

Cohesive Model Applied to Fracture Propagation in Indiana Limestone

SAND2014-20477C



Sandia
National
Laboratories

Thomas Dewers¹, Alex Rinehart^{1,2*} and Joseph Bishop³

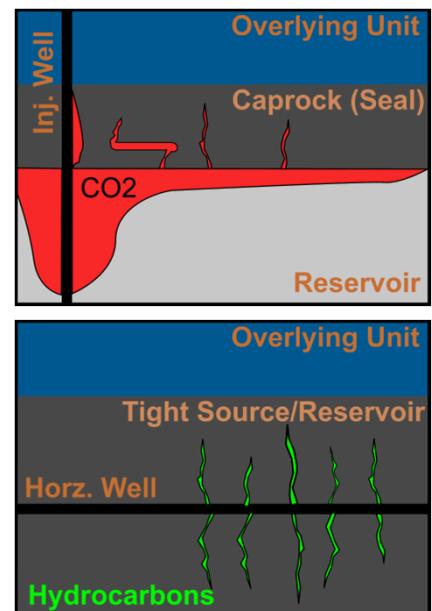
(1) Geomechanics Department, Sandia National Laboratories; (2)* E&ES Department, New Mexico Tech, rinehart@nmt.edu;
(3) Solid Mechanics, Sandia National Laboratories. AGU 2014 Fall Meeting.



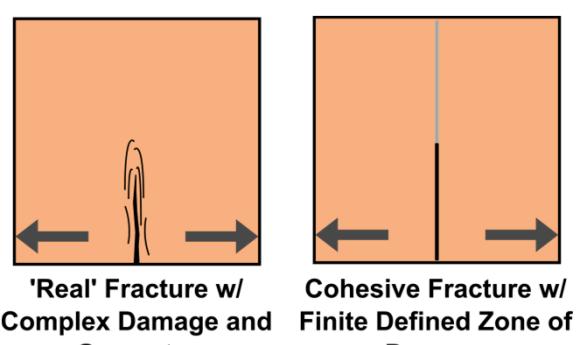
Introduction

Goal 1: Test if linear softening cohesive fracture model (LCFM) can be used to predict fracture propagation in different geometries for geomaterials.

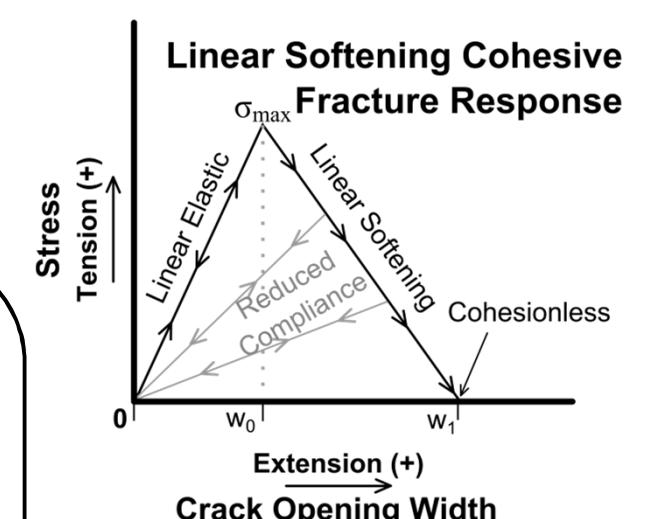
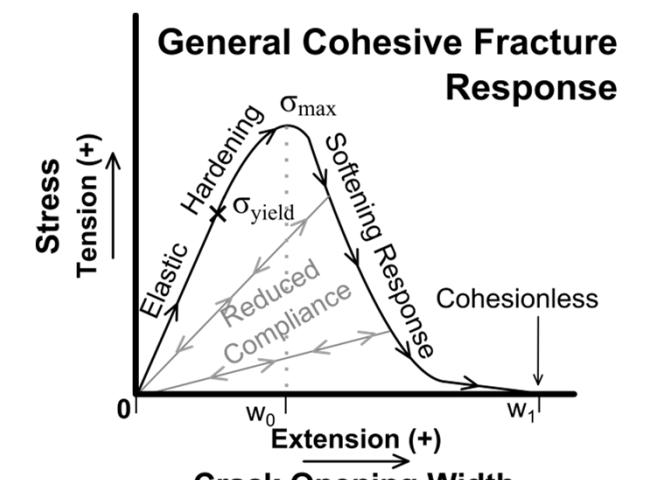
Goal 2: Understand fracture propagation in Indiana Limestone through lens of combined numerical modeling and experiment.



Modeling and process understanding of opening-mode fracture propagation in geomaterials is critical to viability of CO₂ capture and storage, unconventional hydrocarbon extraction, and waste repository design. Fractures provide possible preferential leakage and extraction pathways.



Cohesive fracture models idealize the actual fracture processes into a plane between continuum media (Bazant and Plaňas, 1907).



Workflow

Fix σ_{max} with Brazil tests, Young's mod. (E) with UCS.

Calibrate w_1 and E of LCFM in short-rod geometry.

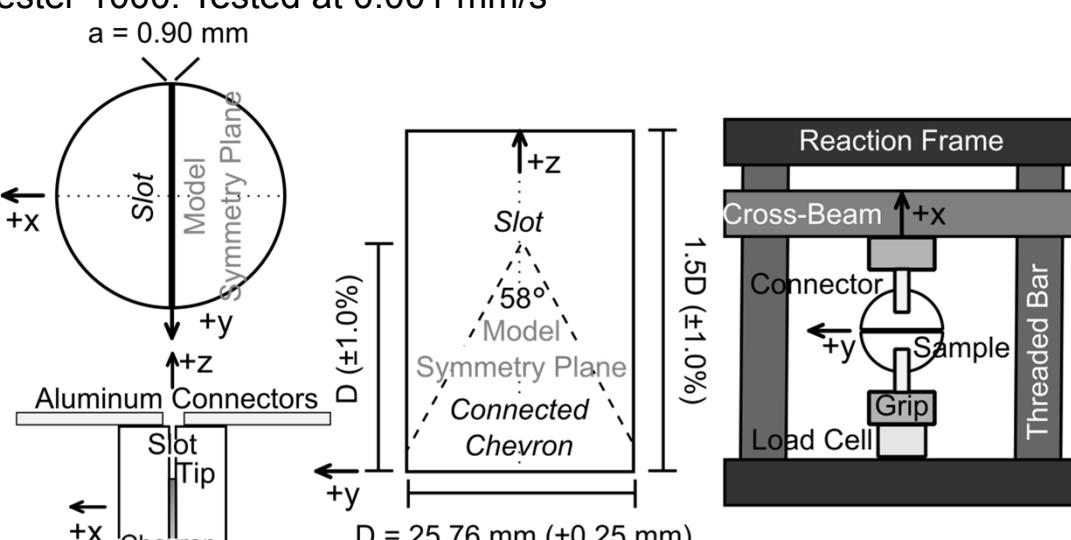
Confirm model in notched 3-point bend (N3PB) geometry.
Propose hypotheses for deviations.

Each 'segment' of fracture responds elastically initially, then may have different formulations of yield and softening. We use the LCFM that has been used to simulate generic Mode I and hydraulic fractures (Yao, 2012)

Methods

Experimental Set-up

Short-rod samples prepared from Indiana Limestone in accord with Ouchterlony (1989, 1990), with force and displacement measured in a MTI SEMTester 1000. Tested at 0.001 mm/s



N3PB tests in accord with ASTM C1421-10 (2011), were tested in a MTS 220-kip frame using frame displacement, and Honeywell 1112.5 N load cell. Tested at 0.001 mm/s.

Brazilian and UCS tests were performed in the MTS 220 kip frame in conformance with ASTM D3967-08 (2008) and ASTM D7012-3 (2013). UCS-test strains were measured with 0.125 universal-T strain gages.

Numerical Model

Short-Rod Mesh: Cohesive zone 0.2 of notch width. Meshing performed in Cubit. Used 8-node hexahedron elements. Yellow elements were linear softening cohesive elements. Gray were linear elastic elements.

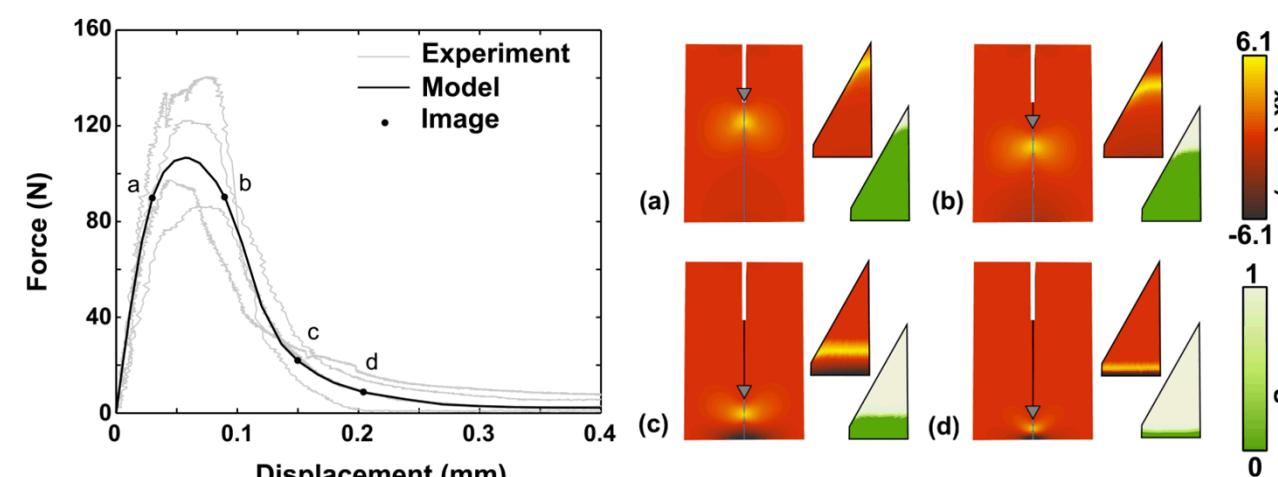
LCFM-finite element simulations performed in Abaqus Standard 13.1 (ref). Displacement boundary conditions enforced on top surface (short-rod) and outer edges (N3PB). Resultant forces found on same nodesets. Symmetry plane ($y=0$) used.

Initial Params. (Brazil, UCS Tests)

$$\sigma_{max} = 5.9 \text{ MPa} \text{ (fixed in simulations). } \pm 15\%.$$
$$E = 39.5 \text{ GPa} \text{ (subs. calibrated). } \pm 10\%$$

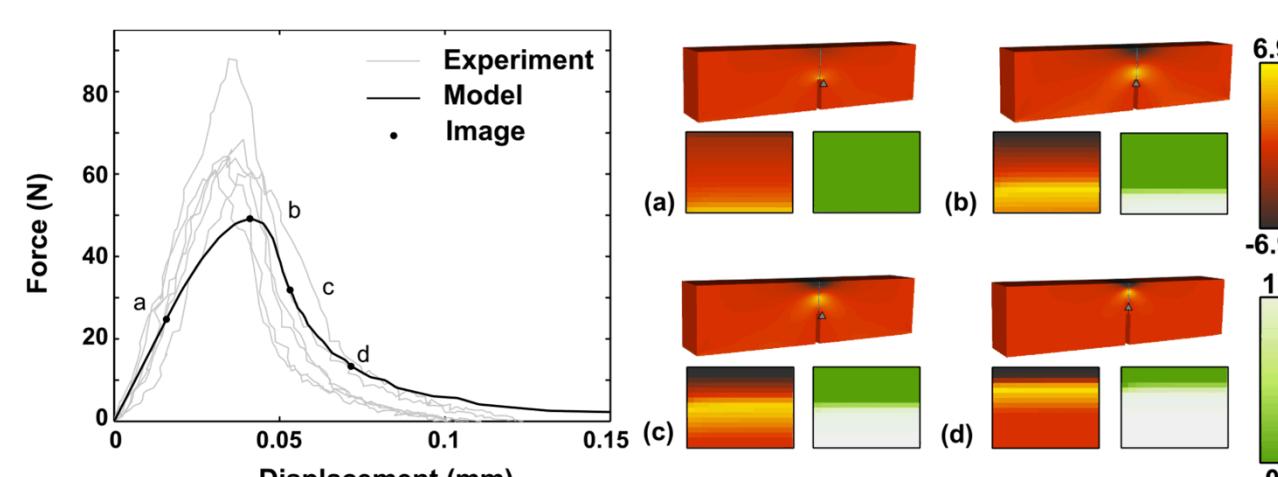
Results

Short-rod Calibrations



- Calibrated successfully.
 - $E_{cal} = 7 \text{ GPa} <> 40 \text{ GPa}$
 - $w_1 = 0.0115 \text{ mm}$
- Observed piece-wise failure similar to qualitative simulated failure pattern.
- Pre-peak yield similar in test and simulation.
- Matched both initial and late softening behavior.

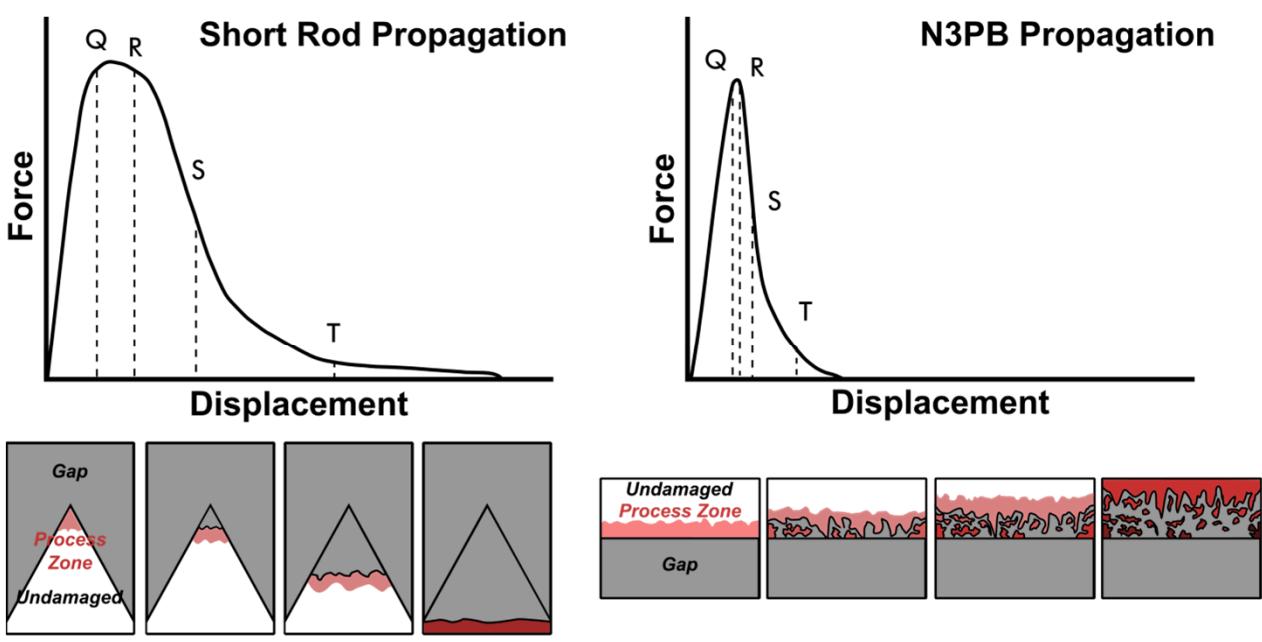
N3PB Confirmation



- Simulation overshoots observed late softening behavior.
- Simulated peak force 80% of minimum observed force.
- Simulated yield proportional to peak force begins much earlier than observed.
- Observed fracture is diffuse through >99% of softening, with coherent unstable fracture forming just before complete failure.
- Simulated elastic response more compliant (lower slope) than observed.
- Simulation has discrete fracture progression sequentially.

Conclusions

Conceptual Model



- LCFM can be reasonably used in different geometry and loading configurations.
- Volumetric proportion of stress in tensile, shear and compressive states control continuum elastic response.
- LCFM assumes uniform fracture process through all of propagation.
- N3PB has 2-stage failure, with stiff and strong shear elements stiffening sample and shielding weak tensile portions.

Acknowledgements

This material is based upon work supported as part of the Center for Frontiers of Subsurface Energy Security, an Energy Frontier Research Center funded by the U.S. Department of Energy, Office of Science, Office of Basic Energy Sciences under Award Number DE-SC0001114. Sandia National Laboratories is a multi-program laboratory managed and operated by Sandia Corporation, a wholly owned subsidiary of Lockheed Martin Corporation, for the U.S. Department of Energy's National Nuclear Security Administration under contract DE-AC04-94AL85000.

Bibliography

Abaqus Software 6.13 (2013a), Abaqus Analysis User's Guide, Dassault Systèmes Simulia Corp., Providence, RI, USA.
ASTM C1421-10 (2011), Standard test methods for determination of fracture toughness of advanced ceramics at ambient temperature, ASTM, West Conshohocken, PA.
ASTM D3967-08 (2008), Standard test method for splitting tensile strength of intact rock core specimens, West Conshohocken, PA.
ASTM D7012-3 (2013), Standard test methods for compressive strength and elastic moduli of intact rock core specimens under varying states of stress and temperatures, ASTM, West Conshohocken, PA.
Bazant, Z.P., and Plaňas, J. (1997), Fracture and Size Effect in Concrete and Other Quasibrittle Materials, CRC Press, Boca Raton, FL.
Hillerborg, A., Modéer, M., and Petersson, P.E. (1976), Analysis of crack formation and crack growth in concrete by means of fracture mechanics and finite elements, Cement Concrete Res. 6, 773-782.
Ouchterlony, F. (1989), On the background to the formulae and accuracy of rock fracture toughness measurements using ISRM Standard core specimens, Int. J. Rock Mech., Min. Sci., Geomech. Abstr., 26 (1), 13-23.
Ouchterlony, F. (1990), Fracture toughness testing of rock with core based specimens, Engineering Fracture Mechanics, 35(1-3), 351-366.
Yao, Y. (2012), Linear elastic and cohesive fracture analysis to model hydraulic fracture in brittle rock and ductile rocks, Rock Mech. Rock Eng. 45, 375-387.