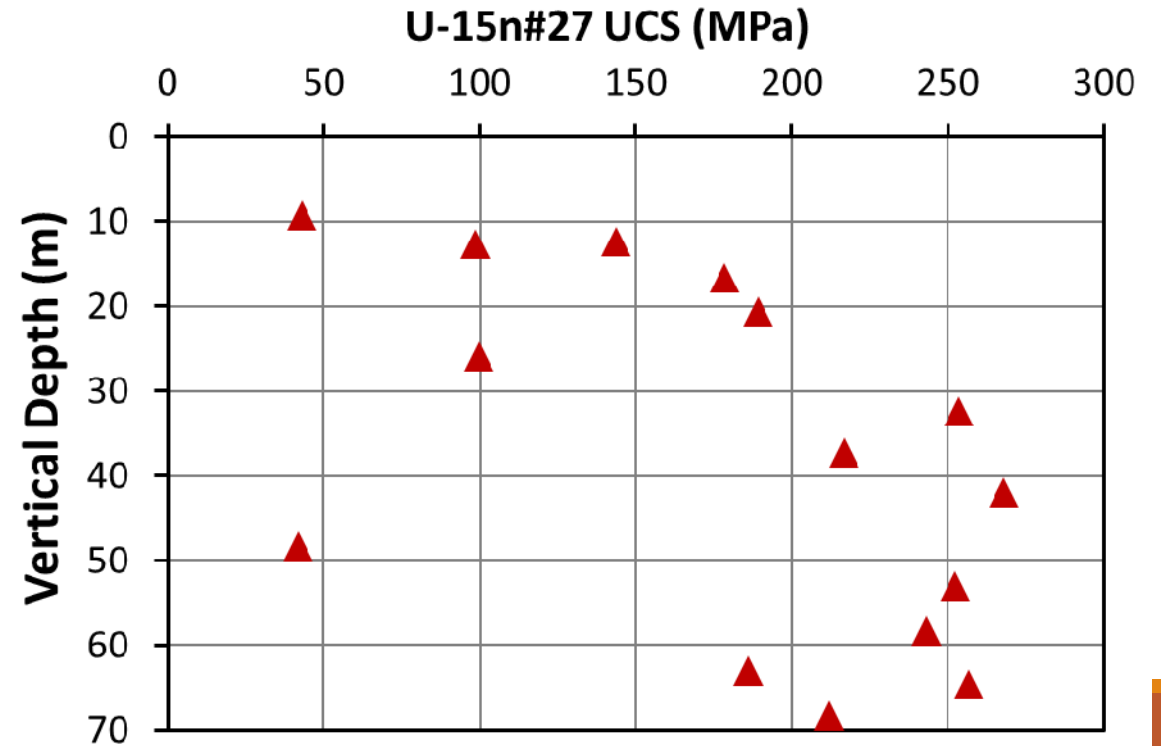
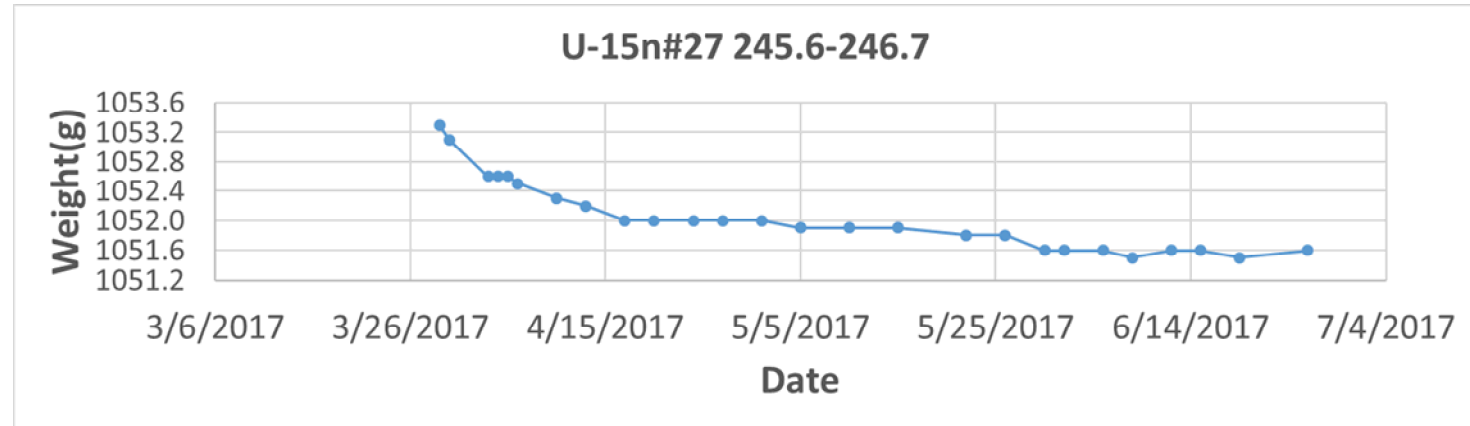
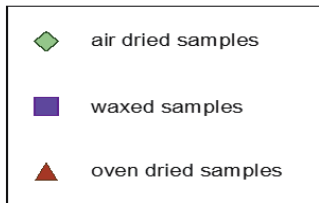
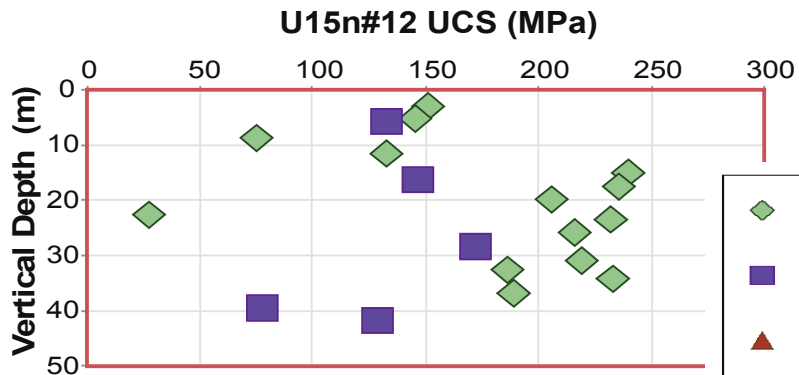
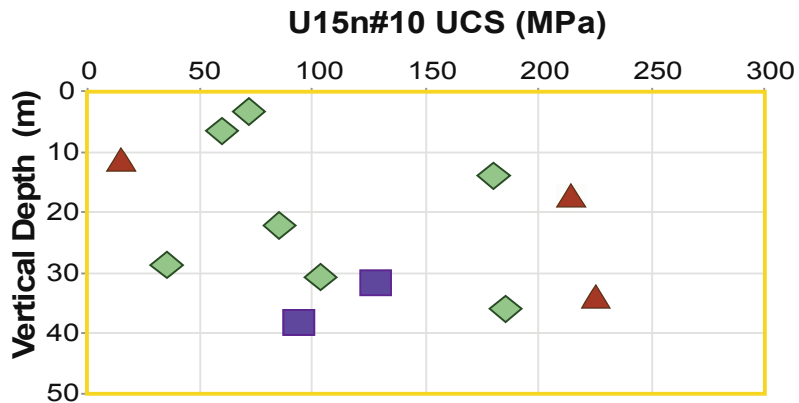
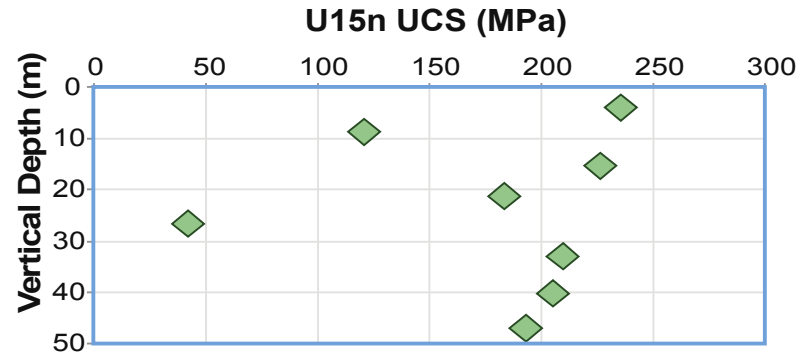
Samples each photo (Left) SPE27-TXC-Perm-186.1-186.8 and (Right) SPE27-TXC-Perm-156.1-156.9
Buttons mount LVDT's to measure radial strain
Pins record Acoustic Emissions



- Baseline permeability $\sim 1\text{E-}04\text{mD}$ taken at 3.5 MPa hydrostatic pressure
- Permeability decreases at 75 MPa Axial Stress
- Permeability increases at 145 MPa (above hydrostatic baseline) and then again at 210 MPa Axial Stress (one order of magnitude above baseline)
- Sample SPE27-TXC-Perm-186.1-186.8 taken to failure. Post failure permeability is seven orders of magnitude above previous measured permeability
- Sample SPE27-TXC-Perm-156.1-156.9 unloaded after Axial Stress of 210 MPa. Hydrostatic permeability now half an order of magnitude greater than undamaged hydrostatic permeability

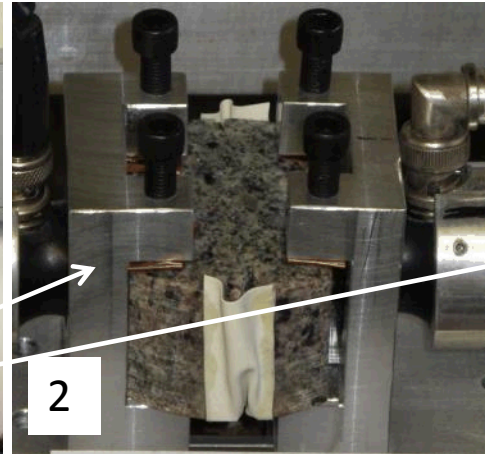
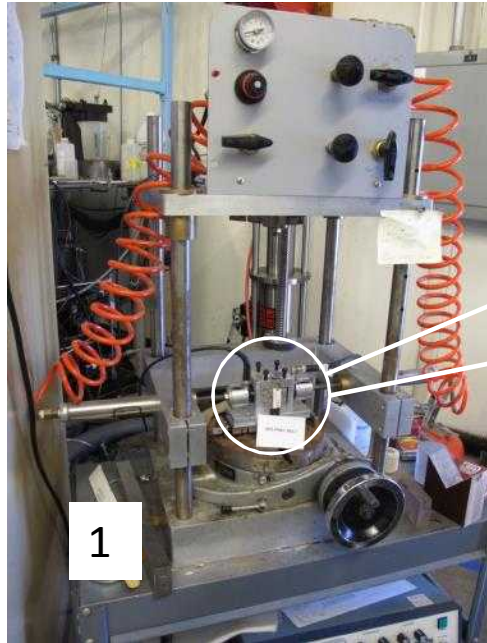
Core Scale Material Property Analyses

Unconfined compressive strength varies with water content

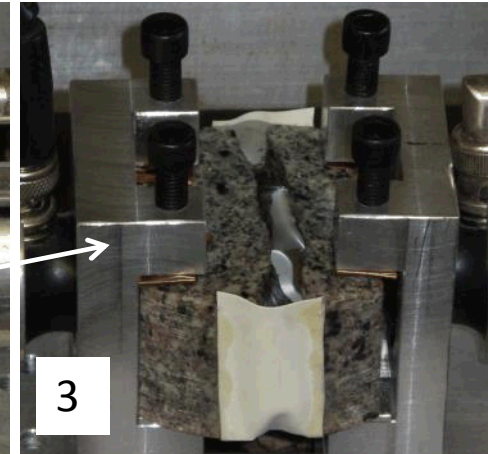


Core Scale Fracture Property Analyses

P&S velocity as a function of fracture aperture with water as coupling medium



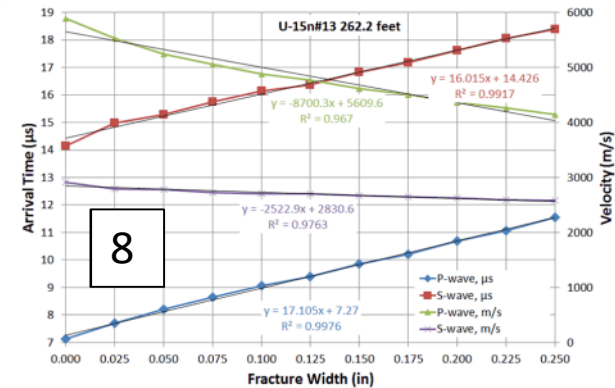
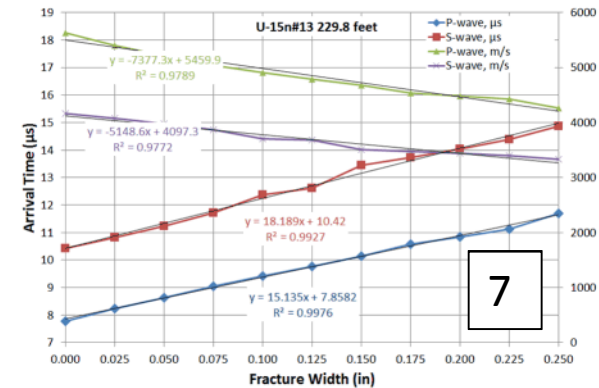
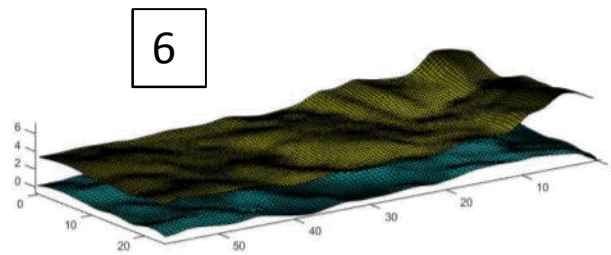
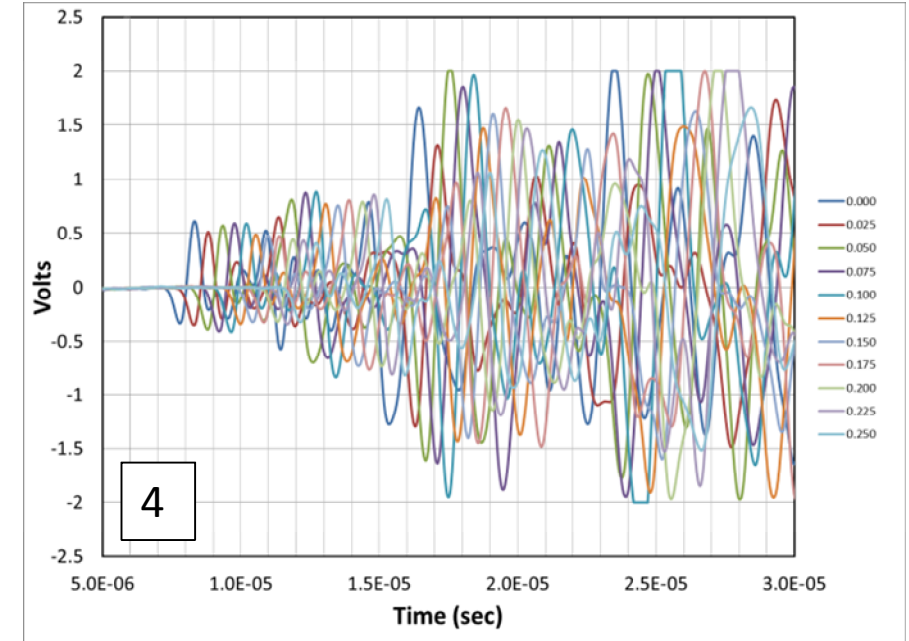
Water filled closed gap.



0.250 inch gap filled with water.

SPE-FRAC-262.2

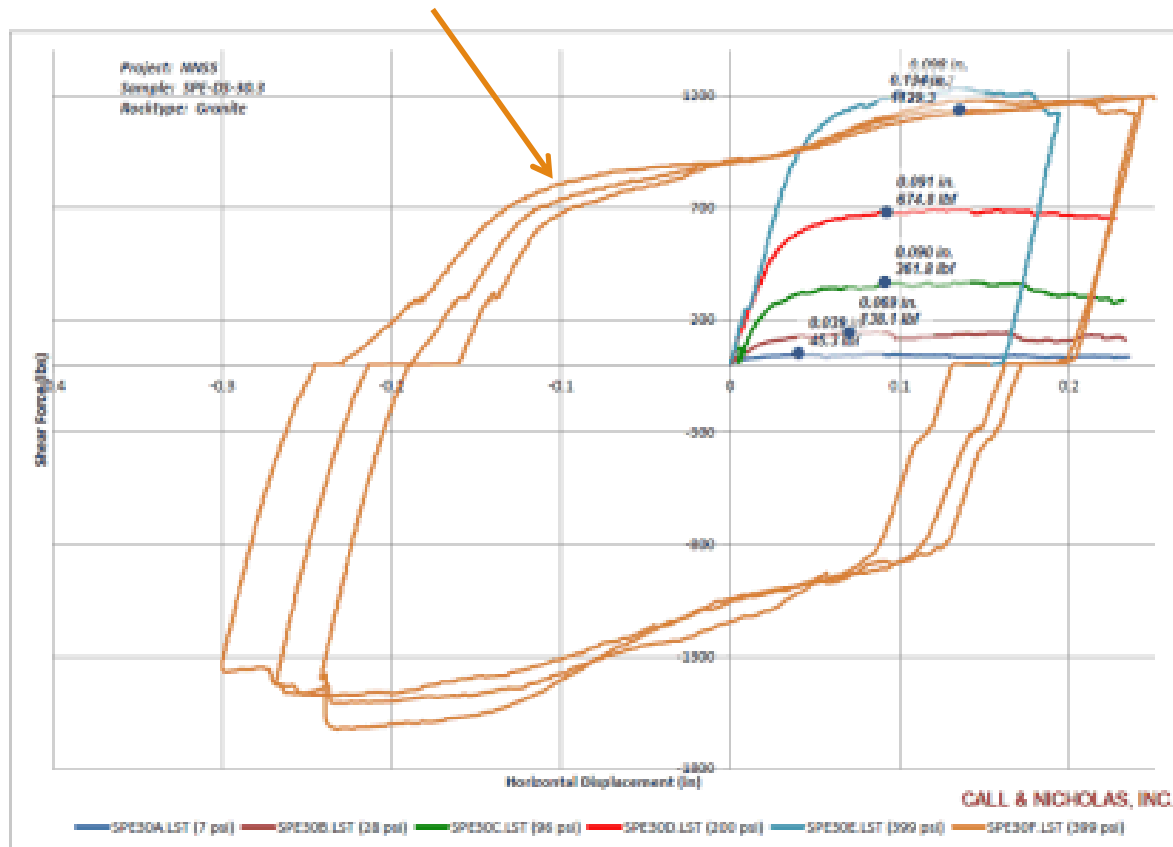
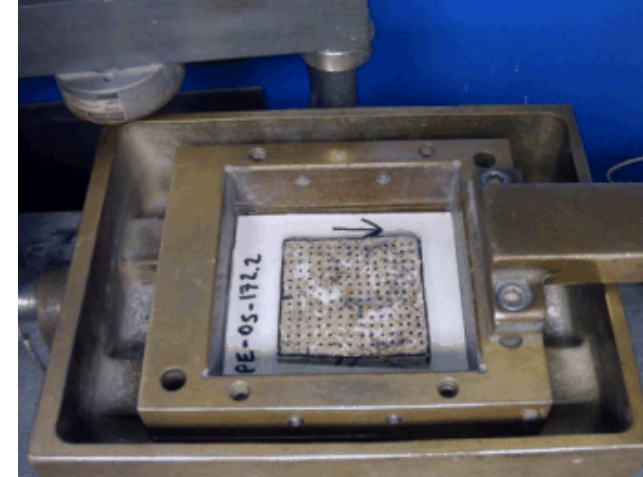
SPE-FRAC-262.2



Core Scale Fracture Property Analyses

Direct Shear on Natural Fractures

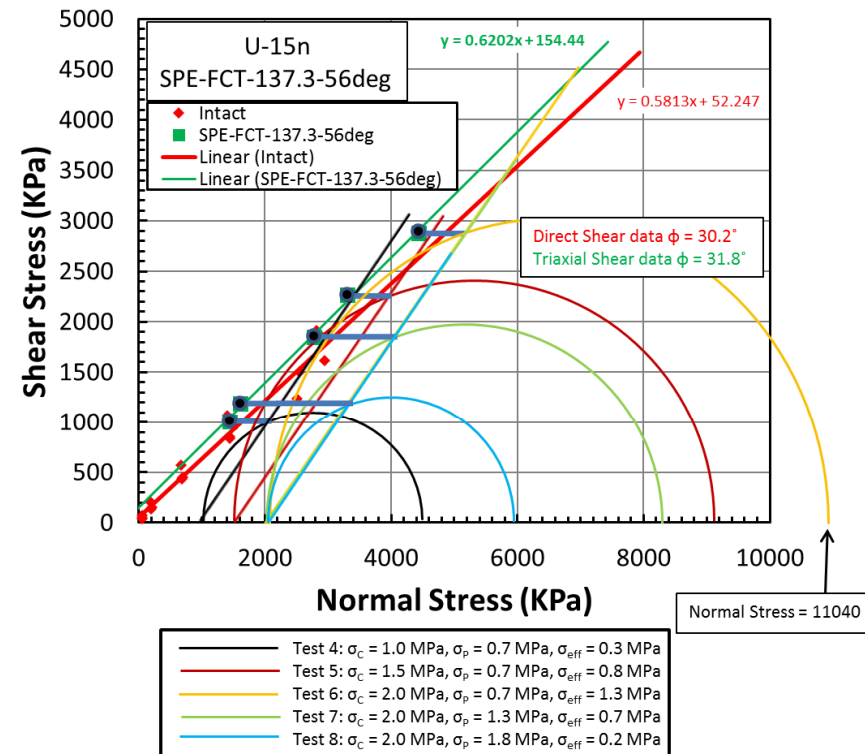
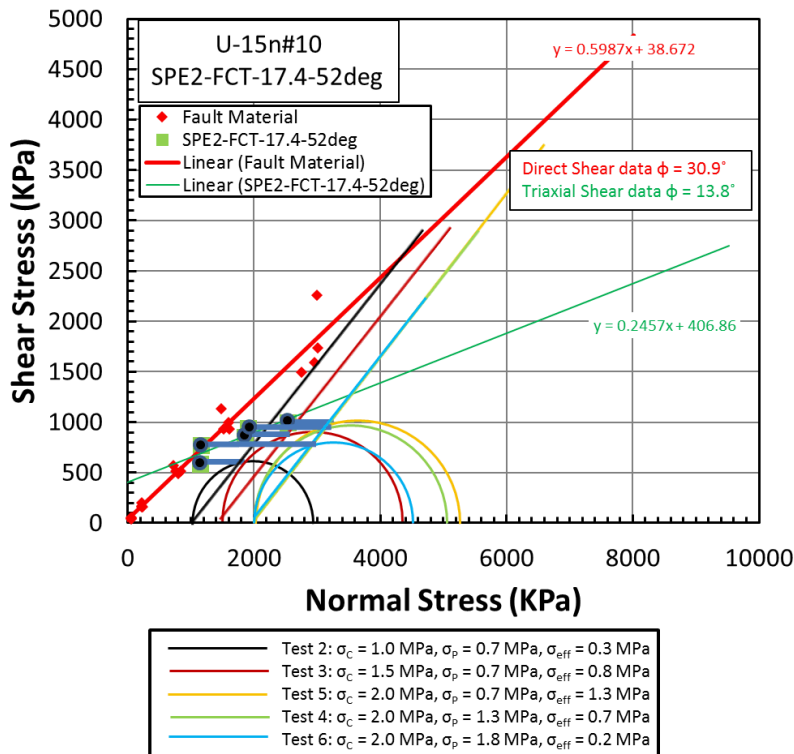
- Core from source hole U-15n (pre SPE) and U-15n#10 (post SPE2)
- Determine Friction angle and cohesion of natural fractures
- Dry and saturated testing conditions
- Repetitive shearing performed



Core Scale Fracture Property Analyses

Triaxial Shear on Natural Fractures

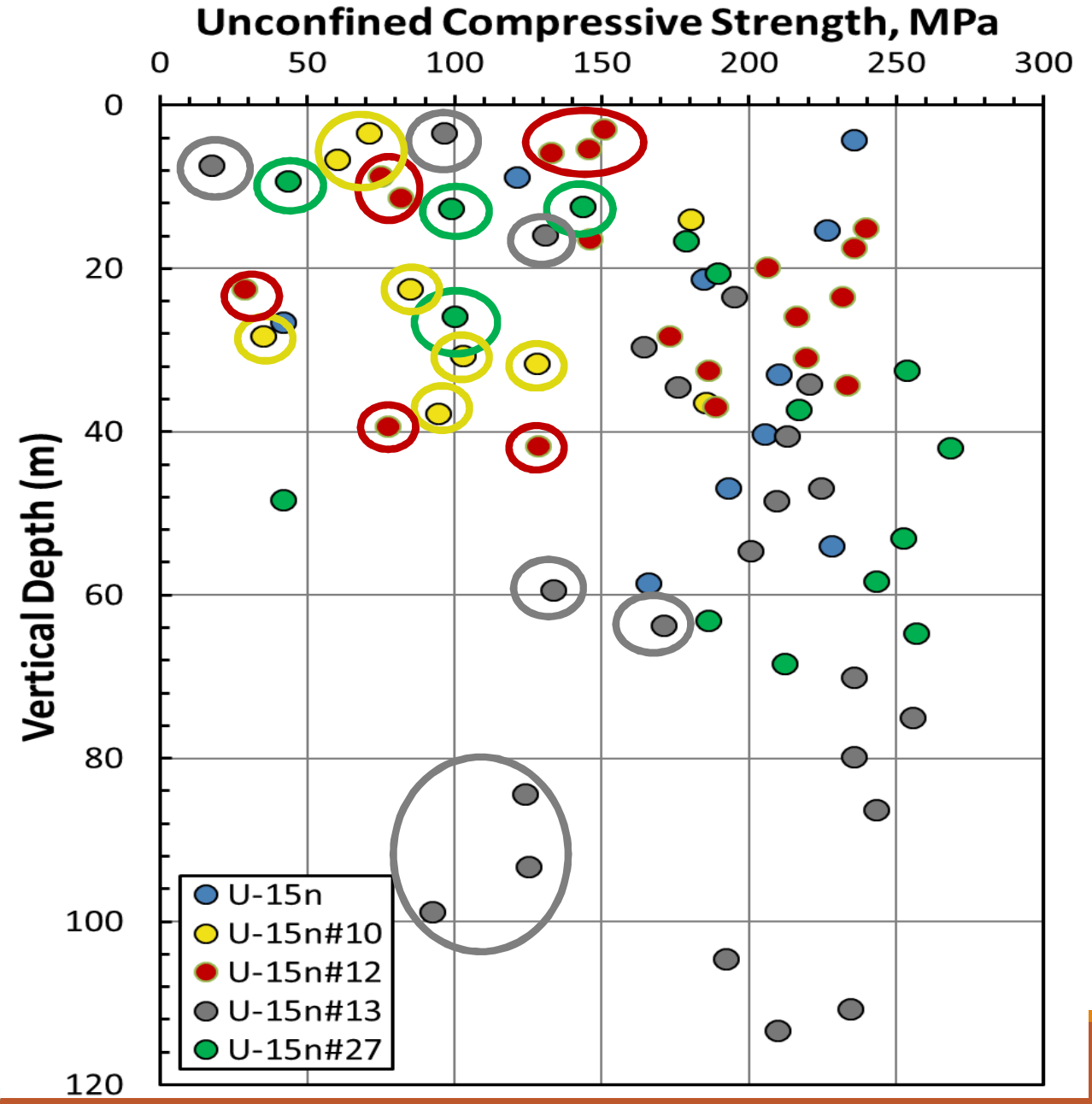
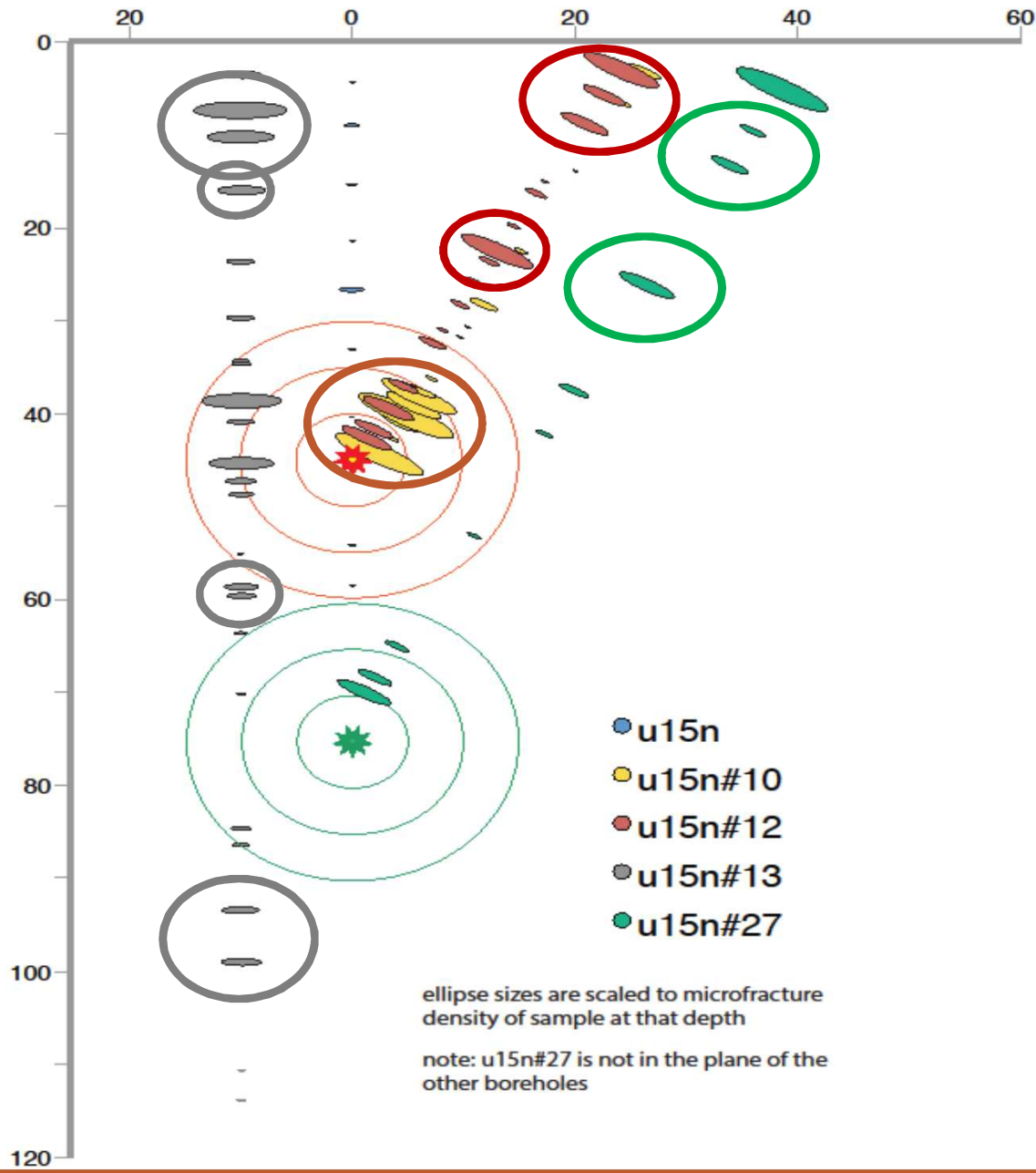
- Core from source hole U-15n (pre SPE) and U-15n#10 (post SPE2)
- Determine Friction angle and cohesion
- Incorporate fracture surface fluid pressure and confining pressure
- Characterization of wet weathered material possible



Grooves cut in rock to facilitate transmission of pore fluid to natural fracture surface.

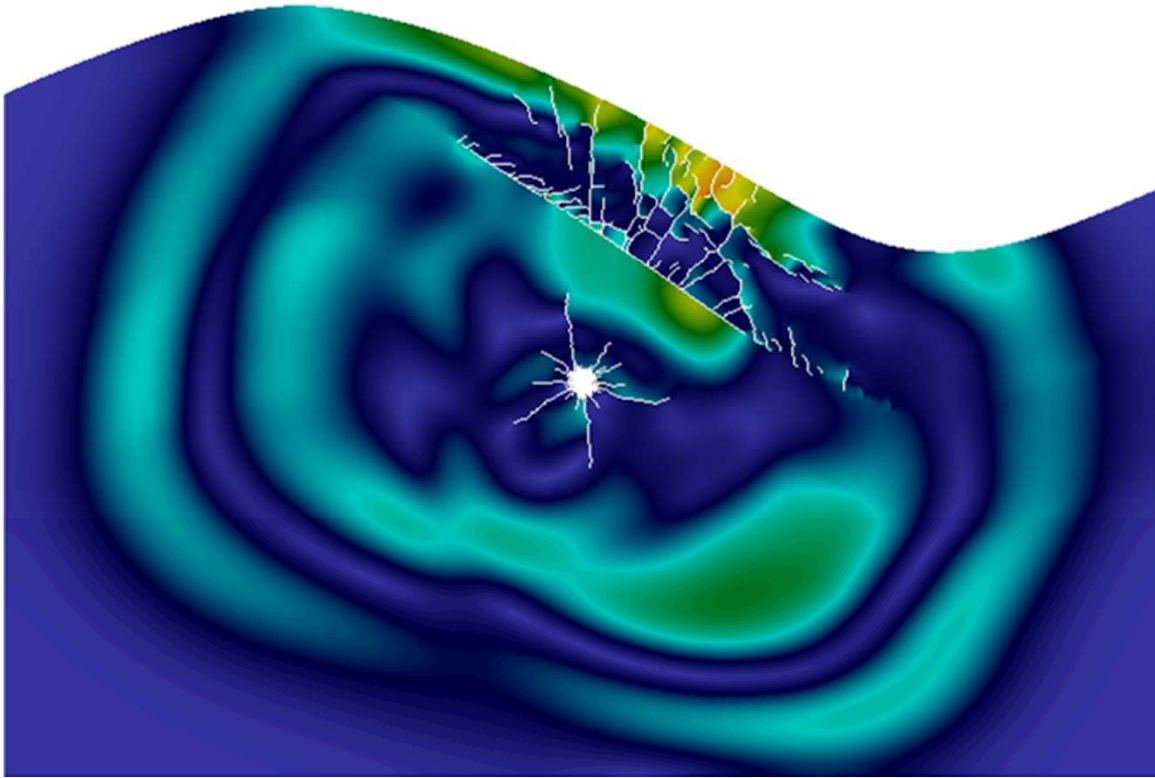


Linking Micro to Core Scale Material Properties

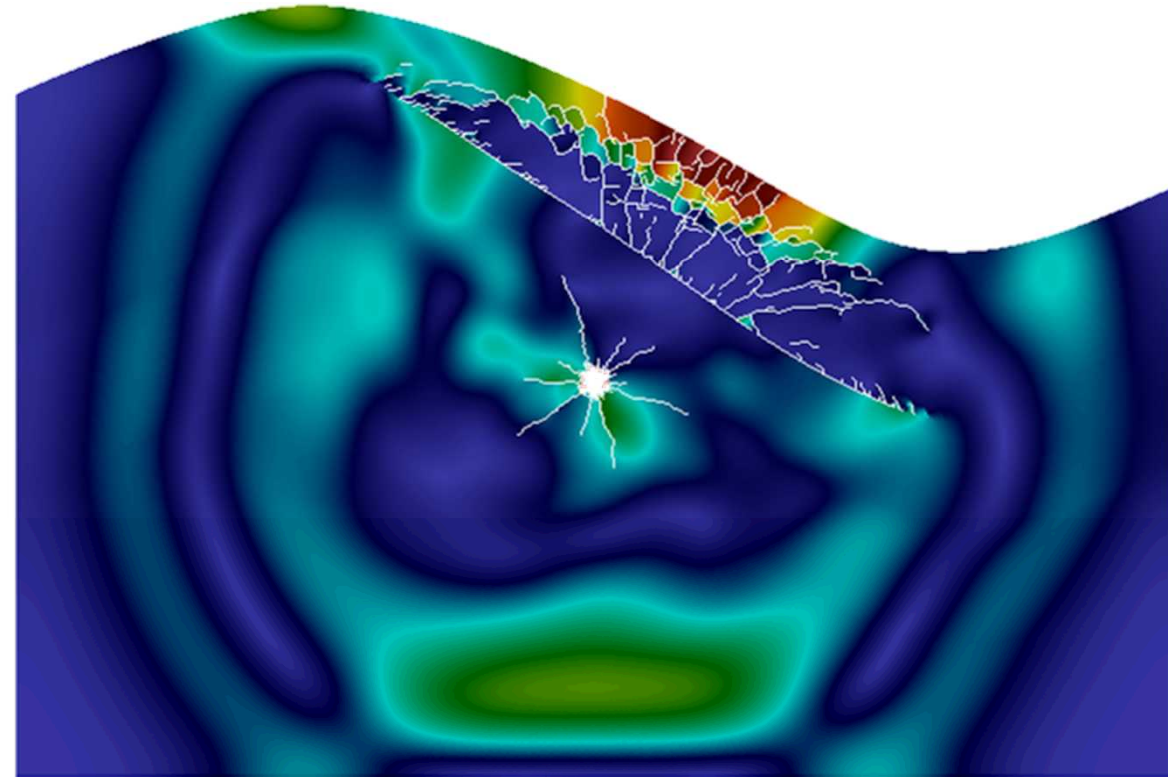


Micro-scale Material and Fracture Property Analyses

Optical microscopy data incorporated into models

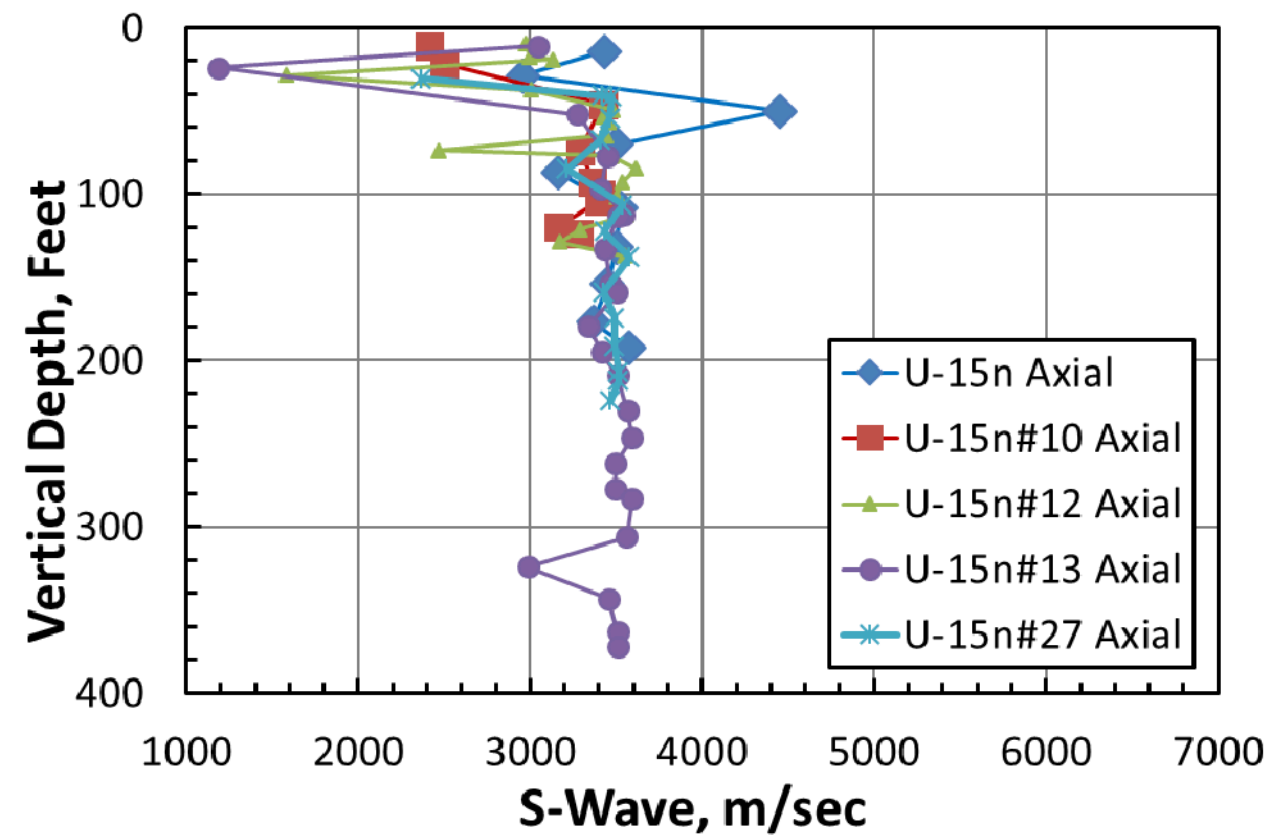
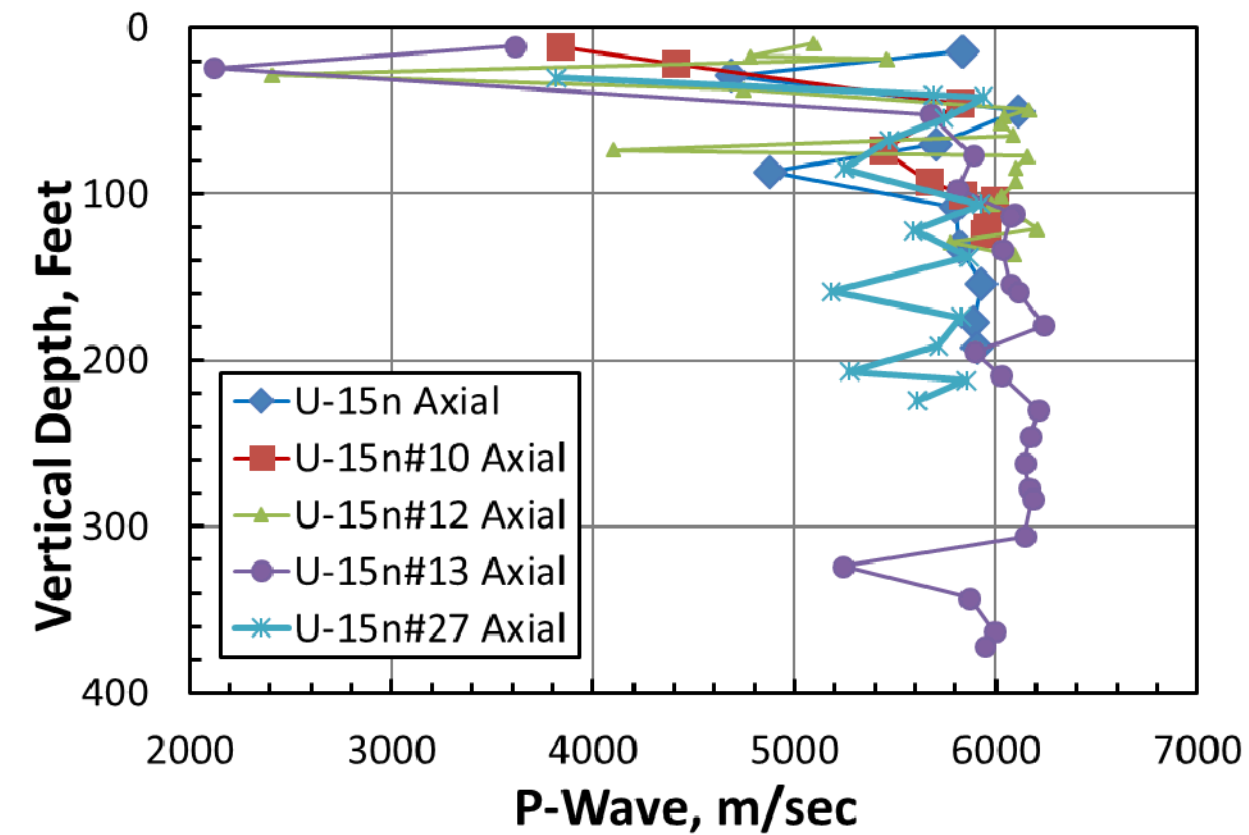


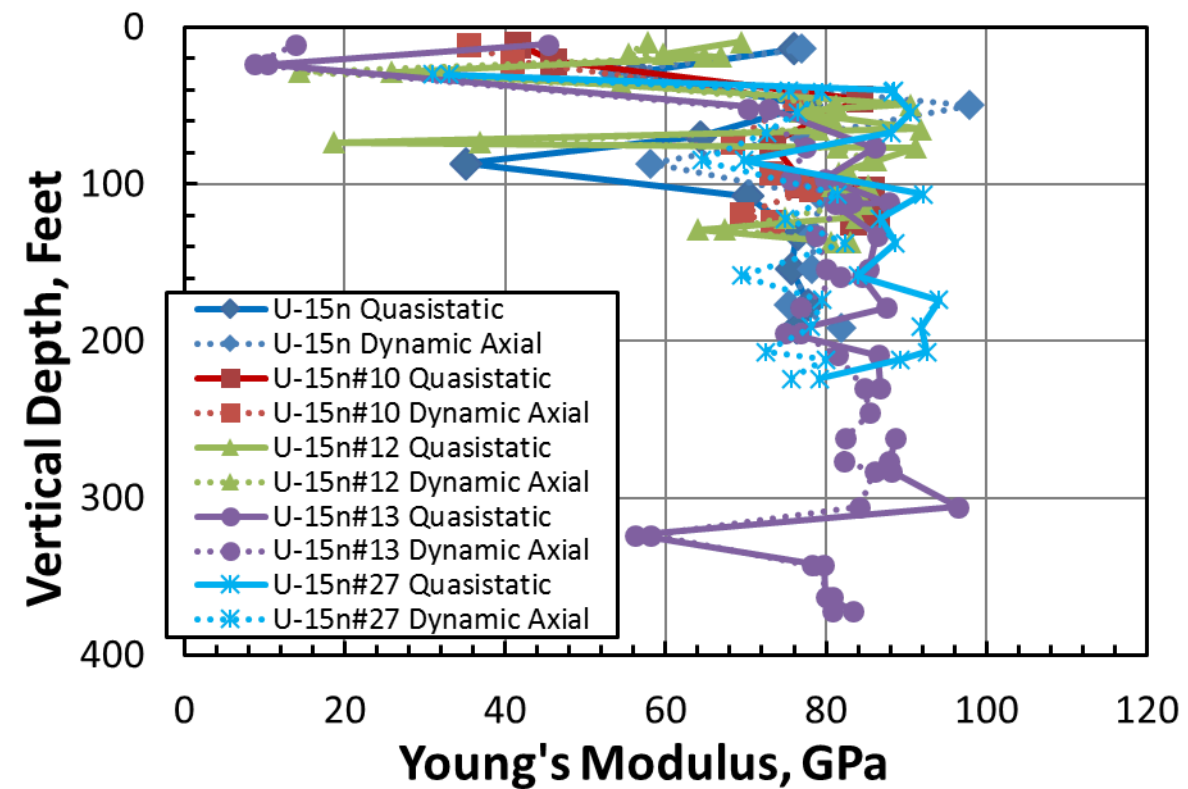
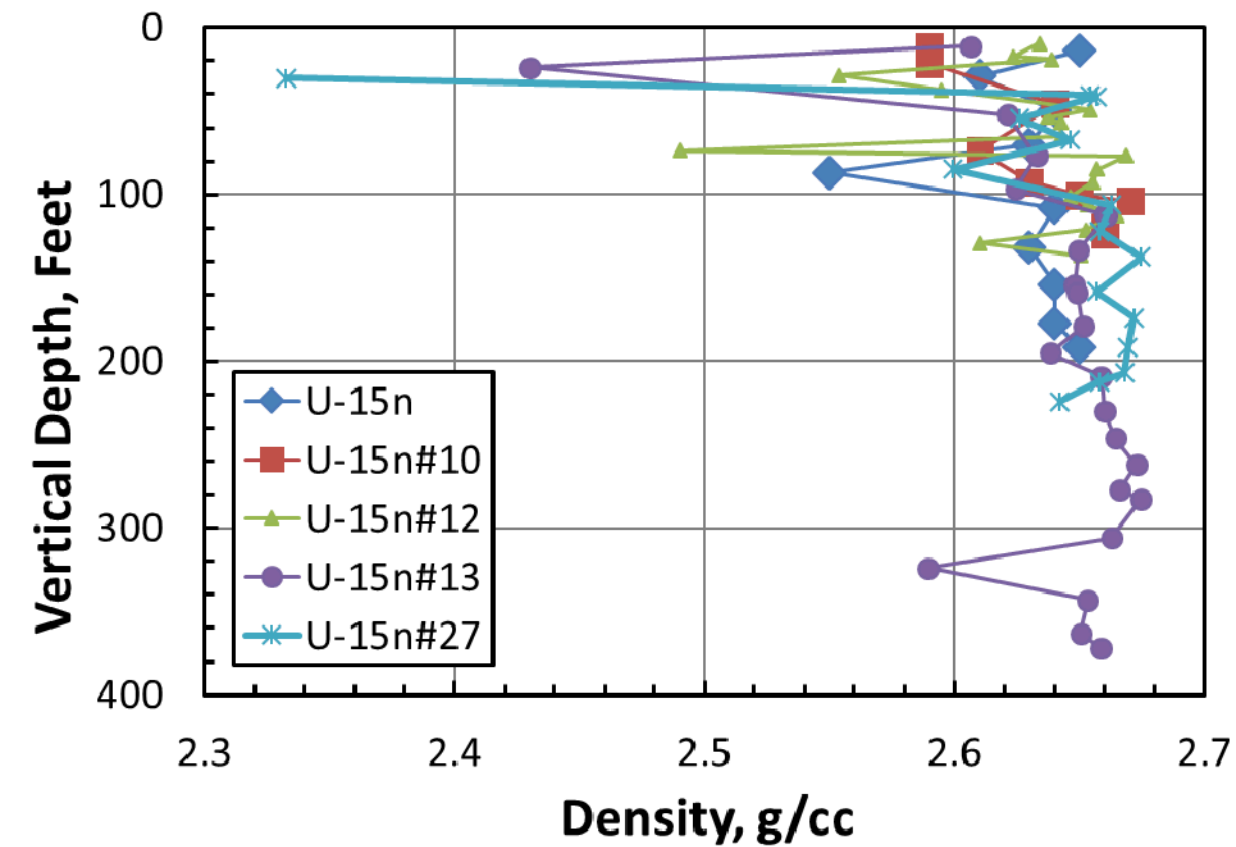
Anisotropic Medium

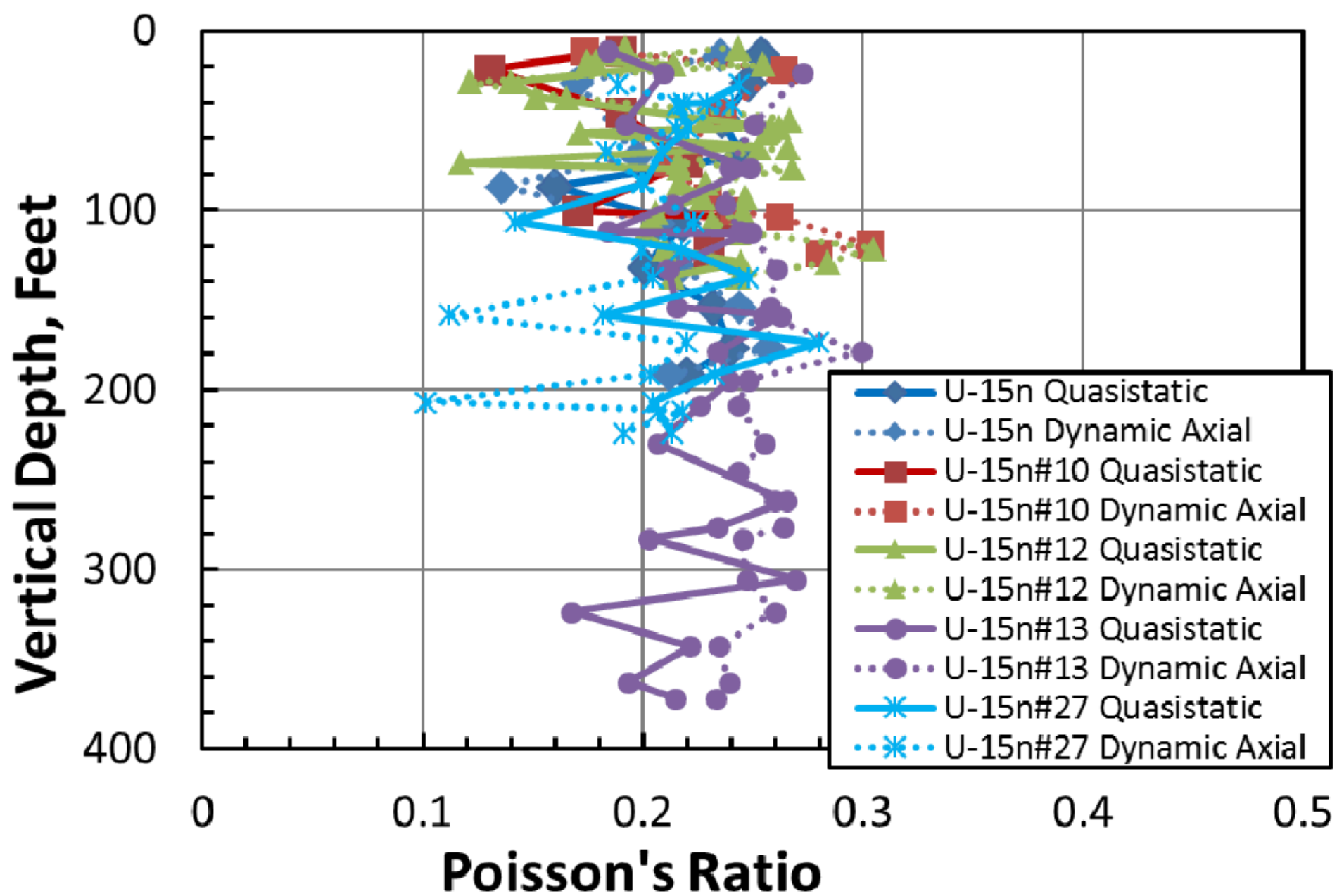


Isotropic Medium

Backup slides







Normalized SPE Phase 1 Material Properties

