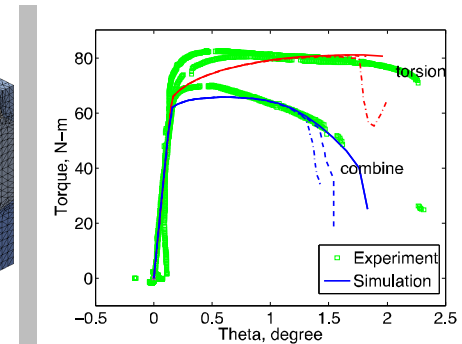
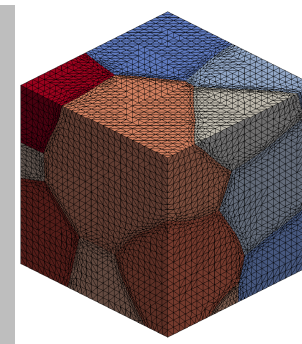
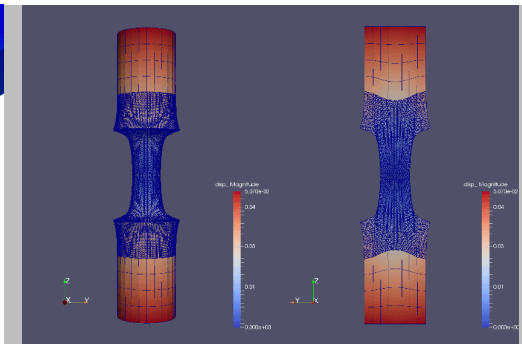
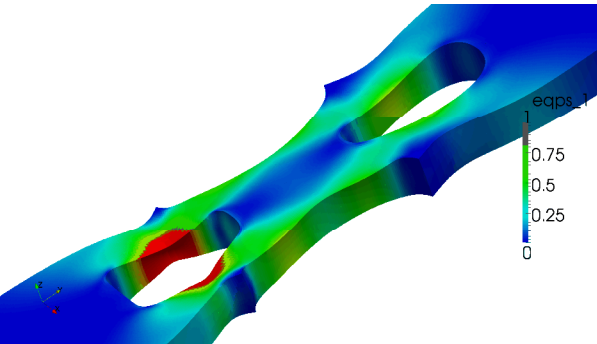


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# Mechanics of Materials Department

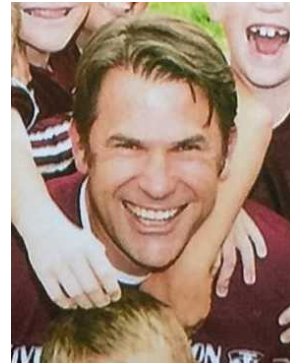
## Computational Solid Mechanics

# Who we are

---



**Alejandro Mota**  
PhD Structural Engineering  
Cornell University



**Jay Foulk**  
PhD Mechanical Engineering  
University of California Berkeley



**Jake Ostien**  
PhD Mechanical Engineering  
University of Michigan Ann Arbor



**Coleman Alleman**  
PhD Civil Engineering  
Johns Hopkins University

# The problems we address

---

- **Transient or steady state behavior of solids and structures.**
- **Materials and structures subjected to very large deformations.**
- **Damage and failure of materials and structures.**
- **Crack initiation and propagation.**
- **Fracture and fragmentation.**

# Our approach

- Use mathematics, solid mechanics and computer science to understand and predict the behavior of solids and structures.
- Start from fundamental physical principles.
- Maintain mathematical rigor.
- Acknowledge that experiment is the ultimate arbiter.

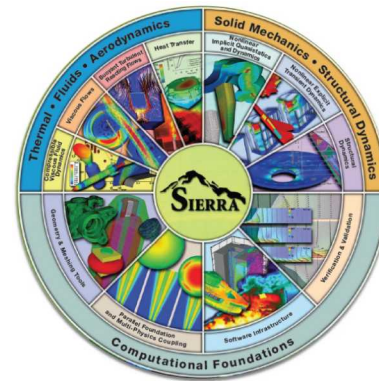


[www.trilinos.org](http://www.trilinos.org)



[github.com/gahansen/Albany](https://github.com/gahansen/Albany)

**Research, Open Source**

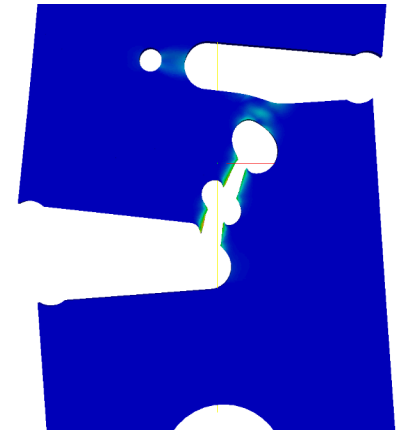
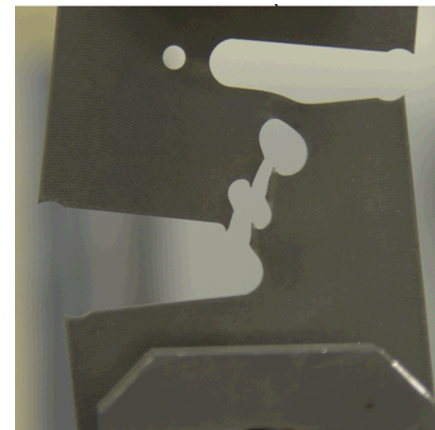
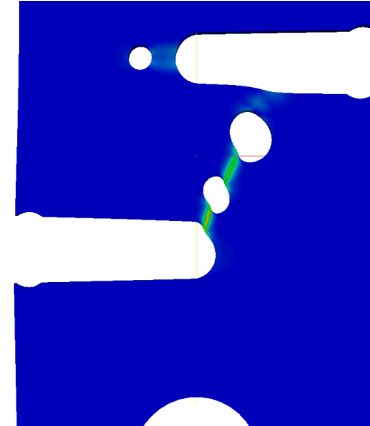
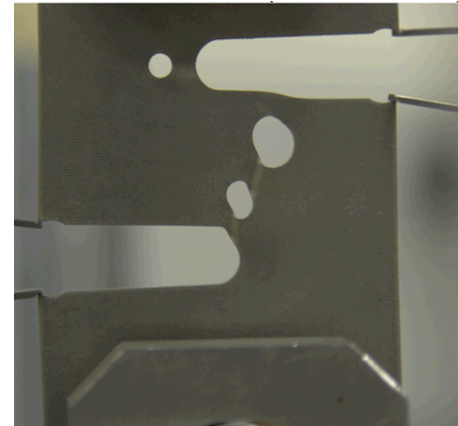
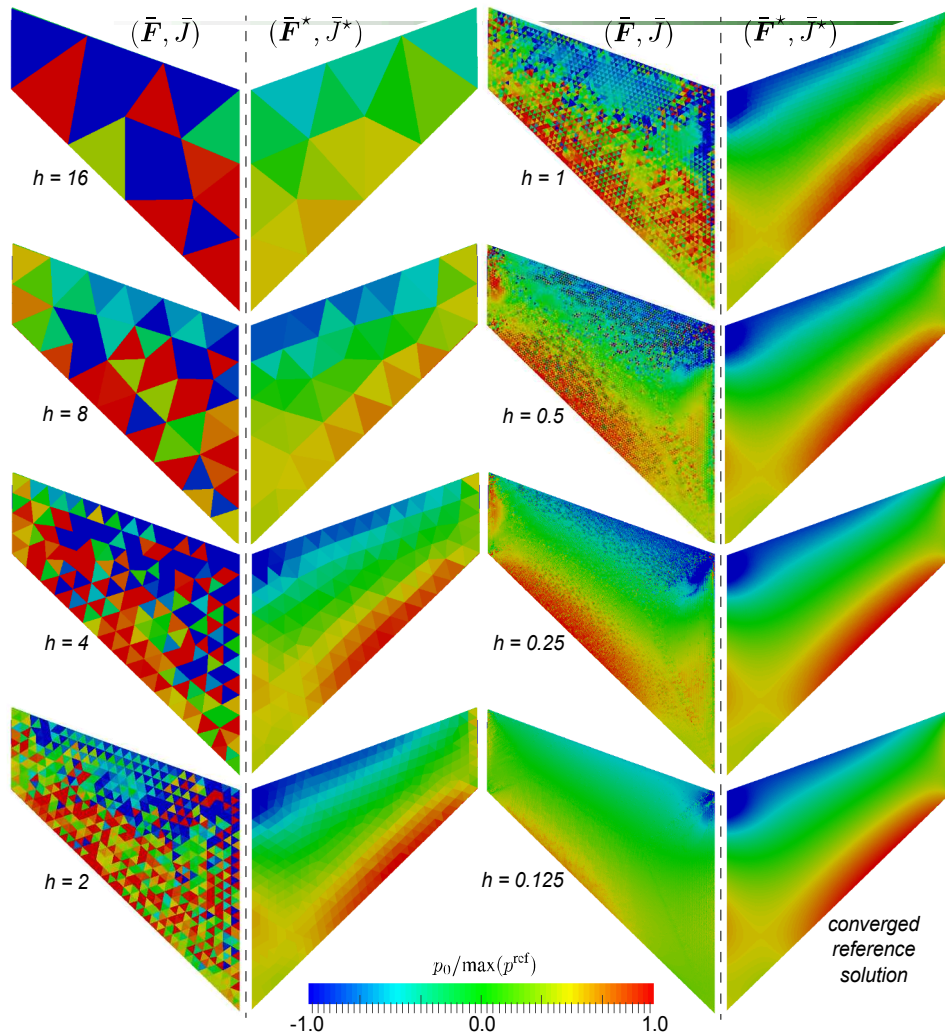


**Production, Sandia Proprietary**



- **Finite-Deformation Solid Mechanics**
- **Constitutive Behavior of Materials**
- **Finite Element Methods**
- **Coupled Physics**
- **Multiscale Modeling and Coupling**
- **Remeshing and Mesh Adaptation**
- **Damage, Failure and Fracture Mechanics**

# Element Technology, Fracture



Accurate Pressure

# Finite Deformation, Regularization, Schwarz Coupling

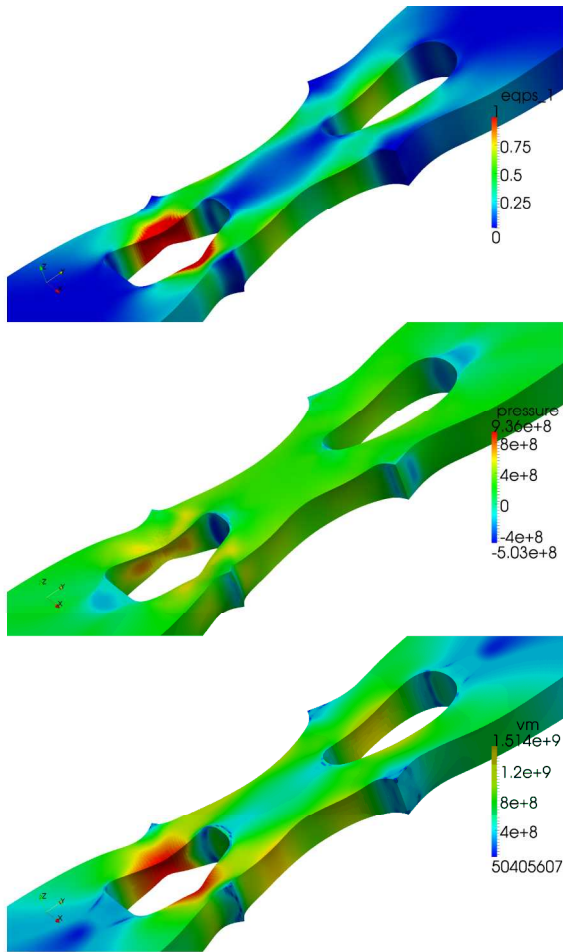
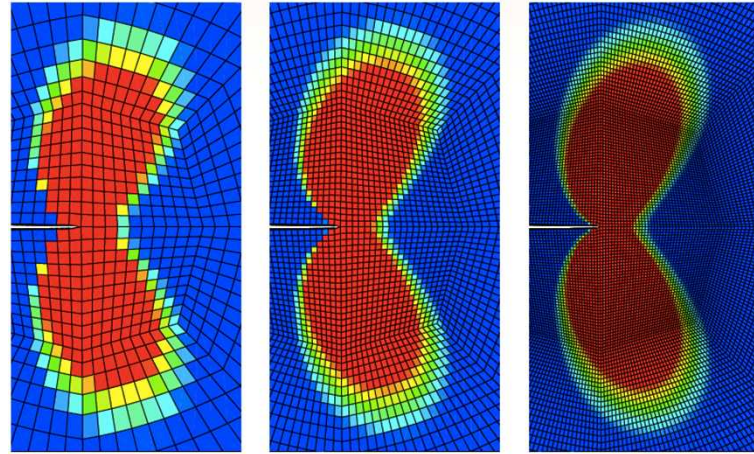
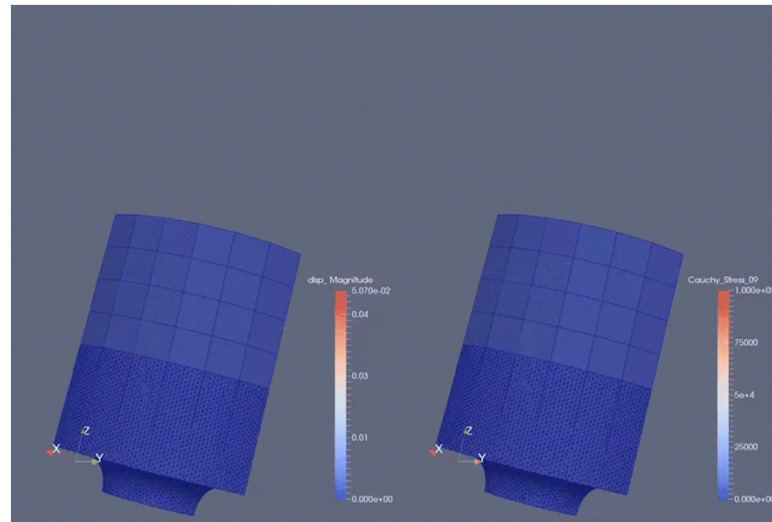


Figure 11: Zoomed view of deformed geometry and fields, equivalent plastic strain (eqps), pressure [Pa], and von Mises stress (vm) [Pa]



Contours of equivalent plastic strain, (0.001), mesh sizes  $60\mu m$ ,  $30\mu m$ ,  $15\mu m$

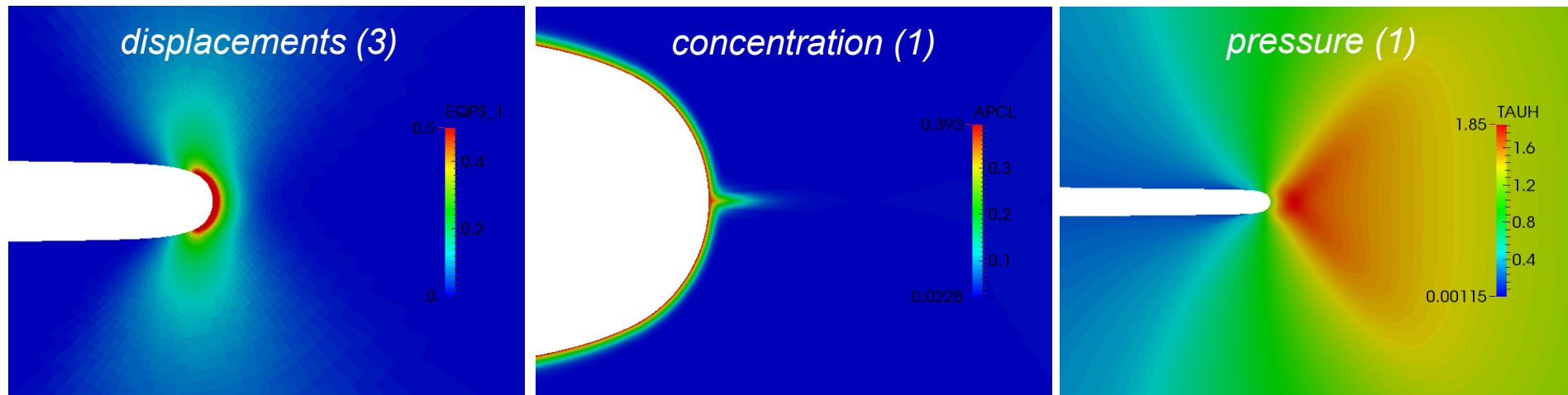


Schwarz Coupling

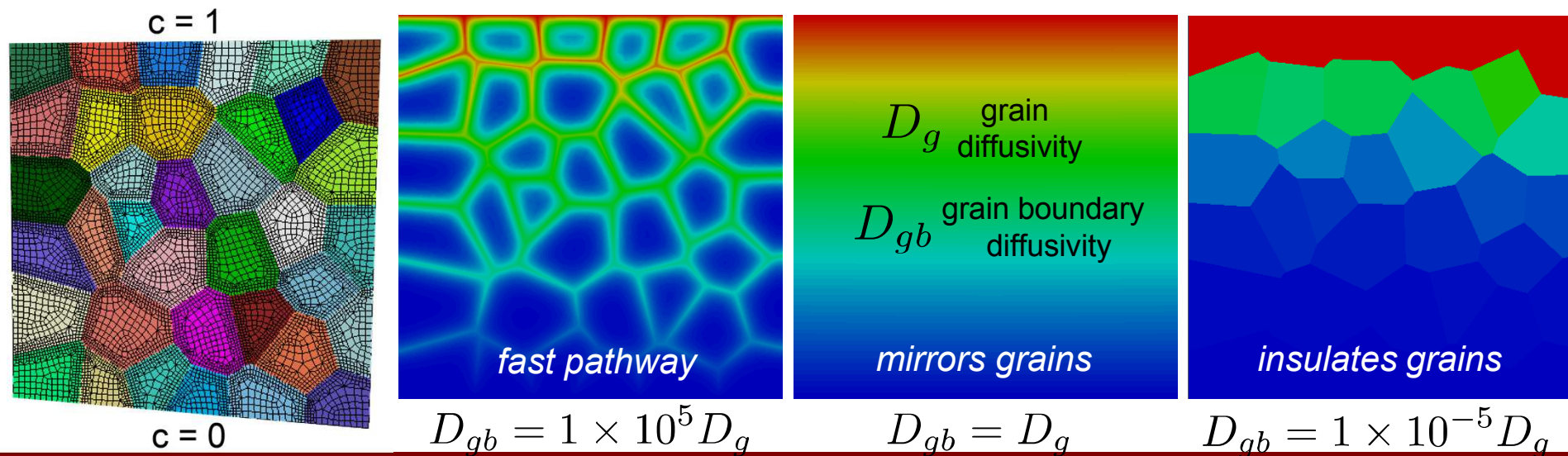


# Strong Chemo-Mechanical Coupling

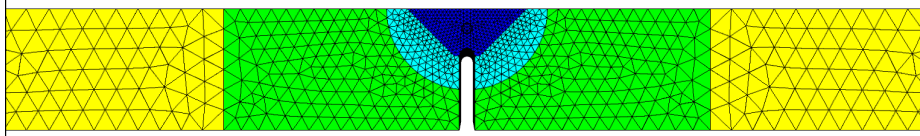
Block solve for displacement, concentration, and pressure at a crack tip



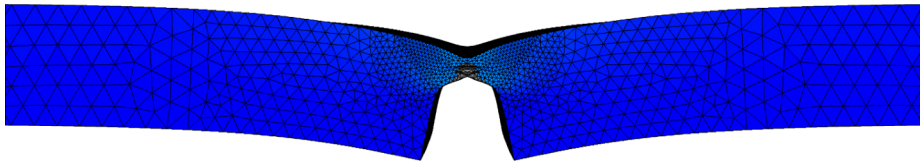
Exploring fast pathways through the inclusion of surface elements on grain boundaries



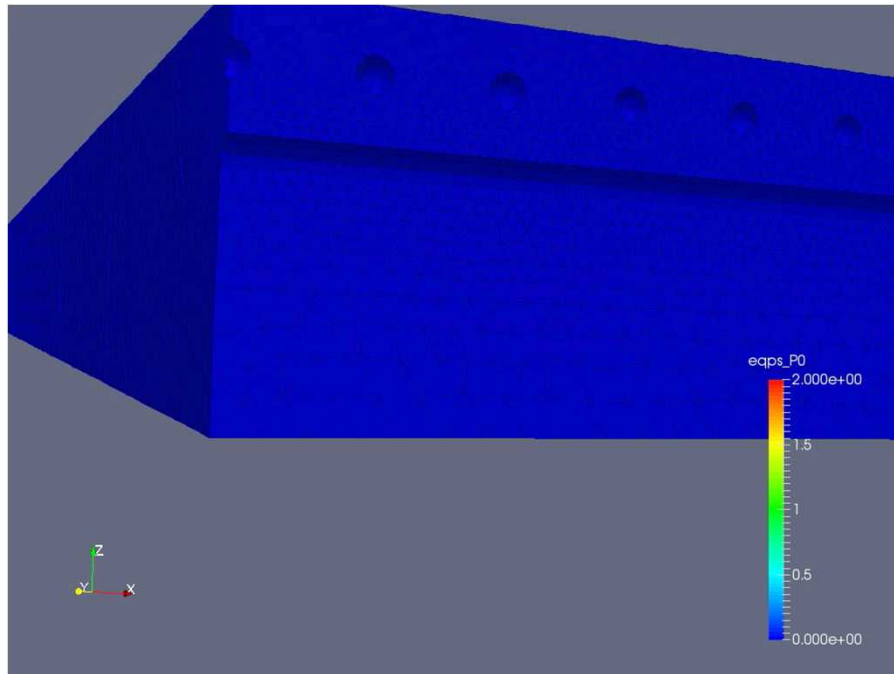
# Mesh Adaptation



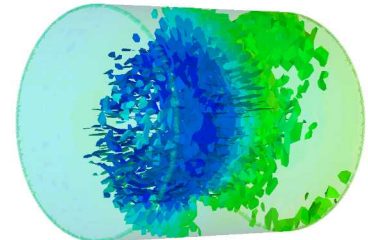
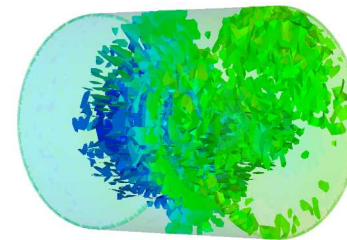
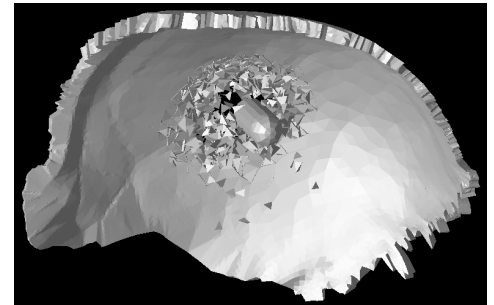
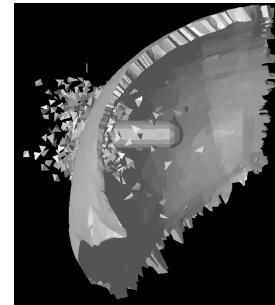
undeformed mesh  
with notch



necking at  
mid-plane

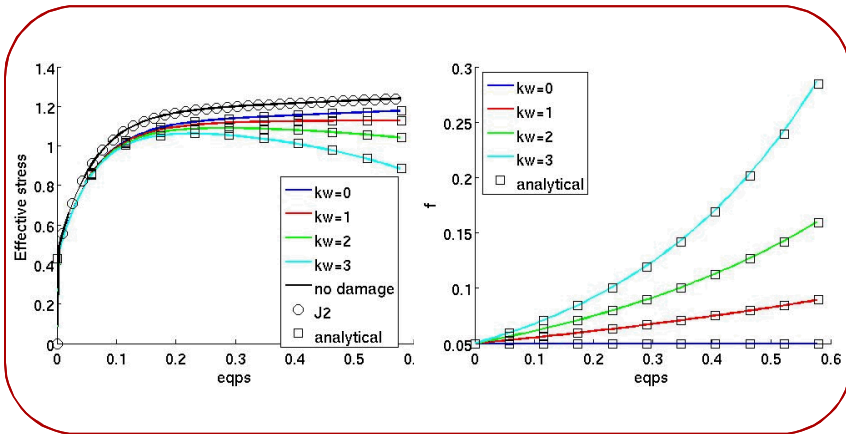


Resolving the evolution of pores in laser welds

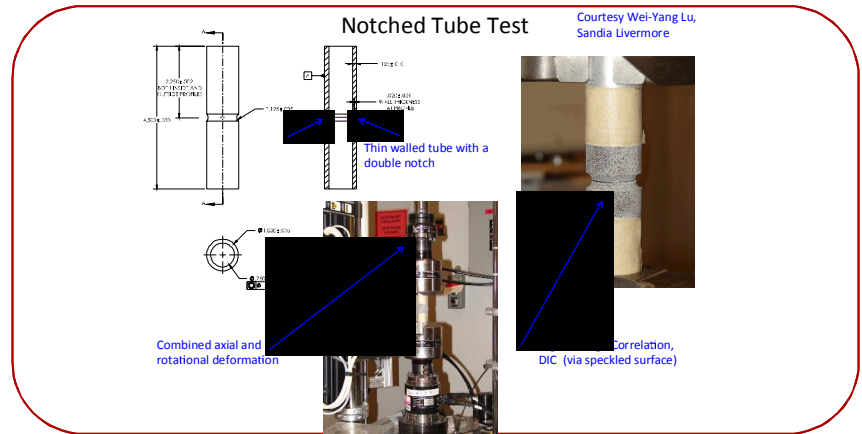


Multiple crack paths and fragmentation

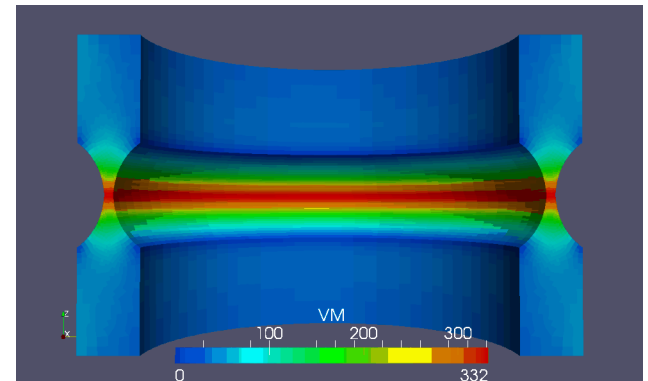
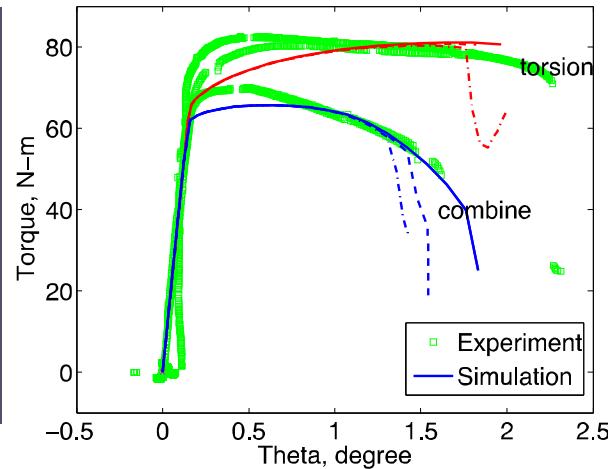
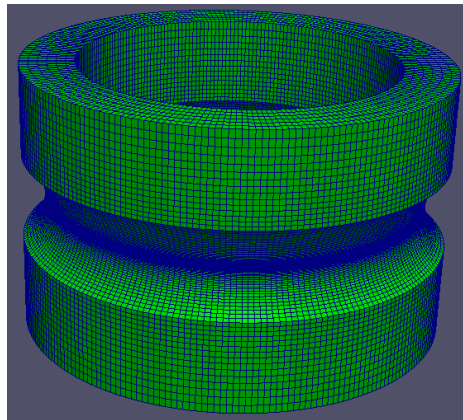
# Constitutive Model Development, Verification, and Validation



Verification in simple shear

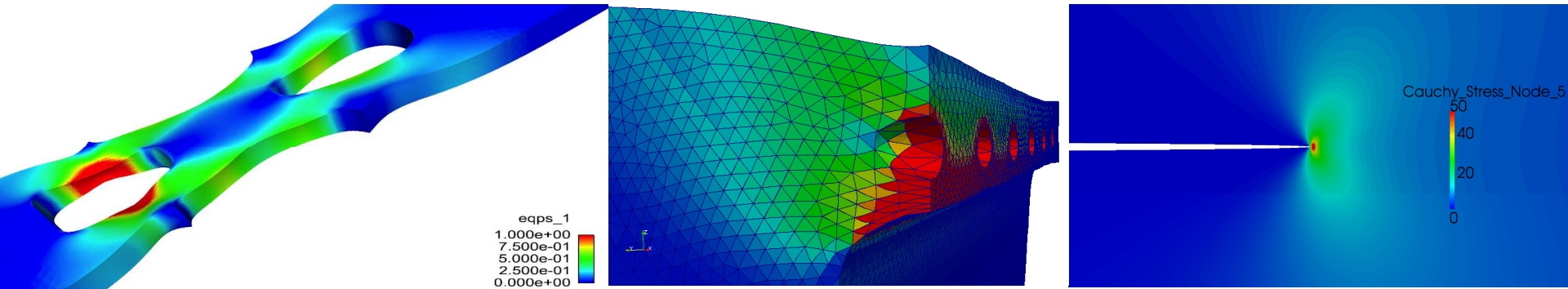


Validation experiments (courtesy W. Lu)



Preliminary simulation results and validation comparisons

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## The Laboratory for Computational Mechanics

Jakob T. Ostien, James W. Foulk III, Alejandro Mota, Glen Hansen, Andy Salinger,  
Mike Veilleux, John Emery, Coleman Alleman



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The graph illustrates the hierarchical and interdependent relationships between various residuals in the HDiffusionDeformationMatterResidual module. The central node is HDiffusionDeformationMatterResidual<Residual>, which is connected to several other residuals. The graph shows a complex network of dependencies, with many residuals having multiple incoming and outgoing edges. The edges are labeled with numbers, likely representing the strength or type of the relationship. The graph is organized into several layers, with the central node at the top and the Gather Coordinate Vector<Residual> at the bottom. The graph is a directed graph, with arrows indicating the direction of the relationships. The graph is a complex network of nodes and edges, with many nodes having multiple incoming and outgoing edges. The graph is a directed graph, with arrows indicating the direction of the relationships. The graph is a complex network of nodes and edges, with many nodes having multiple incoming and outgoing edges. The graph is a directed graph, with arrows indicating the direction of the relationships.

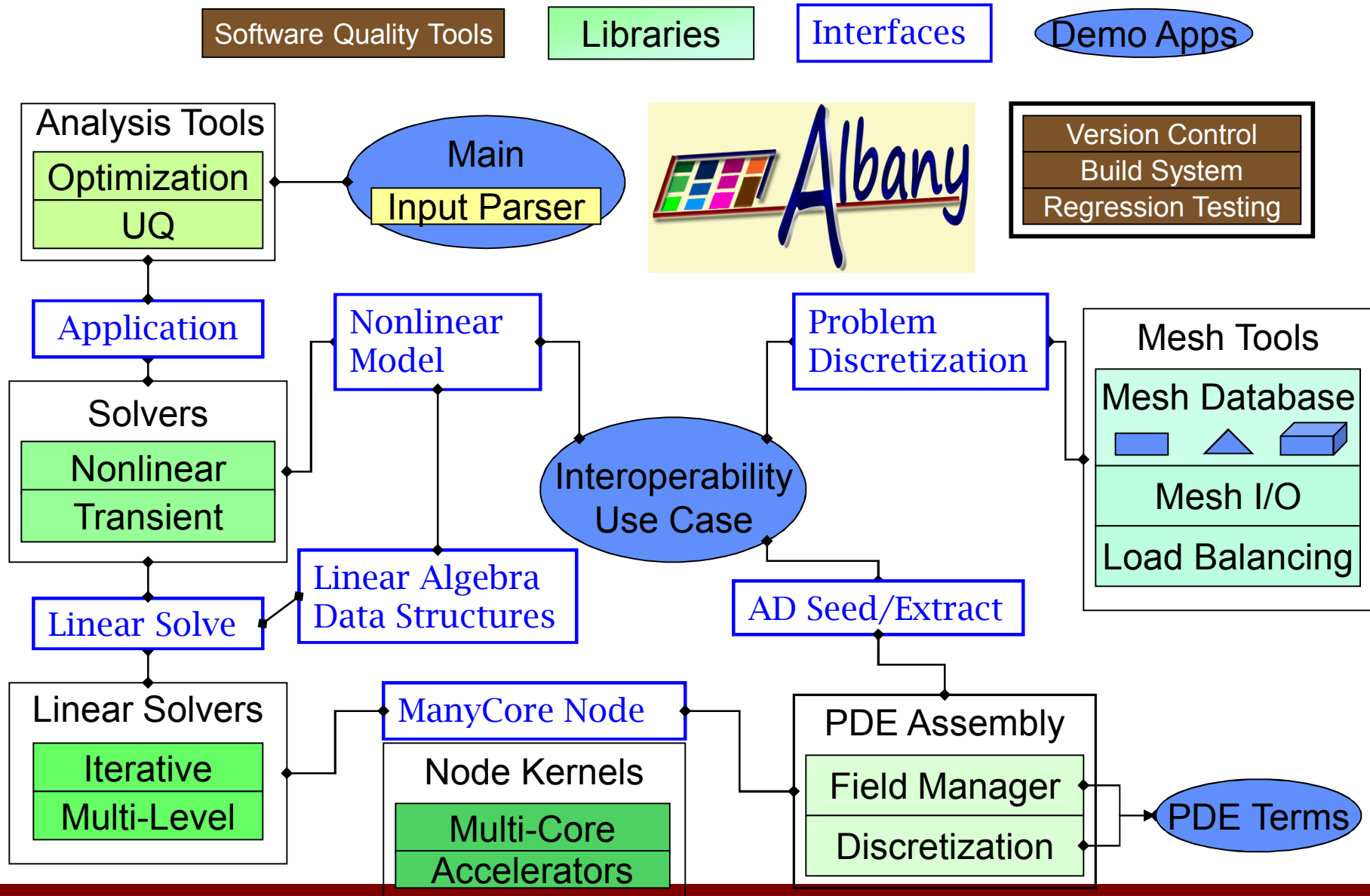
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# Trilinos and Albany

- The Trilinos Project <http://trilinos.sandia.gov>
  - Run out of the Computational Mathematics Center at Sandia NM
  - Consists of a collection of “packages” that address many problems of discretizing and solving PDEs
    - Finite element and topology libraries
    - Nonlinear solvers
    - Linear solvers
  - Additional utilities pertain to advanced and specialized features
    - Template based generic programming → Automatic Differentiation!
    - Multi-physics dependency management
  - And more...
- Albany
  - Originally established to test installations of Trilinos
  - Transitioned to be the testbed and first friendly user for advanced Trilinos packages
  - Was adopted by multiple LDRD projects (as well as the LCM project) as a code base
  - Stable and growing *developer* community
  - Creature comforts for *users* still in early stages

# Anatomy of a Component-Based Application



# Reusable Math Libraries and Software Components

## Analysis Tools (black-box)

Optimization
UQ (sampling)
Parameter Studies
V&V, Calibration
OUU, Reliability

## Analysis Tools (embedded)

Nonlinear Solver
Time Integration
Continuation
Sensitivity Analysis
Stability Analysis
Constrained Solves
Optimization
UQ Solver

## Linear Algebra

Data Structures
Iterative Solvers
Direct Solvers
Eigen Solver
Preconditioners
Matrix Partitioning

## Architecture- Dependent Kernels

Multi-Core
Accelerators

## Composite Physics

MultiPhysics Coupling
System Models
System UQ

## Mesh Tools

Mesh I/O
Inline Meshing
Partitioning
Load Balancing
Adaptivity
Remeshing
Grid Transfers
Quality Improvement
DOF map

## Utilities

Input File Parser
Parameter List
Memory Management
I/O Management
Communicators

## PostProcessing

Visualization
Verification
Model Reduction

## Mesh Database

Mesh Database
Geometry Database
Solution Database

## Data-Centric Algs

Graph Algorithms
SVDs
Map-Reduce
Linear Programming
Network Models

## Software Quality

Version Control
Regression Testing
Build System
Backups
Verification Tests
Mailing Lists
Unit Testing
Bug Tracking
Performance Testing
Code Coverage
Porting
Web Pages
Release Process

## Local Fill

Discretizations
Discretization Library
Field Manager

## Derivative Tools

Sensitivities
Derivatives
Adjoints
UQ / PCE Propagation

## Physics Fill

Element Level Fill
Material Models
Objective Function
Constraints
Error Estimates
MMS Source Terms

# The LCM project

- Project Objectives
  - Provide an open, collaborative computational mechanics research environment
  - Accelerate research transition into the production analysis environment
- Adopted Albany as a code base
  - Creates ties with the 'Math Guys'
  - Leverages existing repository, version control, testing infrastructure
- Infrastructure allows for spatially dependent boundary conditions (e.g. torsion) and parameters (e.g. shear modulus), consistent coupled physics dependence (e.g. temperature dependent flow rule), and some embedded uncertainty algorithms (e.g. stochastic Galerkin)
- Support for linear/nonlinear elasticity, inelasticity
- Current work is focusing on adaptivity and coupled physics
  - Projects in these areas use Albany to test out ideas, provide some funding to enable Albany to do what is necessary for the research
- Current limitations
  - No robust explicit time integration
  - No contact
  - Usability has not been a focus (i.e. XML input files)

# Getting Started

- Abstraction #1 ModelEvaluator
  - Abstraction for a discretized PDE
  - In principle communicates with the solver, time integrator
  - In practice, time integration for LCM is computed in Albany
  - Keeps as data an 'Application', and a key evalModel() method
- Abstraction #2 Application
  - An Application owns a 'Discretization' (finite element mesh, fields)
  - Also owns a 'Problem'
  - General methods to compute system Residual, Jacobian, Tangent
- Abstraction #3 Discretization
  - Interface to allow for multiple mesh representations
  - Currently the primary mesh representation is from the Sierra ToolKit (STK) available from Trilinos
- Abstraction #4 Problem
  - Here be physics, degrees of freedom, implementation of equations
  - Composed of a series of 'Evaluators' that compute intermediate quantities in the Residual
- Abstraction #5 Evaluators
  - Atomic computations with managed dependencies
  - Examples, basis functions, strain, conductivity, permeability, residual forces

## ■ Generic Internal Force Algorithm

- Primary solution variable is the displacement vector
- finite element basis used to compute the displacement and deformation gradient

$$\mathbf{H} = \frac{\partial \mathbf{u}}{\partial \mathbf{X}}; \quad \mathbf{F} = \mathbf{1} + \mathbf{H}$$

- For Hyperelastic constitutive laws, compute Cauchy stress as a function of deformation gradient

$$\boldsymbol{\sigma} = J^{-1} \frac{\partial \Psi}{\partial \mathbf{F}} \mathbf{F}^T$$

- For Hypoelastic constitutive laws, the rate of deformation and stress are computed from the logarithmic mapping of the incremental deformation gradient

$$\mathbf{l} = \dot{\mathbf{F}} \mathbf{F}^{-1} = \frac{\log \mathbf{f}}{\Delta t}; \quad \mathbf{f} = \mathbf{F}_{n+1} \mathbf{F}_n^{-1}; \quad \boldsymbol{\sigma} = \mathcal{F}(\text{sym } \mathbf{l})$$

- Internal force is computed as an integral over the reference configuration

$$\mathbf{f}^{int} = \int_{\Omega_0} \nabla_X \cdot \mathbf{P} dV; \quad \mathbf{P} = J \boldsymbol{\sigma} \mathbf{F}^{-T}$$



- Generic Tensor Library
  - Tensor (rank 1, 2, 3, 4) manipulation is handled via a Tensor class
  - Construction is dynamic and arbitrary
  - Scalar, Vector, Tensor arithmetic operations defined
  - A number invariants, decompositions (SVD, Polar), logarithmic and exponential mappings, and other goodies
  - Sample Code

```
LCM::Tensor<ScalarT> A = LCM::eye<ScalarT>(3);
LCM::Tensor<ScalarT> B(3);
LCM::Tensor<ScalarT> C(3);
LCM::Vector<ScalarT> u(3);

A = 2.0 * A;
A(1, 0) = A(0, 1) = 1.0;
A(2, 1) = A(1, 2) = 1.0;

B = LCM::inverse(A);

C = A * B;

TEST_COMPARE( LCM::norm(C - LCM::eye<ScalarT>(3)), <=,
              LCM::machine_epsilon<ScalarT>());

ScalarT I1 = LCM::I1(A);
ScalarT I2 = LCM::I2(A);
ScalarT I3 = LCM::I3(A);

u(0) = I1 - 6;
u(1) = I2 - 10;
u(2) = I3 - 4;

TEST_COMPARE( LCM::norm(u), <=, LCM::machine_epsilon<ScalarT>());
```

# Code Details

- Local Systems of Equations
  - Target is constitutive models that have internal state variables with evolution equations that produce a nonlinear system at each material point
  - Automatic Differentiation is exploited to compute the Jacobian of the nonlinear system in some cases
  - Matrix and vector are packaged up and shipped to LAPACK
  - Global system sensitivities are preserved by the LocalNonlinearSolver class
  - For usage example reference the utLocalNonlinearSolver unit test, GursonFD, CapImplicit
  - Example code

```
LocalNonlinearSolver<EvalT, Traits> solver;

std::vector<ScalarT> F(1);
std::vector<ScalarT> dFdX(1);
std::vector<ScalarT> X(1);

F[0] = f;
X[0] = 0.0;
dFdX[0] = ( -2. * mubar ) * ( 1. + H / ( 3. * mubar ) );
while (!converged && count < 30)
{
    count++;

    solver.solve(dFdX,X,F);

    ScalarT X0 = X[0];
    alpha2 = eqpsold(cell, qp) + sq23 * X0;
    H2 = K * alpha2 + signf*( 1. - exp( -delta * alpha2 ) );
    dH2 = K + delta * signf * exp( -delta * alpha2 );

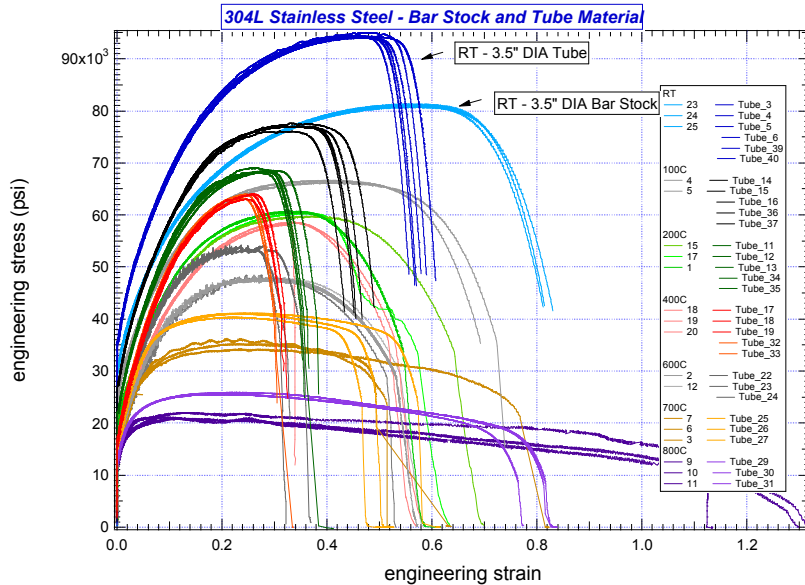
    F[0] = smag - ( 2. * mubar * X0 + sq23 * ( Y + H2 ) );
    dFdX[0] = -2. * mubar * ( 1. + dH2 / ( 3. * mubar ) );

    res = std::abs(F[0]);
    if ( res < 1.e-11 || res/f < 1.E-11 )
        converged = true;

    TEUCHOS_TEST_FOR_EXCEPTION( count > 30, std::runtime_error,
                                std::endl << "Error in return mapping, co
                                "\nres = " << res <<
                                "\nrelres = " << res/f <<
                                "\ng = " << F[0] <<
                                "\ndg = " << dFdX[0] <<
                                "\nalpha = " << alpha2 << std::endl);
}
solver.computeFadInfo(dFdX,X,F);
dgam = X[0];
```

# Motivation for Coupled Physics

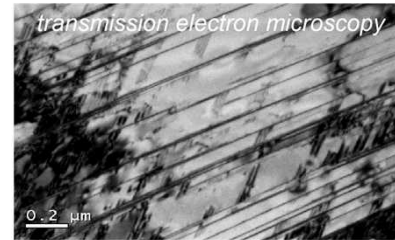
## Thermomechanical behavior



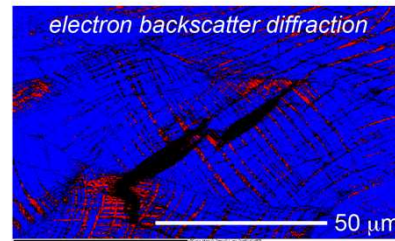
Mechanical response varies strongly with temperature

- Commonly assumptions are made
  - Isothermal at slow rates
  - Adiabatic at high rates
- Strong coupling is required as physics become richer, environment rates may span slow to fast

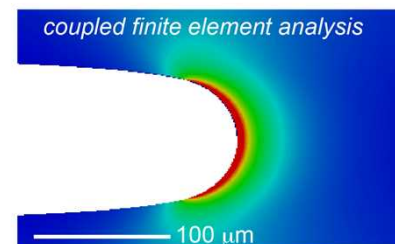
## Hydrogen embrittlement



deformation twins



phase transformations, fcc, bcc



plastic zone at blunted crack tip

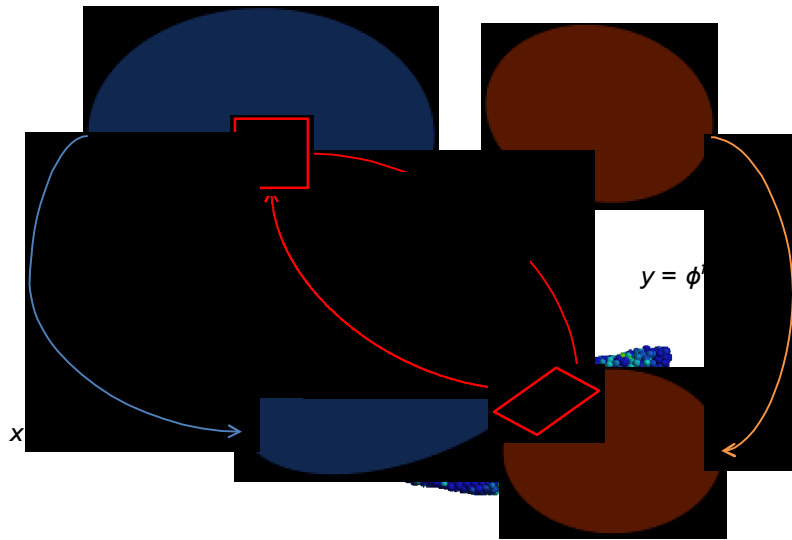
Hydrogen *activates* microstructure and *localizes* deformation processes

- Aids deformation bands/twinning ( $\text{nm}$ )
- Activates phase transformations ( $\text{nm} - \mu\text{m}$ )
- Accentuates grain boundary interactions ( $\text{nm}$ )

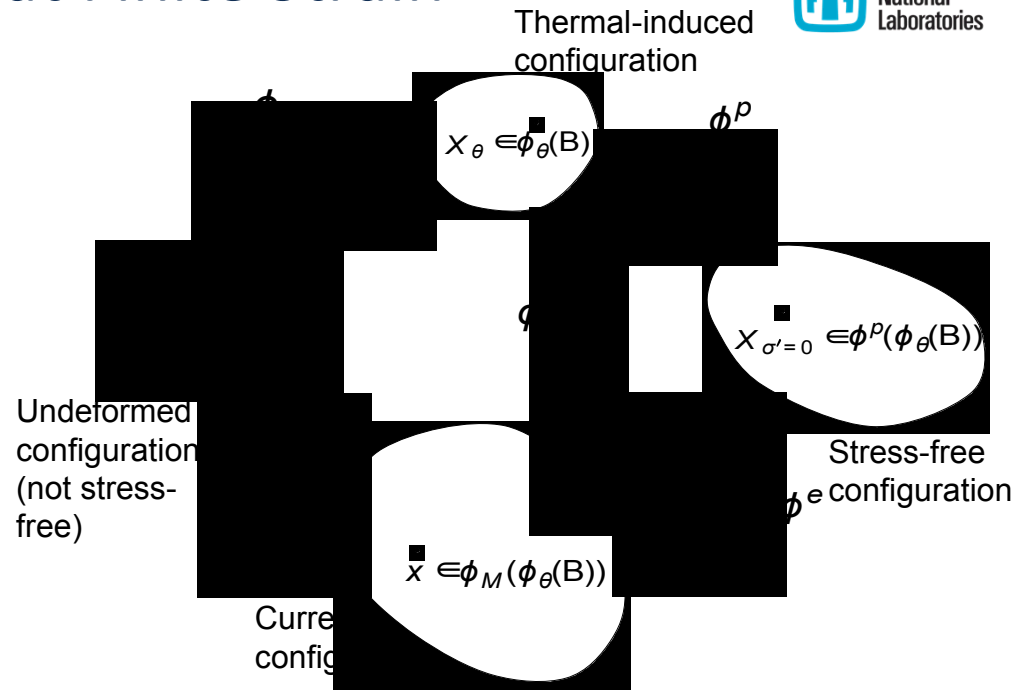
# Coupled Physics Computational Environment – Albany

- Some useful features for rapid development
  - Graph based multiphysics dependency management
  - Analytic Jacobians via Automatic Differentiation (AD), even for the cross coupling terms
  - Access to the whole suite of linear solvers (direct, iterative), nonlinear solver strategies, preconditioners (algebraic multigrid, physics based)
  - Fully parallel, with automatic mesh decomposition capabilities

# Kinematics of THM Problem at Finite Strain



Trajectories of the solid and fluid constituent.



Multiplicative decomposition of the thermo-hydro-mechanics problem

Multiplicative decomposition of skeleton deformation gradient

$$\mathbf{F} = \frac{\partial \varphi(\mathbf{X}, t)}{\partial \mathbf{X}} = \mathbf{F}_M \cdot \mathbf{F}_\theta ; \quad \mathbf{F}_\theta = \frac{\partial \varphi_\theta(\mathbf{X}, t)}{\partial \mathbf{X}} ; \quad \mathbf{F}_M = \frac{\partial \varphi_M(\mathbf{X}_\theta, t)}{\partial \mathbf{X}_\theta}$$

Isotropic tensor

$$\boldsymbol{\sigma} = \boldsymbol{\sigma}' - B p^f \mathbf{I},$$

$$P(\mathbf{F}, z, p^f, \theta) = P'(\mathbf{F}, z, \theta) - J B p^f \mathbf{F}^{-T}$$

Concept of Effective Stress

$$B = 1 - \frac{K}{K_s}$$

$$P(\mathbf{F}_M, z, p^f) = P'(\mathbf{F}_M, z) - J B p^f \mathbf{F}_M^{-T},$$

# Strong Form of THM Problem at Finite Strain

## □ Balance of Linear Momentum

$$\nabla^{\mathbf{X}} \cdot \mathbf{P} + J(\rho^s + \rho^f)\mathbf{G} = \mathbf{0}$$

where  $\mathbf{P}(\mathbf{F}_M, z, p^f) = \mathbf{P}'(\mathbf{F}_M, z) - JBp^f \mathbf{F}^{-T}$ ,

↑
↑
↑

Total Stress                      Effective Stress                      1<sup>st</sup> PK pore pressure

## □ Balance of Mass

$$\left(\frac{B}{J} - 3\alpha_s(\theta - \theta_o)\right)\dot{J} + \frac{1}{M}\dot{p}^f - 3\alpha^m\dot{\theta} + \frac{1}{\rho_f}\nabla^{\mathbf{X}} \cdot \mathbf{W} = 0.$$

where

$$\mathbf{W} = J\mathbf{F}^{-1} \cdot \mathbf{w}. \quad \text{and} \quad \mathbf{w} = \rho_f \mathbf{k} \cdot \left[ -\nabla^{\mathbf{x}} p^f + \rho_f(\mathbf{G} - \mathbf{a}^f) \right] - \rho_f s_T \nabla^{\mathbf{x}} \theta,$$

↑
↑
↑

Piola's Transform                      Darcian Flow                      Soret Effect  
(neglected here)

## □ Balance of Energy

$$c_F \dot{\theta} = \underbrace{[D_{\text{mech}} - H_{\theta}]}_{\text{Dissipation}} + \underbrace{[-\nabla^{\mathbf{X}} \cdot \mathbf{Q}_{\theta}]}_{\text{Heat Flux}} - \underbrace{\frac{\Phi^f c_{Ff}}{\rho_f} \mathbf{W} \cdot \mathbf{F}^{-T}}_{\text{Convection}} \underbrace{\nabla^{\mathbf{X}} \theta}_{\text{Heat source}} + R_{\theta}, \quad \text{where}$$

$$\mathbf{H}_{\theta} = H_{\theta}^s + H_{\theta}^f, \quad \text{and} \quad H_{\theta}^s = -\theta \frac{\partial}{\partial \theta} \mathbf{P}' : \dot{\mathbf{F}} = -3K\alpha_{sk}\theta \frac{\dot{J}}{J} \quad \text{Solid Structural Heating (depending on which constitutive law being used)}$$

↑

Total Structural Heating

$$H_{\theta}^f = -\theta \frac{\partial}{\partial \theta} 3\alpha^m(\theta - \theta_o)\dot{p}^f = -3\alpha^m\theta \dot{p}^f. \quad \text{Fluid contribution}$$

# Remarks on Estimating Effective Thermal Conductivity from Microstructures

- Volume averaging effective thermal conductivity

$$k_{\theta} = \phi^f k_{\theta}^f + (1 - \phi^f) k_{\theta}^s$$

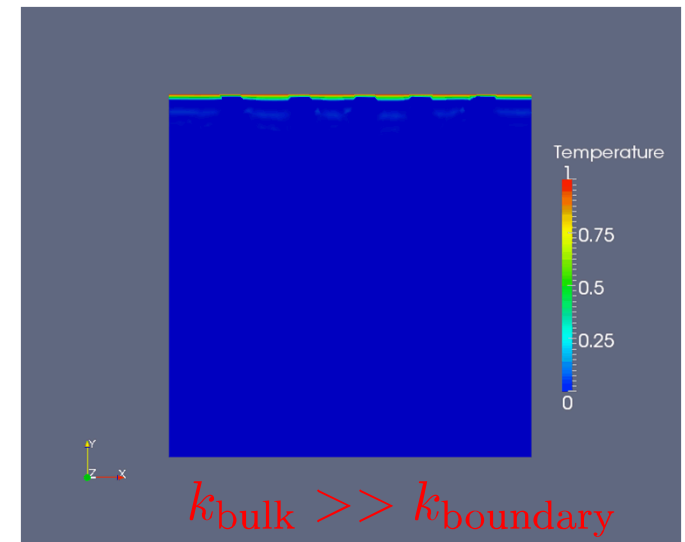
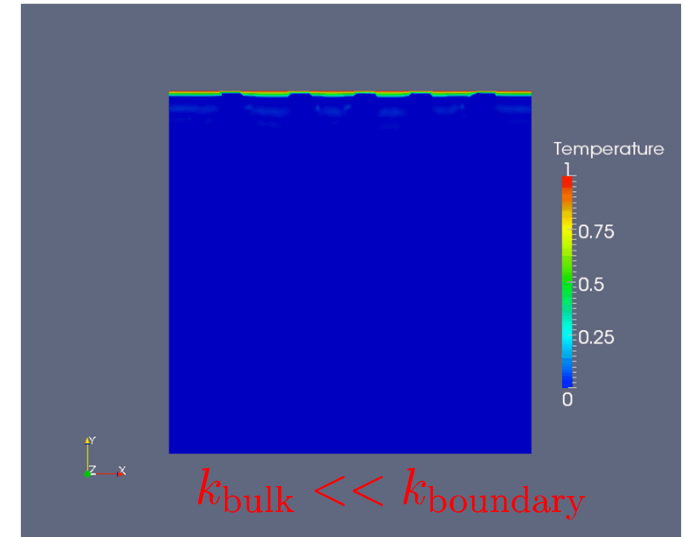
(cf. Preisig & Prevost, IJGGC 2011)

- Homogenized effective conductivity via Eshelby equivalent inclusion method (for spherical inclusions)

$$k_{\theta} = \left( k_{\theta}^f + \frac{\phi^f (k_{\theta}^s - k_{\theta}^f) k_{\theta}^f}{(k_{\theta}^s - k_{\theta}^f) \phi^f + k_{\theta}^f} \right) \mathbf{I}$$

(cf. Zhou & Meschke, IJNAMG 2013)

Important Note: In general, the temperature of the pore-fluid and solid skeleton are **not the same** in the RVE, until after sufficient diffusion takes place. This difference is neglected in current formulation.



Solution of transient heat equation of two-phase materials



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# Laser Weld



U.S. DEPARTMENT OF  
**ENERGY**



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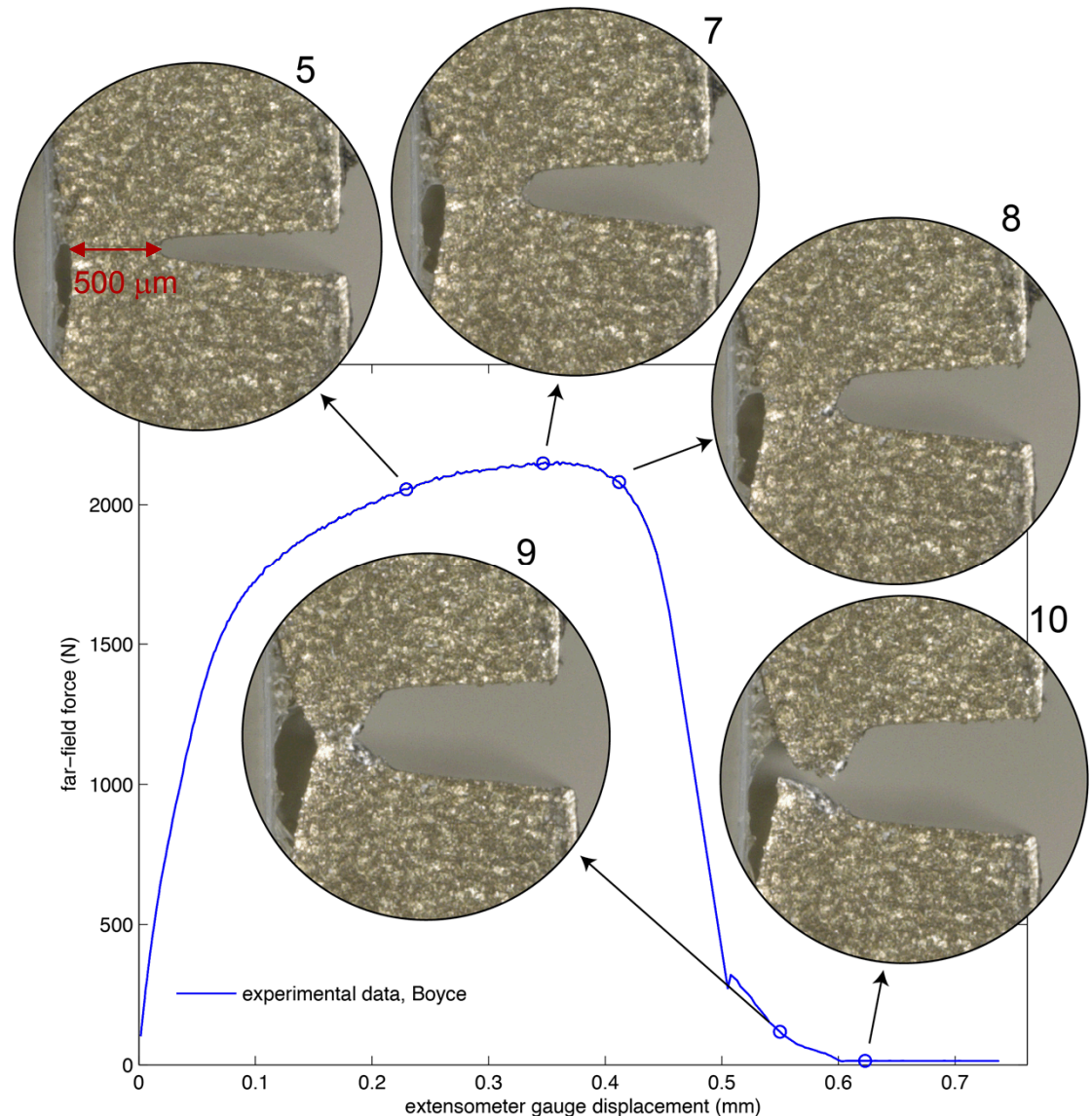
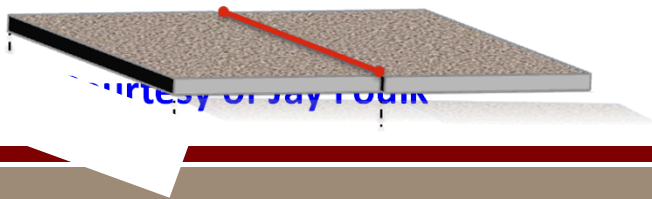
# Large Deformation – Laser Welds

*304L (stainless steel) is one of the most damage tolerant materials on the planet*

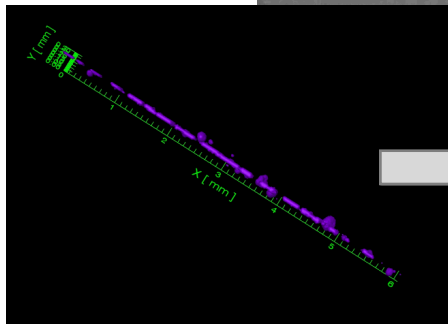
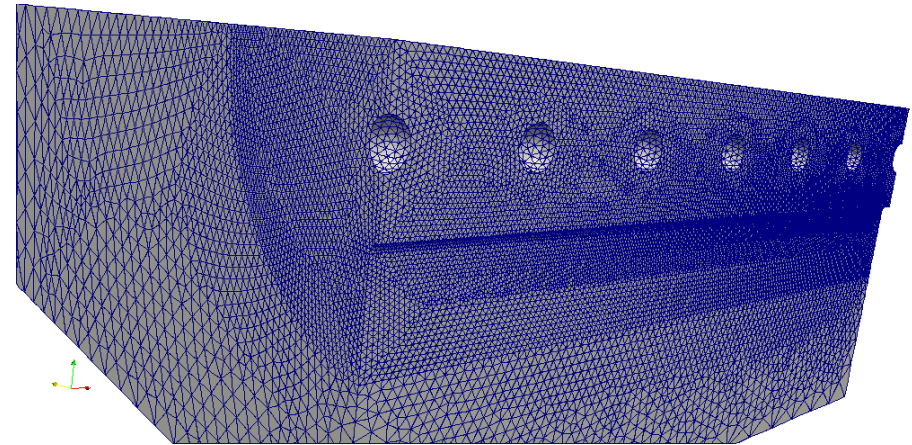
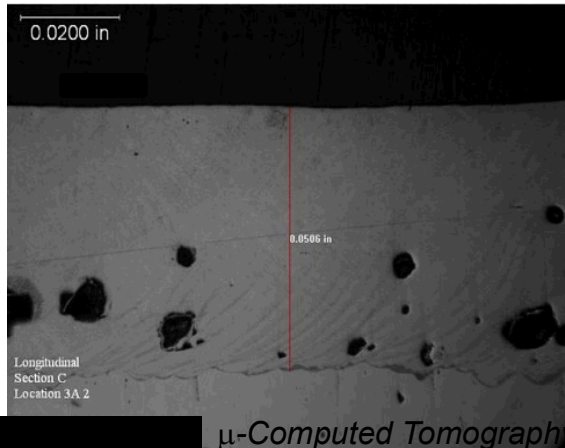
*The failure of 304L is a necking problem. Free surface creation is a 2<sup>nd</sup> order effect.*

*Pore size and distribution can aid the necking process*

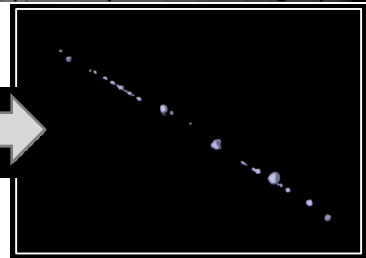
1. Modeling pore growth requires remeshing and mapping
2. Component and system models must model failure through necking



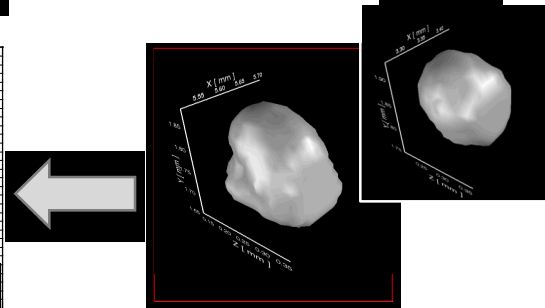
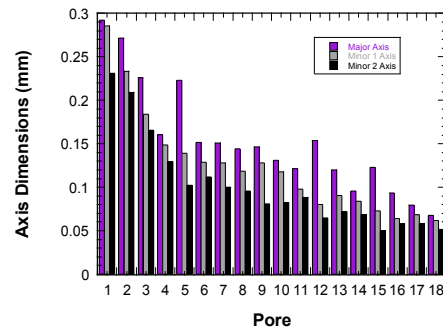
# Large Deformation – Laser Welds



μ-Computed Tomography



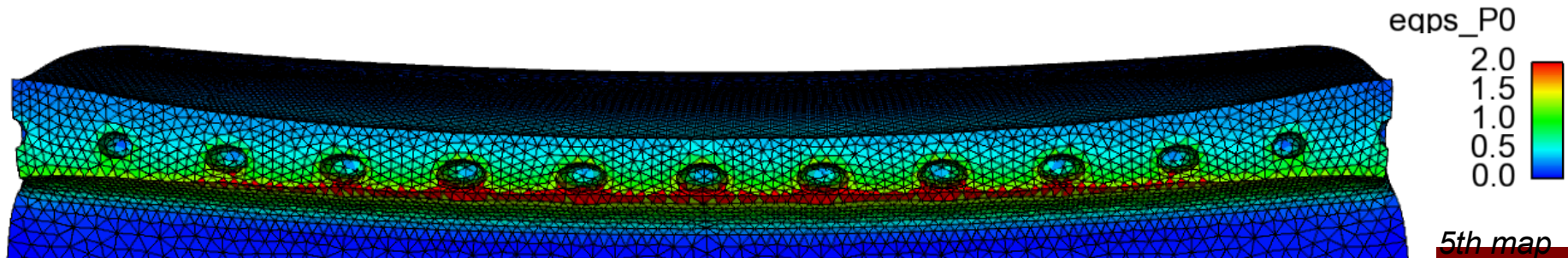
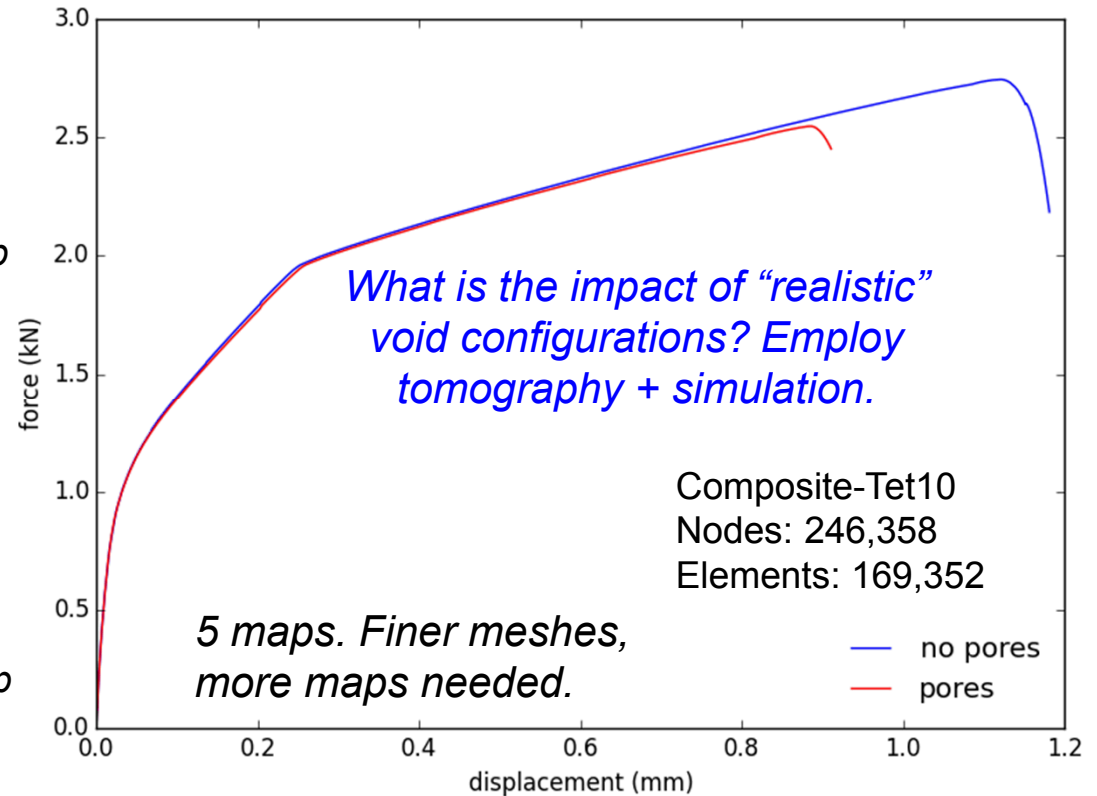
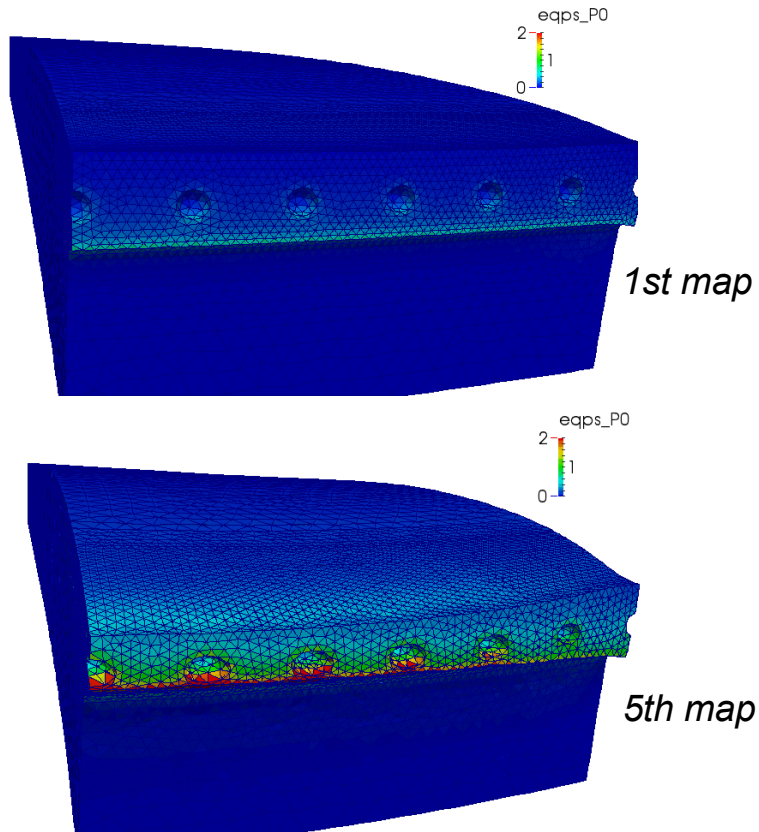
Magnification: 9X  
Voxel size: 14  $\mu\text{m}$   
Energy: 130 keV



J. Madison, L. K. Aagesen, "Quantitative Characterization of Porosity in Laser Welds of Stainless Steel" SCRIPTA MATERIALIA (2012)

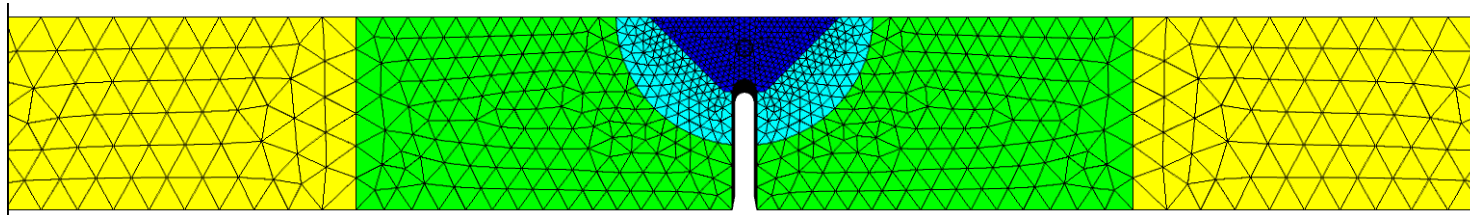
Sheet thickness: 1.6 mm  
Ligament length: 508  $\mu\text{m}$   
Number of voids: 6  
Void diameter: 150  $\mu\text{m}$   
Area fraction: 0.066 (~target)  
Location: centerline of ligament  
Coarse element size: 24  $\mu\text{m}$   
Finer element size: 12  $\mu\text{m}$   
Element type: composite-tet (10 nodes)

# Remeshing/mapping discrete pores

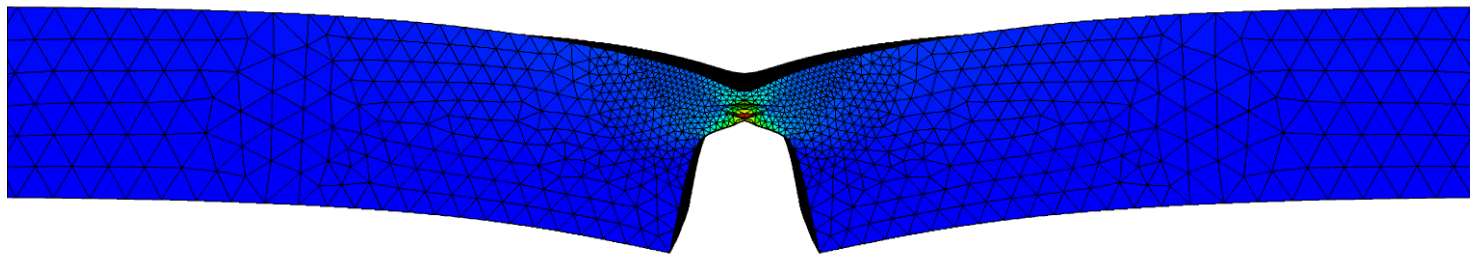




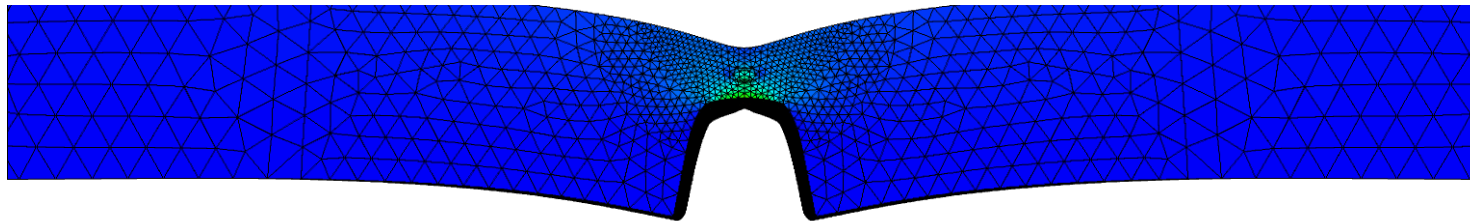
# Additional views of necking process



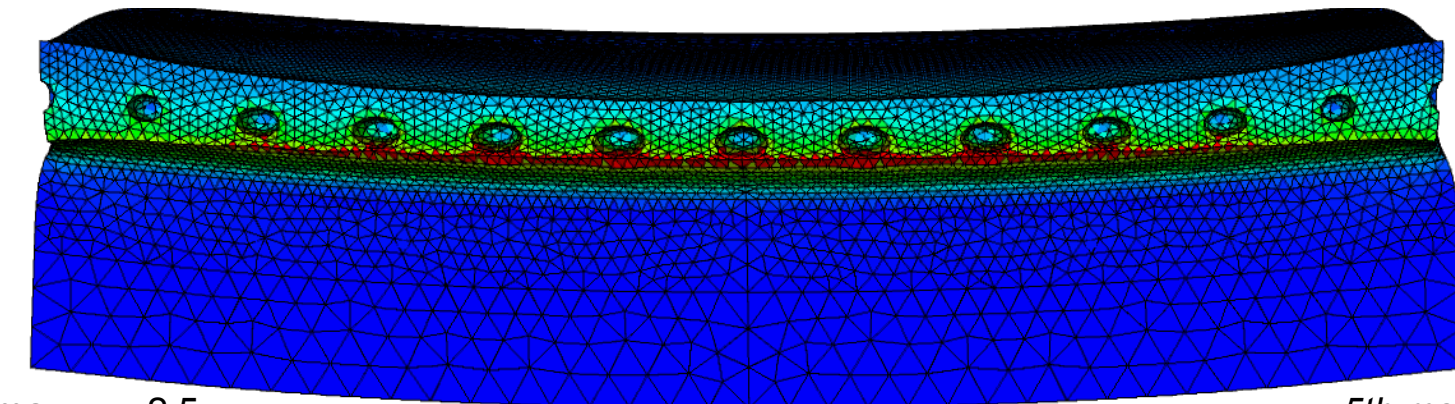
undeformed mesh  
with notch




necking at  
mid-plane



necking at  
surface



eqps\_P0  
2.0  
1.5  
1.0  
0.5  
0.0

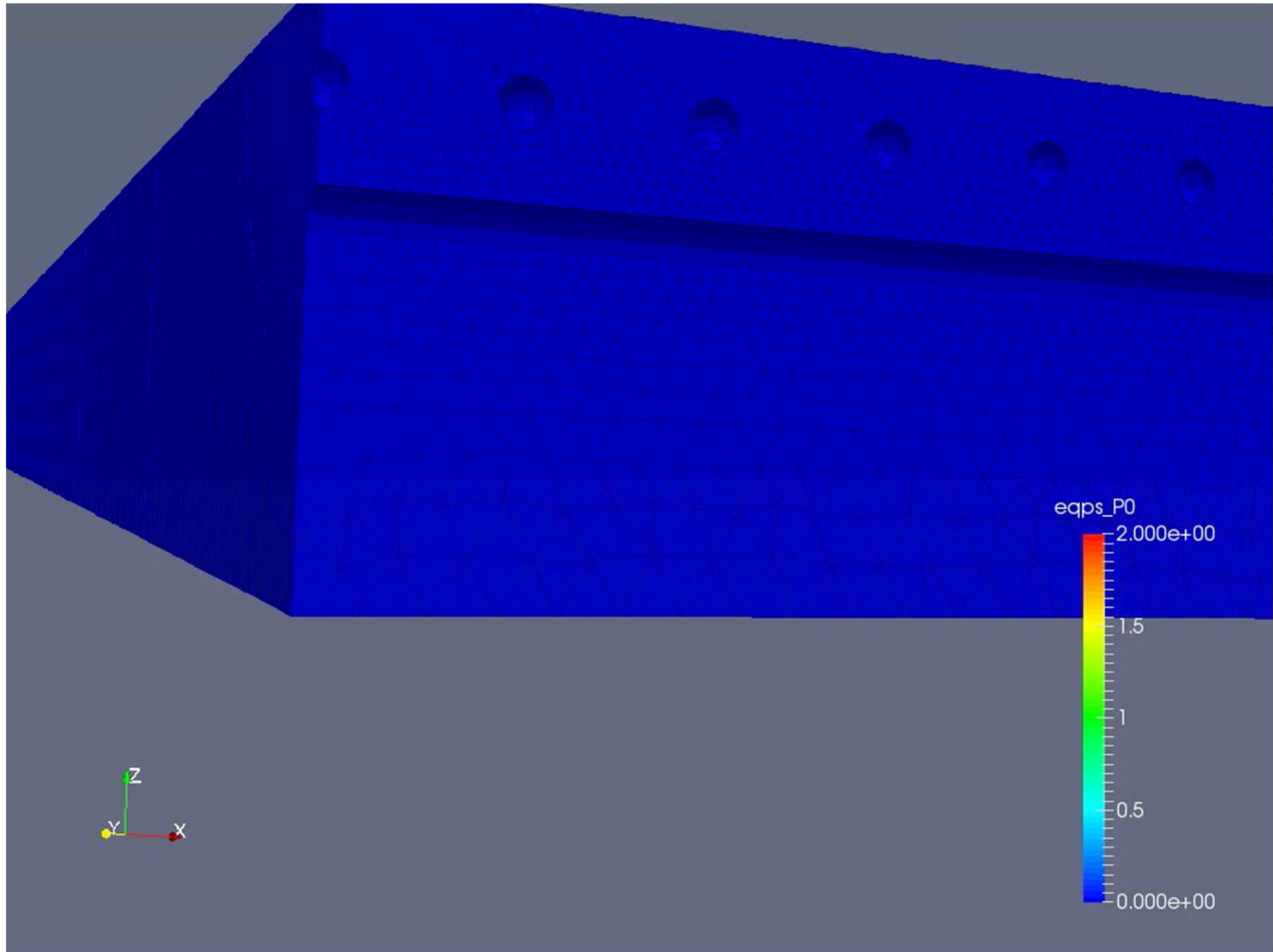


A vertical color bar with a gradient from blue at the bottom to red at the top, corresponding to the values 0.0 to 2.0.

max  $\epsilon_p = 2.5$

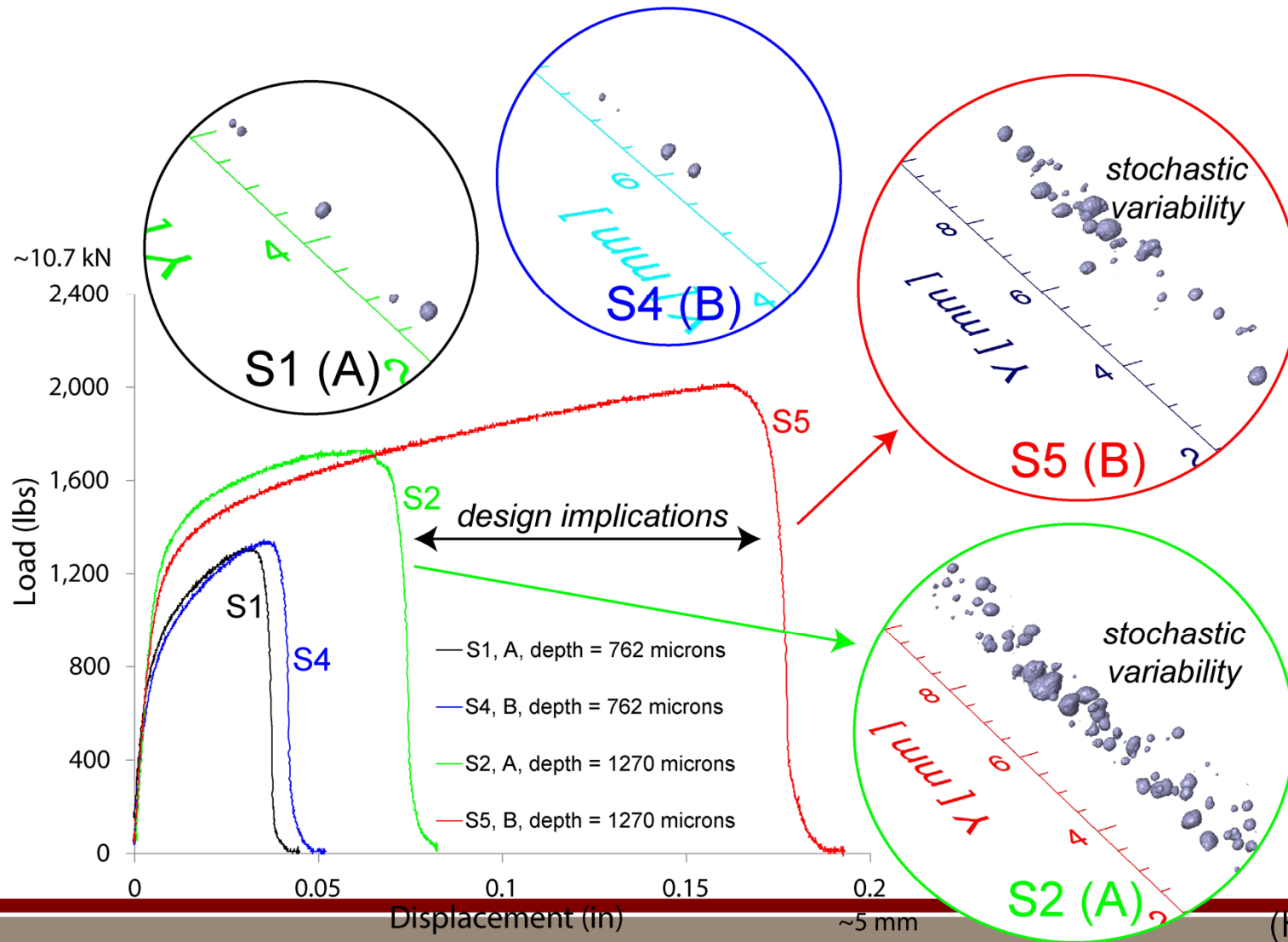
5th map

# Increasing the number of mappings



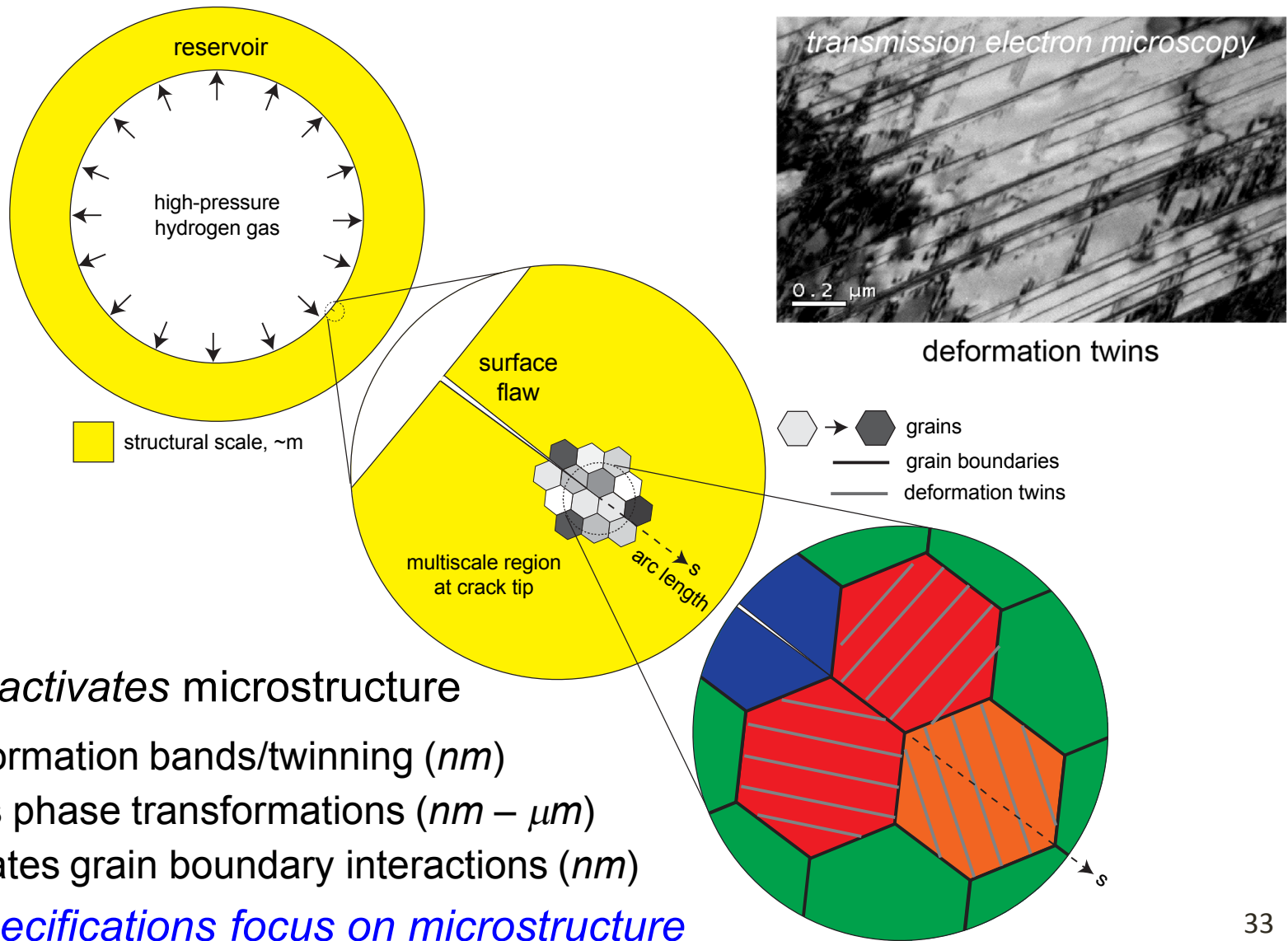
# Deeper welds galvanize efforts

*Weld schedule impacts porosity. Porosity impacts performance.*





# Hydrogen activates microstructure (stainless steel)



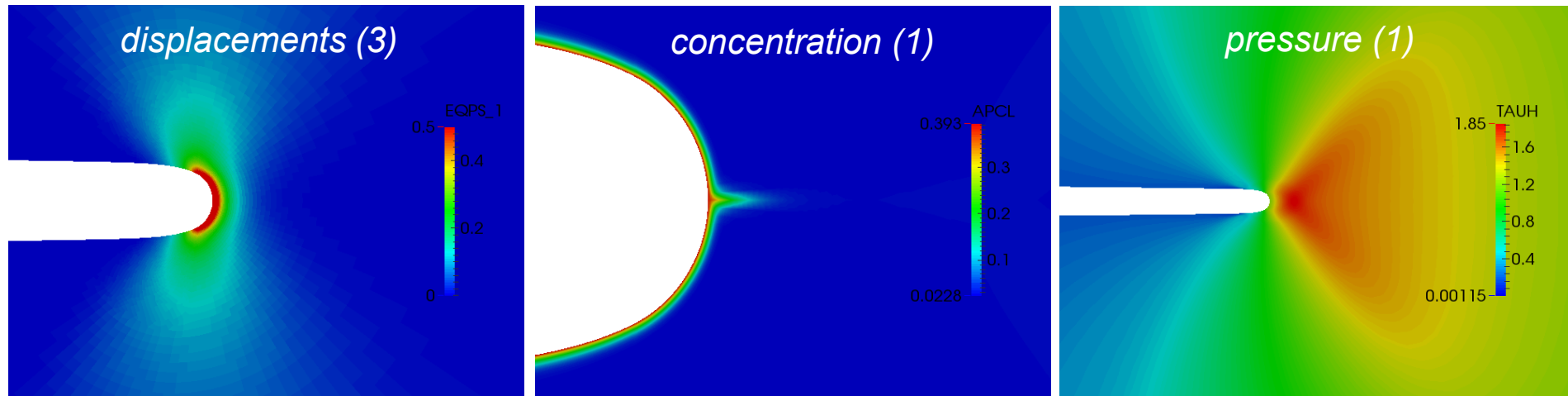
Hydrogen *activates* microstructure

- Aids deformation bands/twinning ( $nm$ )
- Activates phase transformations ( $nm - \mu m$ )
- Accentuates grain boundary interactions ( $nm$ )

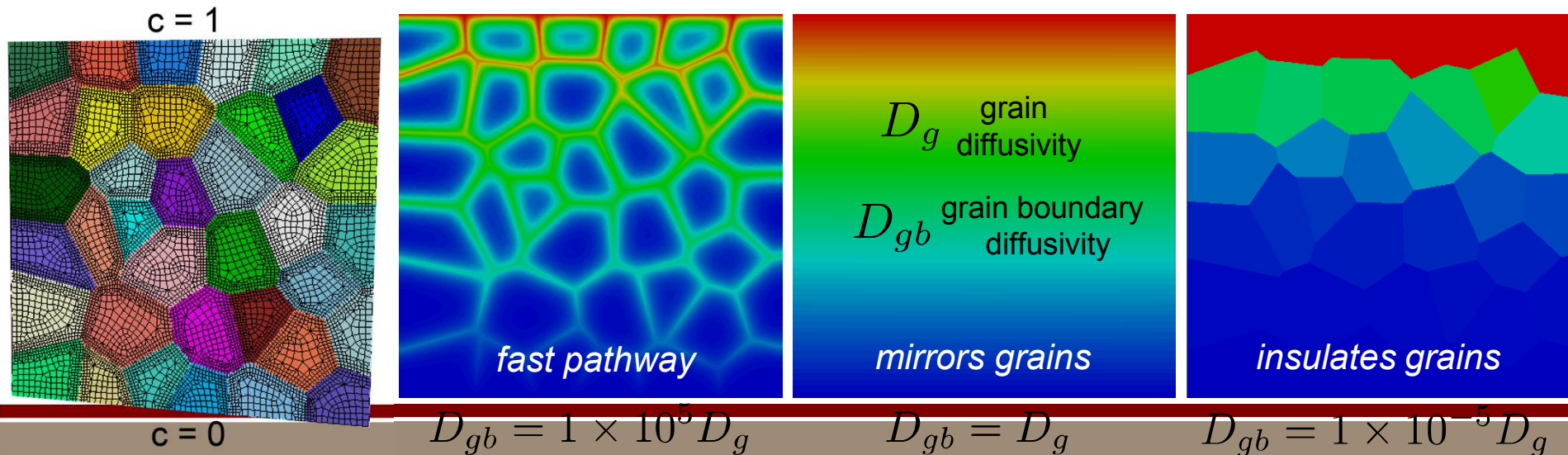
*Material specifications focus on microstructure*

# Demonstrate strong chemo-mechanical coupling

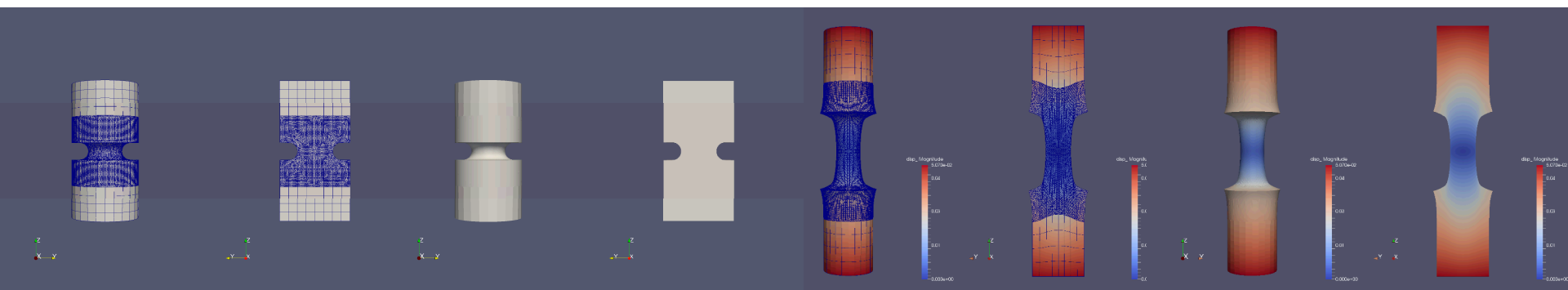
Block solve for displacement, concentration, and pressure at a crack tip



Exploring fast pathways through the inclusion of surface elements on grain boundaries



*Exceptional service in the national interest*



## The Alternating Schwarz Method for Concurrent Multiscale in Finite Deformation Solid Mechanics

**Alejandro Mota, Irina Tezaur, Coleman Alleman**  
Sandia National Laboratories, Livermore, CA

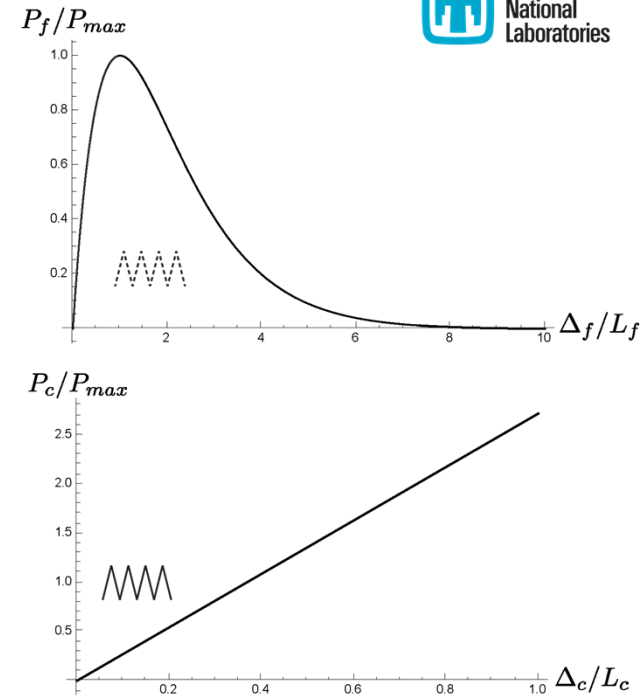
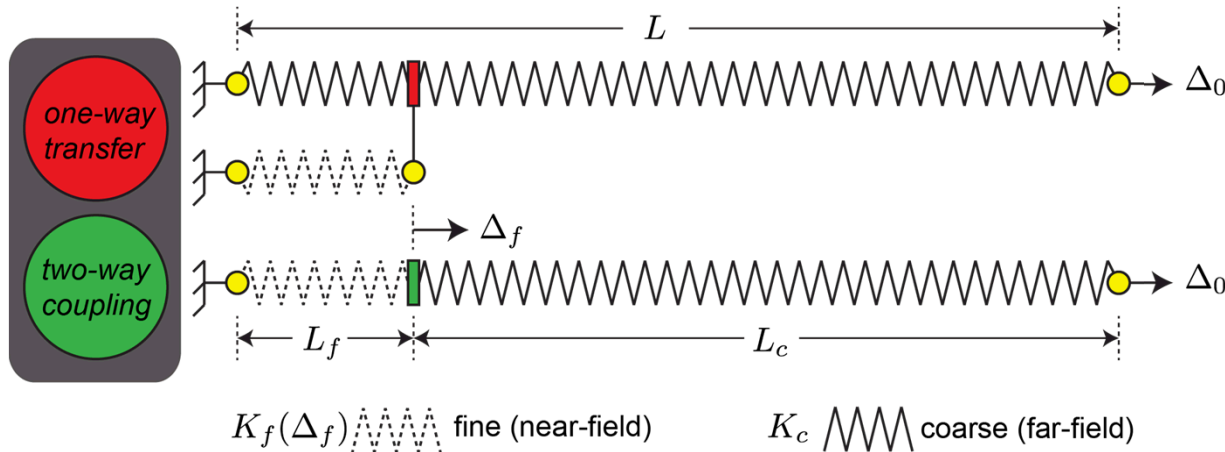


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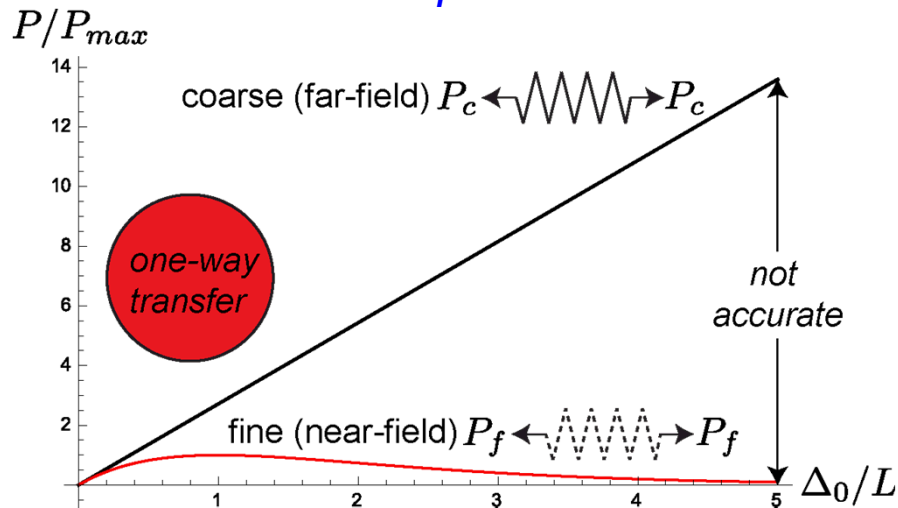
# A Case for Concurrent Coupling

Q: Is a one-way transfer accurate? Conservative?

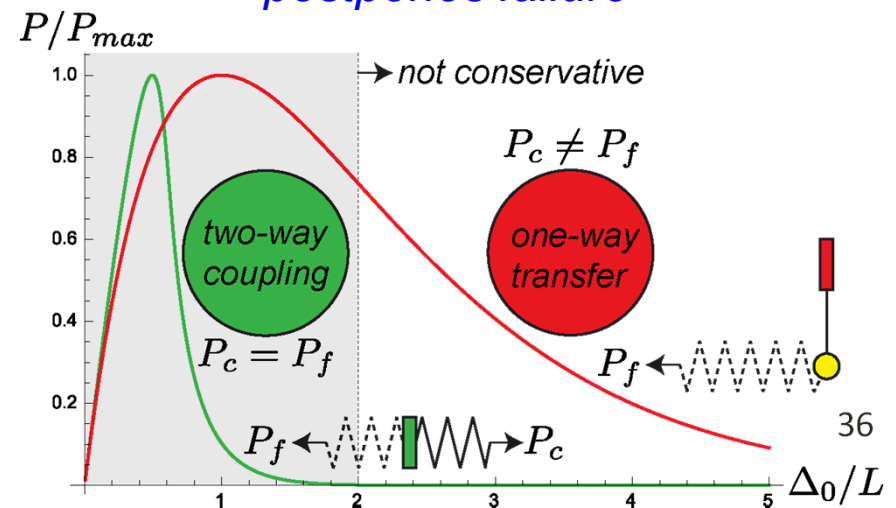
A: For failure processes involving localization, no.



violates equilibrium

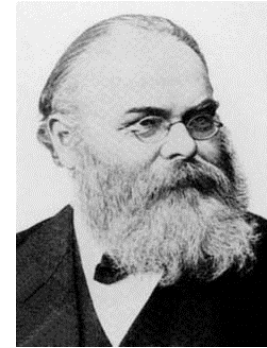


postpones failure

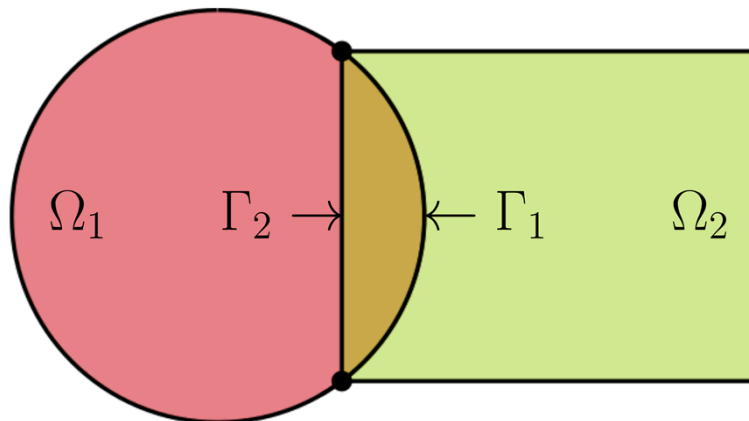


# Alternating Schwarz for Domain Decomposition

- First developed in 1870 for solving Laplace's equation in irregularly shaped domains.
- Simple idea: if the solution is known in regularly shaped domains, use those as puzzle pieces to iteratively build a solution for the more complex domain.



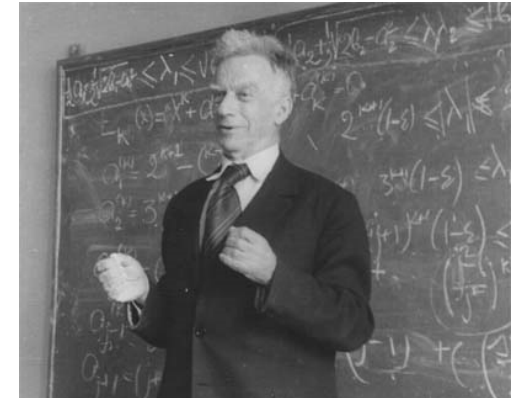
Karl Hermann Amandus Schwarz  
(1843 – 1921). Source: *bibmath.net*



- Initialize:
  - Solve PDE by any method on  $\Omega_1$  using an initial guess for Dirichlet BCs on  $\Gamma_1$ .
- Iterate until convergence:
  - Solve PDE by any method (can be different than for  $\Omega_1$ ) on  $\Omega_2$  using Dirichlet BCs on  $\Gamma_2$  that are the values just obtained for  $\Omega_1$ .
  - Solve PDE by any method (can be different than for  $\Omega_2$ ) on  $\Omega_1$  using Dirichlet BCs on  $\Gamma_1$  that are the values just obtained for  $\Omega_2$ .

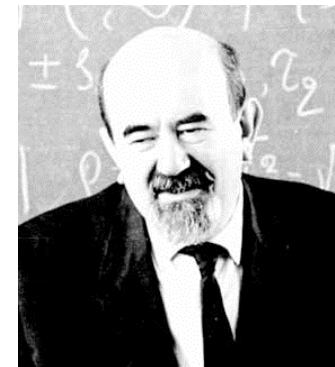
# Alternating Schwarz after Schwarz

- S. L. Sobolev posed the Schwarz method for linear elasticity in variational form.
- He also proved convergence of the method for linear elasticity in 1936 by proposing a convergent sequence of energy functionals.
- Convergence for general linear elliptic partial differential equations was not proved until much later in 1951 by S. G. Mikhlin.
- We have derived a proof of convergence of the alternating Schwarz for the finite deformation, fully nonlinear PDE.
- We have also determined that the alternating Schwarz method converges geometrically for the finite deformation problem.



Sergei Lvovich Sobolev (1908 – 1989).

Source: [www.math.nsc.ru](http://www.math.nsc.ru)



Solomon Grigoryevich Mikhlin (1908 – 1990).

Source: [www-history.mcs.st-andrews.ac.uk](http://www-history.mcs.st-andrews.ac.uk)

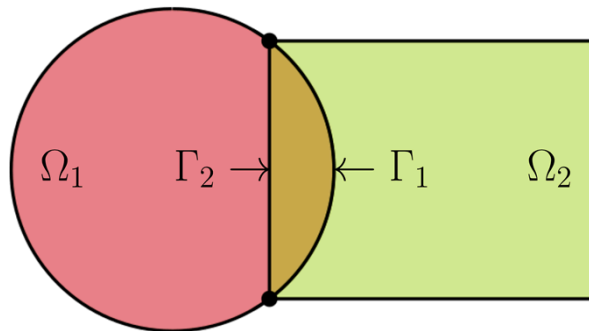
$$\Phi[\varphi] = \int_B W(\mathbf{F}, \mathbf{Z}, T) dV - \int_B \mathbf{B} \cdot \varphi dV - \int_{\partial_T B} \bar{\mathbf{T}} \cdot \varphi dS$$
$$\text{Div } \mathbf{P} + \mathbf{B} = \mathbf{0}$$



# Alternating Schwarz in Albany

- Alternating Schwarz can be posed in the linear case as a block system.
- The block system can have different structures (see Smith et al., Domain Decomposition, 2004).
- In a nonlinear setting, the linear system results from the consistent linearization of the fully nonlinear system.
- The coupling terms appear in the RHS, resulting in a block diagonal system with a simpler structure that we exploit.
- Efficient solution is achieved with iterative linear solver using a block preconditioner.

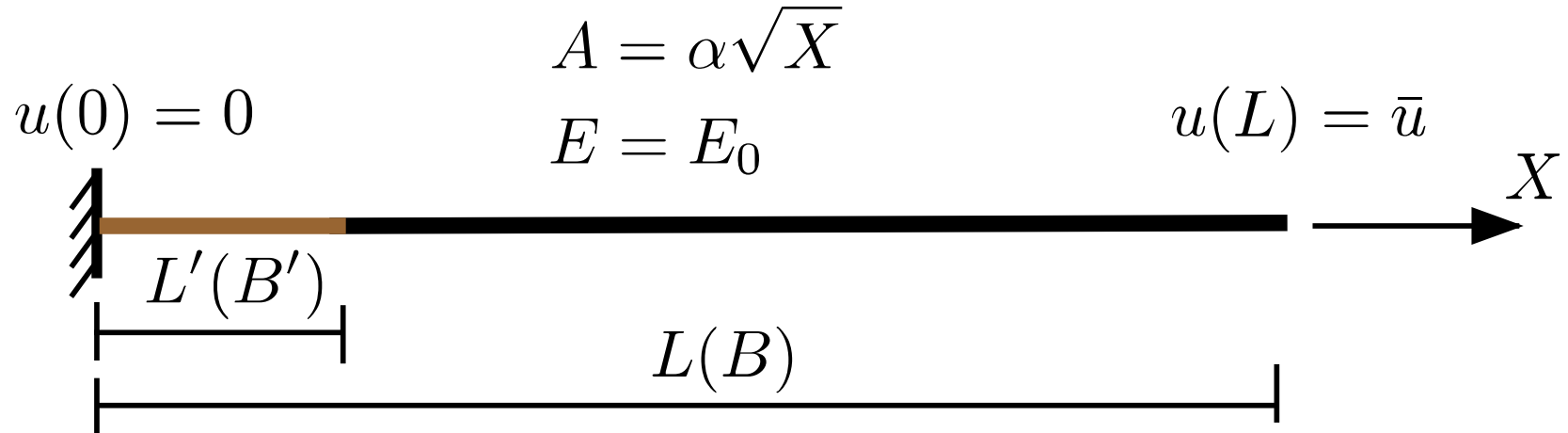
$$\begin{pmatrix} \mathbf{K}_{AB}^1 & \mathbf{0} \\ \mathbf{0} & \mathbf{K}_{AB}^2 \end{pmatrix} \begin{Bmatrix} \Delta \mathbf{x}_B^1 \\ \Delta \mathbf{x}_B^2 \end{Bmatrix} = \begin{Bmatrix} -\mathbf{R}_A^1 - \mathbf{K}_{Ab}^1 (\mathbf{x}_b^1 - \mathbf{x}_b^1) - \mathbf{K}_{A\beta}^1 (P_{\Omega_2 \rightarrow \Gamma_1}[\mathbf{x}^2] - \mathbf{x}_\beta^1) \\ -\mathbf{R}_A^2 - \mathbf{K}_{Ab}^2 (\mathbf{x}_b^2 - \mathbf{x}_b^2) - \mathbf{K}_{A\beta}^2 (P_{\Omega_1 \rightarrow \Gamma_2}[\mathbf{x}^1] - \mathbf{x}_\beta^2) \end{Bmatrix}$$



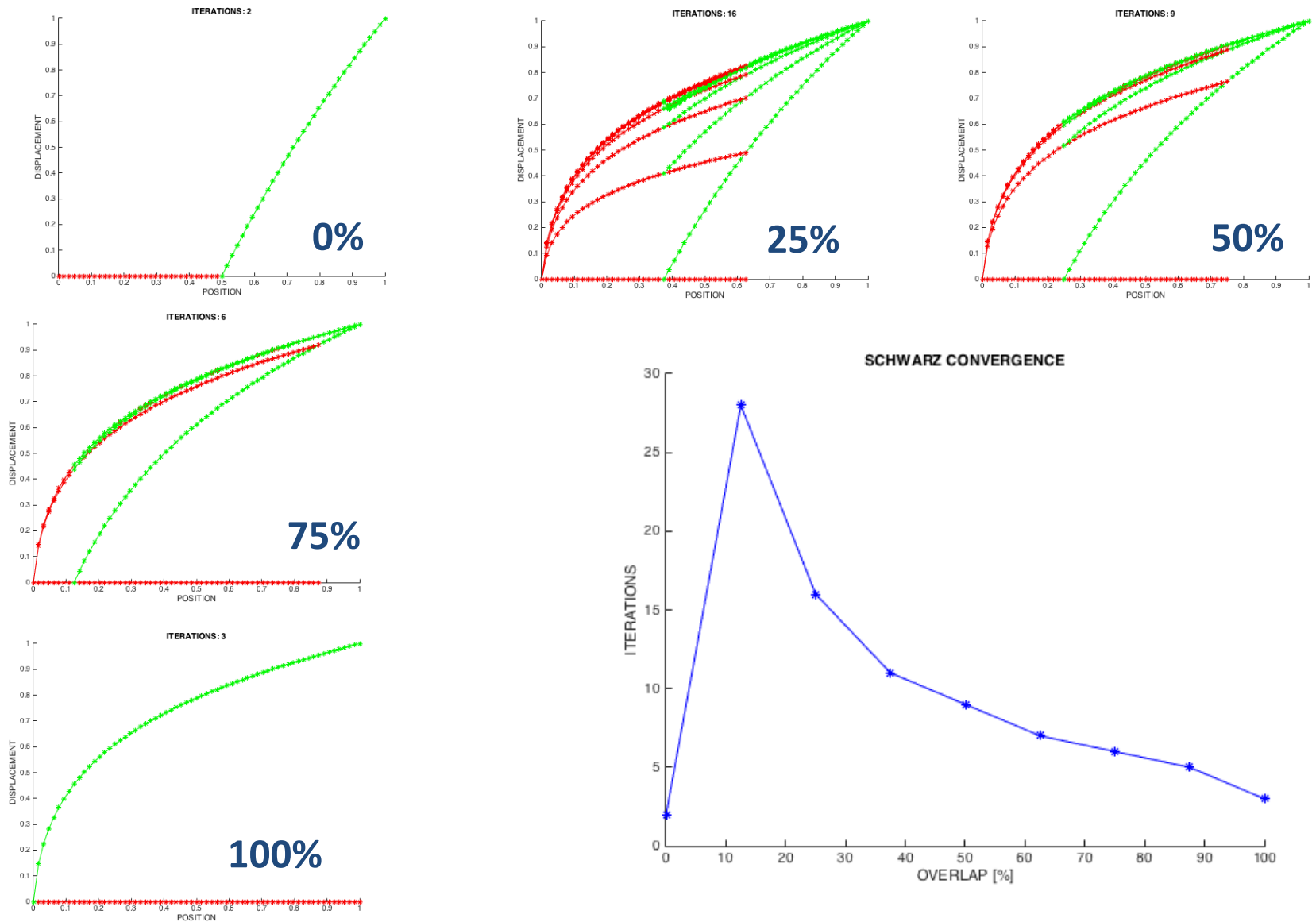
$$\begin{pmatrix} \mathbf{K}_{AB}^1 & \mathbf{K}_{Ab}^1 & \mathbf{K}_{A\beta}^1 & & & \\ & \mathbf{I} & & & \mathbf{0} & \\ & & \mathbf{I} & & & \\ & & & \mathbf{K}_{AB}^2 & \mathbf{K}_{Ab}^2 & \mathbf{K}_{A\beta}^2 \\ & \mathbf{0} & & & \mathbf{I} & \\ & & & & & \mathbf{I} \end{pmatrix} \begin{Bmatrix} \Delta \mathbf{x}_B^1 \\ \Delta \mathbf{x}_b^1 \\ \Delta \mathbf{x}_\beta^1 \\ \Delta \mathbf{x}_B^2 \\ \Delta \mathbf{x}_b^2 \\ \Delta \mathbf{x}_\beta^2 \end{Bmatrix} = \begin{Bmatrix} -\mathbf{R}_A^1 \\ \mathbf{x}_b^1 - \mathbf{x}_b^1 \\ P_{\Omega_2 \rightarrow \Gamma_1}[\mathbf{x}^2] - \mathbf{x}_\beta^1 \\ -\mathbf{R}_A^2 \\ \mathbf{x}_b^2 - \mathbf{x}_b^2 \\ P_{\Omega_1 \rightarrow \Gamma_2}[\mathbf{x}^1] - \mathbf{x}_\beta^2 \end{Bmatrix}$$

# Example: Foulk's Singular Bar

- 1D Proof of concept problem.
- Test convergence and compare with literature (Evans, 1986).
- Expect faster convergence in fewer iterations with increased overlap.
- Strong singularity on left end of bar – area proportional to square root of length
- Simple hyperelastic model with damage

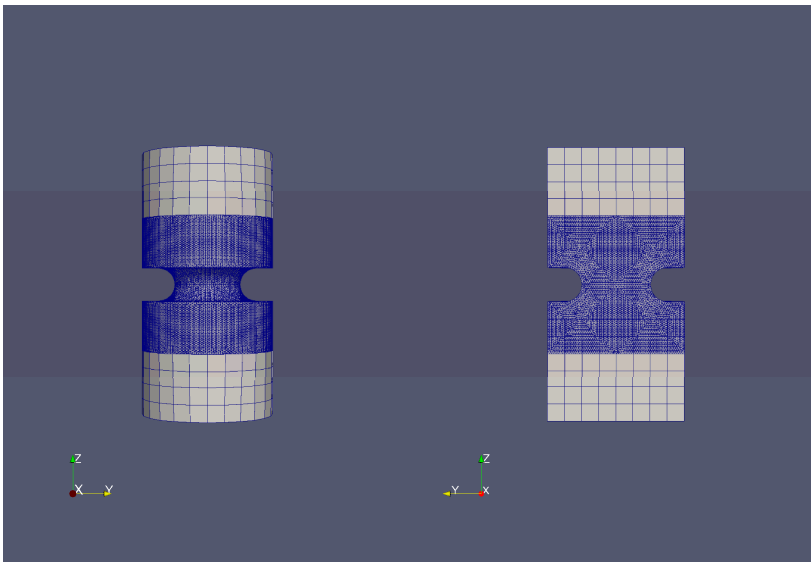


# Schwarz Convergence: Symmetric Overlap

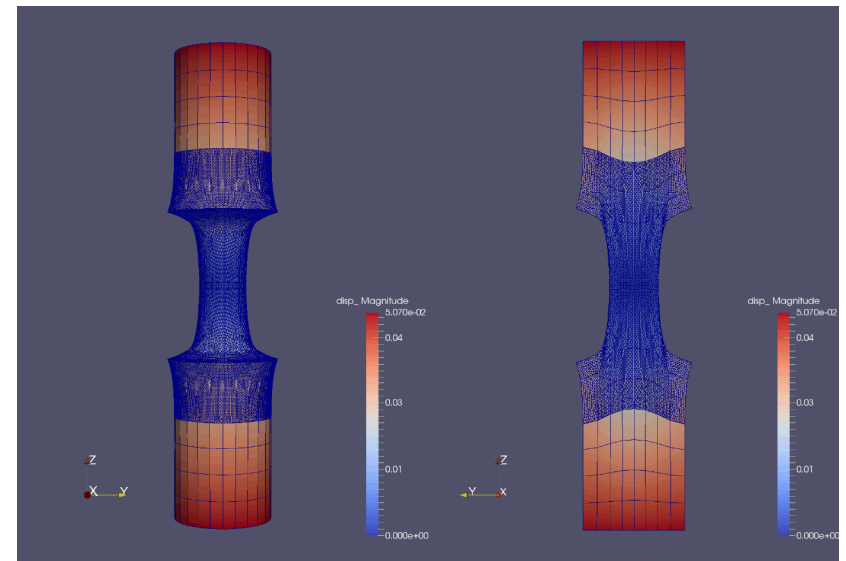


# Hyperelastic Notched Cylinder

- Notched cylinder stretched to twice its original height.
- Stress concentrations and strain localization in the notch require a higher level of mesh refinement.
- The fine and coarse region overlap but the solutions are computed separately.



- The notched region, where stress concentrations are expected, is finely meshed with tetrahedral elements.
- The top and bottom regions, presumably of less interest, are meshed with coarser hexahedral elements.



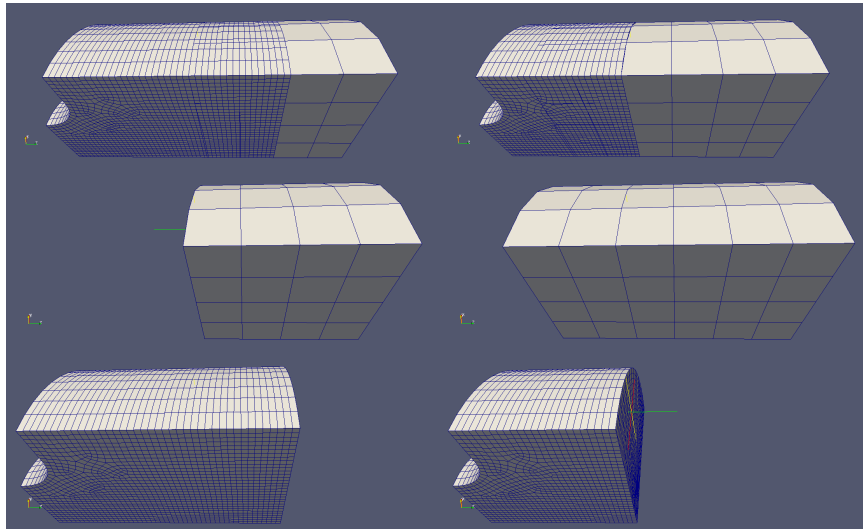
Deformed configuration colored with displacement magnitude.  
Smooth field transition even when the meshes do not match.

# Coupling of Different Material Models

- The Schwarz method is capable of coupling regions with different material models.
  - Notched cylinder subjected to tensile load with an elastic and J2 elasto-plastic regions.
- When the overlap region is far from the notch, no plastic deformation exists in it: the coarse and fine regions predict the same behavior there.
- When the overlap region is near the notch, plastic deformation spills onto it and the two models predict different behavior, affecting convergence adversely. Independent of any method, this kind of solution is questionable.

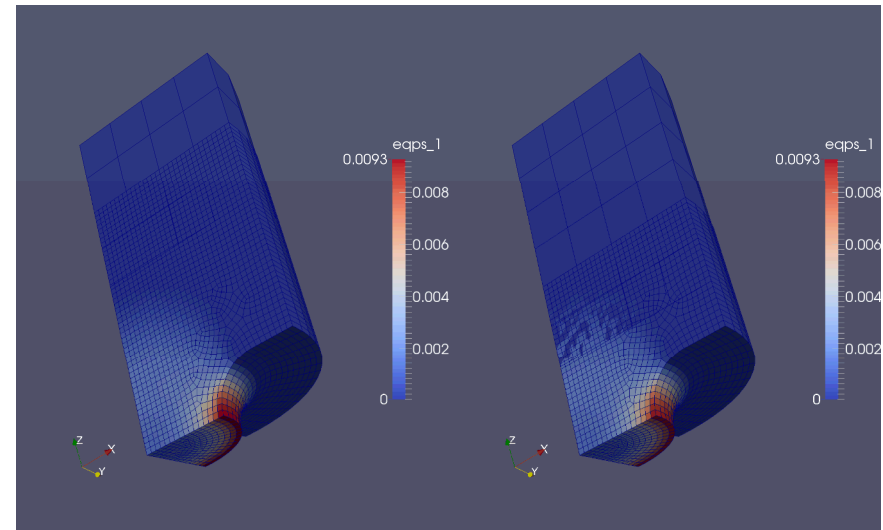
Overlap far from notch

Overlap near notch



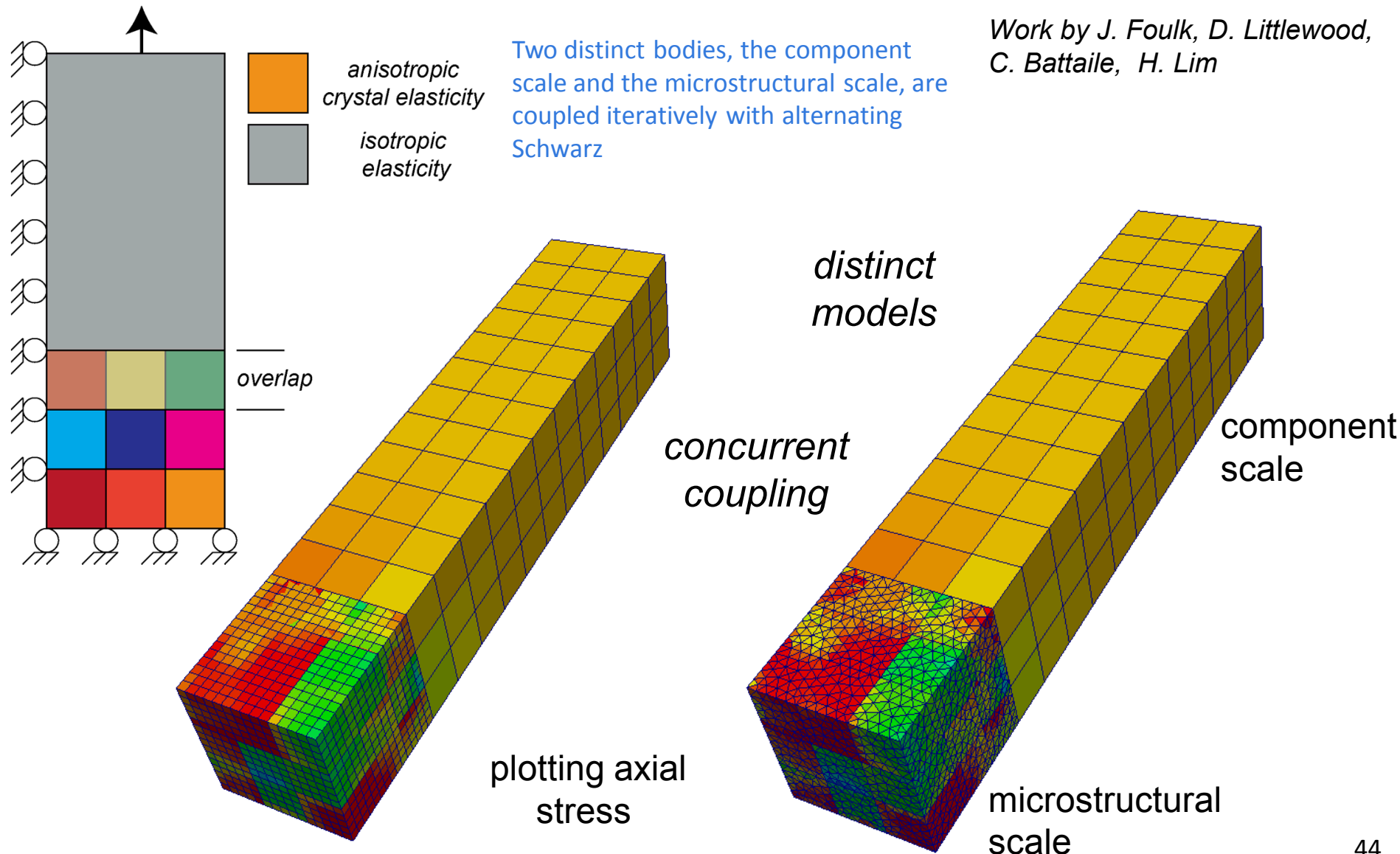
Overlap far from notch

Overlap near notch



# Coupled Isotropic and Crystal Hyperelasticity

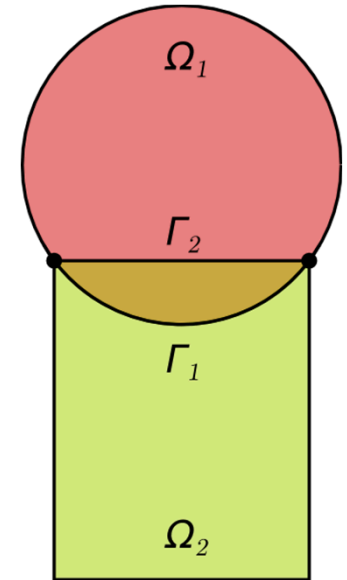
Work by J. Foulk, D. Littlewood,  
C. Battaile, H. Lim





# Dynamics with Schwarz

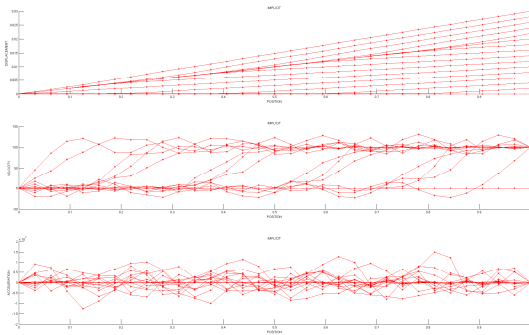
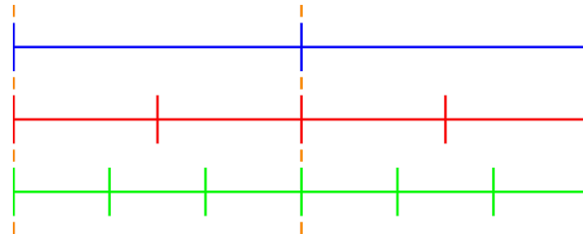
- Extend Schwarz coupling to dynamics using a governing time stepping algorithm to control time integrators in each domain.
- Can use different integrators and time steps in each domain.
- 1D results show smooth coupling; no numerical artifacts such as spurious wave reflections at boundaries of coupled domains.



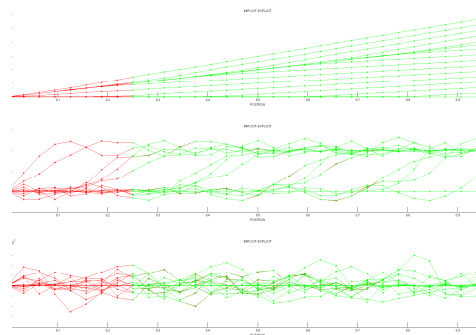
Controller time stepper

Time integrator for  $\Omega_1$

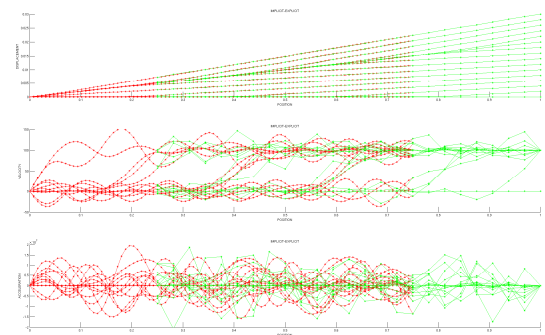
Time integrator for  $\Omega_2$



(a) Reference single-domain solution



(b) Two-domain implicit-explicit solution equal to (a). No dynamic artifacts.



(c) Fine-coarse implicit-explicit coupling

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# Microstructure



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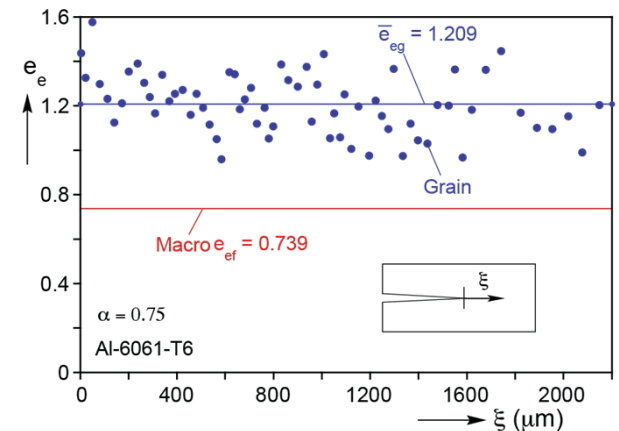
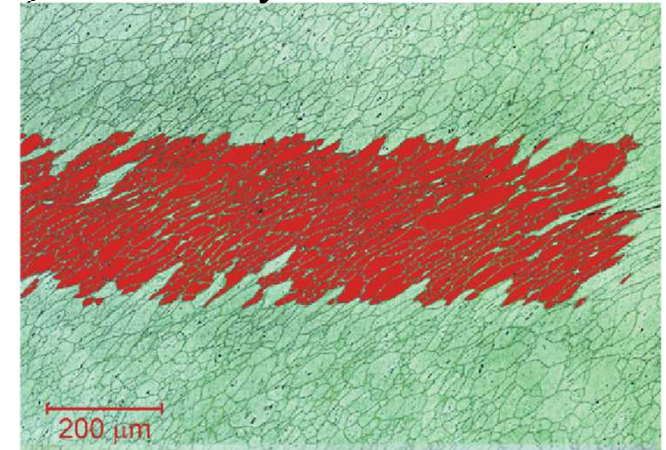
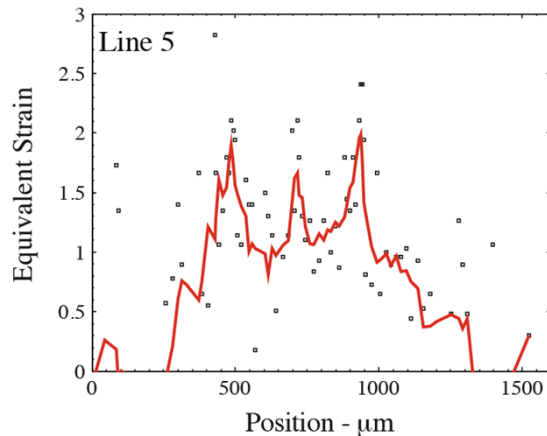
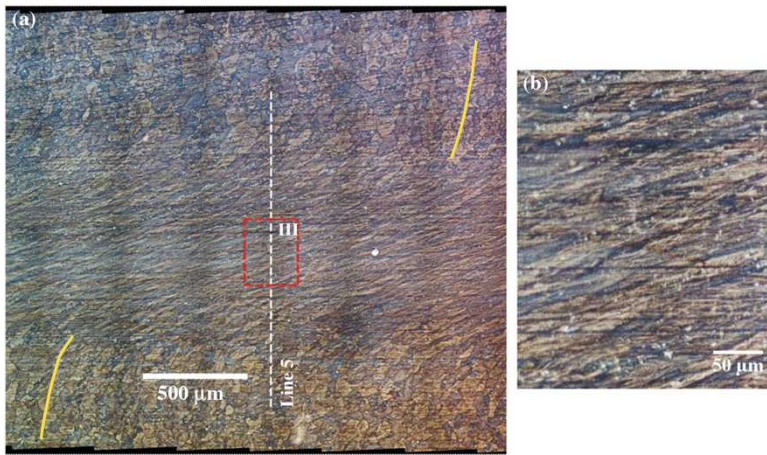
# Microstructural localization requires much more

Q: How does *microstructure influence* conditions for *necking instability*?

Q: *When does microstructure matter, and when can it be ignored in continuum models?*

O: Local deformations at the microstructural level vastly exceed component-scale predictions.

A. Ghahremaninezhad, K. Ravi-Chandar (IJF, 2013) Altom, S. Kyriakides, K. Ravi-Chandar



“strain values measured at the grain level are significantly larger”

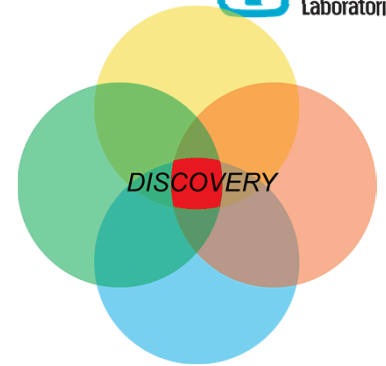
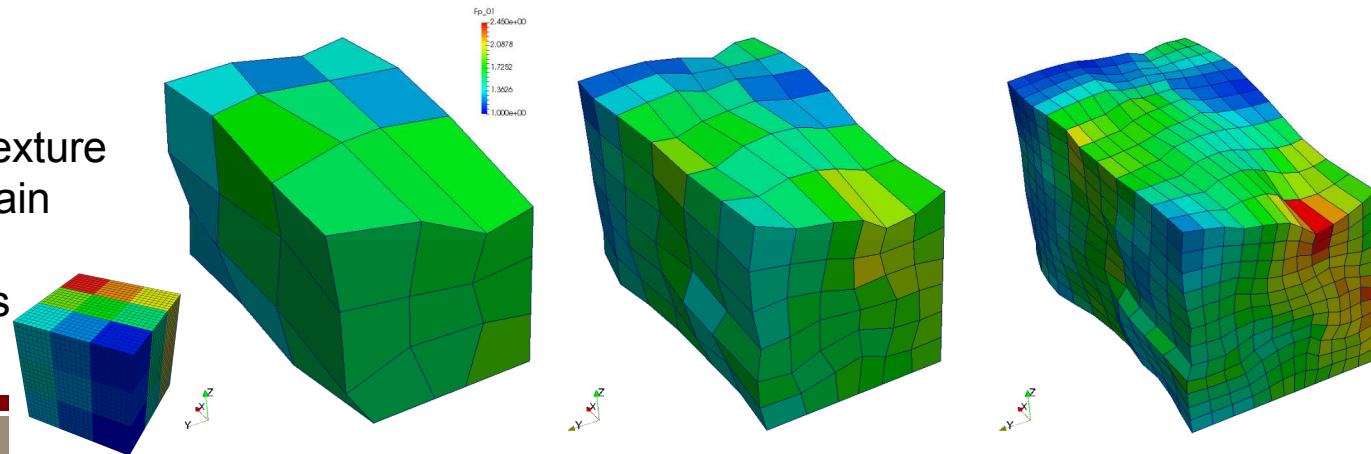
“void formation and coalescence is delayed until the very end of the life of the deforming material”

# Revisiting CP with a focus on agility

- Consolidating crystal physics
- *Implicit implementation with automatic differentiation*
- Agility through plug-n-play interfaces
  - Multiple residuals
    - Solve for slip, slip + hardening (12, 24 unknowns )
    - Solve for  $\mathbf{F}_p$ ,  $\mathbf{F}_p$  + hardening (9, 21 unknowns)
  - Multiple solution schemes (Newton, CG, Trust Region)
  - Evolving physics
    - Models for elasticity, flow rule, and hardening
- Adopting modern software practices with rigorous testing

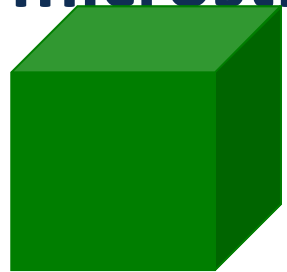
## Rubik's cube case

- 512 elements/grain
- 27 grains w/random texture
- 100% engineering strain
- Vary step size
- Vary solution methods



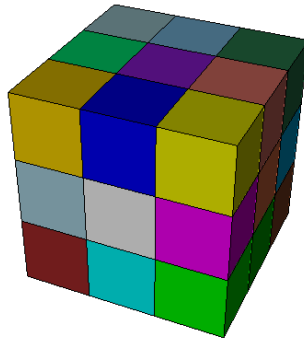


# Focus on verification, robustness and microstructure

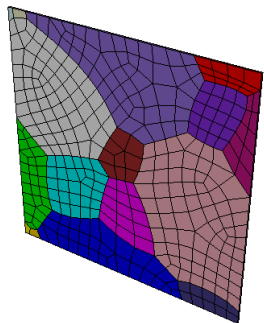


single slip (analytical)

Multiple slip systems (FCC)



Rubik's cube  
3D polycrystals

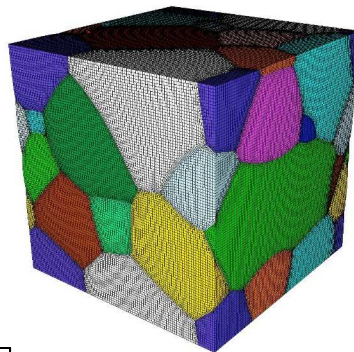


g



2D polycrystals

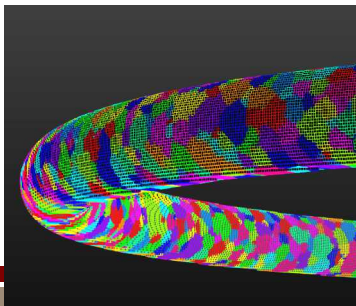
Many elements per grain



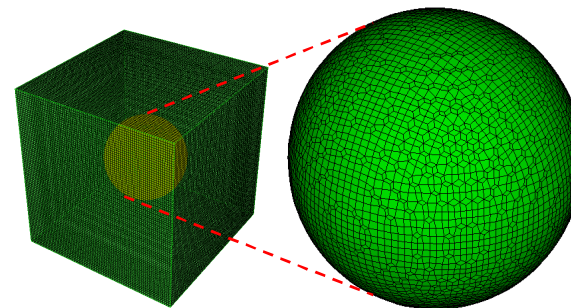
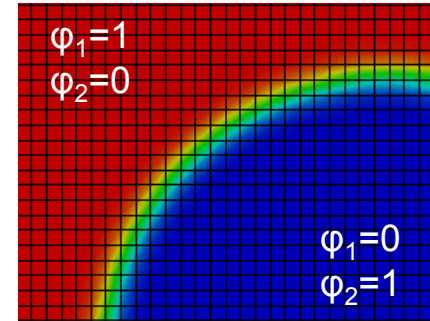
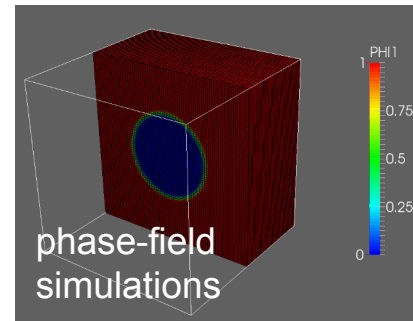
3D polycrystals  
~10<sup>2</sup> grains/~10<sup>6</sup> elements  
anisotropic hardening



Component scale  
~10<sup>6</sup> grains/~10<sup>9</sup> elements  
concurrent coupling



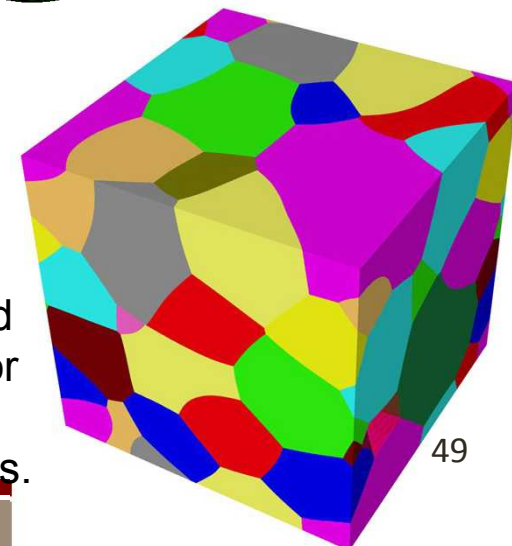
## Leveraging Conformal Microstructures LDRD



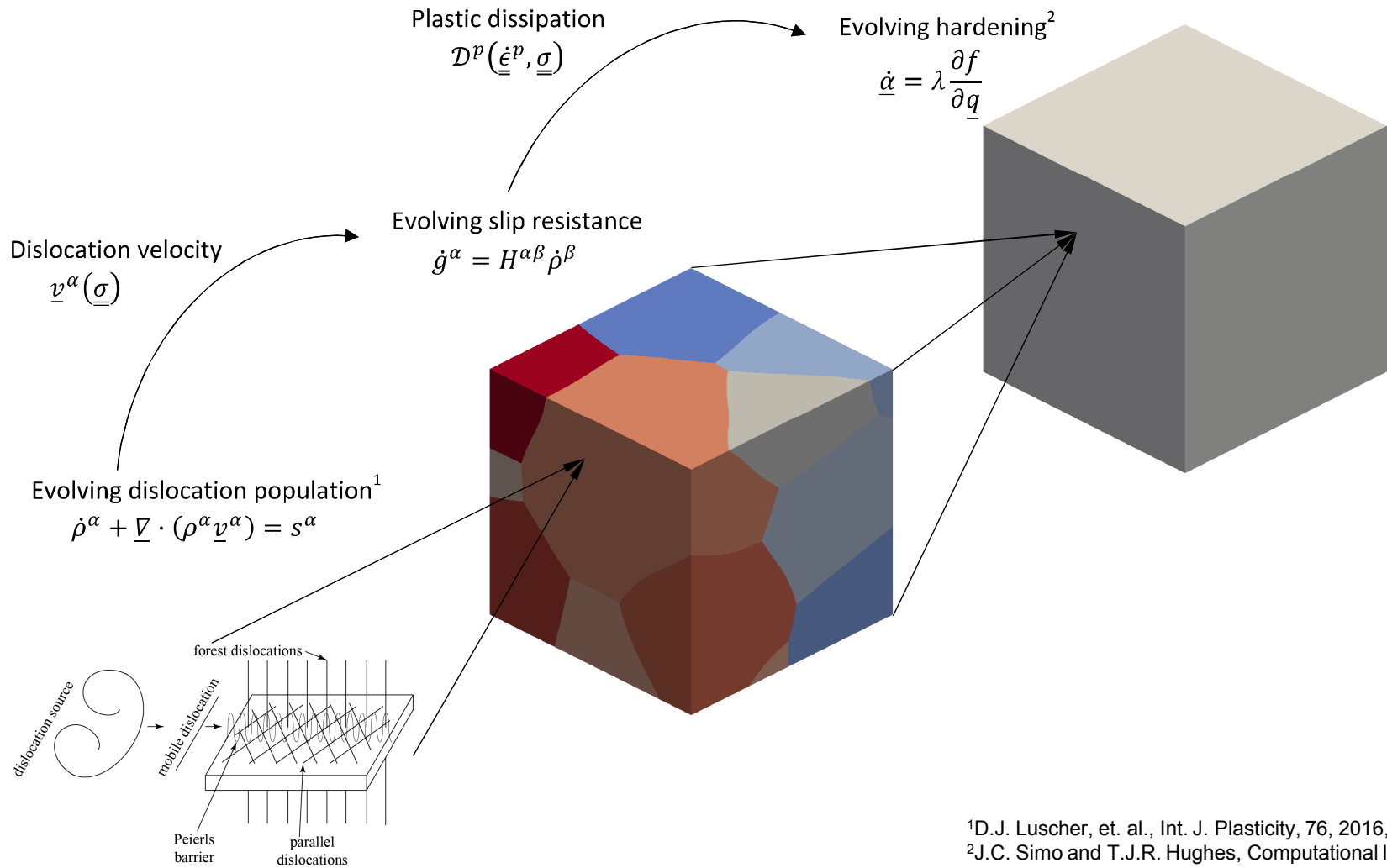
interfacial  
reconstruction  
(SCULPT)

Conformal  
boundaries are  
critical to our effort.

Employing tools and  
providing support for  
the extension to  
tetrahedral elements.



# Meso-Continuum Scale Hierarchy

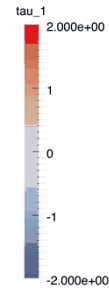
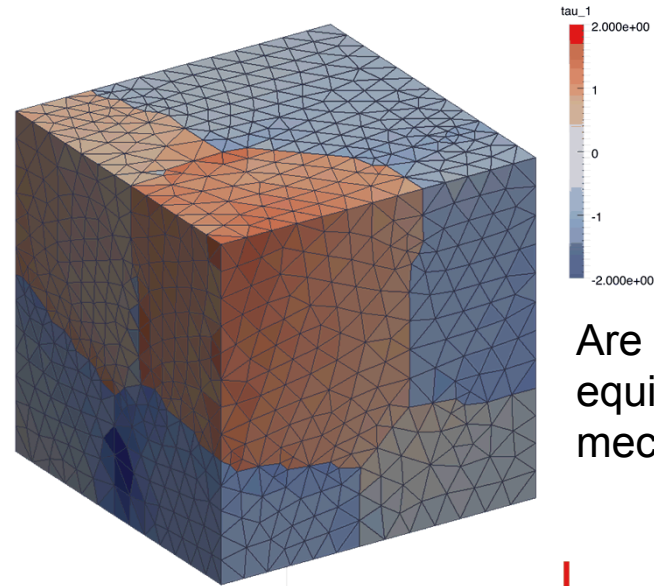
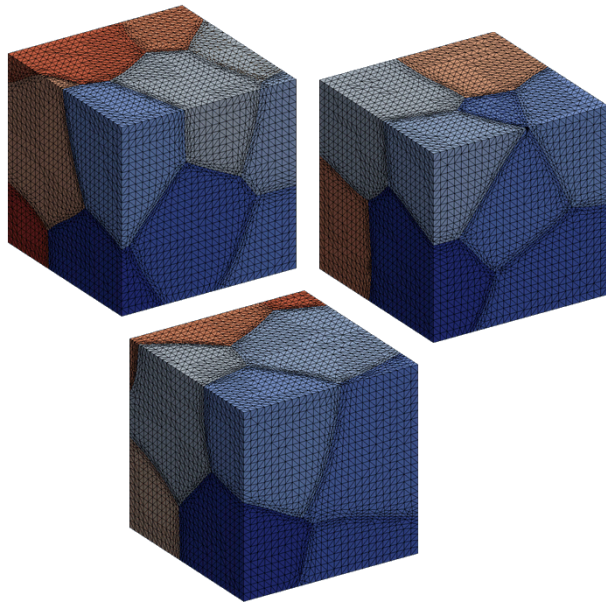


<sup>1</sup>D.J. Luscher, et. al., Int. J. Plasticity, 76, 2016, 111-129.

<sup>2</sup>J.C. Simo and T.J.R. Hughes, Computational Inelasticity, 1998.

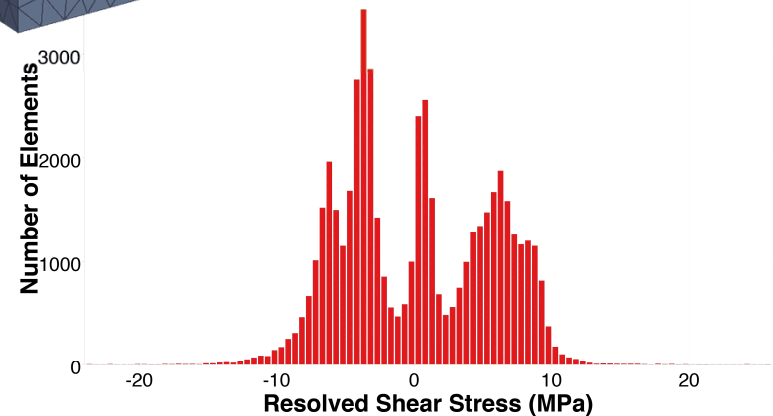
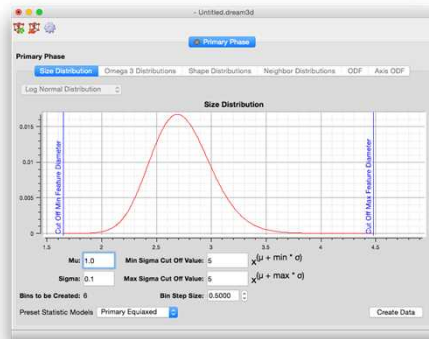


# Microstructural equivalency

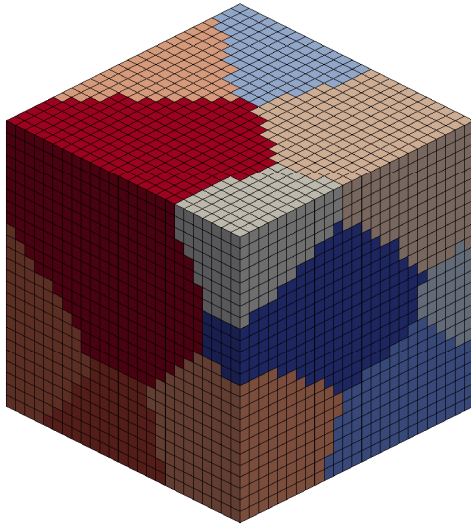


Are the realizations  
equivalent in terms of  
mechanical response?

Microstructural realizations from a single  
set of underlying morphological statistics



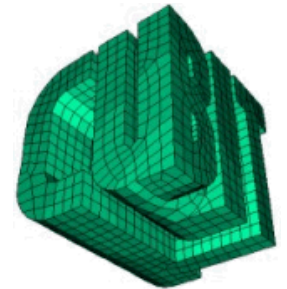
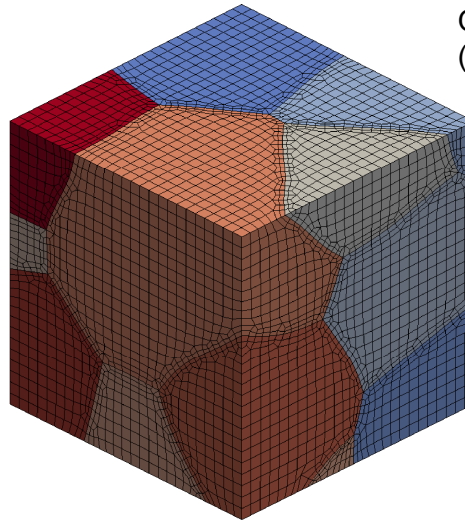
# Finite Element Meshing Workflow



```
SSSSS  CCCCC  UU  UU  LL  PPPPP  TTTTT
SS  SS  CC  CC  UU  UU  LL  PP  PP  TT
SS  CC  UU  UU  LL  PP  PP  TT
SSSSS  CC  UU  UU  LL  PPPPP  TT
SS  CC  UU  UU  LL  PP  TT
SS  CC  CC  UU  UU  LL  PP  TT
SSSSS  CCCCC  UUUUU  LLLLLL  PP  TT
```

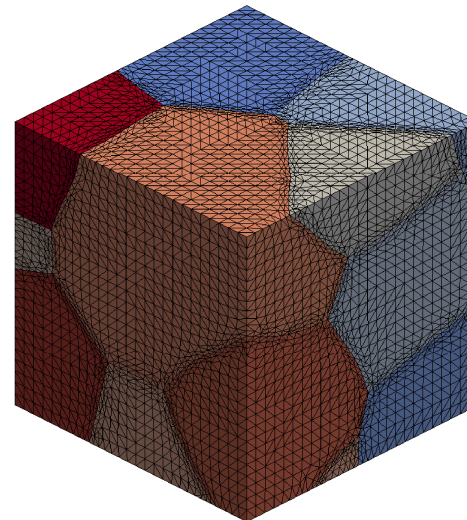
## SCULPT

Input: voxelized microstructure  
Output: finite element mesh  
(hexahedral)



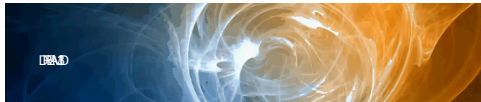
## CUBIT

Input: finite element mesh  
(hexahedral)  
Output: finite element mesh  
(e.g., tetrahedral)



## DREAM.3D

Input: microstructure  
statistics  
Output: voxelized  
microstructure



# Constitutive equation development

## Elasticity

### Micro

Second Piola-Kirchhoff stress

$$\underline{\underline{T}}^* = \underline{\underline{L}}^c : \underline{\underline{E}}^e$$

Elasticity tensor

$$\underline{\underline{L}}^c = \underline{\underline{R}}^T \cdot \underline{\underline{R}}^T \cdot \underline{\underline{L}} \cdot \underline{\underline{R}} \cdot \underline{\underline{R}}$$

Elastic Lagrangian strain

$$\underline{\underline{E}}^e = \frac{1}{2} [(\underline{\underline{F}}^e)^T \underline{\underline{F}}^e - \underline{\underline{I}}]$$

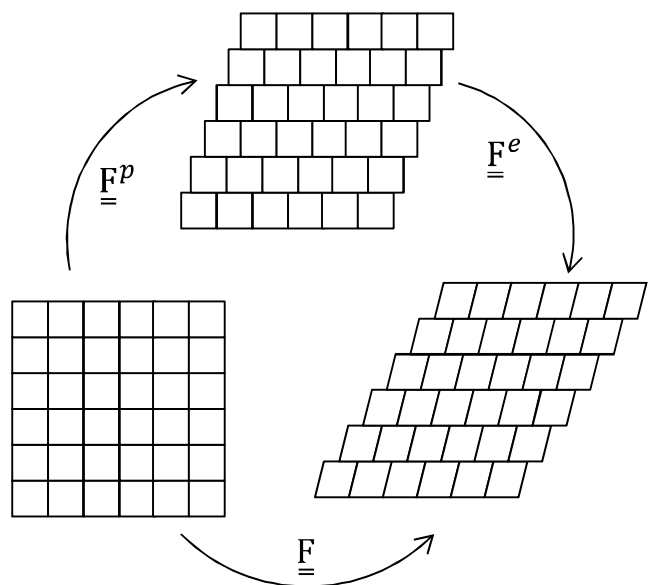
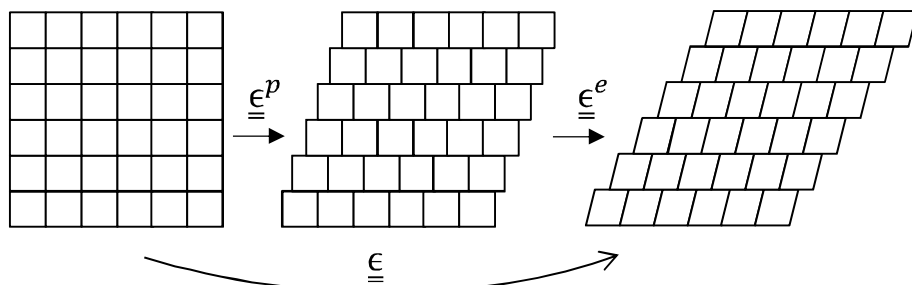
### Macro

Cauchy stress

$$\underline{\underline{\sigma}} = \underline{\underline{C}} : (\underline{\underline{\epsilon}} - \underline{\underline{\epsilon}}^p)$$

Elasticity tensor

$$\underline{\underline{C}} = \lambda \underline{\underline{I}} \otimes \underline{\underline{I}} + 2\mu \underline{\underline{I}}^{sym}$$



## Plasticity

### Micro

Plastic deformation gradient

$$\underline{\underline{F}}^p = \underline{\underline{I}}^{p*} \cdot \underline{\underline{F}}^p$$

Plastic velocity gradient

$$\underline{\underline{I}}^{p*} = \sum_{\alpha} \dot{\gamma}^{\alpha} \underline{\underline{P}}_0^{\alpha}$$

Flow rule

$$\dot{\gamma}^{\alpha} = \dot{\gamma}_0 \left| \frac{\underline{\underline{\tau}} : \underline{\underline{P}}_0^{\alpha}}{\tau_0 + g^{\alpha}} \right|^k \text{sign}(\underline{\underline{\tau}} : \underline{\underline{P}}_0^{\alpha})$$

Evolving slip resistance

$$\dot{g}^{\alpha} = H^{\alpha\beta} \dot{\gamma}^{\beta}$$

Plastic dissipation

$$\mathcal{D}^p = \sum_{\alpha} \tau^{\alpha} \dot{\gamma}^{\alpha} - \dot{g}^{\alpha} q^{\alpha}$$

### Macro

Yield function

$$f(\underline{\underline{\sigma}}) = \phi(\|\underline{\underline{d}}^{ev} : \underline{\underline{\sigma}}\|) - \sigma_Y(\bar{\epsilon}^p)$$

Plastic strain rate

$$\underline{\underline{\epsilon}}^p = \lambda \frac{\partial f}{\partial \underline{\underline{\sigma}}}$$

Evolving hardening

$$\dot{\alpha} = \lambda \frac{\partial f}{\partial \bar{\epsilon}^p}$$

Plastic dissipation

$$\mathcal{D}^p = \underline{\underline{\sigma}} : \underline{\underline{\epsilon}}^p - \dot{\alpha} \bar{\epsilon}^p$$

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# Composite Tet



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# Combined F-bar Formulation

- Isochoric-volumetric split (Hughes 1975, Simo 1975)

$$\mathbf{F} = \mathbf{F}_{\text{vol}} \cdot \mathbf{F}_{\text{iso}}$$

- Replacing volumetric split with assumed term

$$\bar{\mathbf{F}} = \bar{J}^{1/3} \mathbf{F}_{\text{iso}} = \bar{J}^{1/3} J^{-1/3} \mathbf{F}$$

↑  
Modified det(F)

↑  
Original det(F)

Relaxing too much, we get instabilities  
Relaxing too little, we get the volumetric locking

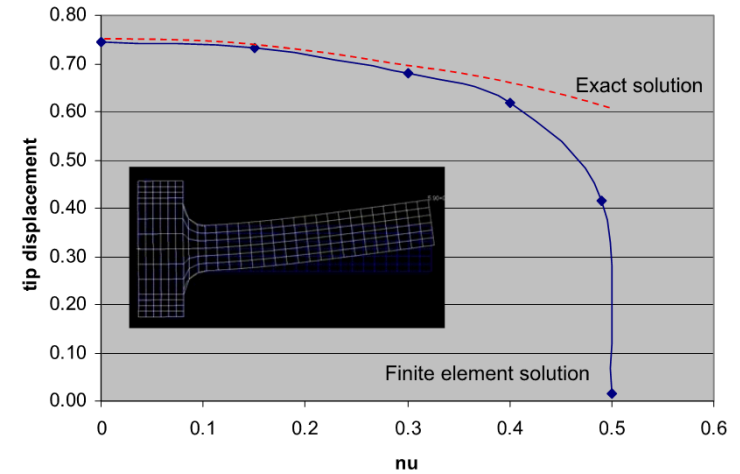
- Combined F-bar approach

$$\tilde{\mathbf{F}} = \alpha \mathbf{F} + (1 - \alpha) \bar{\mathbf{F}}. \quad \leftarrow \text{Invalid operation}$$

- Current Approach via Lie algebra

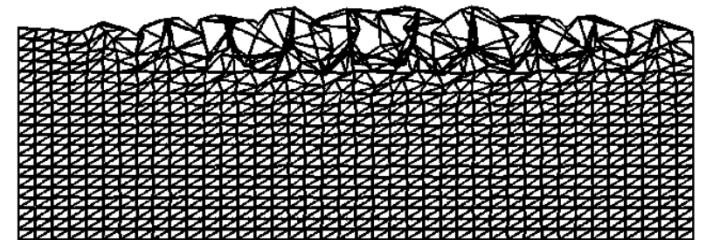
$$\tilde{J} = \exp \left( \frac{1 - \beta}{V_{\mathcal{B}^e}} \int_{\mathcal{B}^e} \log J \, dV + \beta \log J \right).$$

$$\tilde{J}_M = \exp \left( \log \tilde{J} - 3 \left( \frac{1 - \beta}{V_{\mathcal{B}^e}} \int_{\mathcal{B}^e} \alpha_{sk} (\theta - \theta_o) \, dV + \beta \alpha_{sk} (\theta - \theta_o) \right) \right)$$



Standard F leads to  
Volumetric Locking

$$\lambda_{\text{cr}} = 90.3557, \alpha = 0 \times E$$



Pure F-bar leads to instability  
(Brocardo, Micheloni, Krysl, IJNME, 2009)

# 10-Node Composite Tetrahedral Element

Motivated by prior work of Thoutireddy, et. al., IJNME (2002)

$$\Phi[\varphi, \bar{\mathbf{F}}, \bar{\mathbf{P}}] := \int_B A(\bar{\mathbf{F}}) dV + \int_B \bar{\mathbf{P}} : (\mathbf{F} - \bar{\mathbf{F}}) dV - \int_B R\mathbf{B} \cdot \varphi dV - \int_{\partial_T B} \mathbf{T} \cdot \varphi dS$$

$$\bar{\mathbf{P}} = \lambda_\alpha \left( \int_\Omega \lambda_\alpha \lambda_\beta \mathbf{I} dV \right)^{-1} \int_\Omega \lambda_\beta \mathbf{P} dV,$$

$$\bar{\mathbf{F}} = \lambda_\alpha \left( \int_\Omega \lambda_\alpha \lambda_\beta \mathbf{I} dV \right)^{-1} \int_\Omega \lambda_\beta \mathbf{F} dV$$

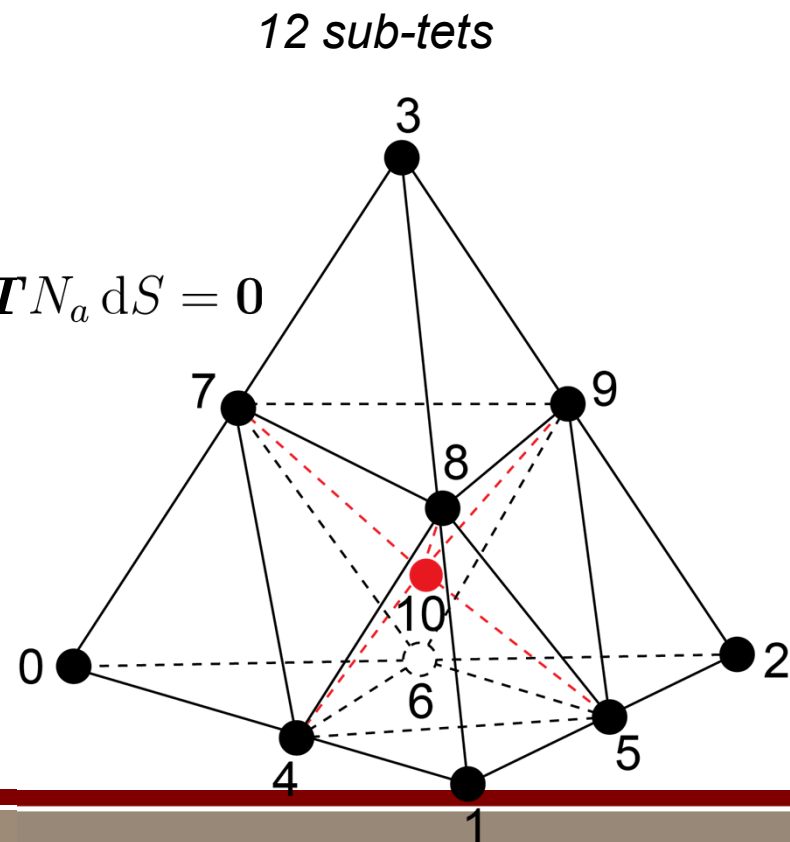
$$\mathbf{R}_a(\varphi) := \int_\Omega \bar{\mathbf{P}} \cdot \mathbf{B}_a dV - \int_\Omega R\mathbf{B} N_a dV - \int_{\partial_T \Omega} \mathbf{T} N_a dS = 0$$

$$\mathbf{B}_a(\mathbf{X}) := \delta_{ik} \frac{\partial N_a(\mathbf{X})}{\partial X_J} \mathbf{e}_i \otimes \mathbf{E}_J \otimes \mathbf{e}_k$$

$\varphi$   $C^0$  piecewise linear

$\bar{\mathbf{F}}$   $C^{-1}$  linear over parent element

$\bar{\mathbf{P}}$   $C^{-1}$  linear over parent element





# Analytical gradient operator

Develop an exact gradient operator that projects and interpolates sub-tet gradients

$$\bar{\mathbf{F}}(\mathbf{X}) := \bar{\mathbf{B}}_a(\mathbf{X}) \mathbf{x}_a$$

$$\bar{\mathbf{B}}_a(\mathbf{X}) := \lambda_\alpha(\mathbf{X}) \left[ \int_{\Omega} \delta_{ik} \lambda_\alpha(\mathbf{X}) \lambda_\beta(\mathbf{X}) dV \right]^{-1} \int_{\Omega} \lambda_\beta(\mathbf{X}) \frac{\partial N_a(\mathbf{X})}{\partial X_J} dV \mathbf{e}_i \otimes \mathbf{E}_J \otimes \mathbf{e}_k$$

$$\bar{\mathbf{B}}_a(\boldsymbol{\xi}) = \lambda_\alpha(\boldsymbol{\xi}) \left[ \int_{\Omega_\xi} \delta_{ik} \lambda_\alpha(\boldsymbol{\xi}) \lambda_\beta(\boldsymbol{\xi}) dV_\xi \right]^{-1} \int_{\Omega_\xi} \lambda_\beta(\boldsymbol{\xi}) \frac{\partial N_a(\boldsymbol{\xi})}{\partial \xi} dV_\xi \left( \frac{\partial \xi}{\partial X_J} \right) \mathbf{e}_i \otimes \mathbf{E}_J \otimes \mathbf{e}_k$$

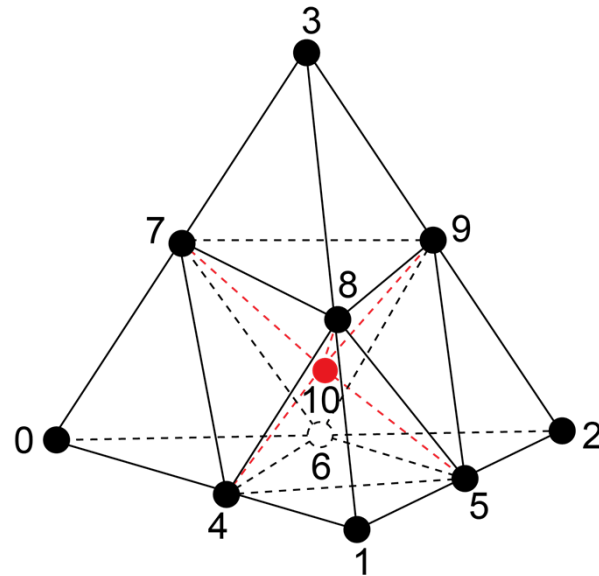
$$\bar{\mathbf{B}}_a(\boldsymbol{\xi}) = \bar{\mathcal{L}}_{a;ilk}(\boldsymbol{\xi}) \left( \frac{\partial \xi_l}{\partial X_J} \right) \mathbf{e}_i \otimes \mathbf{E}_J \otimes \mathbf{e}_k$$

$$\bar{\mathcal{L}}_a(\boldsymbol{\xi}) = \lambda_\alpha(\boldsymbol{\xi}) \delta_{ik} (M_{\alpha\beta})^{-1} \sum_{S=0}^{11} \frac{\partial N_a}{\partial \xi_l} \int_{E_S} \lambda_\beta(\boldsymbol{\xi}) dV_\xi \mathbf{e}_i \otimes \mathbf{a}_l \otimes \mathbf{e}_k$$

$$\bar{\mathbf{B}}_a(\boldsymbol{\xi}) = \bar{\mathcal{L}}_{a;ilk}(\boldsymbol{\xi}) [\bar{\mathcal{L}}_{b;JLM}(\boldsymbol{\xi}) X_{b;M}]^{-1} \mathbf{e}_i \otimes \mathbf{E}_J \otimes \mathbf{e}_k$$

$$\bar{B}_{aJ}(\boldsymbol{\xi}) = \bar{L}_{al}(\boldsymbol{\xi}) [X_{Jb} \bar{L}_{bl}(\boldsymbol{\xi})]^{-1}$$

$$\bar{F}_{iJ}(\boldsymbol{\xi}) = x_{ia} \bar{B}_{aJ}(\boldsymbol{\xi})$$

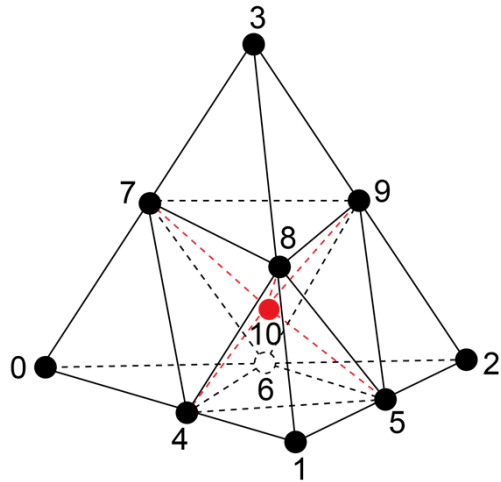


$$\bar{L}_{al}(\boldsymbol{\xi}) \equiv \bar{L}_{10 \times 3} = \frac{1}{24}$$

Evaluate for your  
integration scheme

$$\begin{pmatrix} 9-60\xi_0 & 9-60\xi_0 & 9-60\xi_0 \\ -9+60\xi_1 & 0 & 0 \\ 0 & -9+60\xi_2 & 0 \\ 0 & 0 & -9+60\xi_3 \\ 70(\xi_0-\xi_1) & 2(-4-35\xi_1+5\xi_2+10\xi_3) & 2(-4-35\xi_1+10\xi_2+5\xi_3) \\ 2(-1+5\xi_1+40\xi_2-5\xi_3) & 2(-1+40\xi_1+5\xi_2-5\xi_3) & 10(\xi_0-\xi_3) \\ 2(-4+5\xi_1-35\xi_2+10\xi_3) & 70(\xi_0-\xi_2) & 2(-4+10\xi_1-35\xi_2+5\xi_3) \\ 2(-4+5\xi_1+10\xi_2-35\xi_3) & 2(-4+10\xi_1+5\xi_2-35\xi_3) & 70(\xi_0-\xi_3) \\ 2(-1+5\xi_1-5\xi_2+40\xi_3) & 10(\xi_0-\xi_2) & 2(-1+40\xi_1-5\xi_2+5\xi_3) \\ 10(\xi_0-\xi_1) & 2(-1-5\xi_1+5\xi_2+40\xi_3) & 2(-1-5\xi_1+40\xi_2+5\xi_3) \end{pmatrix}.$$

# Suitable for isochoric motions

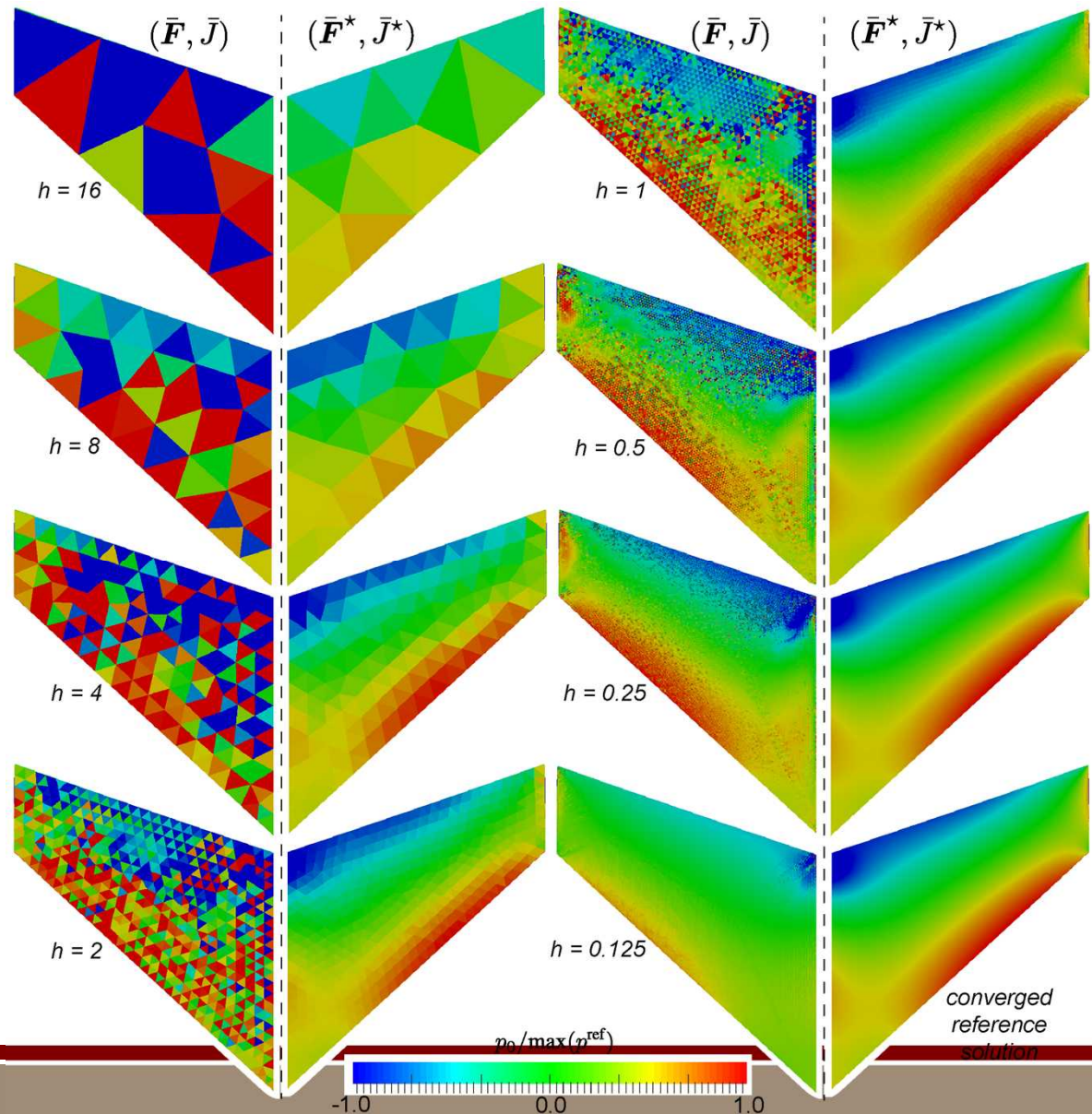


*Volume-averaged formulation  
does not exhibit spurious  
pressure oscillations.*

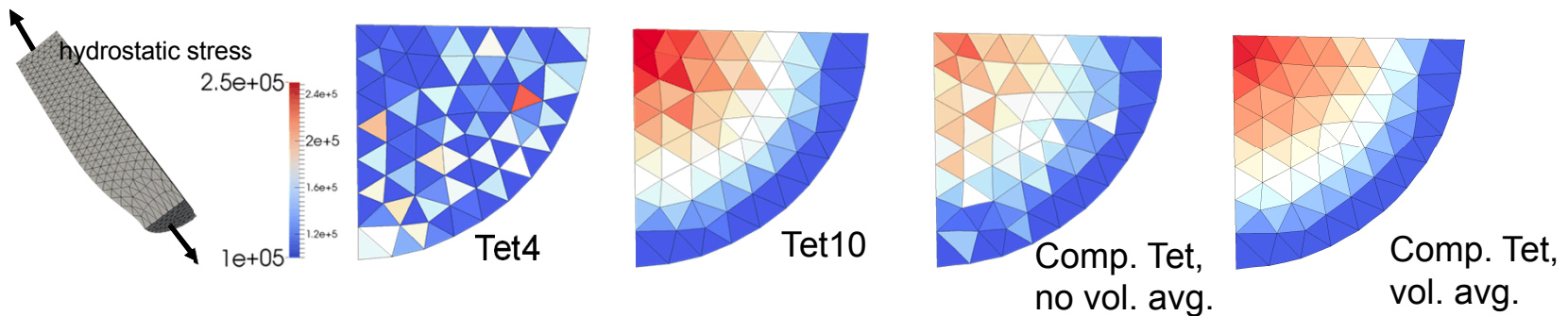
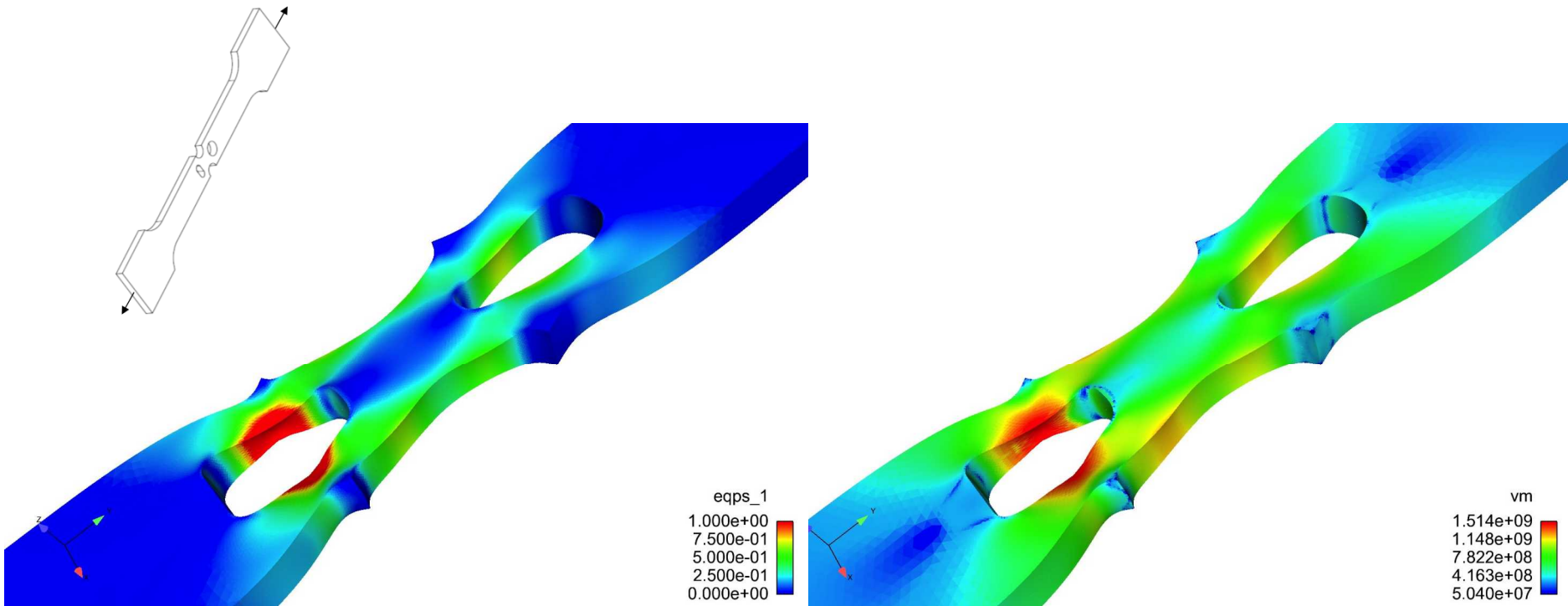
$$\bar{\mathbf{F}}^*(\boldsymbol{\xi}) := \left( \frac{\bar{J}^*}{\bar{J}(\boldsymbol{\xi})} \right)^{\frac{1}{3}} \bar{\mathbf{F}}(\boldsymbol{\xi})$$

$$\bar{J}^* := \frac{\int_{\Omega} \bar{J} \, dV}{\int_{\Omega} dV}$$

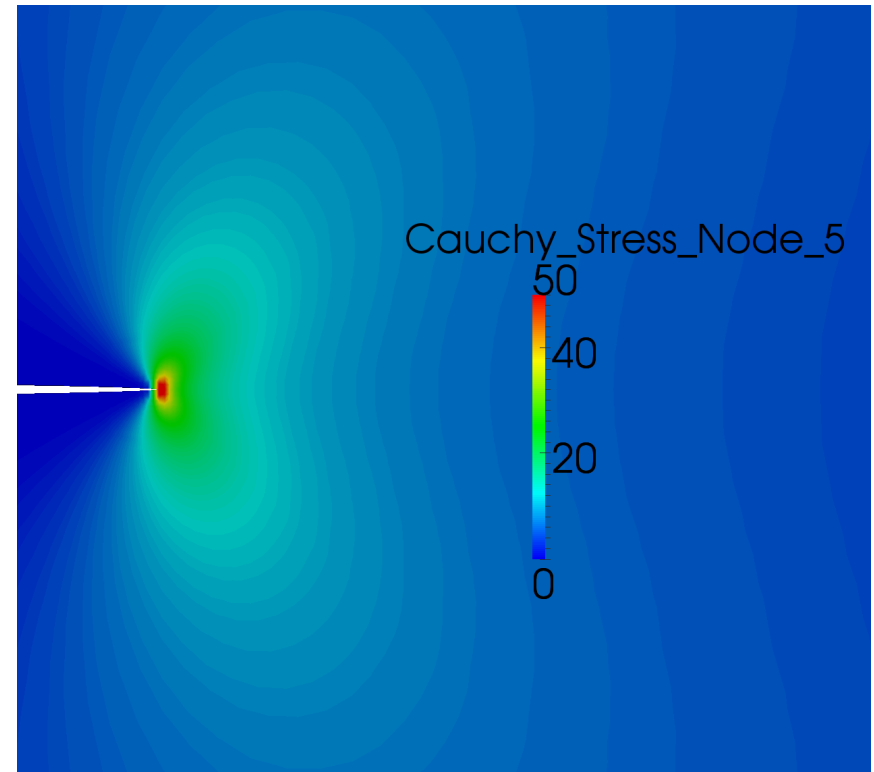
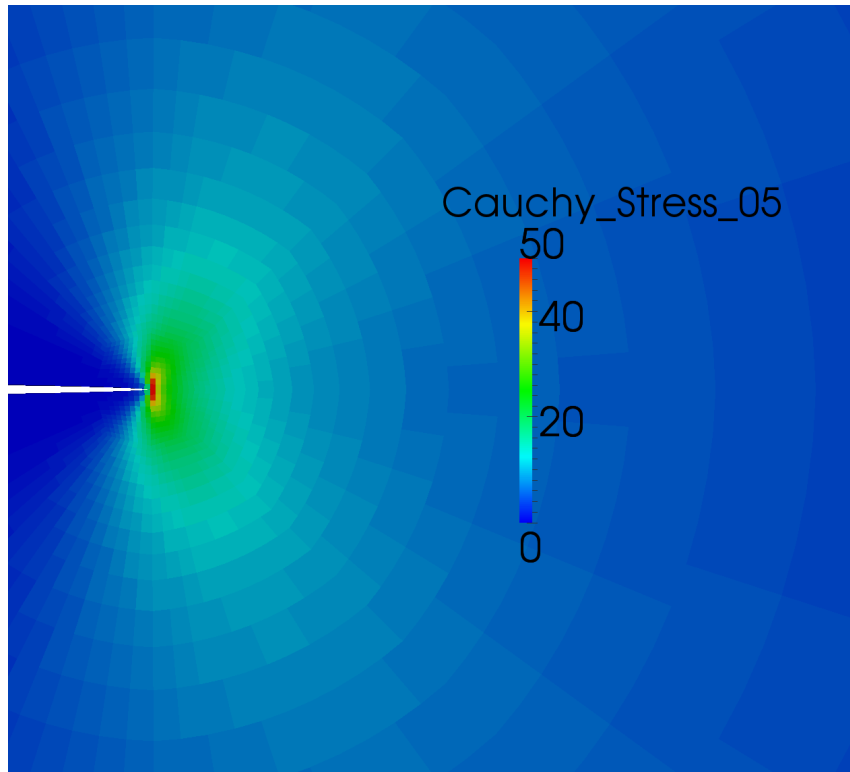
$$\bar{p}^* := \frac{1}{V_{\Omega}} \int_{\Omega} \text{tr} \frac{\partial A(\mathbf{F}^*)}{\partial \mathbf{b}^*} dV$$



# Composite Tet



# Projecting IP Data to Nodes



*To facilitate the mapping procedure, state variable data known at the integration points need to be projected to the nodes. The following interpolation can then use the standard basis functions.*

*Currently in Albany, we can approximate a global L2 projection by computing a nodal volume average for integration point quantities.*

*Exceptional service in the national interest*



# Remeshing/Mapping



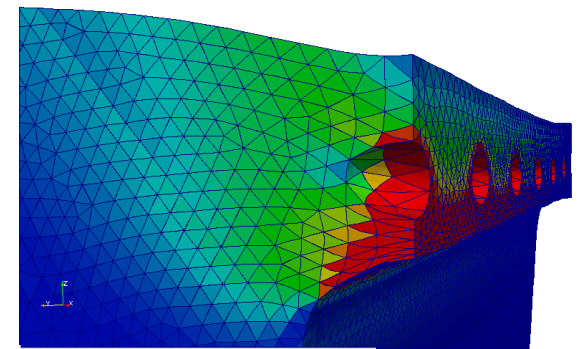
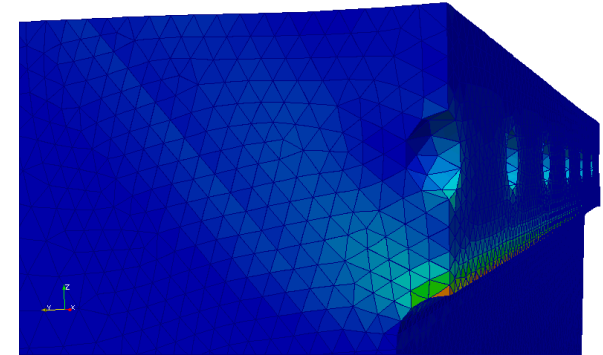
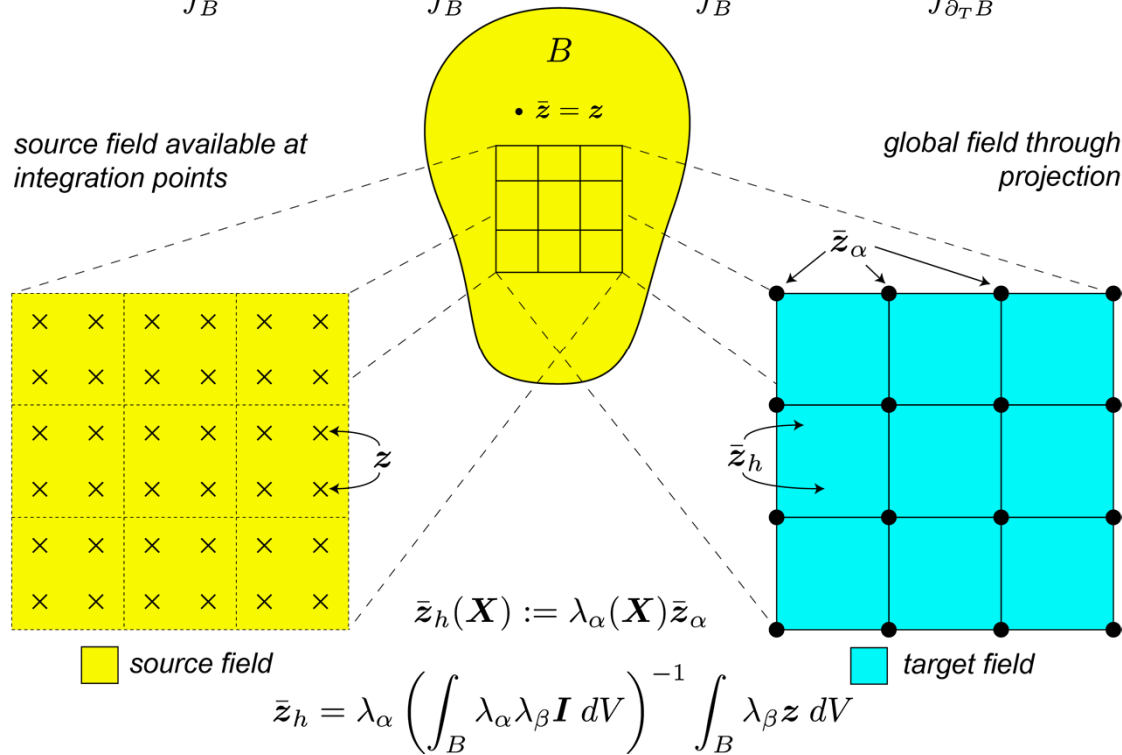
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# ISVs - Remeshing and (Re)mapping

$$\Phi[\varphi, \bar{z}, \bar{y}] := \int_B W(\mathbf{F}, \bar{z}) dV + \int_B \bar{y} \cdot (\bar{z} - z) dV - \int_B \rho_0 \mathbf{B} \cdot \varphi dV - \int_{\partial_T B} \mathbf{T} \cdot \varphi dS$$





# Objectives for FY13

Using the Theory of Groups,  
state variable constraints can be  
maintained during an  
interpolation step!

## ■ Solution and State remapping

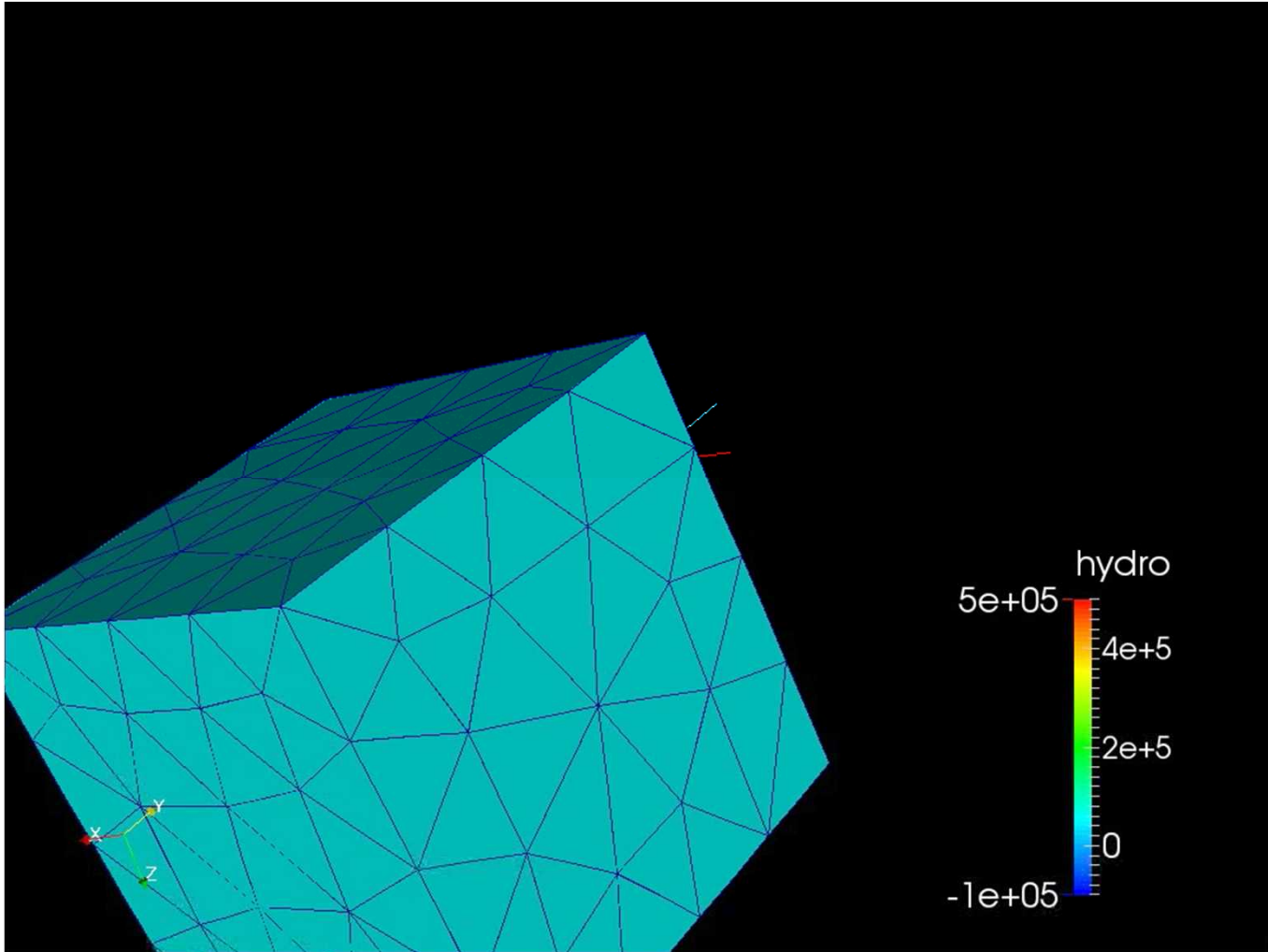
Lie Group $G$	$\mathbf{Z}_1$	$\mathbf{Z}_2$	$\mathbf{Z}(0)$	$\in G$	$\mathbf{Z}(2)$	$\in G$
$\mathbb{R}^+$	0.90	0.10	0.50	yes	-0.70	no
$GL^+(3)$	$\begin{pmatrix} 2 & 0 & 4 \\ 0 & 2 & 0 \\ 0 & 0 & 2 \end{pmatrix}$	$\begin{pmatrix} 2 & 0 & 0 \\ 0 & 2 & 0 \\ 4 & 0 & 2 \end{pmatrix}$	$\begin{pmatrix} 2 & 0 & 2 \\ 0 & 2 & 0 \\ 2 & 0 & 2 \end{pmatrix}$	no	$\begin{pmatrix} 2 & 0 & -2 \\ 0 & 2 & 0 \\ 6 & 0 & 2 \end{pmatrix}$	yes
$SL(3)$	$\begin{pmatrix} 1 & 2 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix}$	$\begin{pmatrix} 1 & 0 & 0 \\ 2 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix}$	$\begin{pmatrix} 1 & 1 & 0 \\ 1 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix}$	no	$\begin{pmatrix} 1 & -1 & 0 \\ 3 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix}$	no
$SO(3)$	$\begin{pmatrix} 1 & 0 & 0 \\ 0 & 0 & -1 \\ 0 & 1 & 0 \end{pmatrix}$	$\begin{pmatrix} 0 & 0 & 1 \\ 0 & 1 & 0 \\ -1 & 0 & 0 \end{pmatrix}$	$\begin{pmatrix} 0.50 & 0.00 & 0.50 \\ 0.00 & 0.50 & -0.50 \\ -0.50 & 0.50 & 0.00 \end{pmatrix}$	no	$\begin{pmatrix} -0.50 & 0.00 & 1.50 \\ 0.00 & 1.50 & 0.50 \\ -1.50 & -1.50 & 0.00 \end{pmatrix}$	no

Table 2: Interpolation and extrapolation without Lie algebras.  $\mathbf{Z}_1 = \mathbf{Z}(-1)$  and  $\mathbf{Z}_2 = \mathbf{Z}(1)$  are given data and belong to the corresponding Lie group.  $\mathbf{Z}(\xi) := N_1(\xi)\mathbf{Z}_1 + N_2(\xi)\mathbf{Z}_2$ .  $N_1(\xi) := \frac{1}{2}(1 - \xi)$  and  $N_2(\xi) := \frac{1}{2}(1 + \xi)$ .

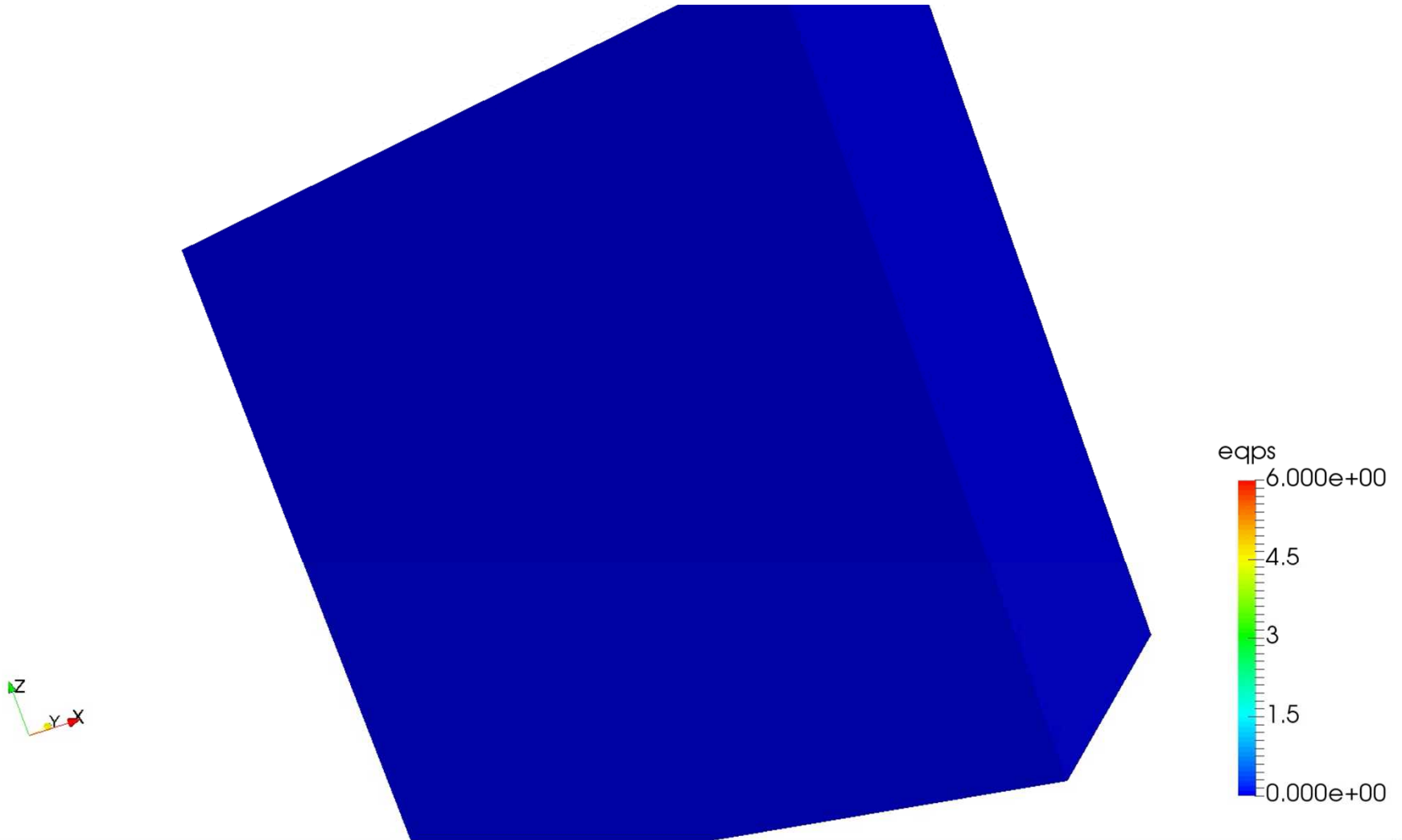
Lie Group $G$	$\mathbf{Z}_1$	$\mathbf{Z}_2$	$\mathbf{Z}(0)$	$\in G$	$\mathbf{Z}(2)$	$\in G$
$\mathbb{R}^+$	0.90	0.10	0.30	yes	0.03	yes
$GL^+(3)$	$\begin{pmatrix} 2 & 0 & 4 \\ 0 & 2 & 0 \\ 0 & 0 & 2 \end{pmatrix}$	$\begin{pmatrix} 2 & 0 & 0 \\ 0 & 2 & 0 \\ 4 & 0 & 2 \end{pmatrix}$	$\begin{pmatrix} 3.09 & 0.00 & 2.35 \\ 0.00 & 2.00 & 0.00 \\ 2.35 & 0.00 & 3.09 \end{pmatrix}$	yes	$\begin{pmatrix} -0.32 & 0.00 & -1.14 \\ 0.00 & 2.00 & 0.00 \\ 3.42 & 0.00 & -0.32 \end{pmatrix}$	yes
$SL(3)$	$\begin{pmatrix} 1 & 2 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix}$	$\begin{pmatrix} 1 & 0 & 0 \\ 2 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix}$	$\begin{pmatrix} 1.54 & 1.18 & 0.00 \\ 1.18 & 1.54 & 0.00 \\ 0.00 & 0.00 & 1.00 \end{pmatrix}$	yes	$\begin{pmatrix} -0.16 & -0.57 & 0.00 \\ 1.71 & -0.16 & 0.00 \\ 0.00 & 0.00 & 1.00 \end{pmatrix}$	yes
$SO(3)$	$\begin{pmatrix} 1 & 0 & 0 \\ 0 & 0 & -1 \\ 0 & 1 & 0 \end{pmatrix}$	$\begin{pmatrix} 0 & 0 & 1 \\ 0 & 1 & 0 \\ -1 & 0 & 0 \end{pmatrix}$	$\begin{pmatrix} 0.72 & 0.28 & 0.63 \\ 0.28 & 0.72 & -0.63 \\ -0.63 & 0.63 & 0.44 \end{pmatrix}$	yes	$\begin{pmatrix} -0.61 & -0.54 & 0.58 \\ -0.54 & 0.82 & 0.19 \\ -0.58 & -0.19 & -0.79 \end{pmatrix}$	yes

Table 3: Interpolation and extrapolation with Lie algebras.  $\mathbf{Z}_1 = \mathbf{Z}(-1)$  and  $\mathbf{Z}_2 = \mathbf{Z}(1)$  are given data and belong to the corresponding Lie group.  $\mathbf{Z}(\xi) := \exp \mathbf{z}(\xi)$ ,  $\mathbf{z}(\xi) := N_1(\xi)\mathbf{z}_1 + N_2(\xi)\mathbf{z}_2$ ,  $\mathbf{z}_1 := \log \mathbf{Z}_1$ ,  $\mathbf{z}_2 := \log \mathbf{Z}_2$ .  $N_1(\xi) := \frac{1}{2}(1 - \xi)$  and  $N_2(\xi) := \frac{1}{2}(1 + \xi)$ .

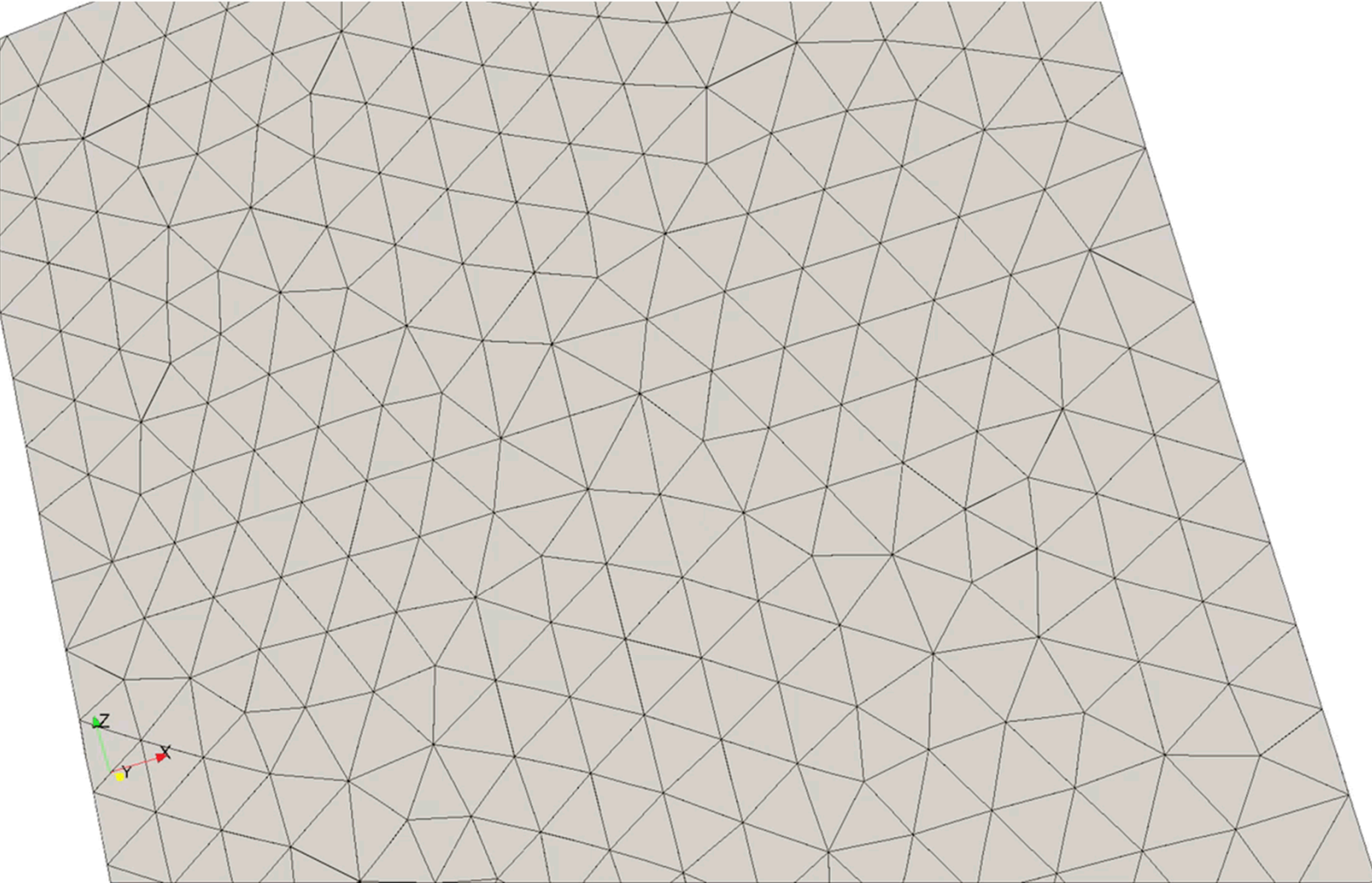
# Adaptivity Example



# Capturing the evolution of inelasticity

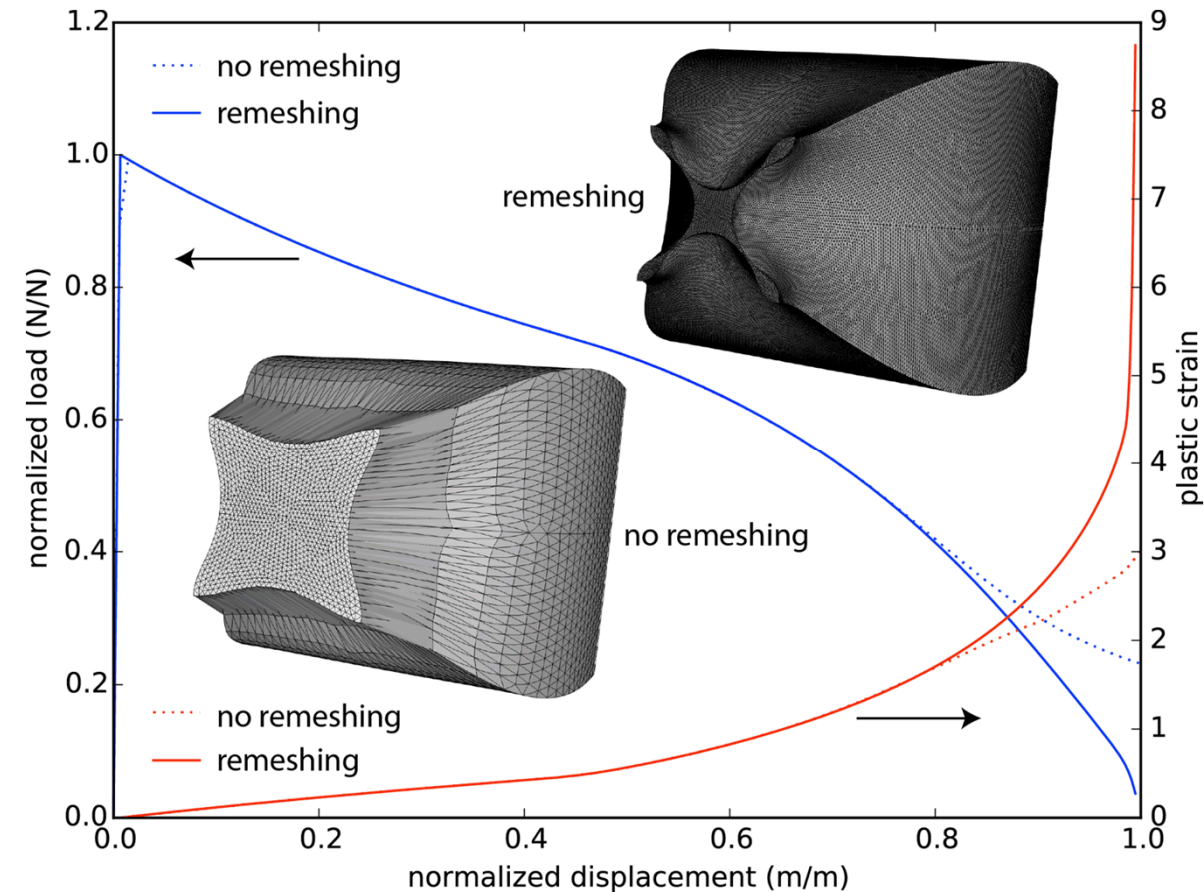


# Resolving the deformation with remeshing/mapping

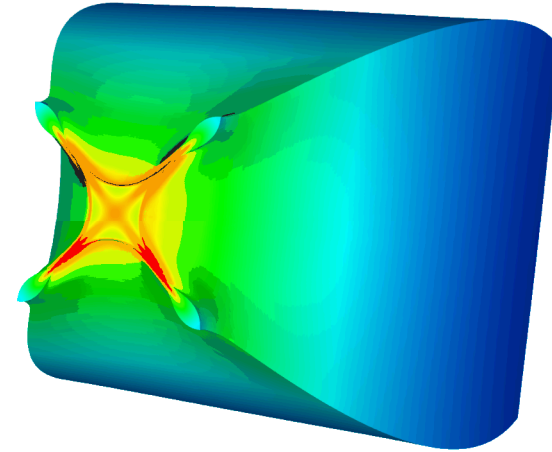


# Local processes are requisite for prediction

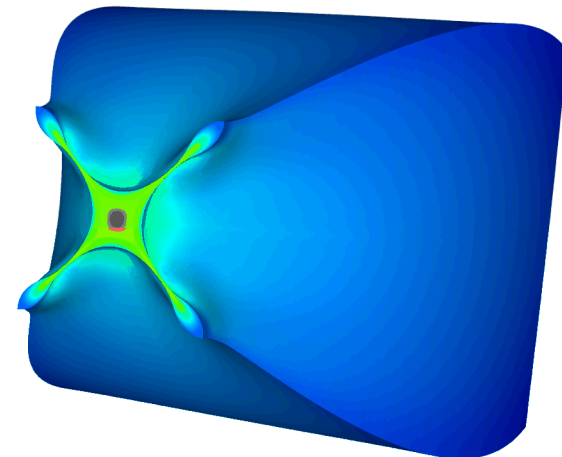
Remeshing the body 30 times in  $\sim 0.25 \varepsilon_p$  increments



eqps  
6.000e+00  
4.500e+00  
3.000e+00  
1.500e+00  
0.000e+00



hydro  
7.344e+03  
5.508e+03  
3.672e+03  
1.836e+03  
0.000e+00



*J<sub>2</sub> plasticity w/linear hardening (PH 13-8 H950)*

*Exceptional service in the national interest*



# Regularized damage evolution for ductile fracture using localization elements

*J.W. Foulk III, A. Lindblad, J.M. Emery, A. Mota, J.T. Ostien, A.A. Brown, T.J. Vogler*

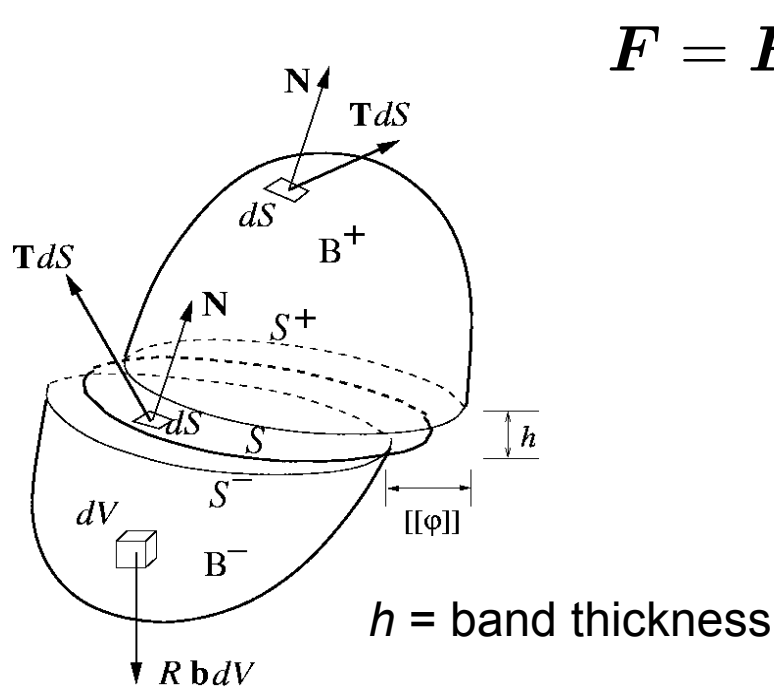


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# Capture sub-grid processes for R(a) Sandia National Laboratories

Goal: Capture sub-grid processes through methods that regularize the jump



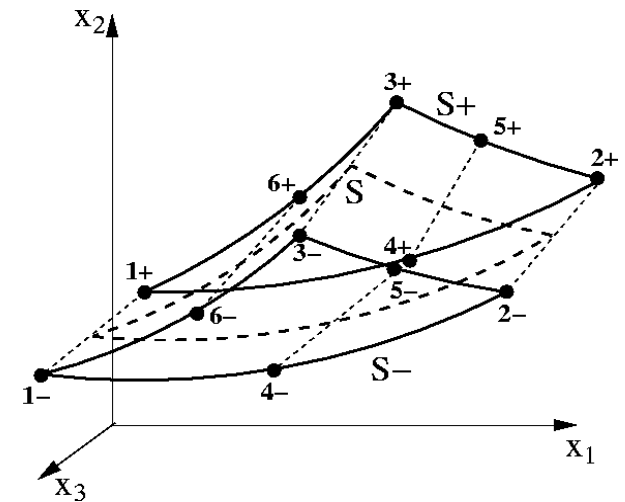
$$\mathbf{F} = \mathbf{F}^{\parallel} \mathbf{F}^{\perp}$$

$$\mathbf{F}^{\parallel} = \mathbf{g}_i \otimes \mathbf{G}^i$$

$$\mathbf{F}^{\perp} = \mathbf{I} + \frac{[[\Phi]]}{h} \otimes \mathbf{N}$$

$$\mathbf{F} = \mathbf{F}^{\parallel} + \frac{[[\varphi]]}{h} \otimes \mathbf{N}$$

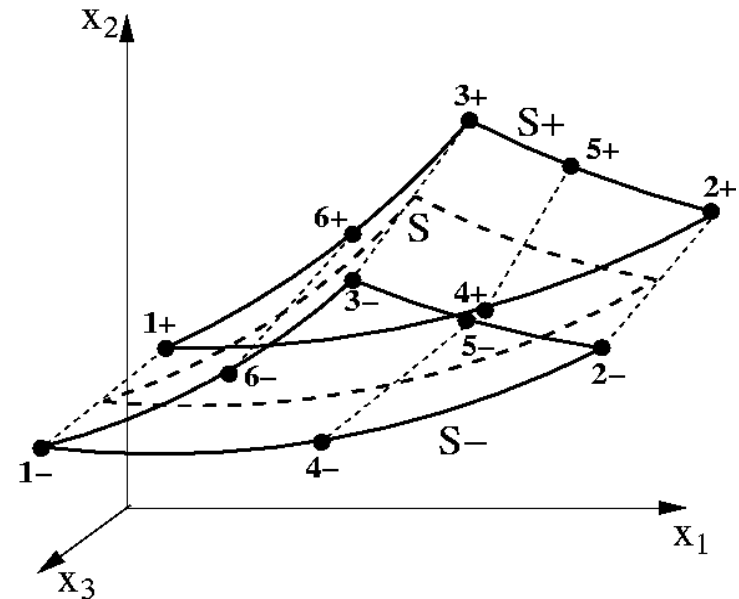
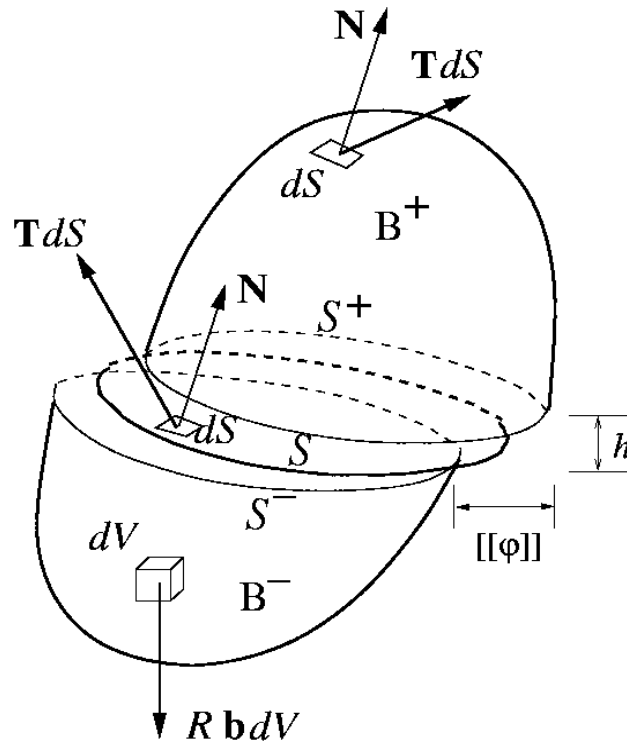
- Finite-deformation kinematics
- Simulation of strain localization
- No additional constitutive assumptions



*Akin to "cohesive" element*

# Kinematic assumptions

*Topical: Plenary talk by Xavier Oliver*

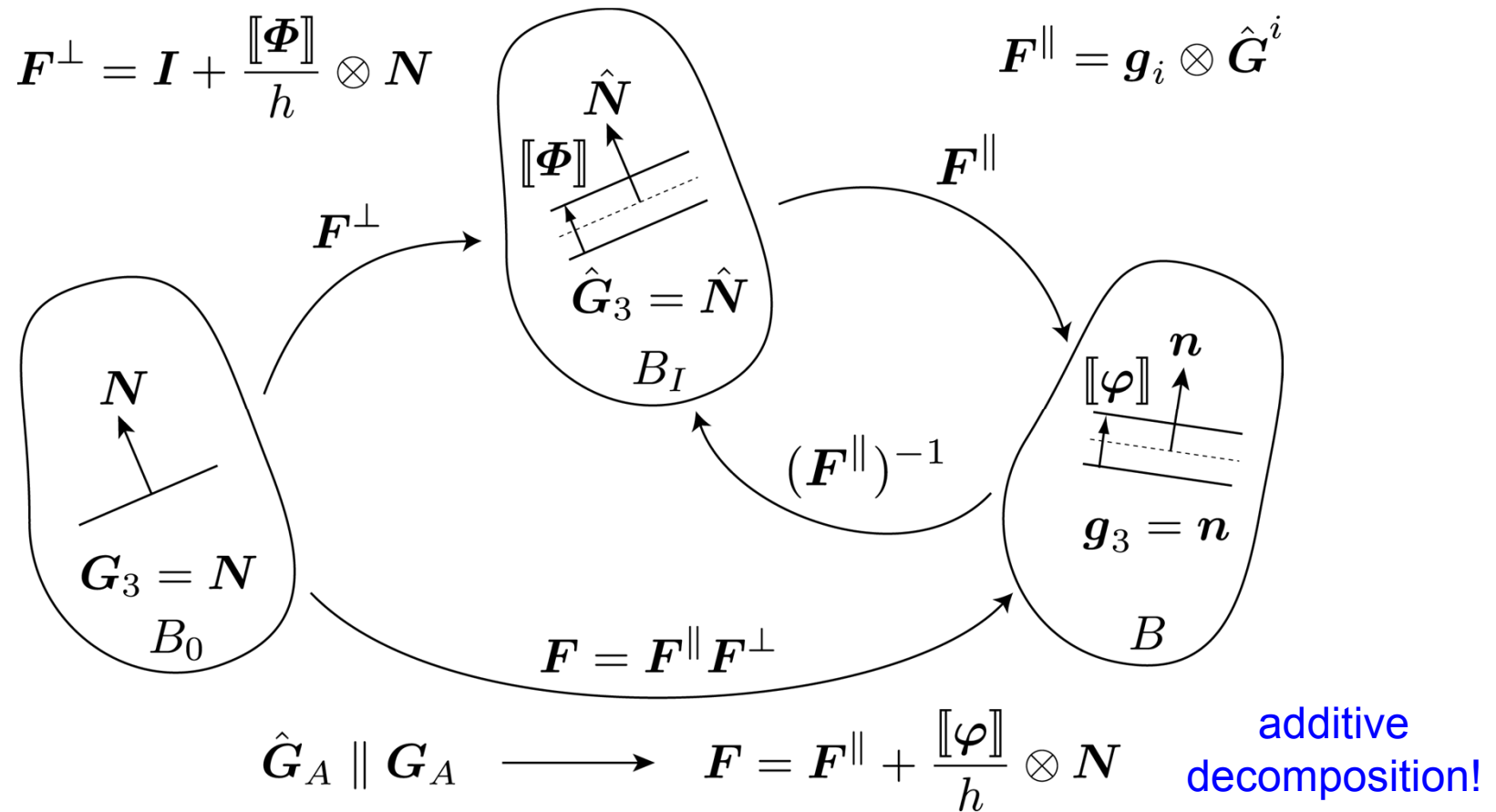


- Finite-deformation kinematics.
- Simulation of strain localization.
- No additional constitutive assumptions

$$\mathbf{F}^\perp = \mathbf{I} + \frac{[[\varphi]]}{h} \otimes \mathbf{N} \quad \mathbf{F}^\parallel = \mathbf{g}_i \otimes \mathbf{G}^i$$

$$\mathbf{F} = \mathbf{F}^\parallel \mathbf{F}^\perp$$

# An intermediate configuration



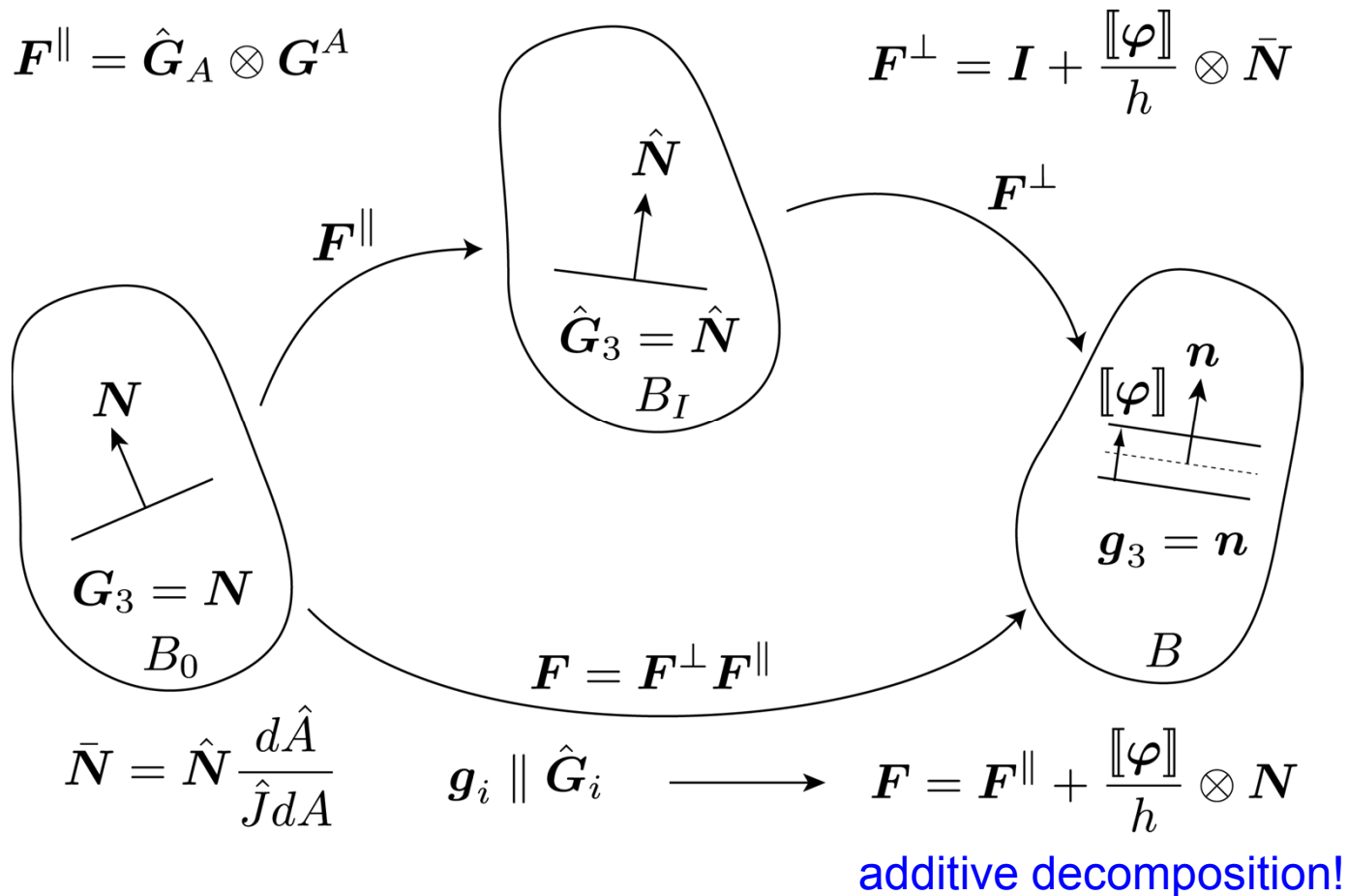
The jump is pushed backwards

$$[[\Phi]] = (F^\parallel)^{-1} [[\varphi]]$$

Retain definition of membrane def.  
grad.

$$F^\parallel = g_i \otimes G^i$$

# The order is not unique



The normal used for construction

$$\bar{N} = (F^\parallel)^{-T} N$$

Retain definition of membrane def.  
grad.

$$F^\parallel = g_i \otimes G^i$$

# Extending surface to multiphysics

*Our work in finite-deformation diffusion has extended elements to multiphysics (LCM)*

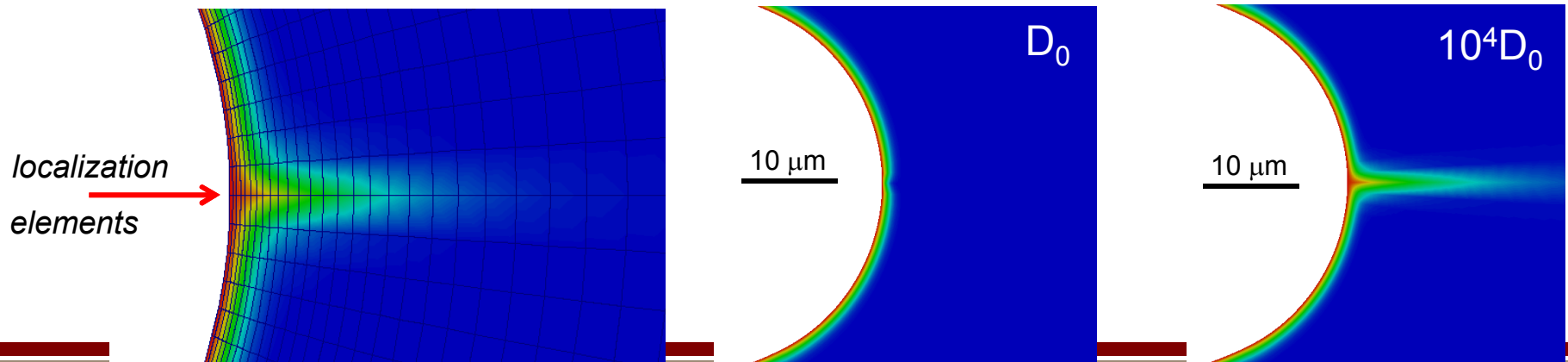
Fox and Simo (1990), Callari, Armero, Abati (2010)

redefine space  $\mathbf{X} = \Phi(\xi^1, \xi^2, \xi^3) = \bar{\Phi}(\xi^1, \xi^2) + \mathbf{N}(\xi^1, \xi^2)\xi^3$   $\mathbf{G}_i = \Phi_{,i} = \frac{\partial \mathbf{X}}{\partial \xi^i}$

include jump in  $C$   $C(\mathbf{X}) = \bar{C}(\phi[\xi^1, \xi^2]) + \frac{[[C]](\phi[\xi^1, \xi^2])}{h}\xi^3$   $\nabla_{\mathbf{X}} C = (\nabla \Phi)^{-T} \frac{\partial C}{\partial \xi^i}$

*Given this gradient operator, we can use the same PDE for finite-deformation diffusion*

$$D^* \dot{C}_L - \nabla_{\mathbf{X}} \cdot d_l \mathbf{C}^{-1} \nabla_{\mathbf{X}} C_L + \nabla_{\mathbf{X}} \cdot \frac{d_l V_H}{RT} \mathbf{C}^{-1} \nabla_{\mathbf{X}} \tau_h C_L + \theta_T \frac{dN_T}{d\epsilon_p} \dot{\epsilon}_p$$



*Exceptional service in the national interest*



# Sandia Fracture Challenge



U.S. DEPARTMENT OF  
**ENERGY**

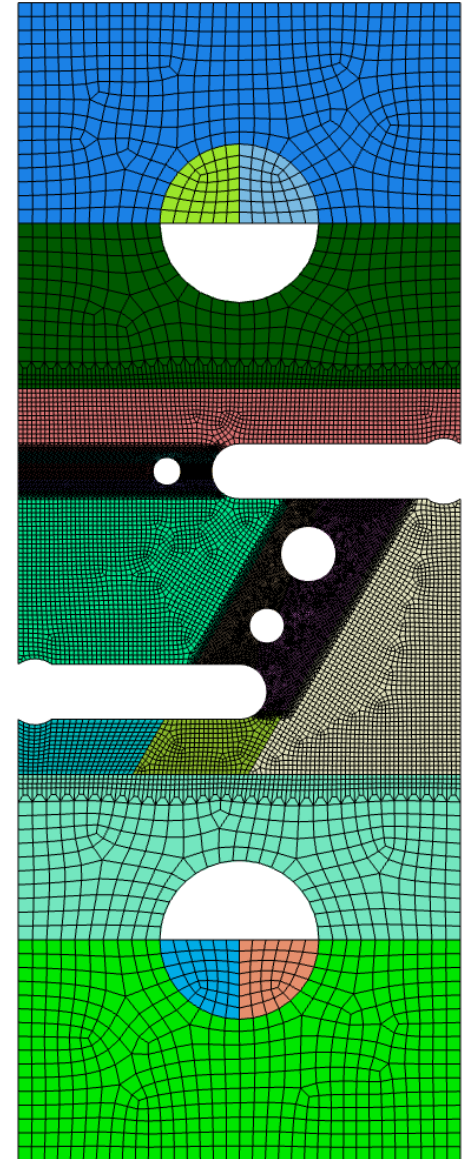


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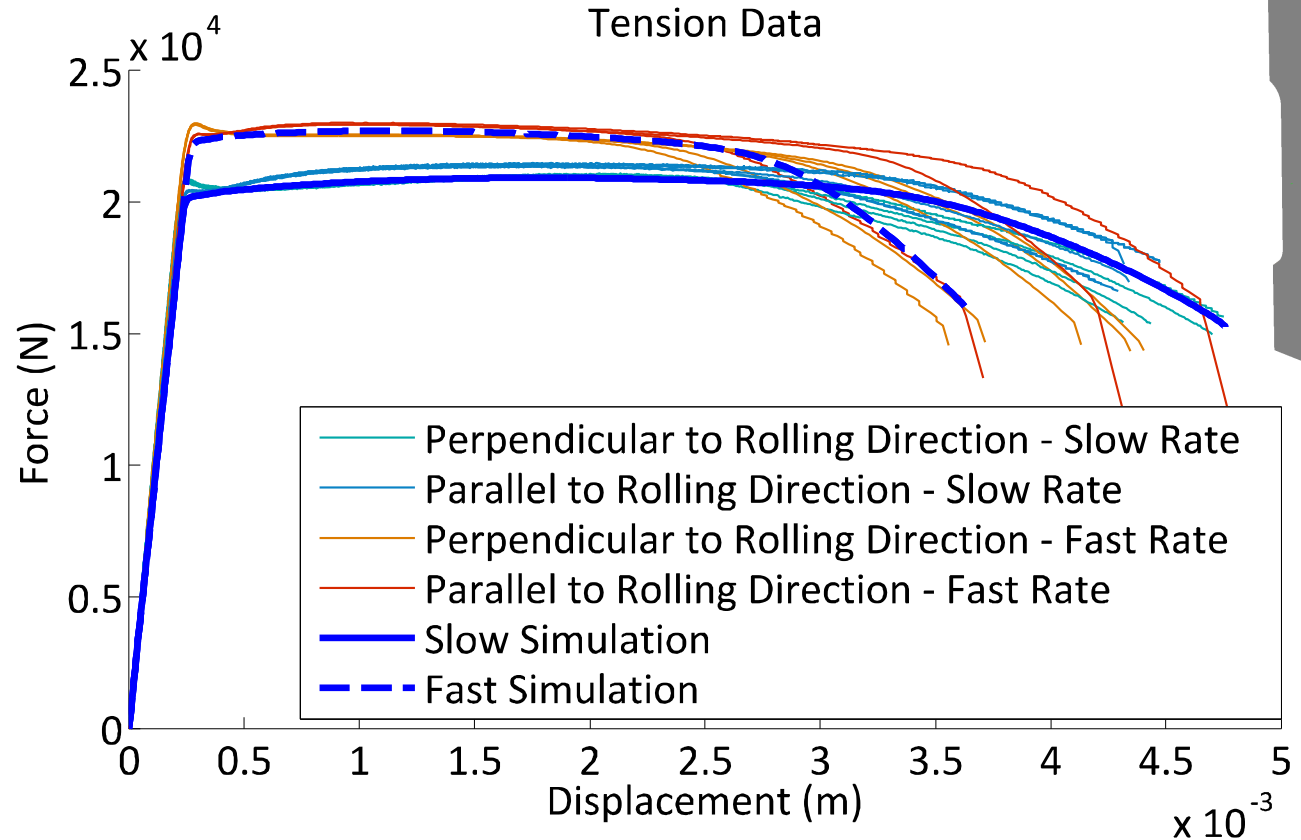
# Initial approach to SFC geometry

- Production codes (Sierra) employed for all calculations
- Simulations employ segregated coupling (Adagio/Aria)
- Implicit solution for long time scales (statics & dynamics)
- Isotropic poro-thermo-viscoplasticity model
- Hexahedral elements (SD, constant pressure)
- Elastic pins are contiguously meshed w/BC on centerline
- Element death was employed when the first integration point reached the coalescence criterion  $\phi_{\text{coal}}$  (0.15)
- Learn with local damage and coarse meshes
- Employ techniques for regularizing the solution
  - Variational nonlocal method
  - Localization elements



# Initial model calibration (tension)

$\beta = 0.8$   
 $H = 3084 \times 10^6 \text{ (Pa)}$   
 $R_d = 13$   
 $f = 1 \times 10^{-6}$   
 $n = 26$   
 $Y_{RT} = 493 \times 10^6 \text{ (Pa)}$   
 $\phi_0 = 1 \times 10^{-4}$   
 $\phi_{coal} = 0.15$   
 $m = 6$   
*Thermo-mechanical  
simulations employed for  
model calibration*



$$\sigma_y = (1 - \phi) \left[ Y(\theta) + \frac{H}{R_d} (1 - e^{-R_d \epsilon_p}) \right] \left\{ 1 + \sinh^{-1} \left[ \left( \frac{\dot{\epsilon}_p}{f} \right)^{1/n} \right] \right\}$$

$$\dot{\phi} = \sqrt{\frac{3}{2}} \dot{\epsilon}_p \frac{1 - (1 - \phi)^{m+1}}{(1 - \phi)^m} \sinh \left[ \frac{2(2m - 1)}{2m + 1} \frac{\langle \frac{I_1}{3} \rangle}{\sqrt{3J_2}} \right]$$

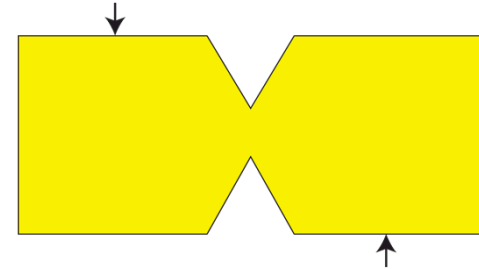
$\dot{q} = \beta \bar{\sigma} : D^p$   
 uncertainty in conversion  
 of plastic work to heat  $\beta$

isotropic damage  $\phi$  taken from Cocks and Ashby (1972)

NOTE: Temperature-dependent thermal conductivity and specific heat also taken from MMPDS-08.

# Incorporating shear data

- Calibrated model did not predict the shear behavior
- Anisotropy evident in yield, hardening and damage evolution
- Focused on orientations relevant (// to RD) to the SFC
- Reduced the initial yield  $Y_{RT}$  and the recovery  $R_d$
- Incorporated void nucleation through  $J_3$  ( $n$  is the evolving void density)



$$\frac{\dot{n}}{n} = N_1 \dot{\epsilon}_p \left( \frac{4}{27} - \frac{J_3^2}{J_2^3} \right)$$

(Horstemeyer, Gokhale, 1999)

$$\frac{\dot{\phi}_n}{\phi} = k_w \dot{\epsilon}_p \left( 1 - \frac{27 J_3^2}{4 J_2^3} \right)$$

(Nahshon, Hutchinson, 2008)

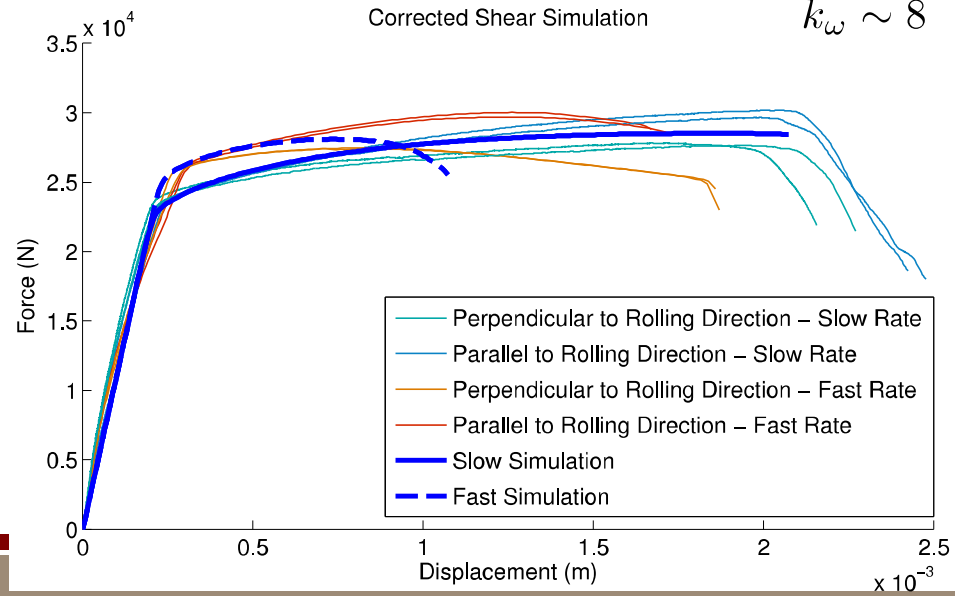
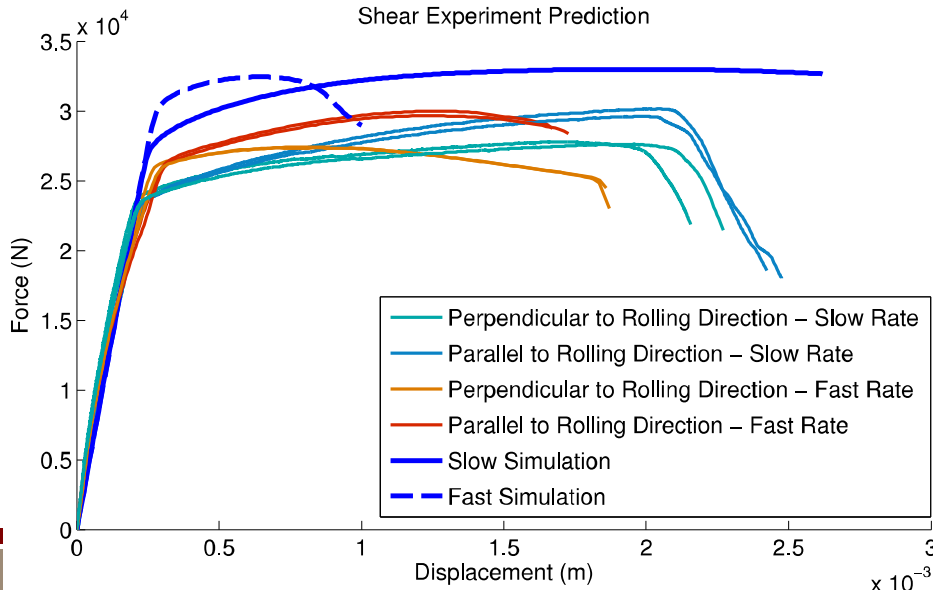
$$N_1 = \frac{27}{4} \left( \frac{1}{1 - \phi} \right) k_w$$

$$Y_{RT}^s = 0.87 Y_{RT}$$

$$R_d^s = 0.92 R_d$$

$$N_1 = 54$$

$$k_w \sim 8$$

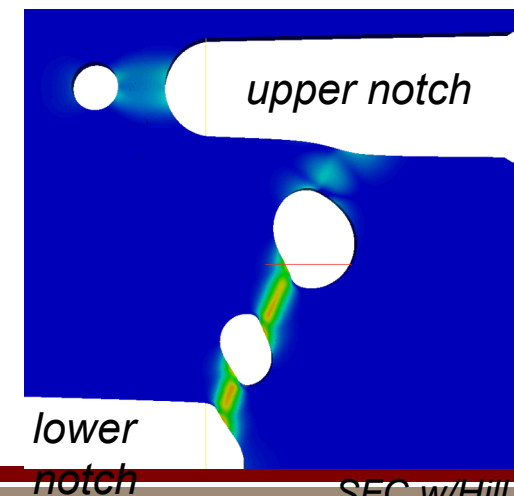
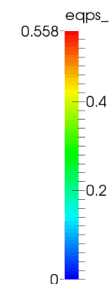
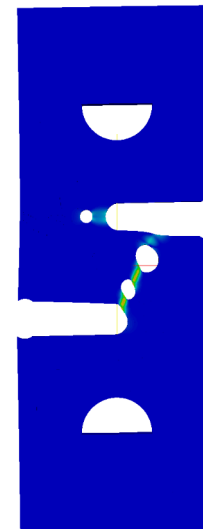
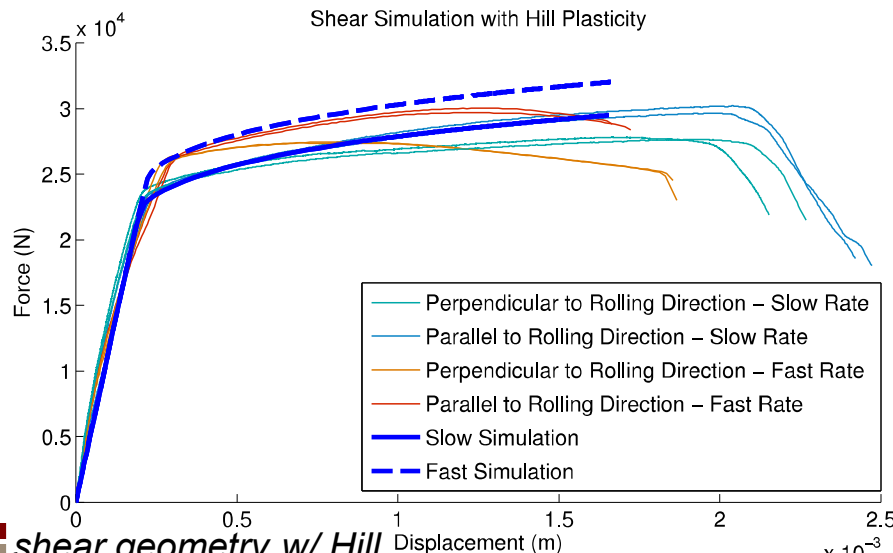
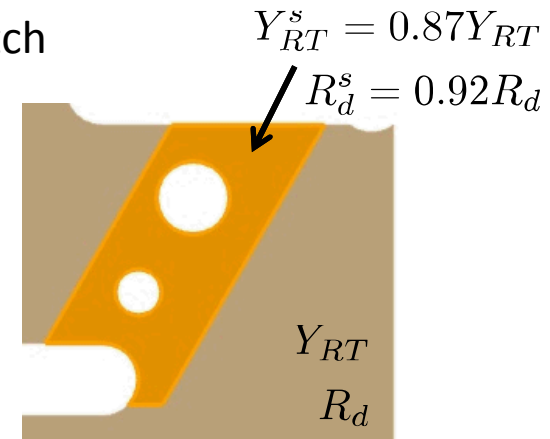


# Revised approach to SFC geometry Sandia National Laboratories

- Calibrated a Hill, anisotropic yield surface to the shear and tensile data
- Although rate and temperature independent, modest agreement at lower rates
- Anisotropic yield predicted SFC would localize in the lower notch

*Idealization.* Keep poro-thermo-viscoplasticity. Accept isotropy. Assign different isotropic material parameters to regions being sheared.

*Goal.* Mimic Hill at lower notch, add physics

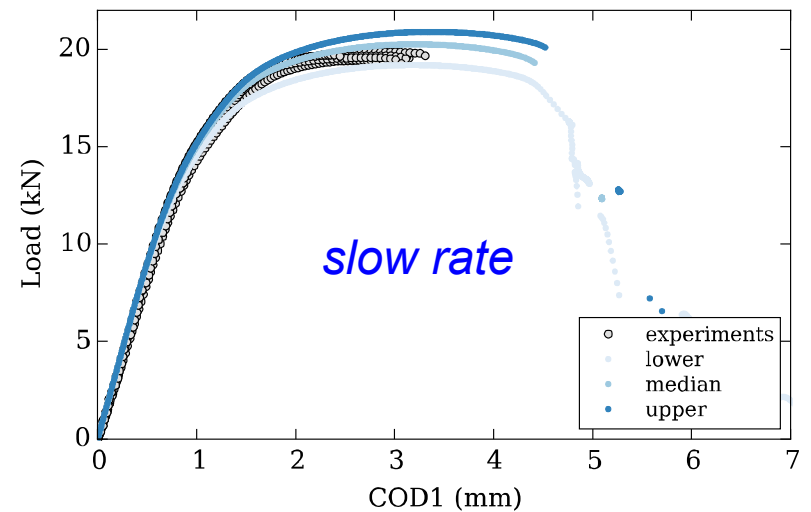
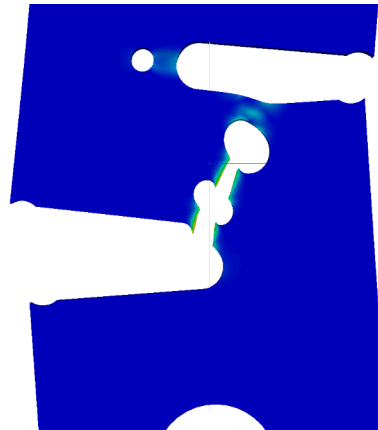
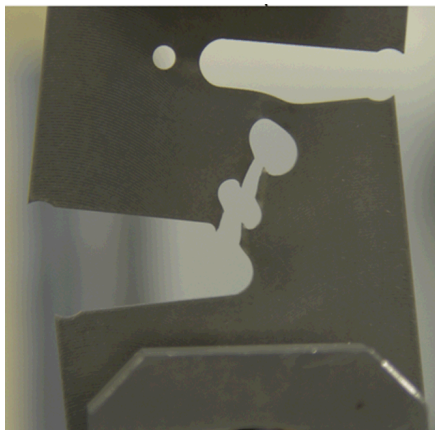
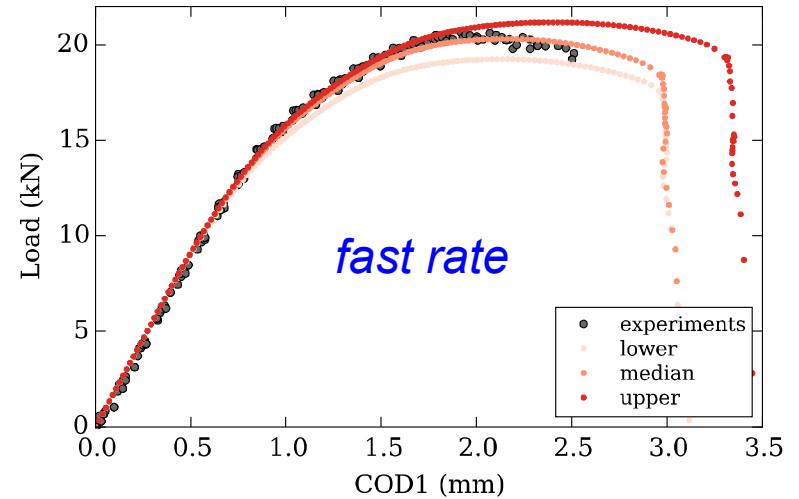
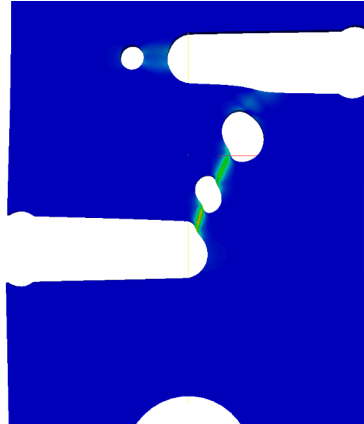
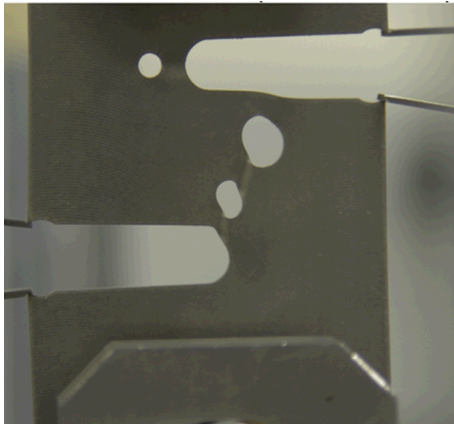


shear geometry w/ Hill

SFC geometry w/Hill

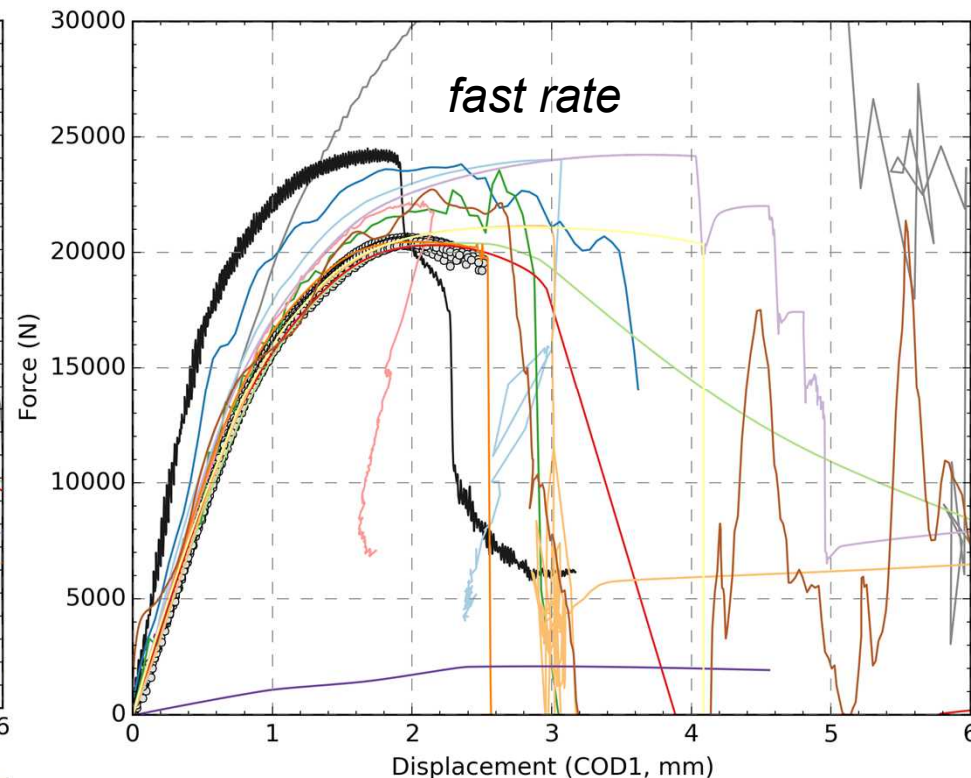
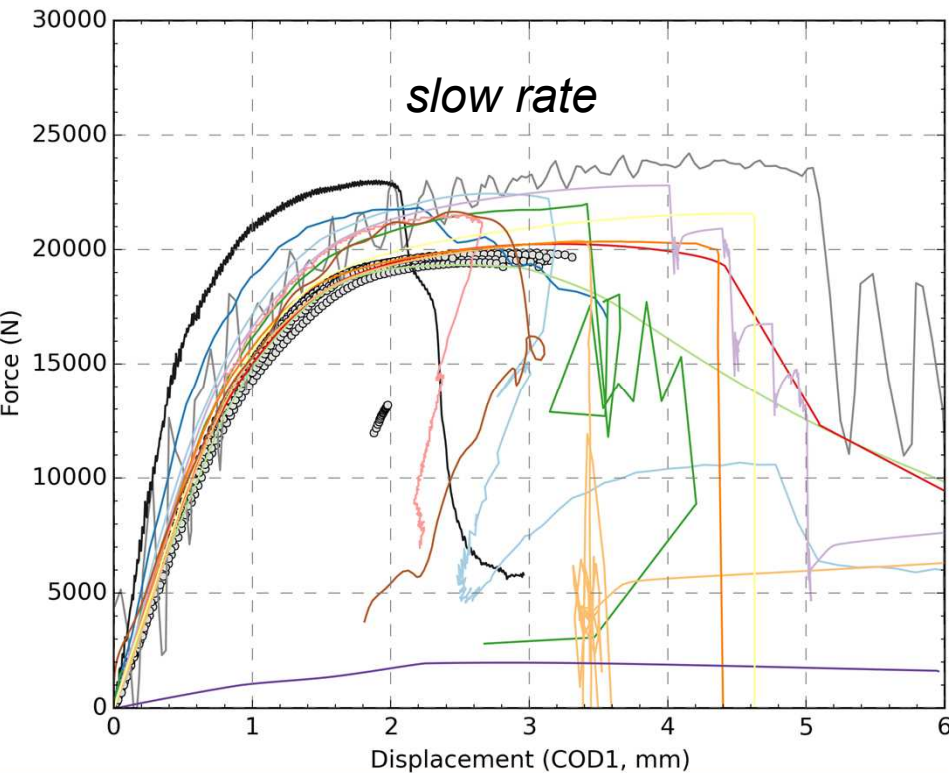
SFC w/Hill

# Blind predictions



# In good company...

- Majority of teams predicted the correct crack path w/error in load-displacement
- Majority of teams over-predicted both the loads and displacements to failure
- We believed that the role of plastic anisotropy would improve our predictions
- Sensitivity studies and new physics were pursued





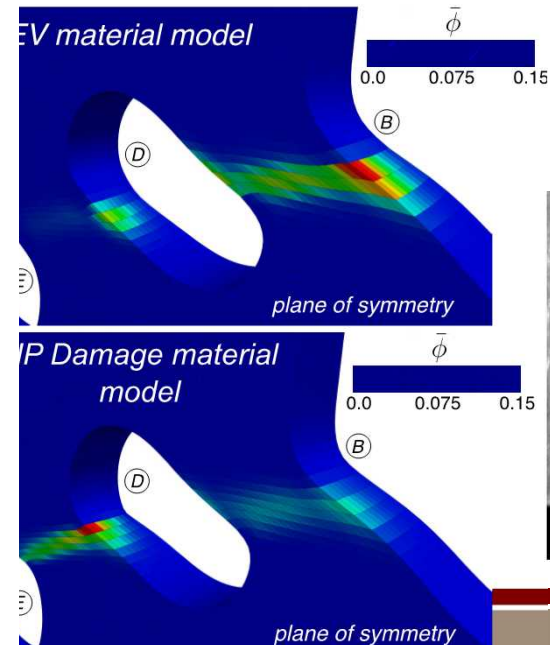
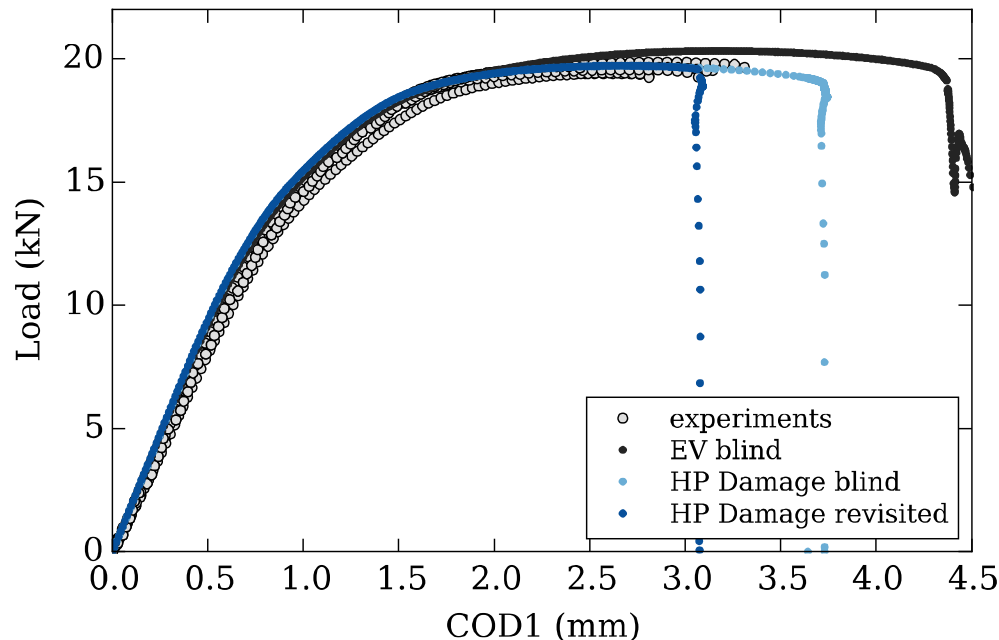
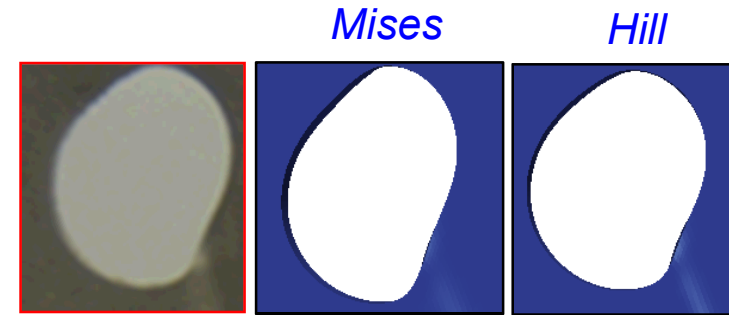
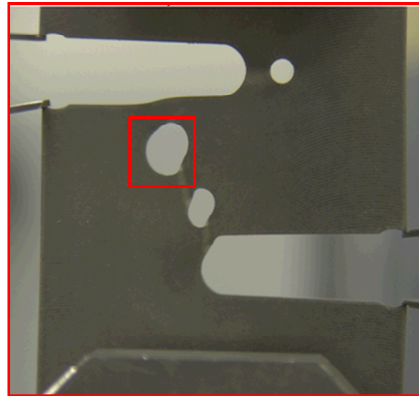
# Revisiting anisotropy

- Keep micromechanics (damage)
- Add Hill yield surface
- Aids understanding

$$\dot{\phi} = \sqrt{\frac{3}{2}} \dot{\epsilon}_p \frac{1 - (1 - \phi)^{m+1}}{(1 - \phi)^m} \sinh \left[ \frac{2(2m - 1)}{2m + 1} \frac{\langle \frac{I_1}{3} \rangle}{\sqrt{3} J_2} \right]$$

$$\dot{\eta} = \eta \dot{\epsilon}_p N_1 \left[ \frac{4}{27} - \frac{J_3^2}{J_2^3} \right]$$

$$f_Y^2(\sigma_{ij}) \equiv F(\sigma_{22} - \sigma_{33})^2 + G(\sigma_{33} - \sigma_{11})^2 + H(\sigma_{11} - \sigma_{22})^2 + 2L\sigma_{23}^2 + 2M\sigma_{31}^2 + 2N\sigma_{12}^2 = \bar{\sigma}_c^2(\epsilon_p)$$



*Ravi-Chandar  
Gross (UT)*

