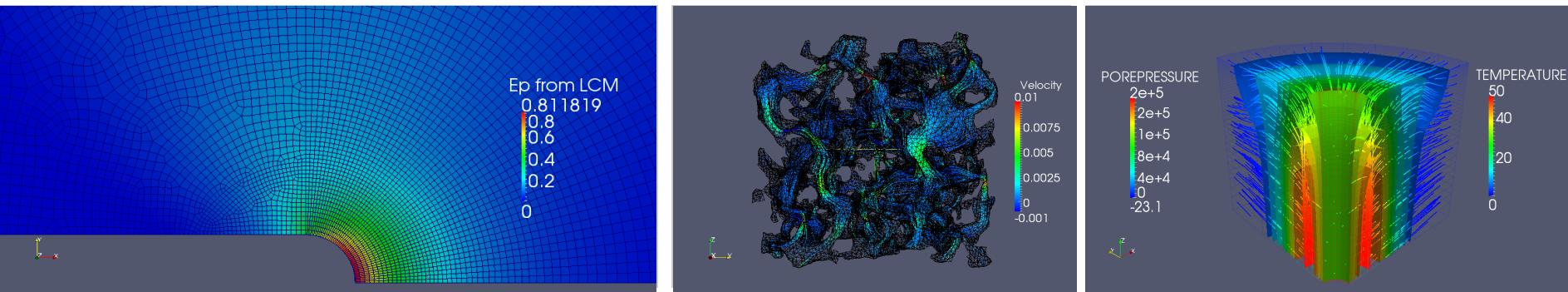


Exceptional service in the national interest



Computational Geomechanics across the pore, grain, specimen and field scales



Steve Sun¹

¹Senior Member of Technical Staff, Mechanics of Materials Department
Sandia National Laboratories, Livermore, California

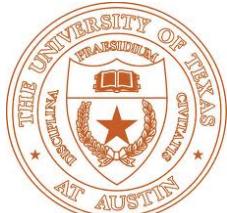


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Collaborators (current and past)



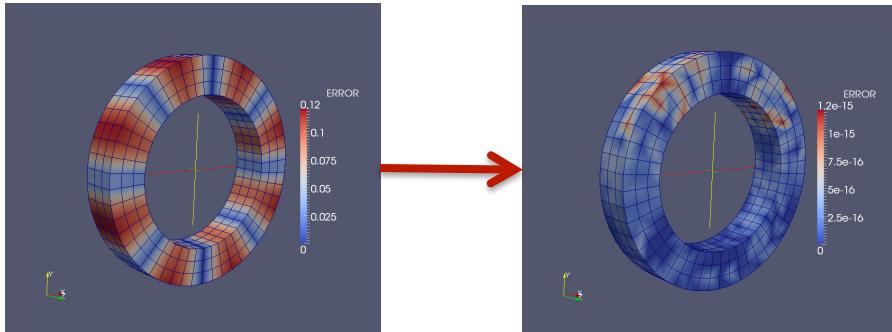
**Sandia
National
Laboratories**



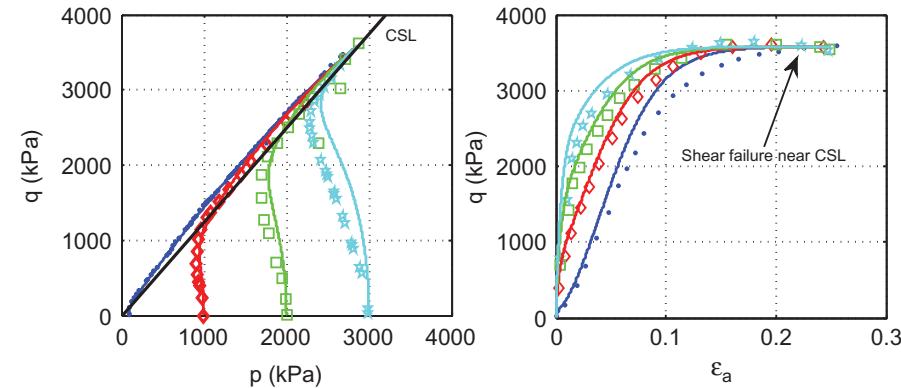
**STONY
BROOK
UNIVERSITY**

- ❑ James W Foulk III (Sandia, CA), localization element for hydrogen embrittlement
- ❑ Alejandro Mota (Sandia, CA), multiscale modeling, internal variable mapping
- ❑ Jakob T Ostien (Sandia, CA), finite strain FEM for thermo-hydro-mechanics
- ❑ Joseph E Bishop (Sandia, NM), mortar finite element method
- ❑ John W Rudnicki (Northwestern, IL), Mechanical and hydraulic properties of real and simulated compaction bands
- ❑ Jose E Andrade (Northwestern, IL; now at Caltech, CA), lattice Boltzmann/finite element hybrid simulations
- ❑ Matthew R Kuhn (University of Portland, OR), Shear band modeling with Discrete element method; granular physics
- ❑ Ronaldo I Borja (Stanford, CA), Monte-Carlo simulations for co-seismic deformations
- ❑ Teng-fong Wong (Stony-Brook University, NY; now at Chinese University of Hong Kong), Modeling and analysis with CT Images
- ❑ Peter Eichhubl (UT Austin), compaction band characterizations
- ❑ Yin Lu Young (Princeton University, NJ; now at University of Michigan, Ann Arbor, MI), Monte-Carlo simulation of sedimentation-consolidation process

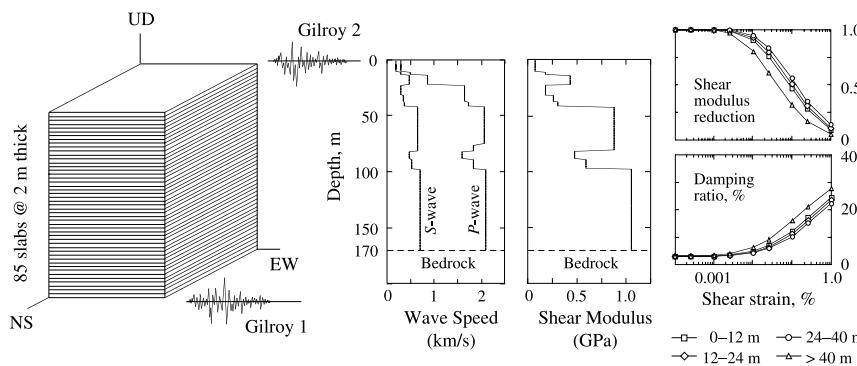
Research Experience



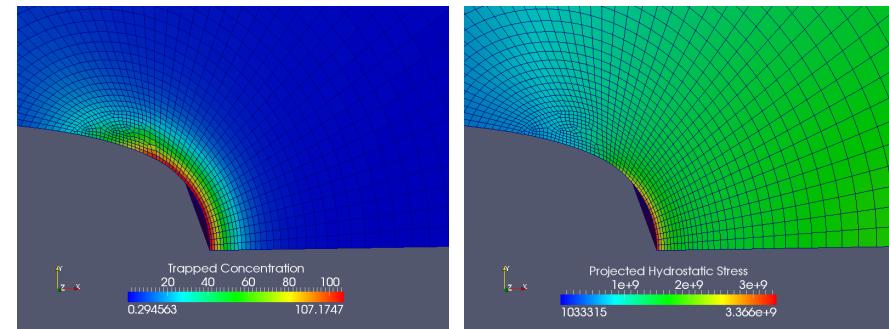
- Variational internal variable recovery with Lie algebra for adaptive finite element method
- Mota, Sun, Ostien, Foulk, Long, Computational Mechanics 2013



- Predicting liquefaction and shear localization with a critical state plasticity model
- Sun, Geomechanics and Geoengineering, 2013

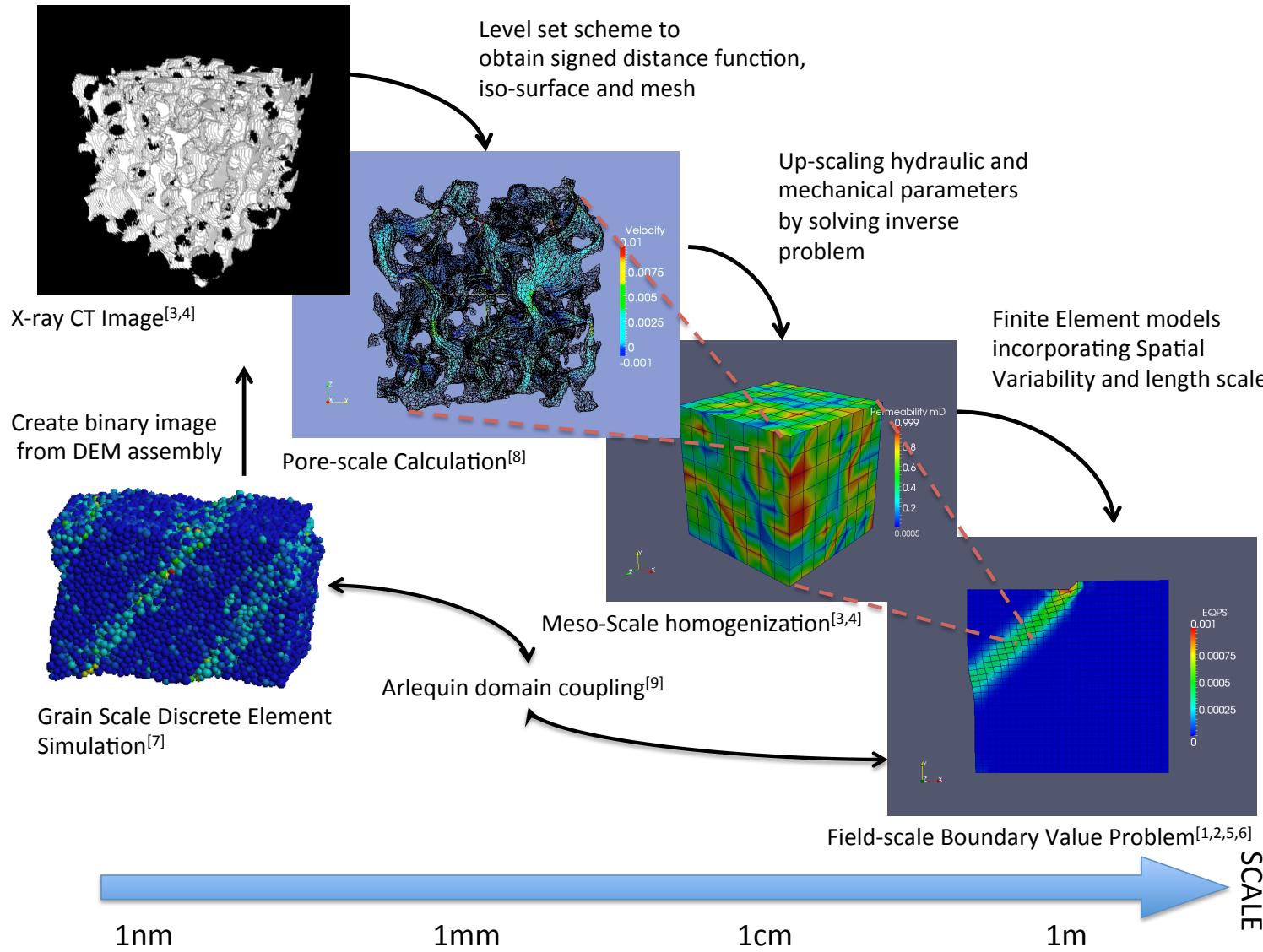


- Estimating co-seismic deformation of sediment during Loma Prieta earthquake with Monte Carlos simulations
- Borja, Sun, Acta Geotechnica, 2007; Borja, Sun, Journal of Geophysical Research 2008



- Hydrogen embrittlement in stainless steel
- Foulk, Sun, Wagner, International Journal of Numerical Methods in Engineering, 2013

Computational Geomechanics Across the Pore, Grain, Specimen and Field scales



Structure of the Presentation

- Motivations
- How to model large deformation thermo-hydro-mechanics at the field scale?
- How to use micromechanics models to predict the macroscopic mechanical and hydraulic responses?
- How to connect micro- and field- scale models with domain coupling methods?
- Conclusion and future Perspectives

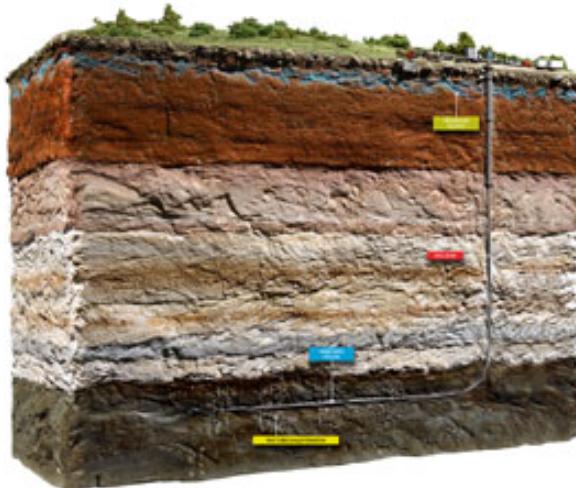
Motivations



Geothermal energy



Rainfall induced slope instability

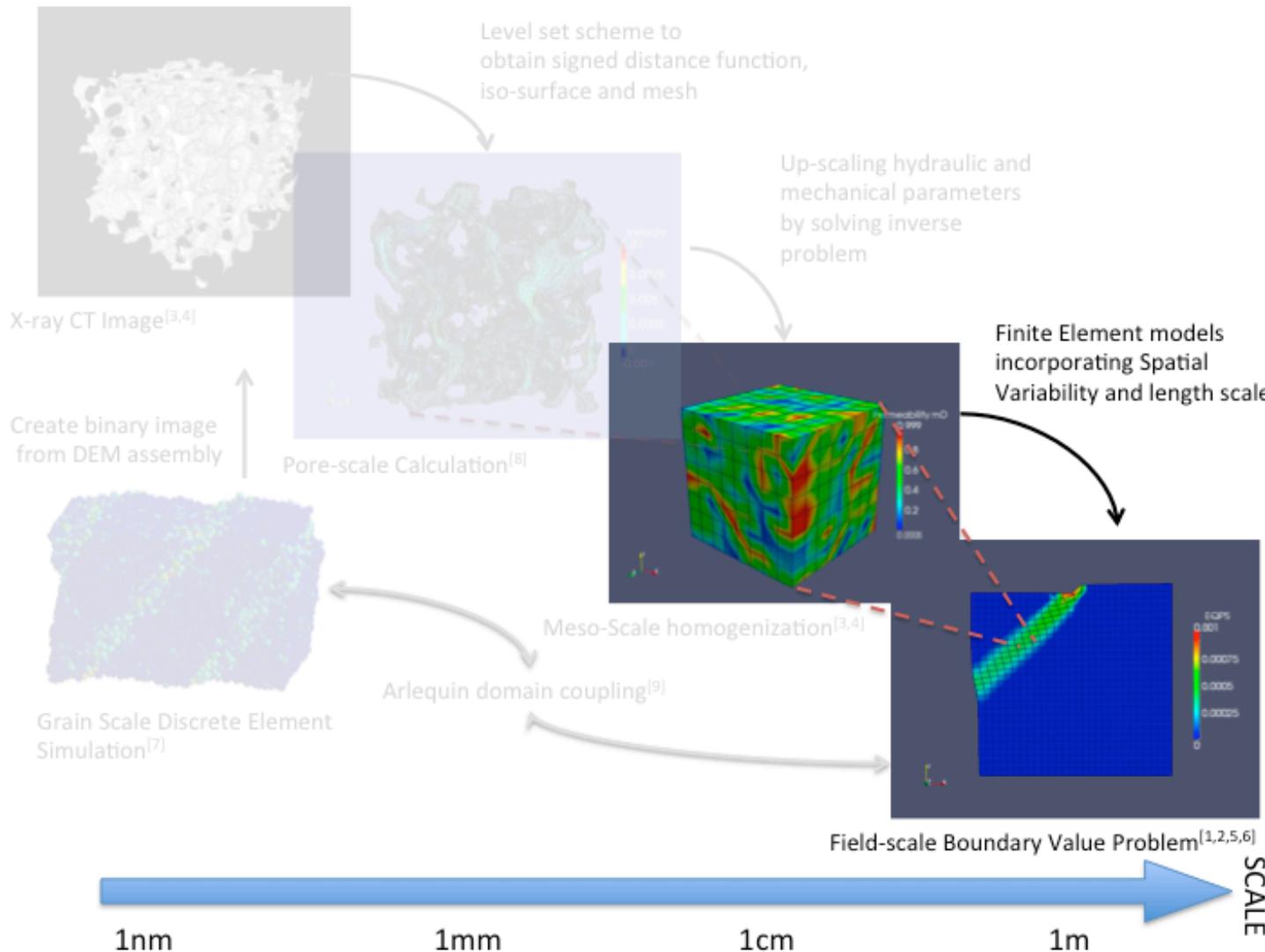


Hydraulic fracture (fracking)

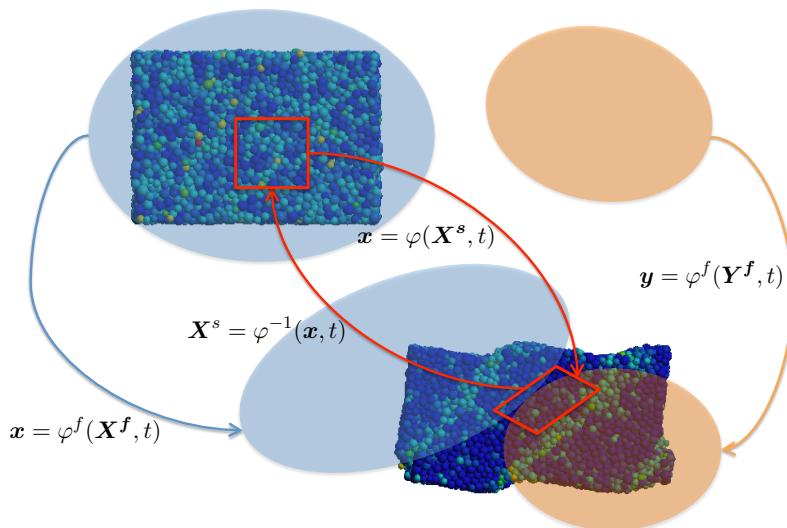


Soil liquefaction

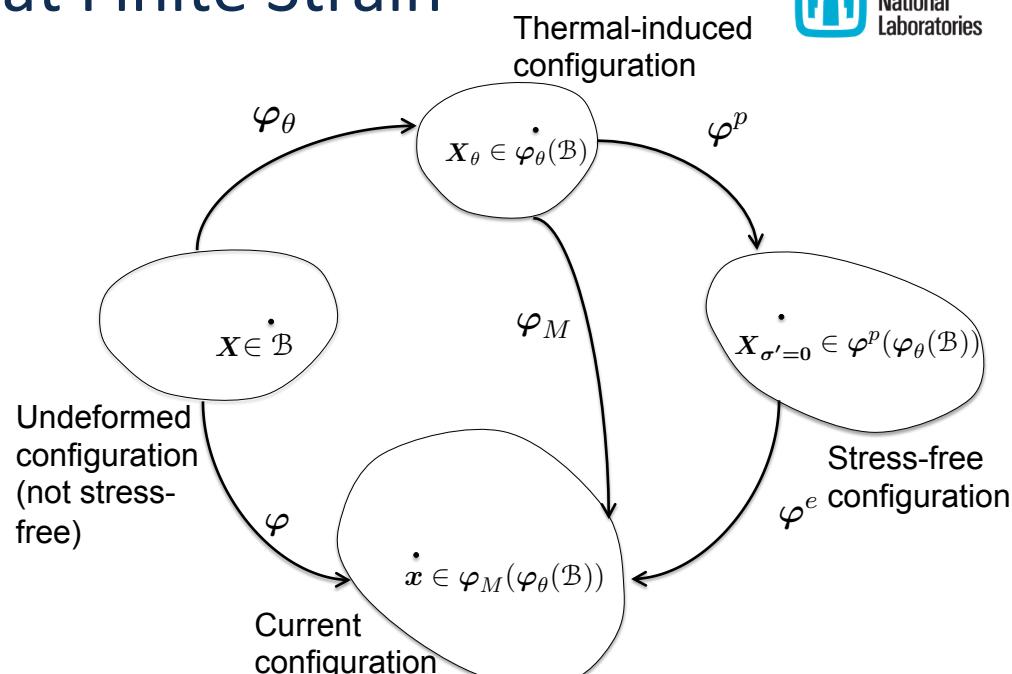
Thermo-hydro-mechanics modeling in the continuum scale



Kinematics of THM Problem at Finite Strain



Trajectories of the solid and fluid constituent.



Multiplicative decomposition of the thermo-hydro-mechanics problem

Multiplicative decomposition of skeleton deformation gradient

$$\mathbf{F} = \frac{\partial \varphi(\mathbf{X}, t)}{\partial \mathbf{X}} = \mathbf{F}_M \cdot \mathbf{F}_\theta ; \quad \mathbf{F}_\theta = \frac{\partial \varphi_\theta(\mathbf{X}, t)}{\partial \mathbf{X}} ; \quad \mathbf{F}_M = \frac{\partial \varphi_M(\mathbf{X}_\theta, t)}{\partial \mathbf{X}_\theta}$$

↑
Isotropic tensor

$$\boldsymbol{\sigma} = \boldsymbol{\sigma}' - B p^f \mathbf{I},$$

$$B = 1 - \frac{K}{K_s}$$

$$\mathbf{P}(\mathbf{F}, \mathbf{z}, p^f, \theta) = \mathbf{P}'(\mathbf{F}, \mathbf{z}, \theta) - J B p^f \mathbf{F}^{-T}$$

$$\mathbf{P}(\mathbf{F}_M, \mathbf{z}, p^f) = \mathbf{P}'(\mathbf{F}_M, \mathbf{z}) - J B p^f \mathbf{F}^{-T},$$

Strong Form of THM Problem at Finite Strain

□ Balance of Linear Momentum

$$\nabla^{\mathbf{X}} \cdot \mathbf{P} + J(\rho^s + \rho^f) \mathbf{G} = \mathbf{0}$$

where $\mathbf{P}(\mathbf{F}_M, z, p^f) = \mathbf{P}'(\mathbf{F}_M, z) - JBp^f \mathbf{F}^{-T}$,

↑
Total Stress ↑
Effective Stress ↑
1st PK pore pressure

□ Balance of Mass

$$\left(\frac{B}{J} - 3\alpha_s(\theta - \theta_o) \right) \dot{J} + \frac{1}{M} \dot{p}^f - 3\alpha^m \dot{\theta} + \frac{1}{\rho_f} \nabla^{\mathbf{X}} \cdot \mathbf{W} = 0.$$

where

$$\mathbf{W} = J \mathbf{F}^{-1} \cdot \mathbf{w}. \quad \text{and} \quad \mathbf{w} = \rho_f \mathbf{k} \cdot \left[-\nabla^{\mathbf{X}} p^f + \rho_f (\mathbf{G} - \mathbf{a}^f) \right] - \rho_f s_T \nabla^{\mathbf{X}} \theta,$$

↑
Piola's Transform ↑
Darcian Flow ↑
Soret Effect
(neglected here)

□ Balance of Energy

$$c_F \dot{\theta} = [D_{\text{mech}} - H_\theta] + \left[-\nabla^{\mathbf{X}} \cdot \mathbf{Q}_\theta - \frac{\Phi^f c_{F^f}}{\rho_f} \mathbf{W} \cdot \mathbf{F}^{-T} \nabla^{\mathbf{X}} \theta + R_\theta \right], \quad \text{where}$$

Dissipation Heat Flux Convection Heat source

$$H_\theta = H_\theta^s + H_\theta^f, \quad \text{and} \quad H_\theta^s = -\theta \frac{\partial}{\partial \theta} \mathbf{P}' : \dot{\mathbf{F}} = -3K\alpha_{sk}\theta \frac{\dot{J}}{J}$$

↑
Total
Structural
Heating Solid Structural Heating (depending
on which constitutive law being used)

$$H_\theta^f = -\theta \frac{\partial}{\partial \theta} 3\alpha^m(\theta - \theta_o) \dot{p}^f = -3\alpha^m \theta \dot{p}^f. \quad \text{Fluid contribution}$$

Remarks on Estimating Effective Thermal Conductivity from Microstructures

- Volume averaging effective thermal conductivity

$$\mathbf{k}_\theta = \phi^f \mathbf{k}_\theta^f + (1 - \phi^f) \mathbf{k}_\theta^s$$

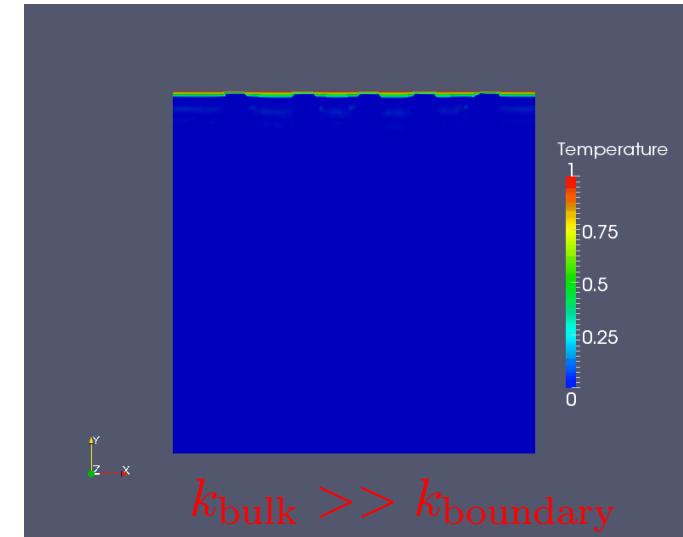
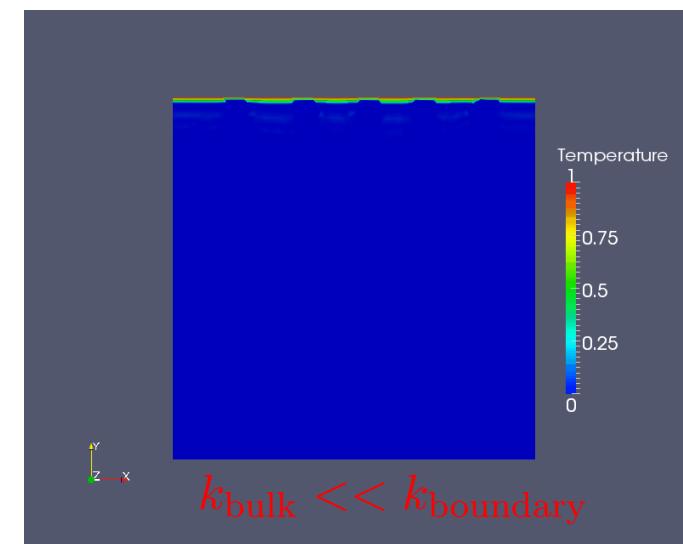
(cf. Preisig & Prevost, IJGGC 2011)

- Homogenized effective conductivity via Eshelby equivalent inclusion method (for spherical inclusions)

$$\mathbf{k}_\theta = \left(k_\theta^f + \frac{\phi^f (k_\theta^s - k_\theta^f) k_\theta^f}{(k_\theta^s - k_\theta^f) \phi^f + k_\theta^f} \right) \mathbf{I}$$

(cf. Zhou & Meschke, IJNAMG 2013)

Important Note: In general, the temperature of the pore-fluid and solid skeleton are **not the same** in the REV, until after sufficient diffusion takes place. This difference is neglected in current formulation.



Stabilization of Two-fold Saddle Point Problem for Equal-order THM Finite Element Model

❑ Combined Inf-sup Condition (not satisfied)

$$\sup_{\mathbf{w}^h \in V_u^h} \frac{\int_{\mathcal{B}} \left(p^{fh} B + 3\theta K \alpha_{sk} \right) \nabla^{\mathbf{x}} \cdot \mathbf{w}^h \, dV}{\|\mathbf{w}^h\|_{V_u^h}} \geq C_o \left(\|p^{fh}\|_{V_p^h} + \|\theta^h\|_{V_{\theta}^h} \right)$$

❑ Combined Weak Inf-sup Condition (still satisfied)

$$\sup_{\mathbf{v}^h \in V_u^h, \mathbf{v} \neq 0} \frac{\int_{\mathcal{B}} \left(p^{fh} B + 3K \alpha_{sk} \theta^h \right) \nabla^{\mathbf{x}} \cdot \mathbf{v}^h \, dV}{\|\mathbf{v}^h\|_1} \geq C_1 \left(\|p^{fh}\|_{V_{p^f}^h} + \|\theta^h\|_{V_{\theta}^h} \right)$$

$- C_2 h \left(\|\nabla^{\mathbf{x}} p^{fh}\|_{V_{p^f}^h} + \|\nabla^{\mathbf{x}} \theta^h\|_{V_{\theta}^h} \right)$

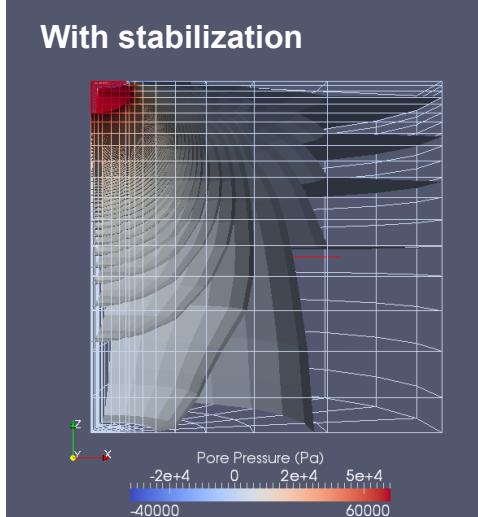
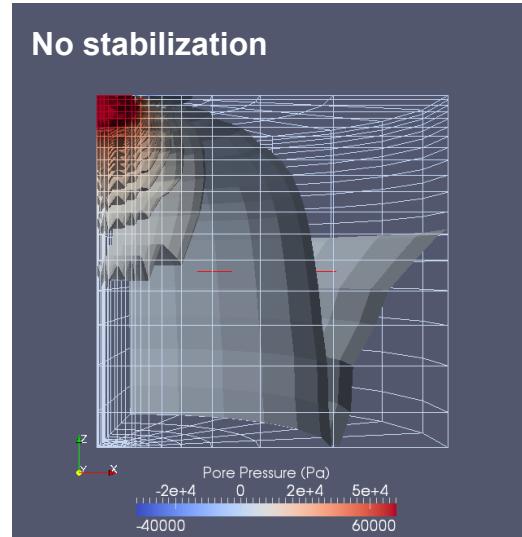
❑ Projection-based Stabilization

$$\begin{aligned} \hat{H}^{\text{stab}}(\psi, p_{n+1}^{fh}, \theta_{n+1}^h) = & \sum_{K \in \mathcal{B}} \int_K C(\psi - \Pi \psi) B(p_{n+1}^{fh} - p_n^{fh} - \Pi(p_{n+1}^{fh} - p_n^{fh})) \, dV \\ & + \sum_{K \in \mathcal{B}} \int_K C(\psi - \Pi \psi) (3\alpha^m) (\theta_{n+1}^h - \theta_n^h - \Pi(\theta_{n+1}^h - \theta_n^h)) \, dV . \end{aligned}$$

\uparrow
 Stabilization parameter

\uparrow
 Interpolated temperature changes

\uparrow
 Projected element-wise constant temperature changes



For isothermal small strain poromechanics, see White and Borja, 2009

Combined F-bar Formulation

Isochoric-volumetric split (Hughes 1975, Simo 1975)

$$F = F_{\text{vol}} \cdot F_{\text{iso}}$$

☐ Replacing volumetric split with assumed term

$$\overline{F} = \bar{J}^{1/3} F_{\text{iso}} = \bar{J}^{1/3} J^{-1/3} F$$

↑ ↑
 Modified $\det(F)$ Original $\det(F)$

Relaxing too much, we get instabilities

Relaxing too little, we get the volumetric locking

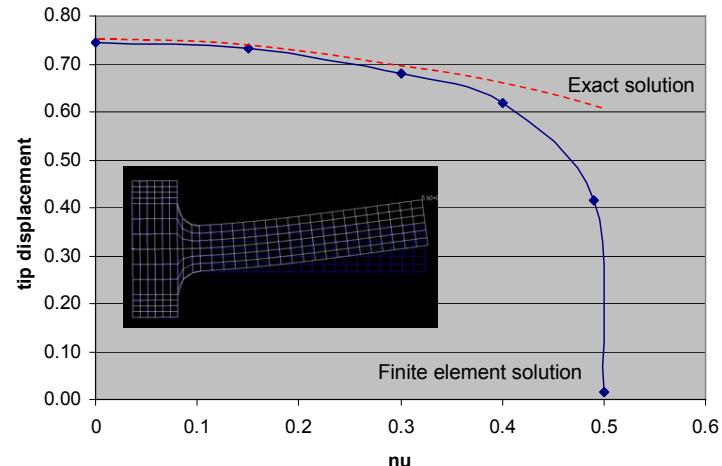
□ Combined F-bar approach

$\tilde{\mathbf{F}} = \alpha \mathbf{F} + (1 - \alpha) \overline{\mathbf{F}}$. ← Invalid operation

□ Current Approach via Lie algebra

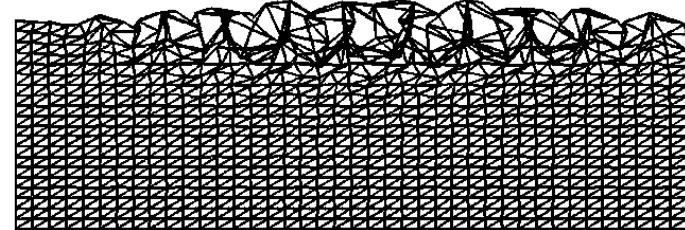
$$\widetilde{J} = \exp \left(\frac{1-\beta}{V_{\mathcal{B}^e}} \int_{\mathcal{B}^e} \log J \, dV + \beta \log J \right).$$

$$\tilde{J}_M = \exp \left(\log \tilde{J} - 3 \left(\frac{1-\beta}{V_{\mathcal{B}^e}} \int_{\mathcal{B}^e} \alpha_{sk}(\theta - \theta_o) \, dV + \beta \alpha_{sk}(\theta - \theta_o) \right) \right)$$



Standard F leads to Volumetric Locking

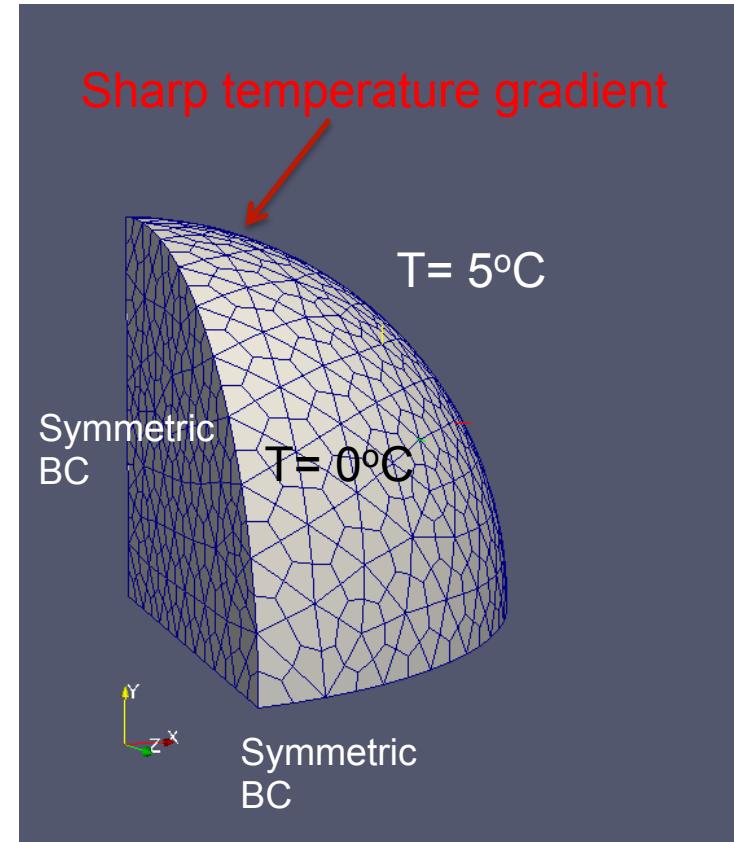
$$\lambda_{\text{cr}} = 90.3557, \alpha = 0 \times E$$



Pure F-bar leads to instability (Brocardo, Micheloni, Krysl, IJNME, 2009)

Thermo-hydro-mechanical Responses of Porous Sphere in Thermal Reservoir

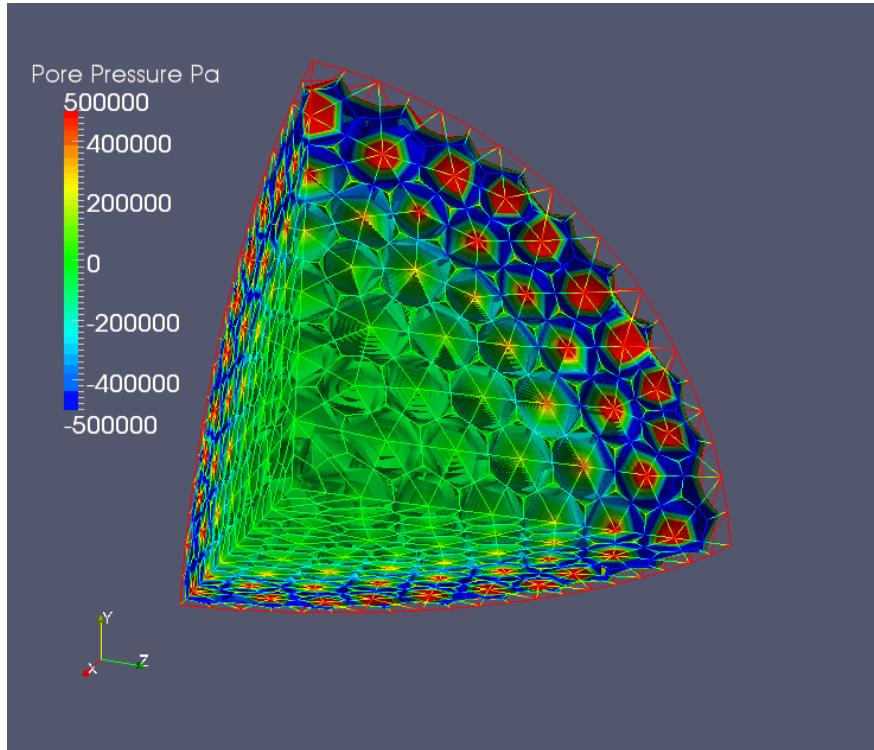
- Material
 - Neo-Hookean
 - Kozeny-Carman Permeability
 - Homogenized Thermal Conductivity
 - Very low Pore-fluid diffusivity
 - High thermal diffusivity
- Boundary Condition
 - Temperature at outer surface = 5°C
 - Initial Temperature = 0°C
 - Globally Undrained
- Analytical Solution is available for linear THM problem (see Belotserkovets & Prevost 2011, Selvadurai and Suvorov 2013)



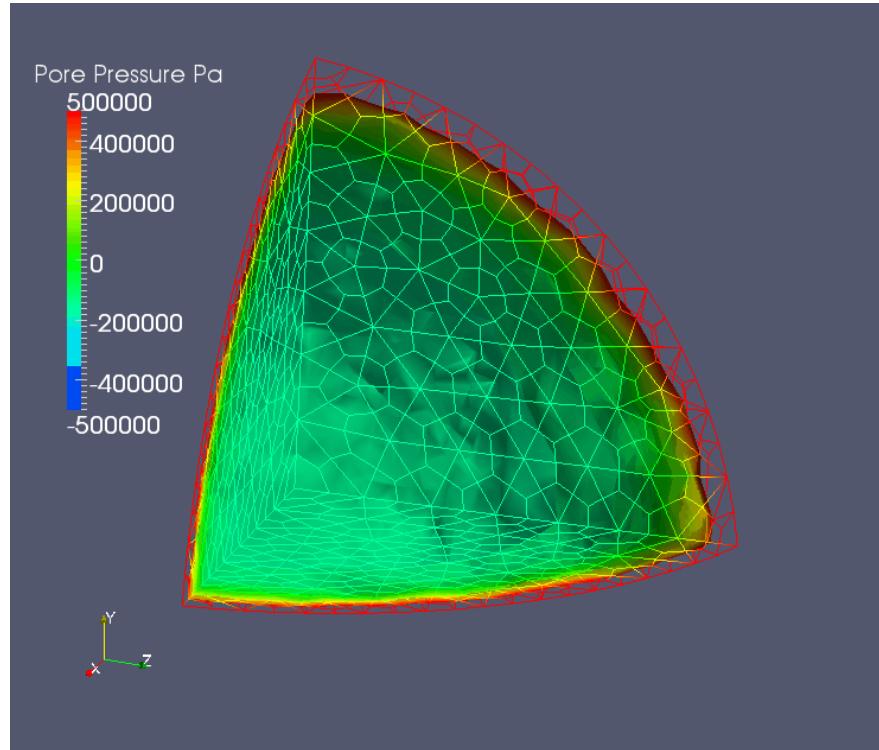
Thermo-hydro-mechanical Responses of Porous Sphere in Thermal Reservoir

X Under-diffusion with spurious patterns

✓ Diffusion with estimated optimal stabilization



Pore Pressure computed with standard Galerkin FEM



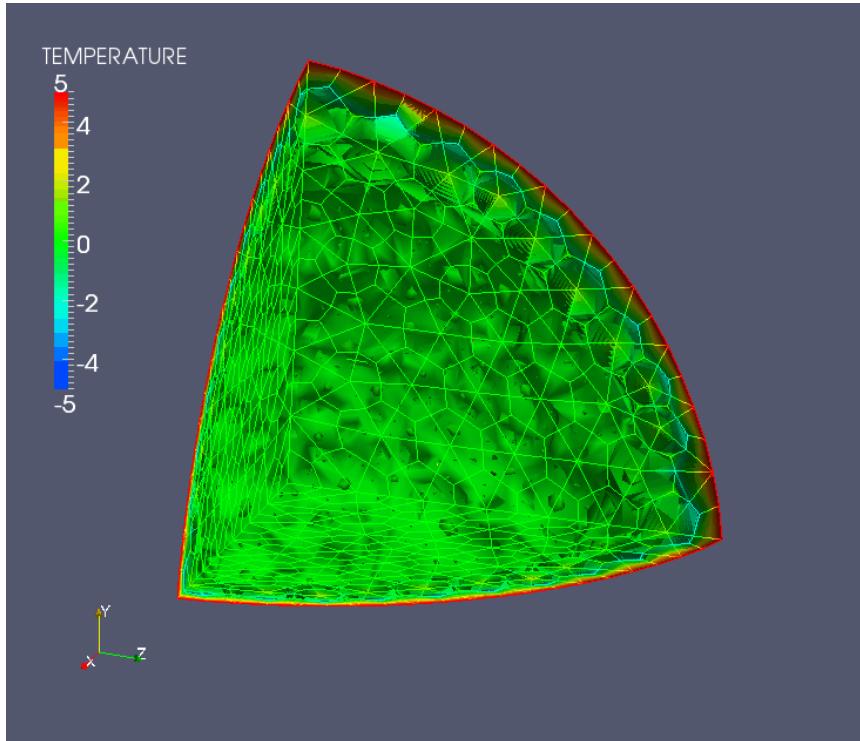
Pore Pressure computed with Stabilized FEM

Note: we do not impose shape pore pressure gradient.

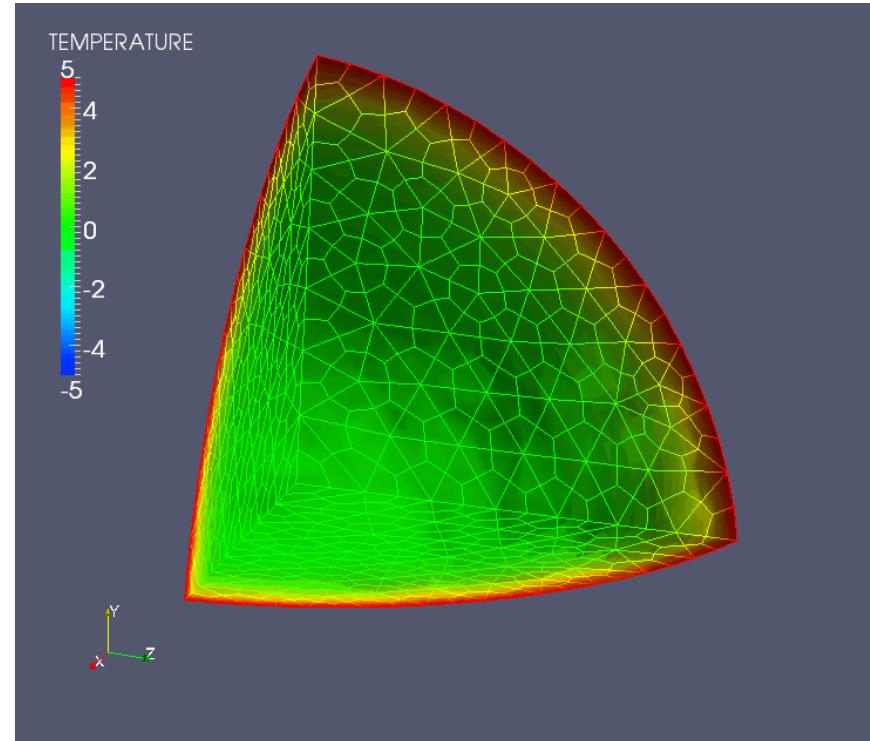
Thermo-hydro-mechanical Responses of Porous Sphere in Thermal Reservoir

Under-diffusion with spurious patterns

Diffusion with estimated optimal stabilization



Temperature computed with standard
Galerkin FEM



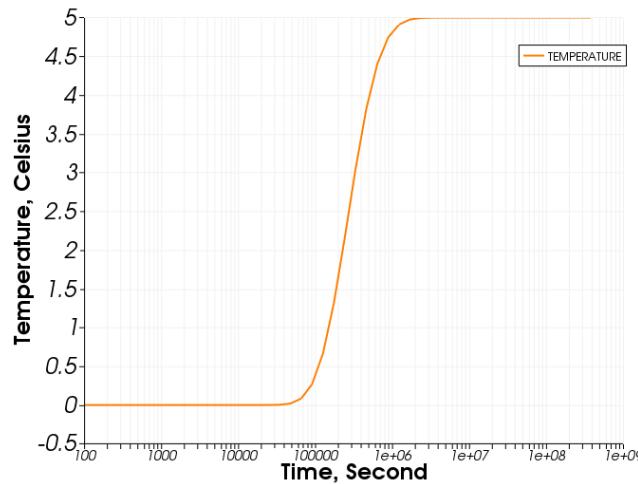
Temperature computed with Stabilized
FEM

Generalized Mandel-Cryer Effect for THM Spherical problems

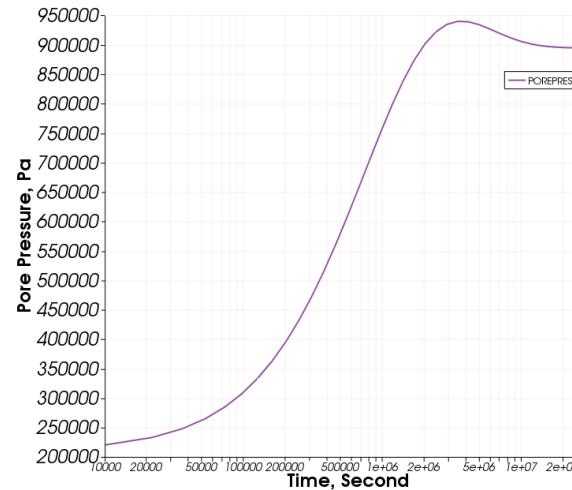
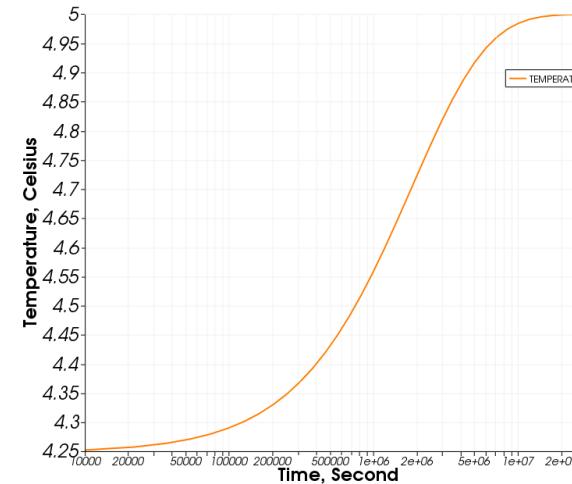
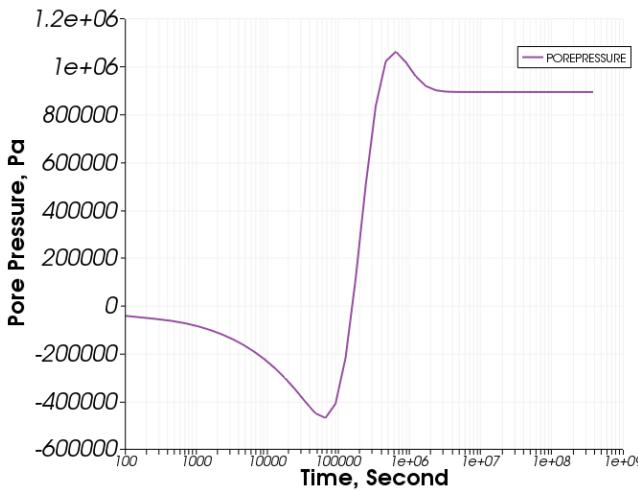
✓ Solution with estimated optimal stabilization

✗ Over-diffused solution (Constant stabilization parameter C=200000)

Temperature
Time History of
the centroid



Pore pressure
Time History of
the centroid



Optimal Stabilization Parameter Estimation

1D poromechanics governing equation

$$c \frac{\partial^2 \hat{p}}{\partial x^2} = \frac{\partial \hat{p}}{\partial t}, \quad c = \frac{k}{\mu} \frac{M' H}{H + \nu \beta M'}; M' = \frac{M(K + 4G/3)}{K + 4G/3 + B^2 M}$$

Three node stencil (standard Galerkin method)

$$-\hat{p}_{A-1} + 2\hat{p}_A - \hat{p}_{A+1} + \frac{h^2}{6\vartheta c \Delta t} (\hat{p}_{A-1} + 4\hat{p}_A + \hat{p}_{A+1}) = 0$$

Three node stencil (Stabilized Galerkin method)

$$-\hat{p}_{A-1} + 2\hat{p}_A - \hat{p}_{A+1} + \frac{h^2}{12\vartheta c \Delta t} [(2 - \gamma)\hat{p}_{A-1} + (8 + 2\gamma)\hat{p}_A + (2 - \gamma)\hat{p}_{A+1}] = 0$$

Growth/decay rate

$$\cosh \frac{h}{(\sqrt{\vartheta c \Delta t})^h} = \frac{(1 + h^2/\vartheta c \Delta t)(4 + \gamma)/6}{(1 - h^2/\vartheta c \Delta t)(2 - \gamma)/12}$$

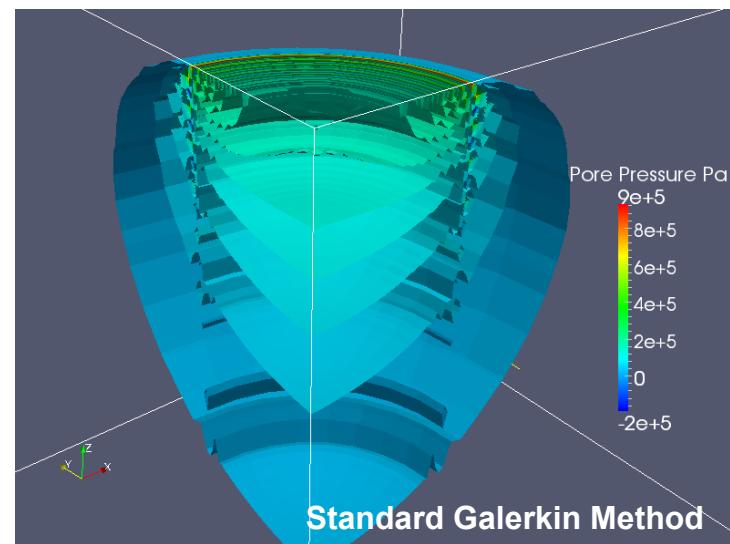
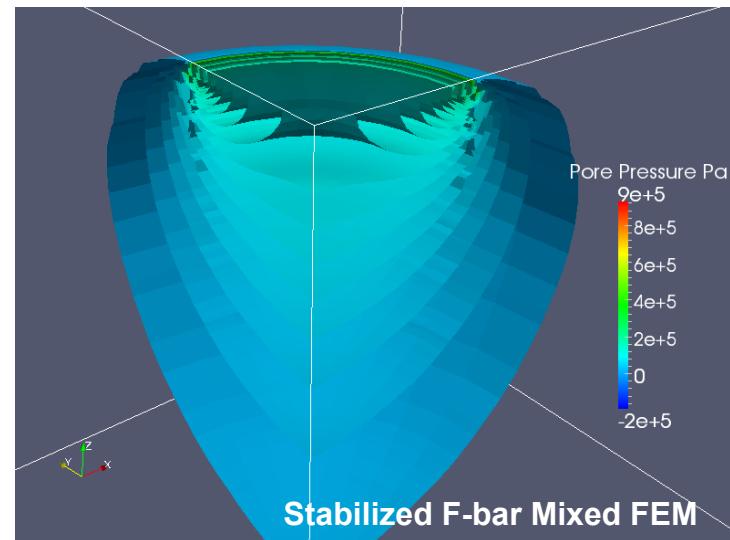
To have real growth/decay rate, we need

$$\gamma > 2 - 12 \frac{\vartheta c \Delta t}{h^2} > 0$$

$$\gamma = (2 - 6 \frac{\vartheta c \Delta t}{h^2}) \left(\frac{1}{2} + \frac{1}{2} \tanh(2 - 12 \frac{\vartheta c \Delta t}{h^2}) \right)$$

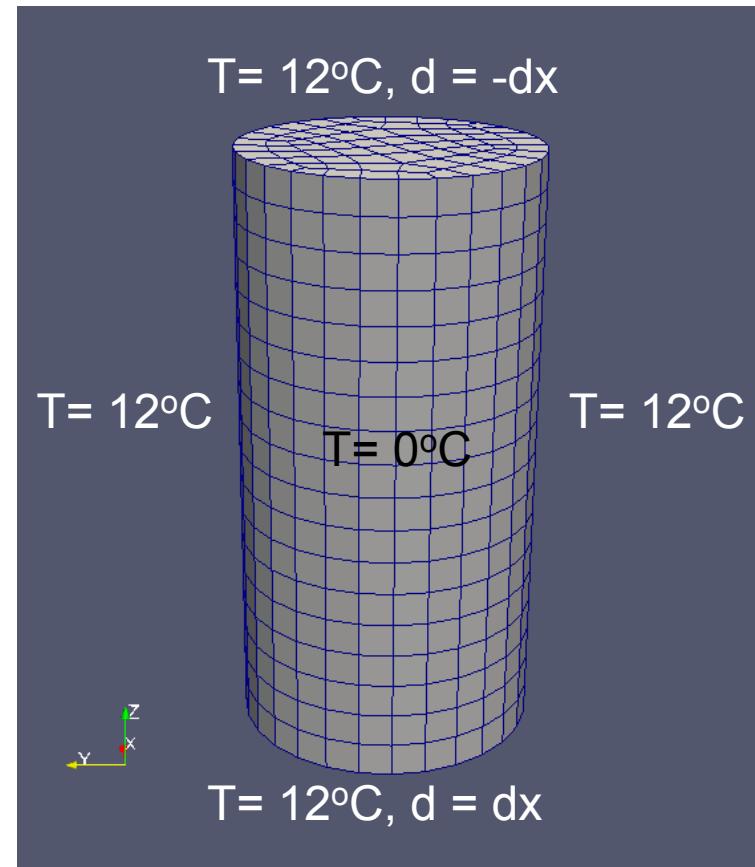
Safety factor

Turn off stabilization
without introducing switch



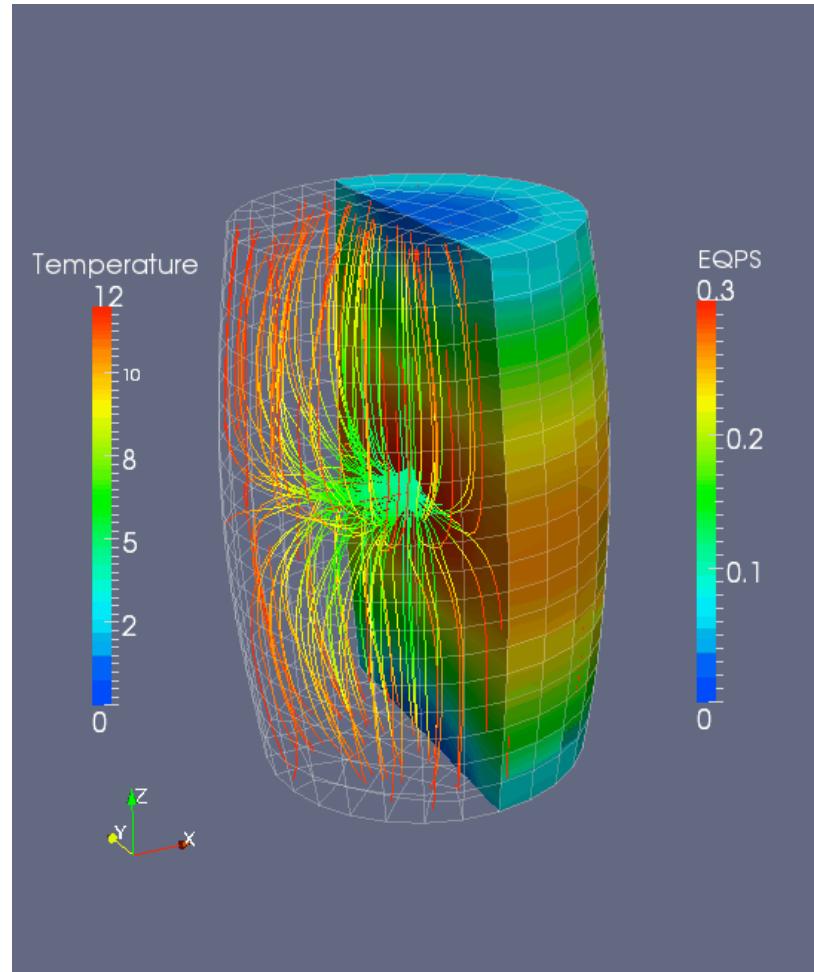
Unconfined Compression Test of Cold Thermo-sensitive Porous Media at Room Temperature

- Material
 - J2 perfect plasticity
 - Kozeny-Carman permeability
 - Homogenized thermal conductivity
 - Pore-fluid diffusivity < thermal diffusivity
- Boundary Condition
 - Temperature at surface = 12°C
 - Initial temperature = 0°C
 - Globally undrained
 - Unconfined side surface
 - Prescribed displacement on the top and bottom
 - Under constant rate, apply 16.67% vertical strain in 10 hours

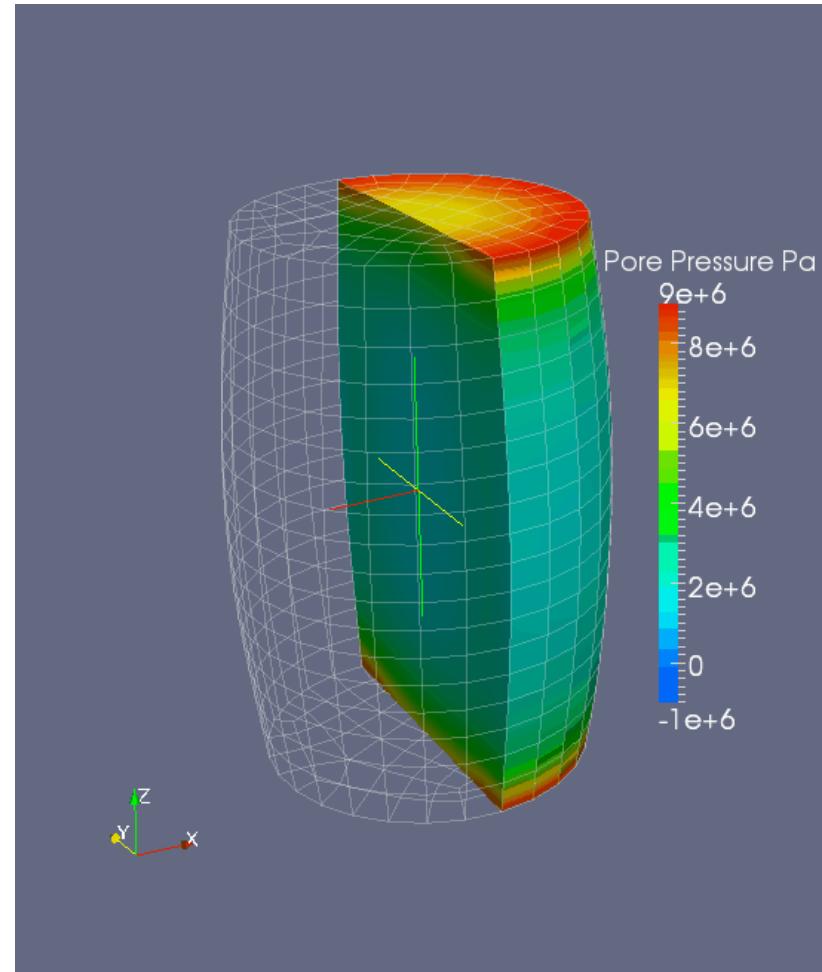


Coupling effects of Multiphysical Responses in Geometrically Nonlinear Regime

➤ Temperature & equivalent plastic strain



➤ Pore Pressure



Why localization elements?



Figure from Hirschberger, Sukumar and Steinmann, Philosophical Magazine, 2008

- Model surface, rock joint and bulk materials without the need to discretize them in the same mesh.

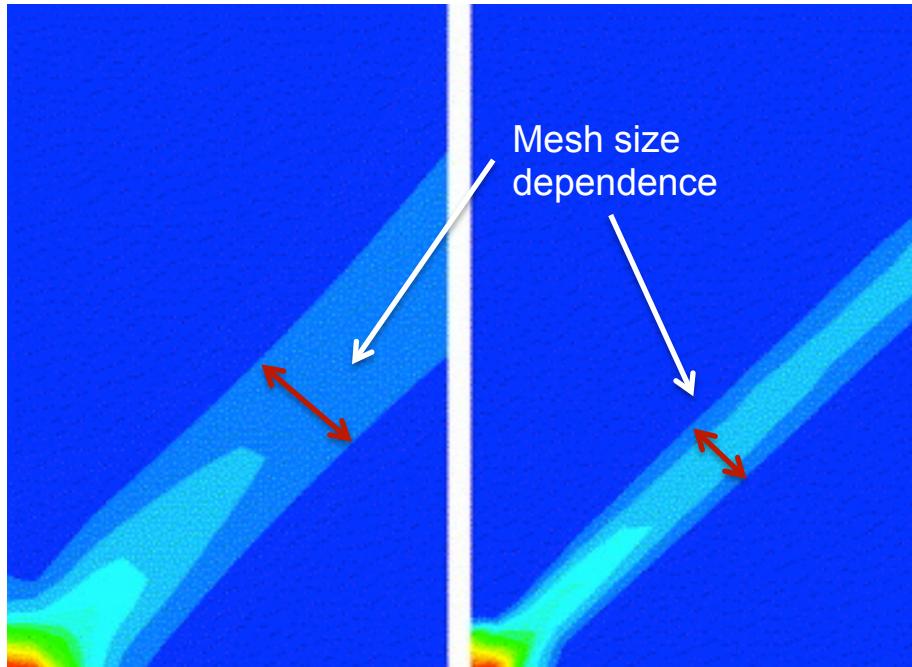


Figure from Khoei et al, Communications in Nonlinear Science and Numerical Simulation 2005

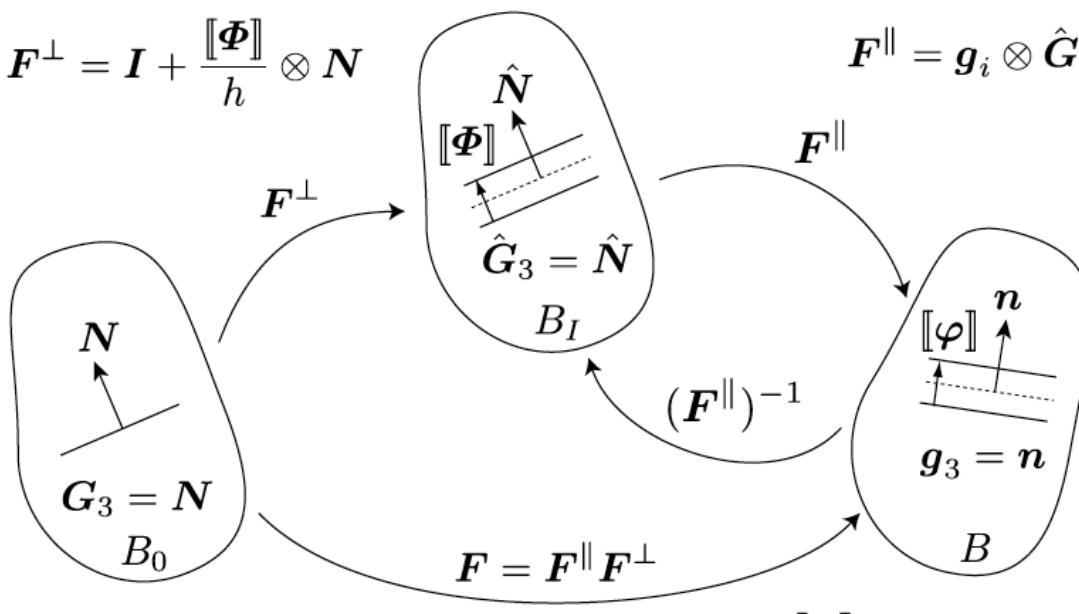
- Cure mesh dependence by introducing length scale.

Formulation of the localization element

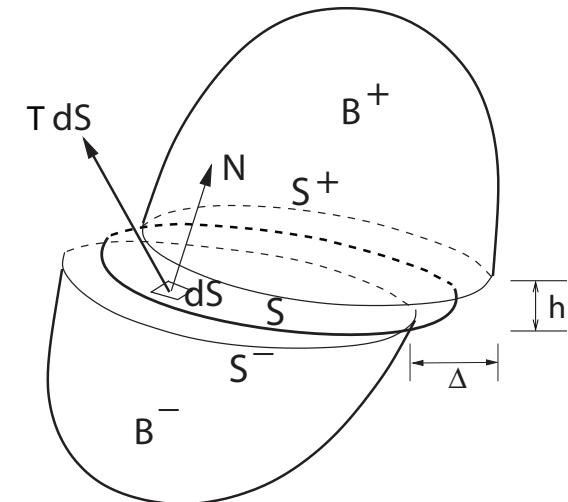
$$\mathbf{F}^\perp = \mathbf{I} + \frac{[\![\Phi]\!]}{h} \otimes \mathbf{N}$$

$$\mathbf{F}^\parallel = \mathbf{g}_i \otimes \hat{\mathbf{G}}^i$$

$$[\![\Phi]\!] = (F^\parallel)^{-1} [\![\varphi]\!]$$



$$\hat{\mathbf{G}}_A \parallel \mathbf{G}_A \longrightarrow \mathbf{F} = \mathbf{F}^\parallel + \frac{[\![\varphi]\!]}{h} \otimes \mathbf{N}$$



From Yang, Mota Ortiz, 2006;
Foulk et al 2013

Deformation power of the solid skeleton

$$\begin{aligned} P^D &= \sum_{\pm} \int_{B_0^{\pm}} \mathbf{P} \cdot \dot{\mathbf{F}} dV + \int_{S_0} \mathbf{P} \cdot \dot{\mathbf{F}} h dS \\ &= \sum_{\pm} \int_{B_0^{\pm}} \mathbf{P} \cdot \dot{\mathbf{F}} dV + \int_{S_0} \mathbf{P} \cdot \left[\dot{\mathbf{F}}^\parallel h + [\![\dot{\varphi}]\!] \otimes \mathbf{N} \right] dS \\ &= \sum_{\pm} \int_{B_0^{\pm}} \mathbf{P} \cdot \dot{\mathbf{F}} dV + \int_{S_0} \left[h \mathbf{P} \cdot \dot{\mathbf{F}}^\parallel + \mathbf{T} \cdot [\![\dot{\varphi}]\!] \right] dS \end{aligned}$$

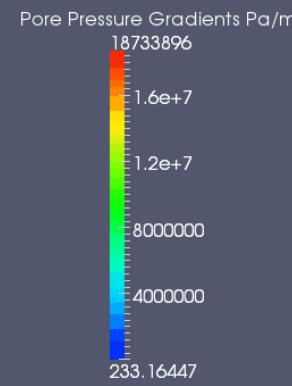
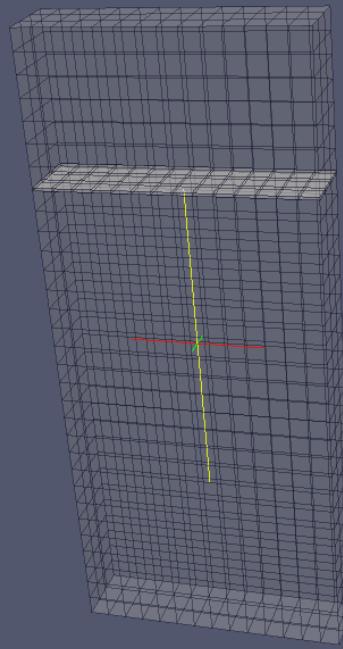
$$\boxed{\mathbf{F} = \mathbf{F}^\parallel \mathbf{F}^\perp}$$

deformation of
mid-surface
membrane

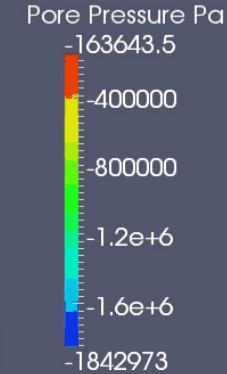
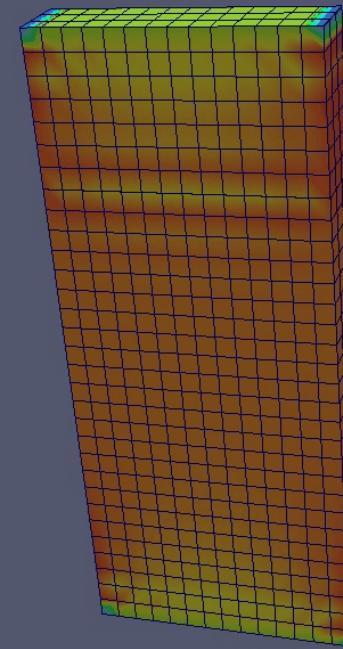
homogenized
displacement
jump

Globally undrained Simple Shear Test of Fully saturated media with flow barrier in the surface element (Pore Pressure)

Without sealed rock joint



With sealed rock joint

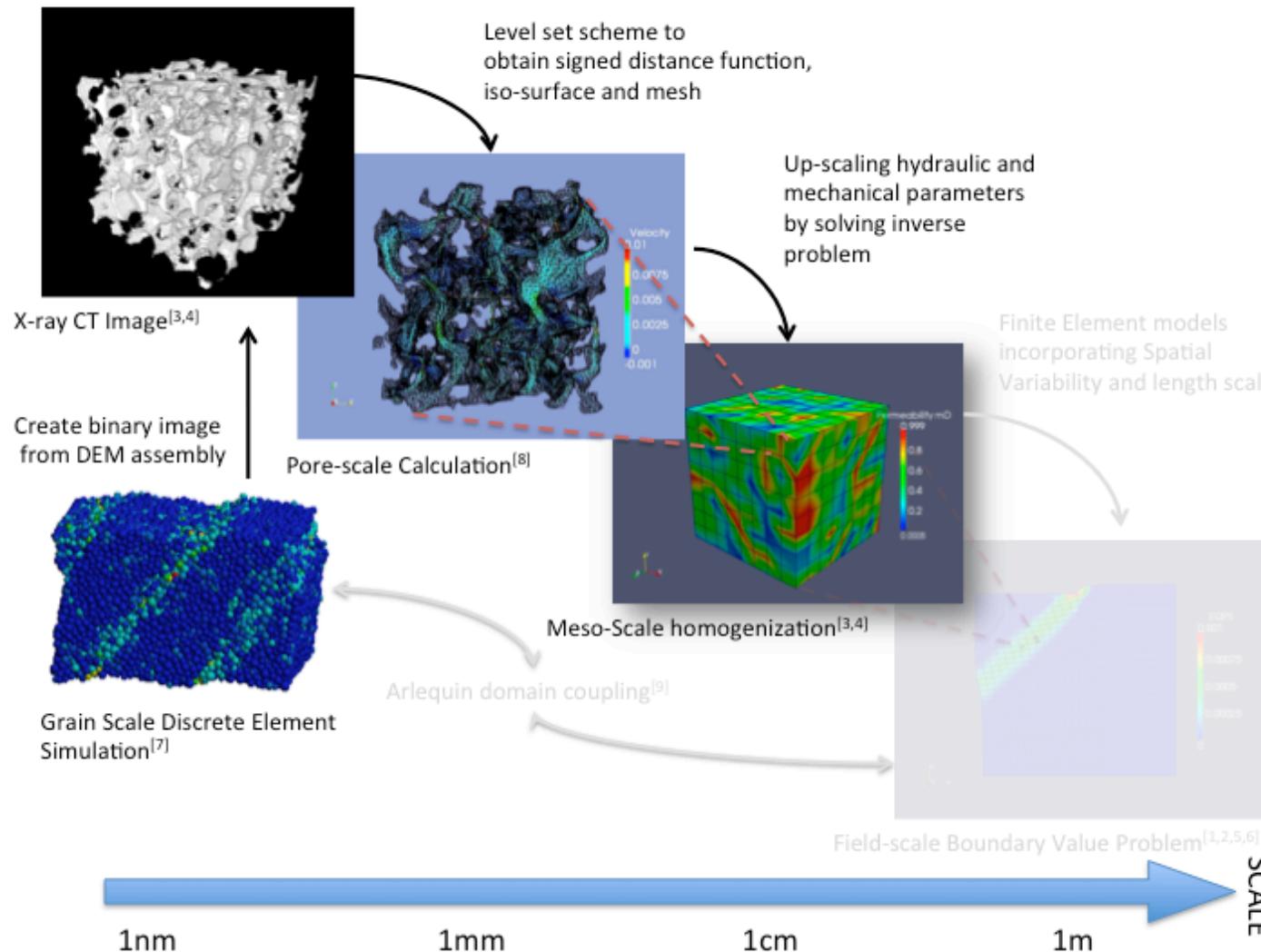


- In geometrical nonlinear regime, pore-fluid flow is significantly influenced by geometrical changes.
- Capturing localized hydraulic features triggered by deformation is important to analyze overall reservoir properties.

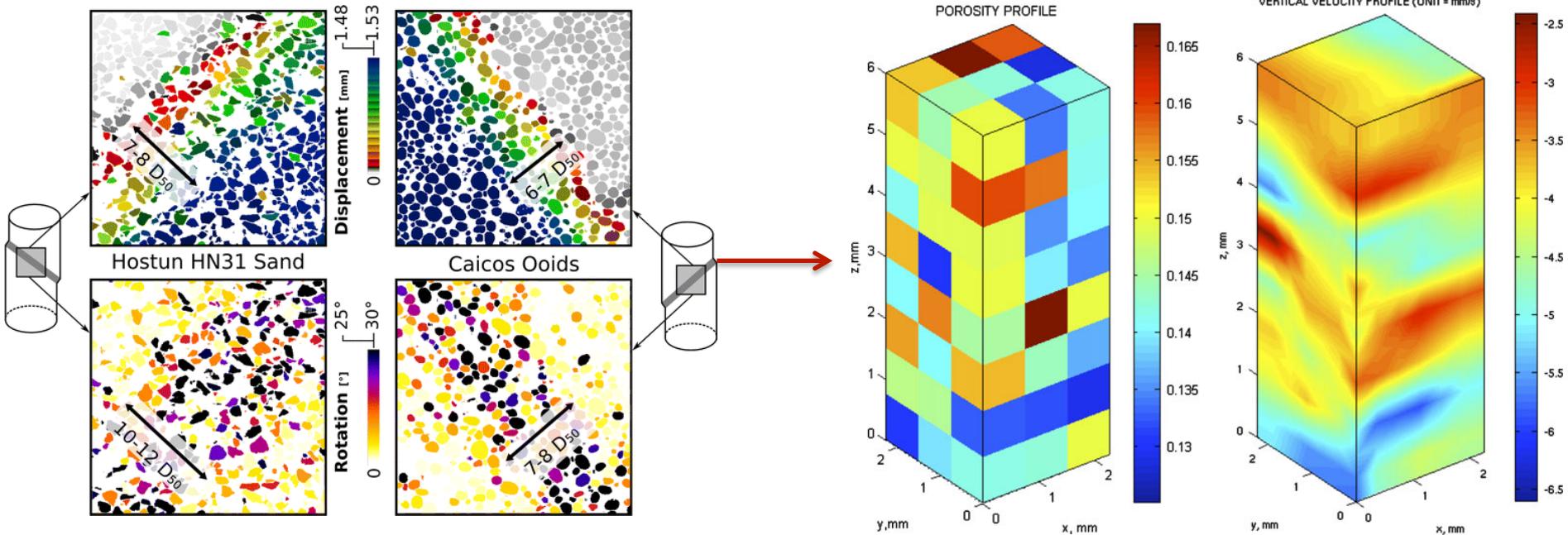
Conclusion and Future Perspective

- For the 1st time, A **fully coupled**, finite deformation, stabilization thermo-hydro-mechanics finite element model is implemented.
- This model preserves Mandel-Cryer effect, and is able to **eliminate spurious oscillation** due to the lack of inf-sup condition.
- Thermo-poro-plasticity model is extended and tested.
- **Localization element** is introduced as localization limiter to cure mesh dependence.
- **Unsaturated flow** will be further tested against analytical solutions and classical problems in the literatures.

Connecting micro-structural attributes and macroscopic mechanical and hydraulic responses



State-of-art on Analyzing Stress Induced Permeability Changes on Granular Materials



1. ID tracking approach performed on tomographic images
2. Obtain porosity profile
3. Compute permeability

Ando, Hall, Viggiani, Desrues, Besuelle, Acta Geotechnica, 2012

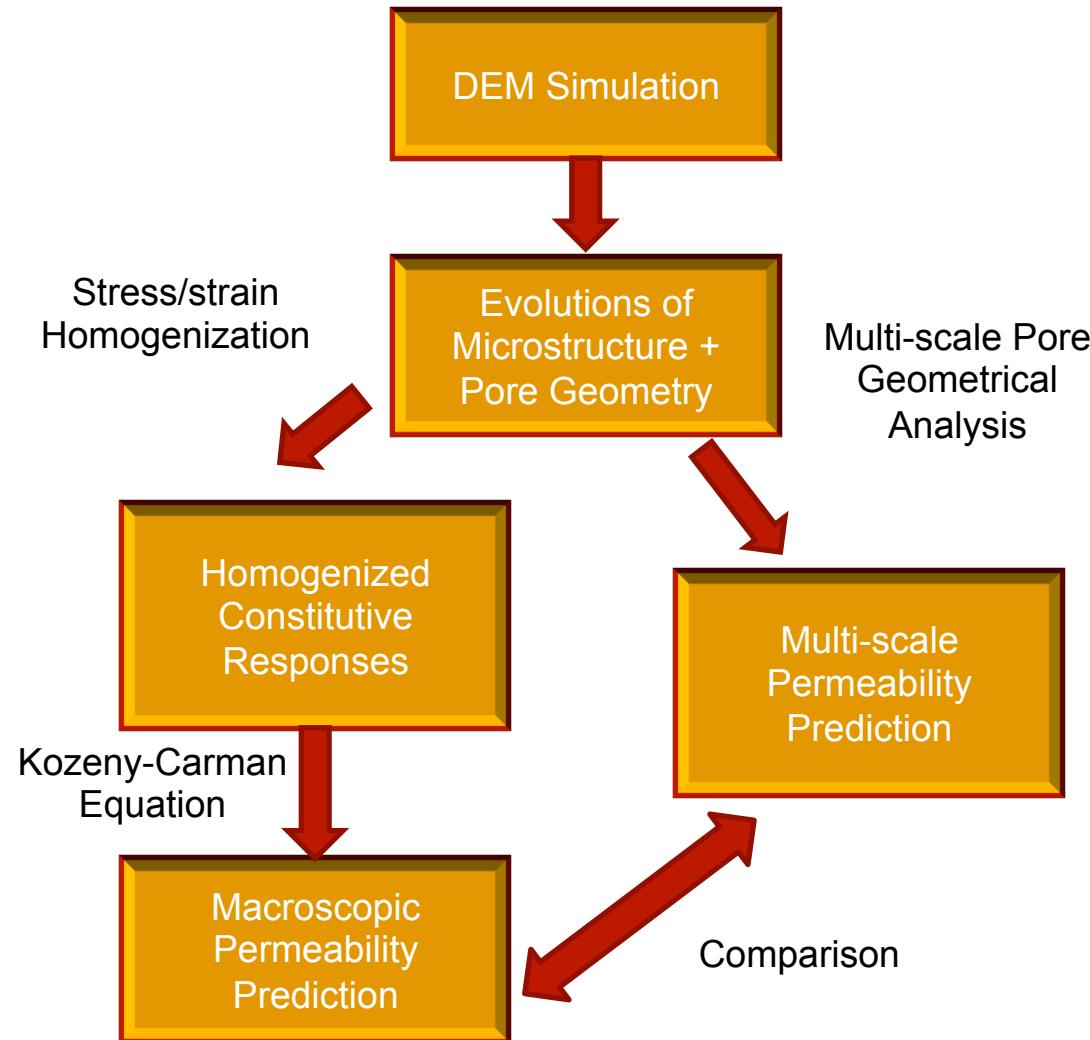
Sun, Andrade, Rudnicki, IJNME, 2011

Some problems with this approach....

1. Not **repeatable** for different stress path (It is impossible to prepare two identical granular assemblies in lab)
2. Technically, volumetric digital image correlation **during** mechanical test is **difficult** and very **rare** (Hall et al 2011).
3. Permeability changes due to changes of pore geometry (**shape**) is lost if porosity dependent empirical relation (i.e., Kozeny-Carman relation) is used to analyze pore-scale images.

Analyzing Microstructure Evolution of Shear Banding in Simple Shear Test

1. Run Discrete Element Simulation.
2. Extract 3D assembles inside and outside deformation bands
3. Analyze rotations, translations and damages of grains
4. Obtain macroscopic constitutive responses
5. Analyze pore geometry and estimate permeability with the multi-scale framework discussed before.



Discrete Element Simulation of Simple Shear Test

Balance of linear and angular momentum

$$m \ddot{u} + C^m \dot{u} + P(u) = F_{ext}$$

force balance

$$I \cdot \ddot{\omega} = M_{ext}$$

moment balance

Homogenized Cauchy stress and infinitesimal strain

$$\sigma_{ij} = \frac{1}{V_\Omega} \sum_{\alpha=1}^{N_c} f_i^\alpha l_j^\alpha; \quad \epsilon_{ij} = \frac{1}{2} (F_{ik} F_{kj} - I_{ij})$$

Hertz's contact model

$$df^n = k^n d\delta; \quad k^n = \frac{\sqrt{2}G_g \sqrt{R^e}}{1-\nu} \delta^{1/2}$$

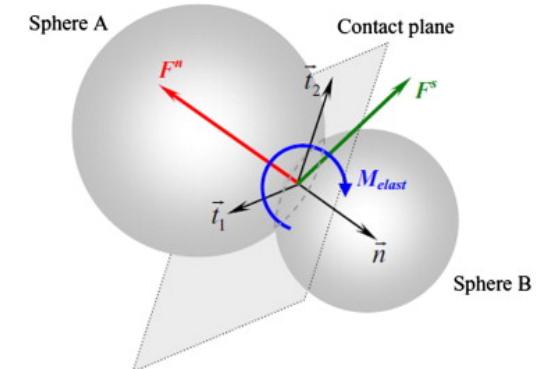
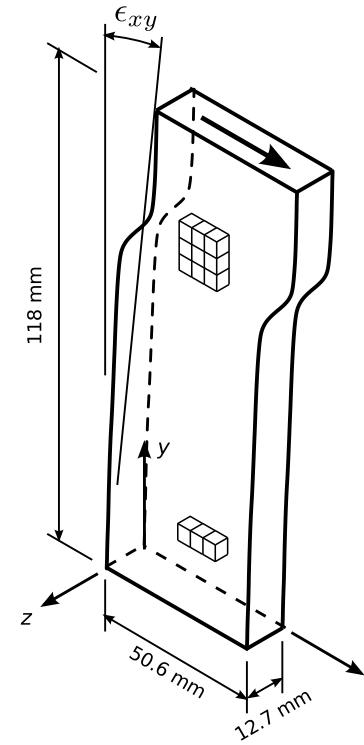
normal stiffness

$$df^t = k^s ds; \quad k^s = \frac{2\sqrt{2}G_g \sqrt{R^e}}{2-\nu} \delta^{1/2}$$

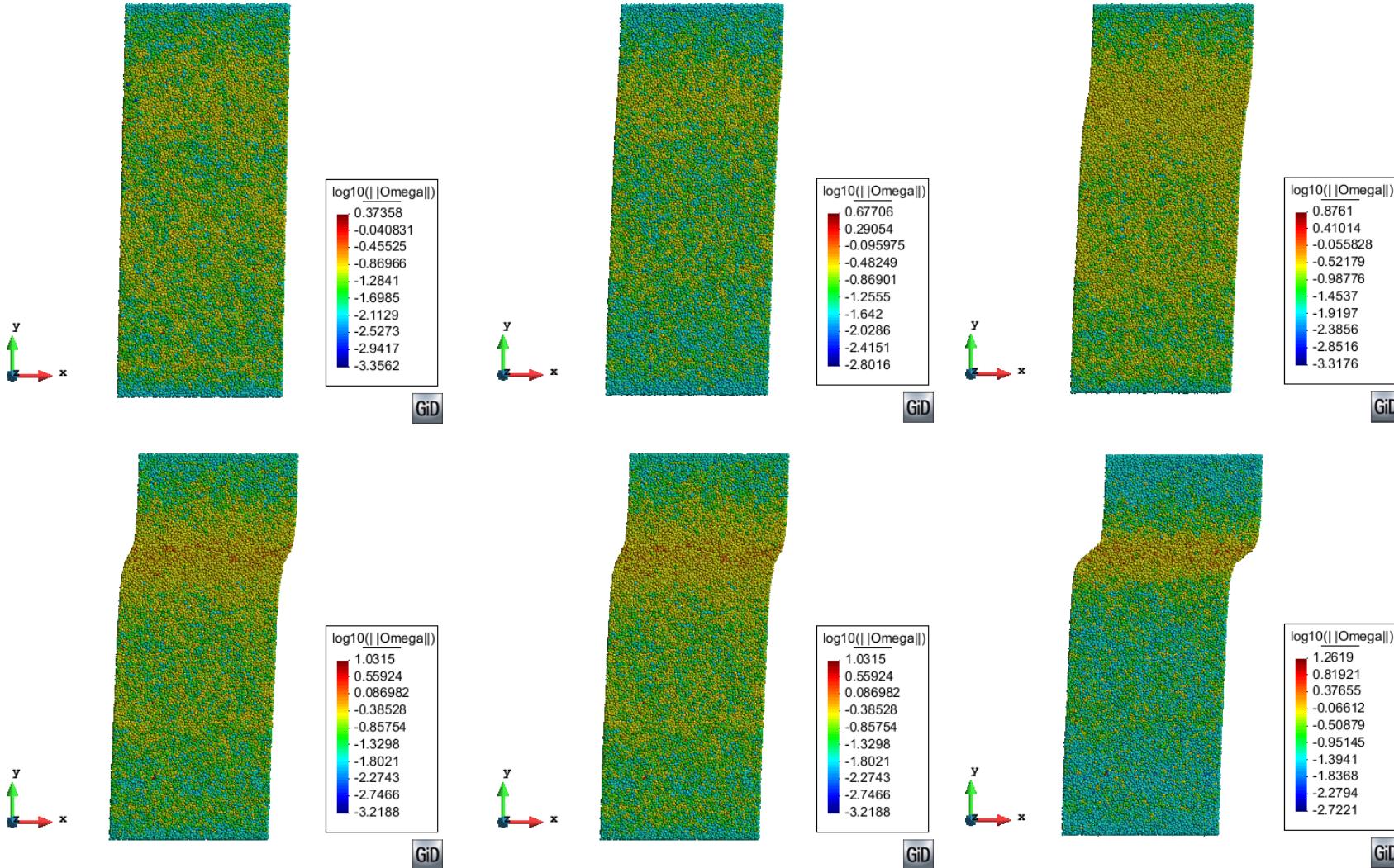
tangential stiffness

$$R^e = \frac{2R_1 R_2}{R_1 + R_2}$$

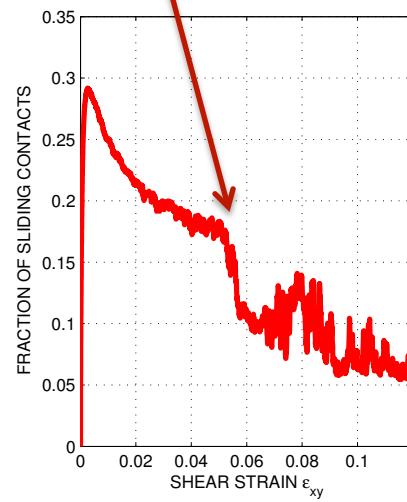
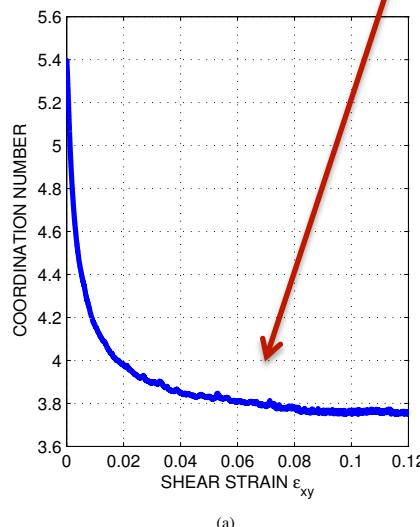
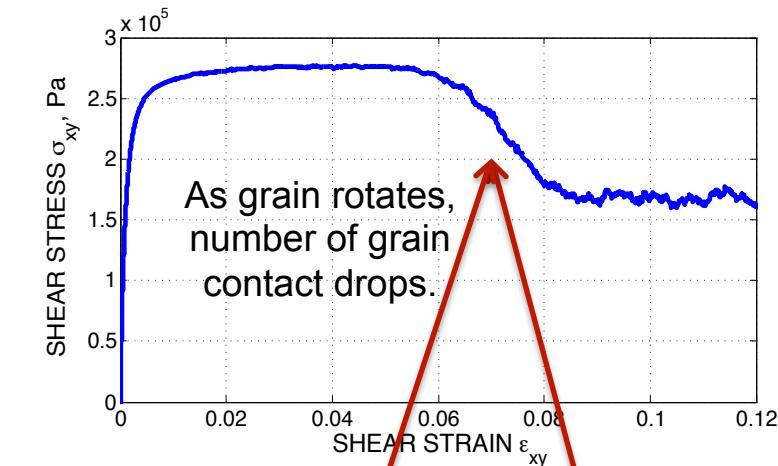
effective radius



Rotation Magnitude of Grains Obtained From The Discrete Element Simulation of Simple Shear Test



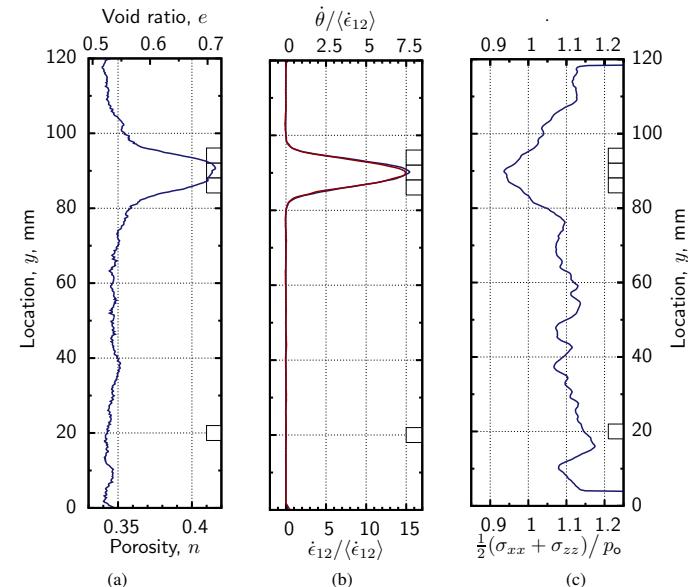
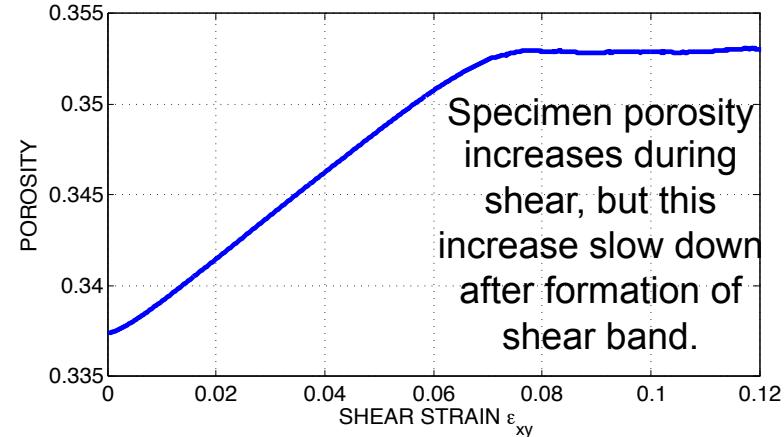
Granular and Macroscopic Responses



(a)

(b)

Number of grain contacts decreases as shear band develops.



Locally, porosity increase is concentrated inside shear band.

Question:

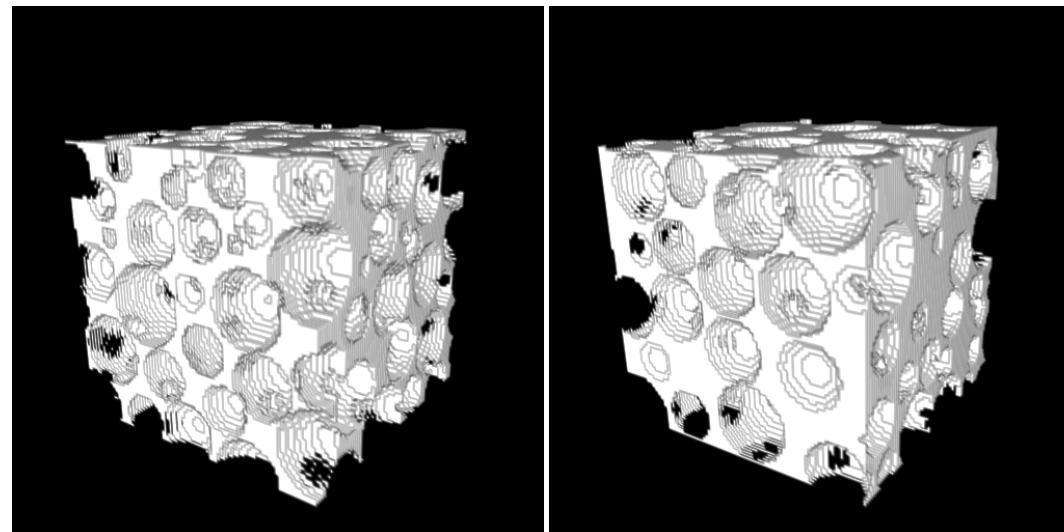
How does this granular motion affects pore geometry and hydraulic properties of deformation bands?

Obtain Pore Volume Binary Images from DEM Assemblies

Algorithm 1 Seed-fill (node, void-flag, solid-flag)

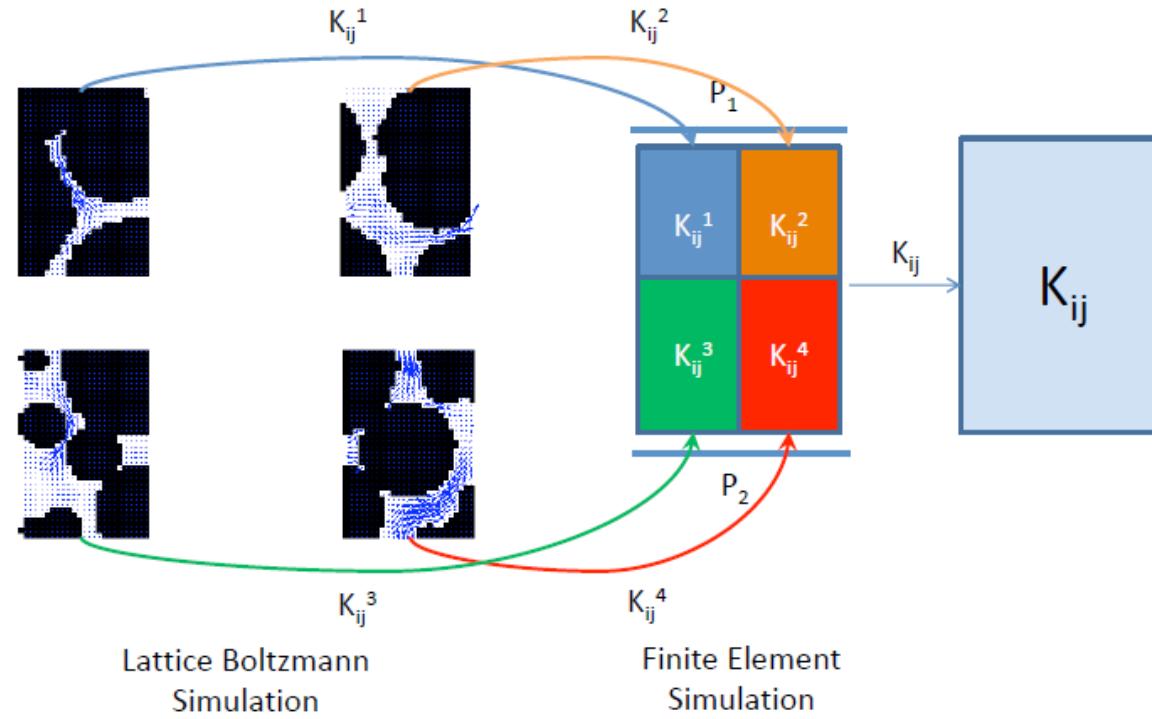
```

if the current node is not a void then
  return
else
  search the spherical particle closest to the current node
  Compute Euclidean distance between current node and the centroid of the closest spherical particle
  if Euclidean distance  $\neq$  radius of the closest spherical particle then
    Perform Flood-fill (the west neighbor of the current node, void-flag, solid-flag)
    Perform Flood-fill (the east neighbor of the current node, void-flag, solid-flag)
    Perform Flood-fill (the north neighbor of the current node, void-flag, solid-flag)
    Perform Flood-fill (the south neighbor of the current node, void-flag, solid-flag)
    Perform Flood-fill (the upper neighbor of the current node, void-flag, solid-flag)
    Perform Flood-fill (the lower neighbor of the current node, void-flag, solid-flag)
  end if
end if
  
```



Binary images generated from DEM assemblies

Multi-scale Lattice Boltzmann/Finite Element Simulations



$$f_i(\mathbf{x} + \mathbf{e}_i, t + \Delta t) - f_i(\mathbf{x}, t) = -\frac{1}{\tau} (f_i(\mathbf{x}, t) - f_i^{\text{eq}}(\mathbf{x}, t)),$$

$$f_i^{\text{eq}} = w_i \rho \left(1 + \frac{3\mathbf{e} \cdot \mathbf{v}}{c^2} + \frac{9(\mathbf{e} \cdot \mathbf{v})^2}{2c^4} - \frac{3(\mathbf{v})^2}{2c^2} \right)$$

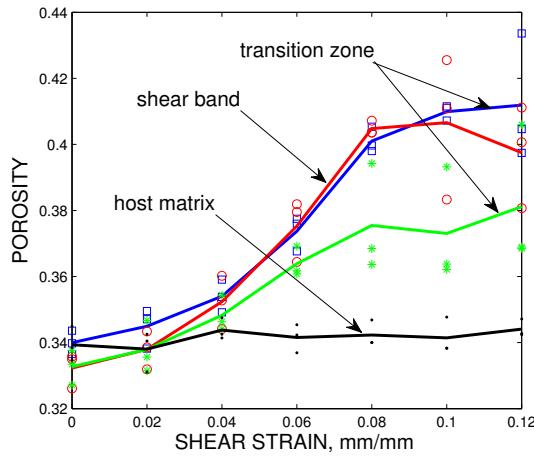
$$\mathbf{v} = \frac{1}{\rho} \sum_{i=1}^{\alpha} f_i \mathbf{e}_i, \quad p = c^2 \rho, \quad \rho = \sum_{i=1}^{\alpha} f_i$$

$$\nabla^{\mathbf{x}} \cdot \mathbf{v}(\mathbf{x}) = 0$$

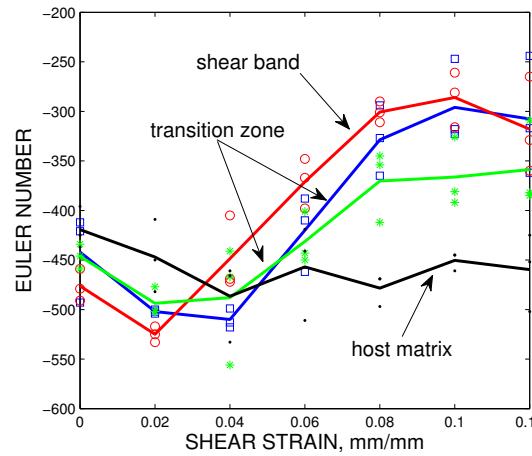
$$\mathbf{v}(\mathbf{x}) = -\frac{1}{\mu^v} \mathbf{k}(\mathbf{x}) \cdot \nabla^{\mathbf{x}} p(\mathbf{x})$$

See W.C Sun, J.E. Andrade, J.W. Rudnicki, A multiscale method for characterization of porous microstructures and their impact on macroscopic effective permeability, *International Journal of Numerical Methods in Engineering*, Vol. 88, No.12, 1260-1279, 2011.

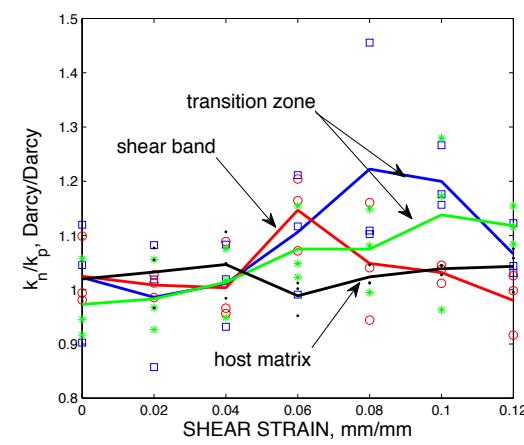
Geometrical Changes of Pore Space Induced By Dilation Inside Shear Band



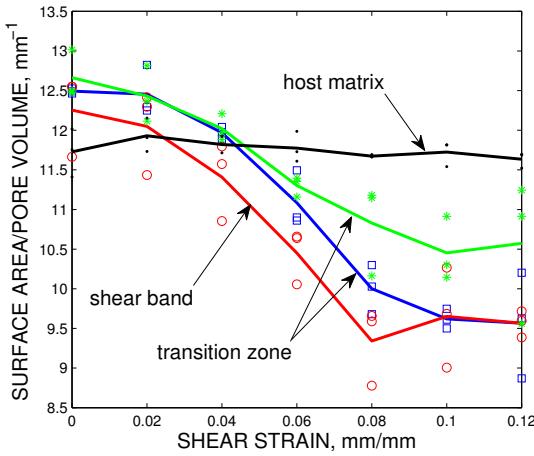
(a) Porosity



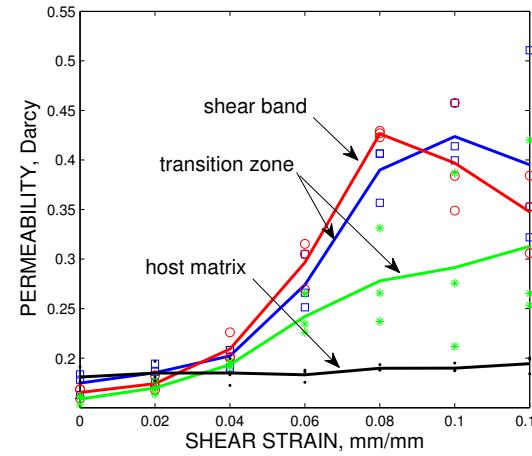
(b) Euler number



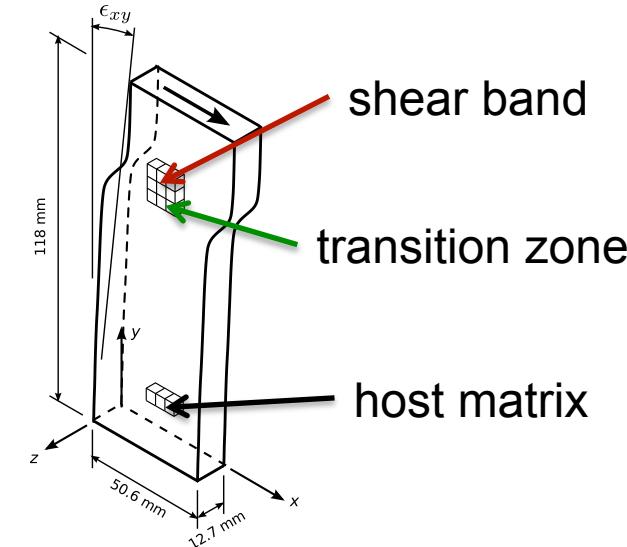
SHEAR STRAIN, mm/mm



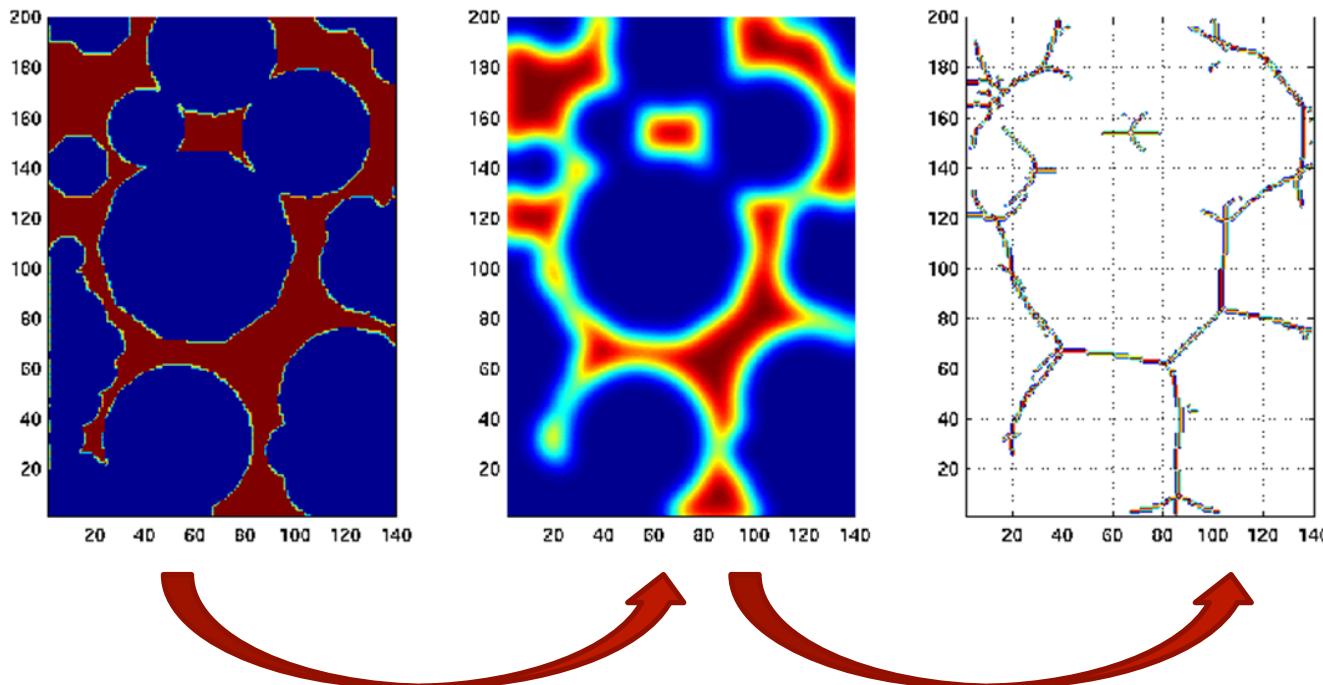
(c) Surface area/pore volume



(d) Permeability



Locating Medial Axis of Flow Path via Level Set

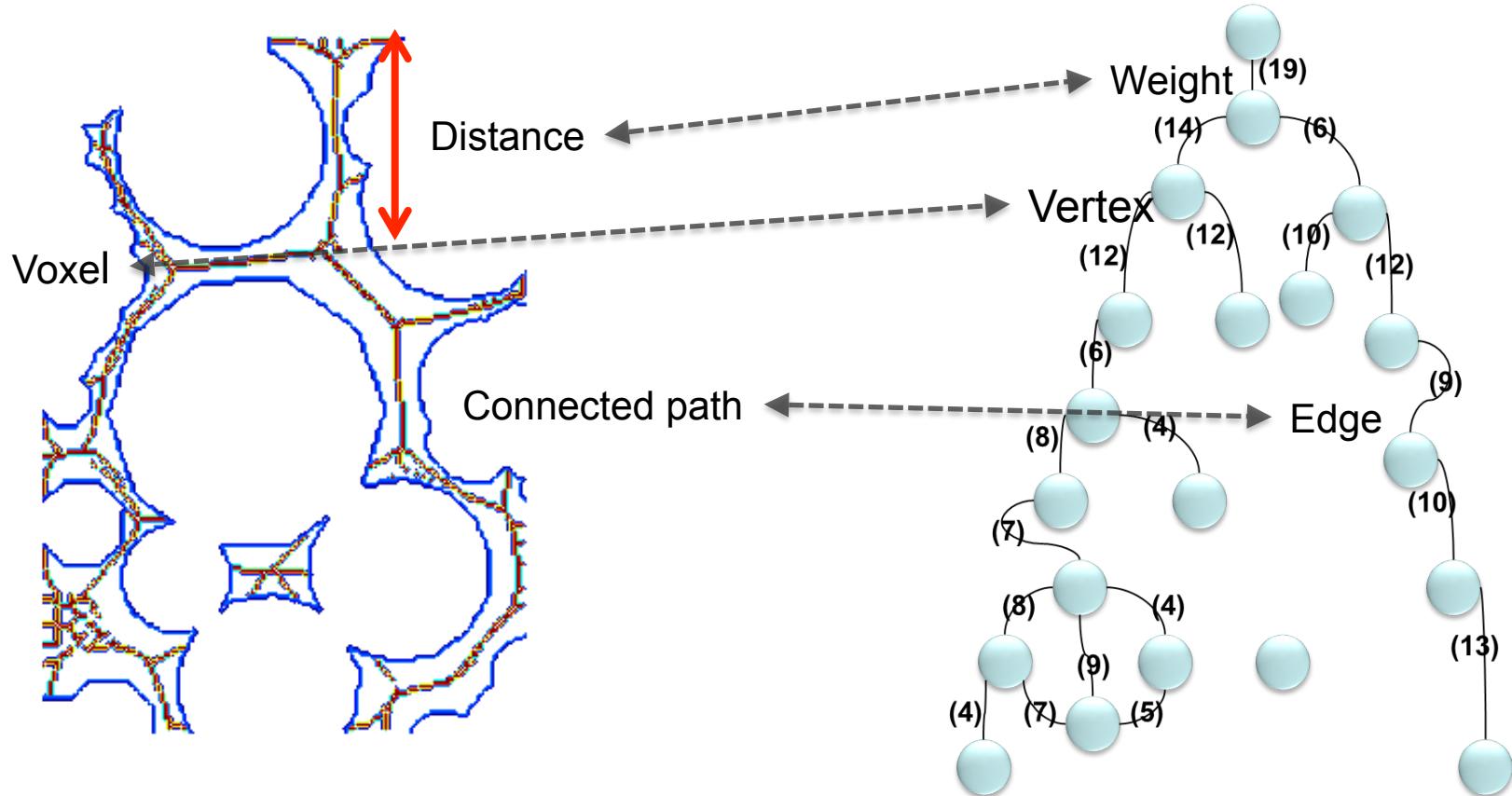


Convert binary
image into level
set via semi-
implicit scheme

Extract Local
minimum of level
set function

See W.C Sun, J.E. Andrade, J.W. Rudnicki, A multiscale method for characterization of porous microstructures and their impact on macroscopic effective permeability, *International Journal of Numerical Methods in Engineering*, Vol. 88, No.12, 1260-1279, 2011.

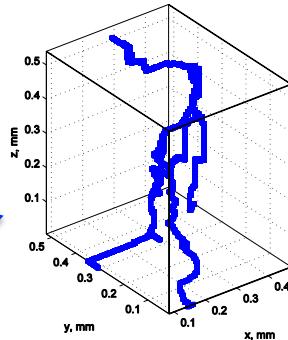
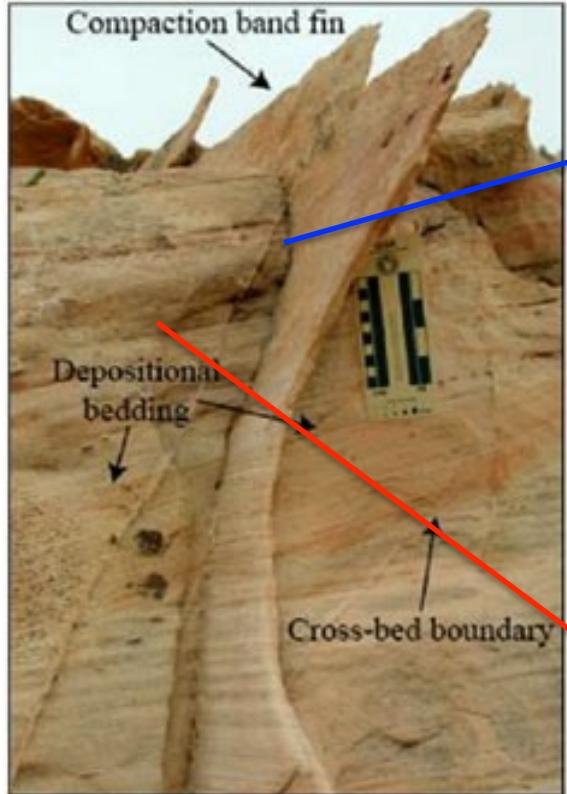
Pore Network and Weighed Graph



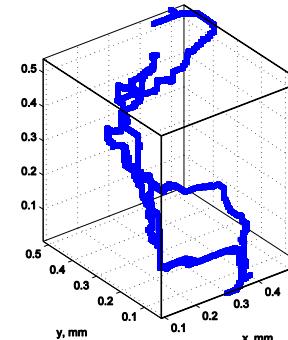
1. Use shortest path algorithm to determine tortuosity.
2. Use region growing method to distinguish connected and isolated pore space.

See W.C Sun, J.E. Andrade, J.W. Rudnicki, A multiscale method for characterization of porous microstructures and their impact on macroscopic effective permeability, *International Journal of Numerical Methods in Engineering*, Vol. 88, No.12, 1260-1279, 2011.

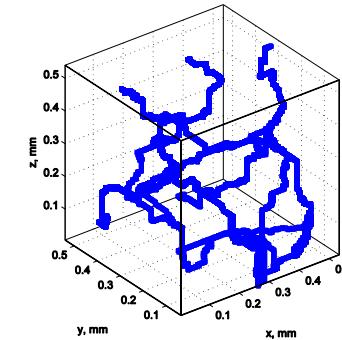
Shortest Flow Path Inside and Outside Compaction Bands (Aztec Sandstone)



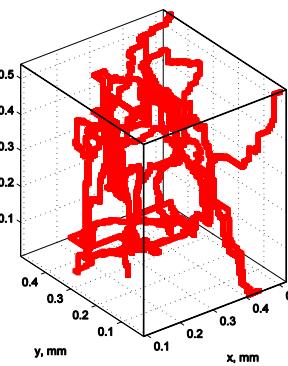
$$\tau = 2.79$$
$$K = 3.4e-13 \text{ m}^2$$



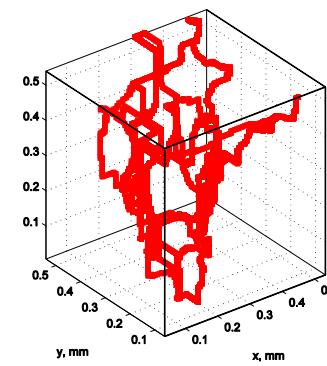
$$\tau = 2.15$$
$$K = 5.3e-13 \text{ m}^2$$



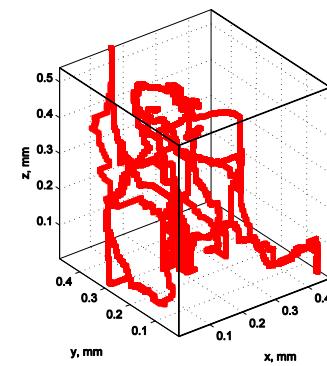
$$\tau = 2.56$$
$$K = 4.4e-13 \text{ m}^2$$



$$\tau = 1.77$$
$$K = 1.3e-12 \text{ m}^2$$

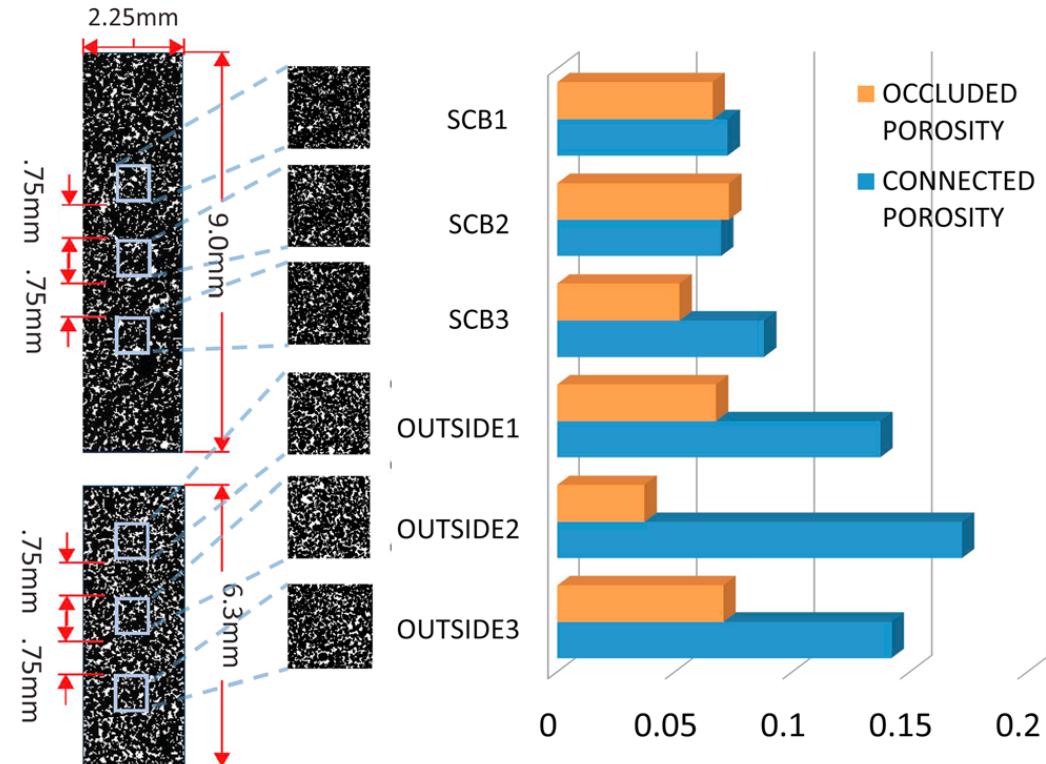
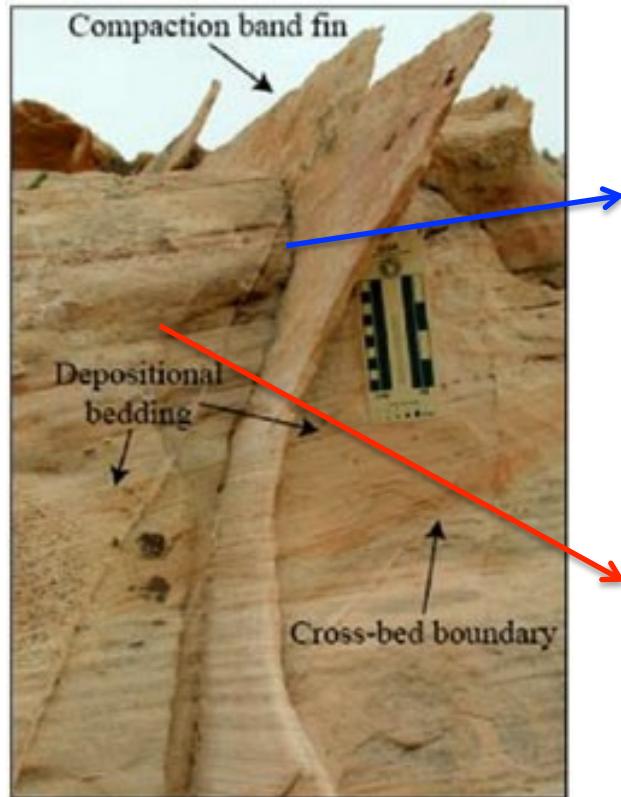


$$\tau = 1.76$$
$$K = 1.2e-12 \text{ m}^2$$



$$\tau = 1.81$$
$$K = 1.3e-12 \text{ m}^2$$

Occluded and Connected Pore Space in Shear Enhanced Compaction Bands

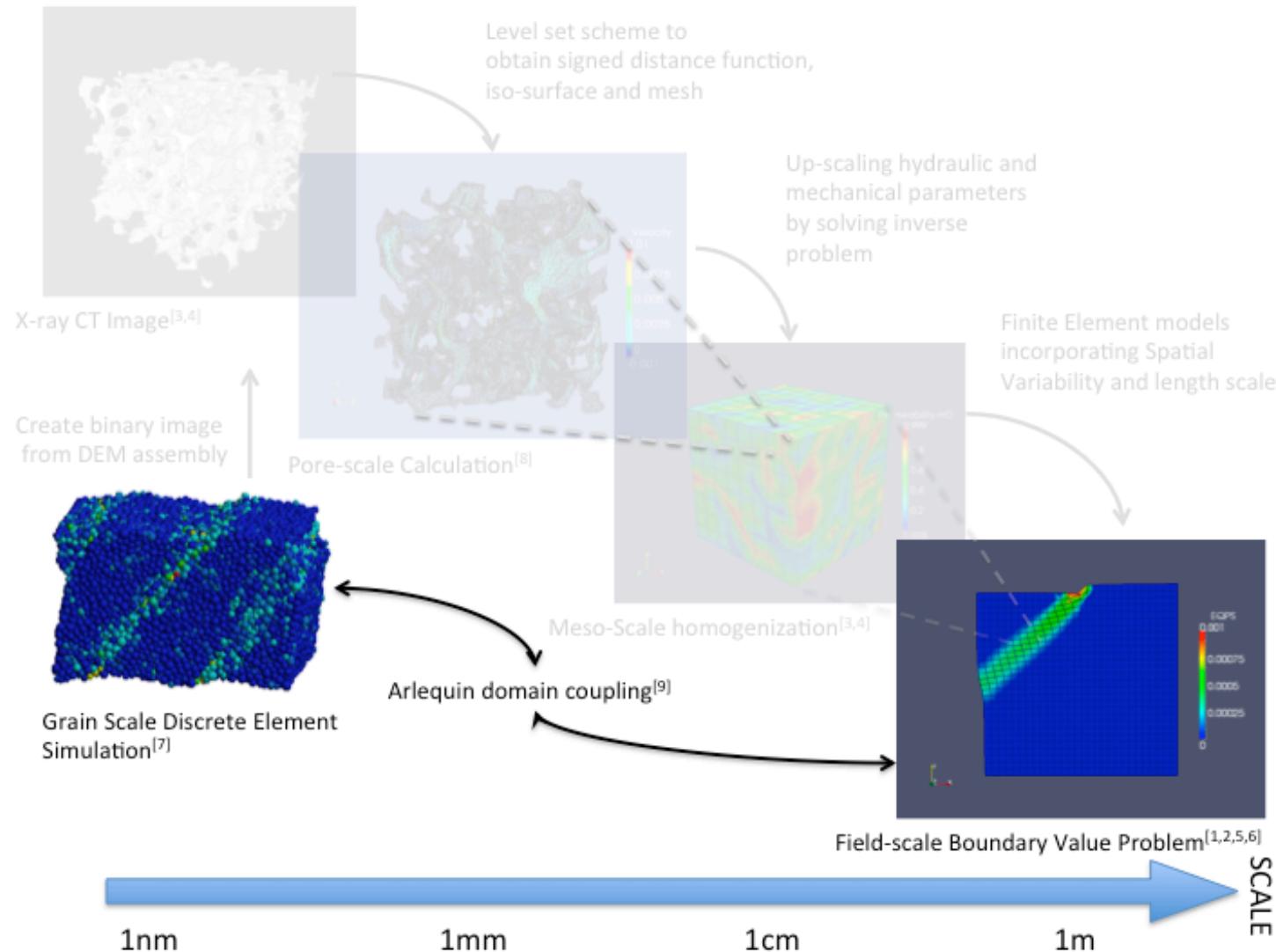


See W.C. Sun, J.W. Rudnicki, J.E. Andrade and P. Eichhubl, Connecting microstructural attributes and permeability from 3-D tomographic images of in situ compaction bands using multi-scale computation, *Geophysical Research Letter*, doi : 10.1029/2011GL047683, 2011.

Conclusion

1. Multiscale geometrical analysis and flow simulation indicates that **increased tortuosity, isolated pore space and reduction in porosity** leads to the **permeability reduction in compaction band** in Aztec sandstone specimen.
2. Discrete element simulations suggest that **increased porosity, more interconnected pore space** (as indicated by the increased Euler number), and **less tortuous flow paths** (as indicated by the decrease in surface area/volume) leads to **the permeability increase inside dilatant shear band**

Concurrent Domain Coupling Method



Overview of Domain overlapped method

■ Background

- Concurrently using different numerical methods/models designed for different scales
- Very flexible : not limited to the type of material models (e.g. can couple multiple model with different set of internal variables, multiple meshes)

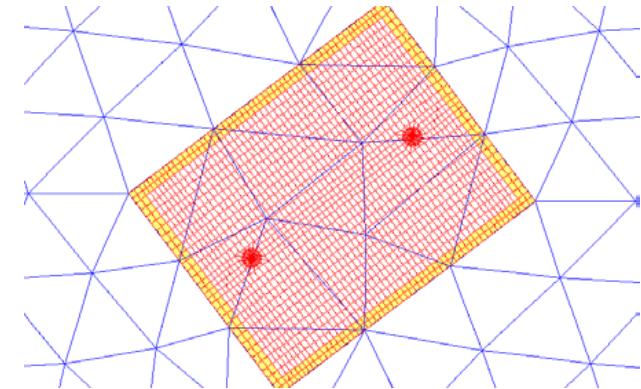
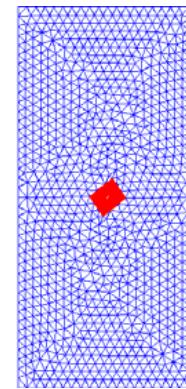


Figure from Hachmi Ben Dhia

■ New Contribution

1. Extension to large deformation problems (completed) and multiphysics problems (work-in-progress).
2. Introduce coupling between local and nonlocal constitutive laws
3. Introduce inf-sup tests to analyze stability of the energy blending domain overlapped method

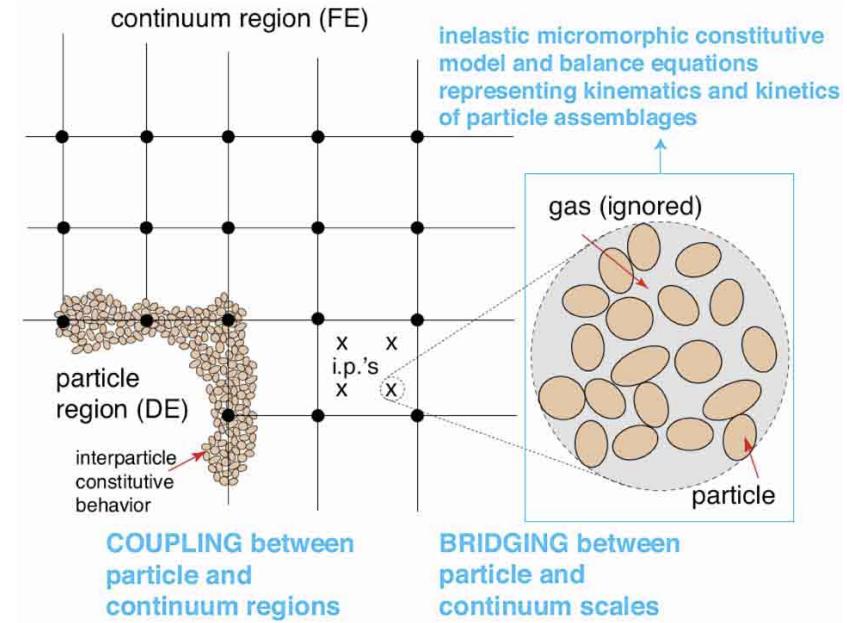
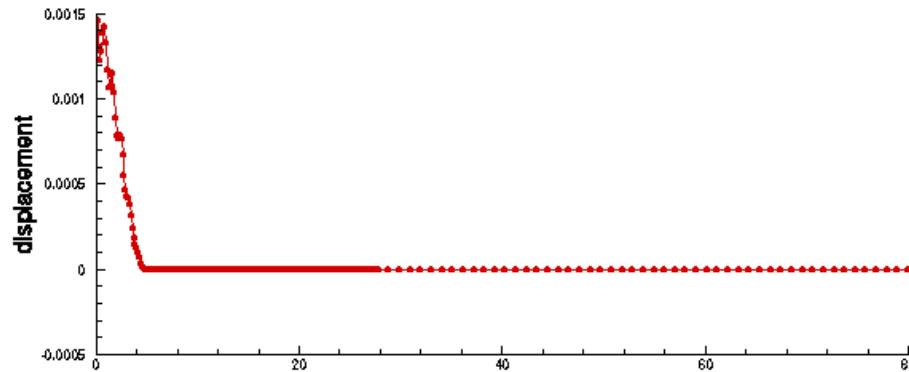


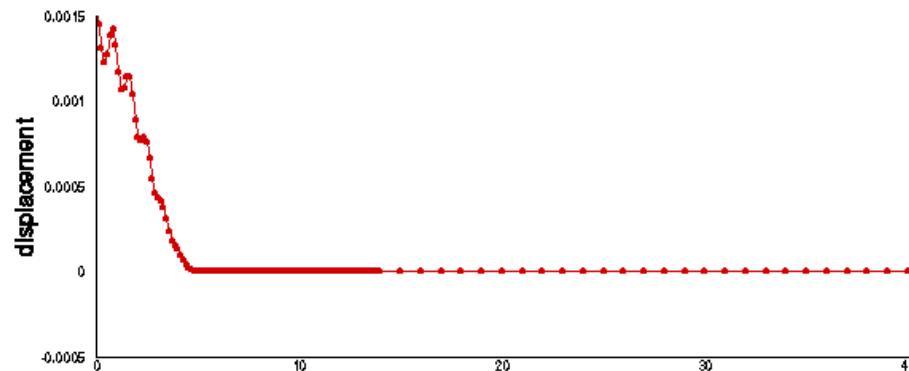
Figure from Richard A. Regueiro

Challenge

Surface Coupling



Domain Coupling



From Prof. Belytschko's webpage

Energy Partition – Method to Describe One Mechanical Response in One domain with Multiple Energy Functionals

Partitioned Incremental Energy Functional

$$\Phi[\bar{\varphi}, \tilde{\varphi}, \phi] = \Phi^{\text{int}}[\bar{\varphi}, \tilde{\varphi}] - \Phi^{\text{ext}}[\bar{\varphi}, \tilde{\varphi}] + \Lambda[\bar{\varphi}, \tilde{\varphi}, \phi]$$

Partitioned internal
energy

Partitioned
external energy

Compatibility
constraint energy

Partition of Unity for energy functional

$$\alpha(\mathbf{X}) = \beta(\mathbf{X}) = \begin{cases} 1 & \mathbf{X} \in \bar{\mathcal{B}} \setminus \mathcal{B}^c \\ 0 & \mathbf{X} \in \tilde{\mathcal{B}} \setminus \mathcal{B}^c \end{cases}$$

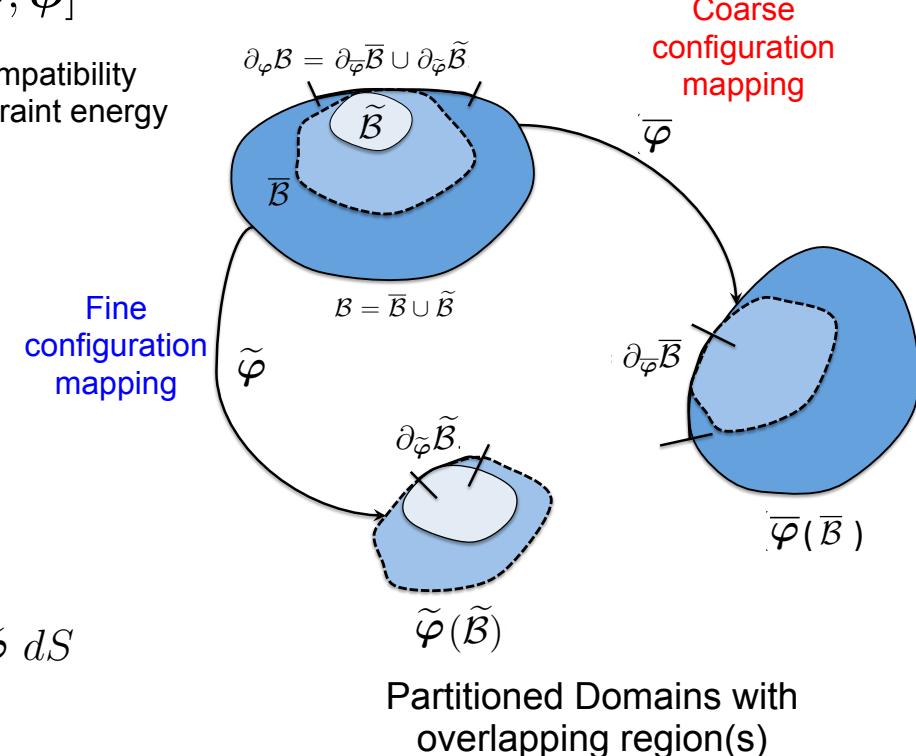
$$\Phi^{\text{int}}[\bar{\varphi}, \tilde{\varphi}] = \int_{\mathcal{B}} \alpha \bar{W}(\bar{\mathbf{F}}, \bar{\mathbf{Z}}) + (1 - \alpha) \tilde{W}(\tilde{\mathbf{F}}, \tilde{\mathbf{Z}}) \, dV$$

$$\begin{aligned} \Phi^{\text{ext}}[\bar{\varphi}, \tilde{\varphi}] = & \int_{\mathcal{B}} \beta \bar{\mathbf{B}} \cdot \bar{\varphi} + (1 - \beta) \tilde{\mathbf{B}} \cdot \tilde{\varphi} \, dV \\ & + \int_{\partial_T \mathcal{B}} \beta \bar{\mathbf{T}} \cdot \bar{\varphi} \, dS + \int_{\partial_T \mathcal{B}} (1 - \beta) \tilde{\mathbf{T}} \cdot \tilde{\varphi} \, dS \end{aligned}$$

$$\Lambda[\bar{\varphi}, \tilde{\varphi}, \phi] = \int_{\mathcal{B}^c} \phi \cdot (\bar{\varphi} - \tilde{\varphi}) + \kappa l^2 \text{Grad } \bar{\phi} : (\text{Grad } \bar{\varphi} - \text{Grad } \tilde{\varphi}) \, dV$$

Minimizing displacement
difference

Minimizing displacement
gradient difference



Linearization System of Equations

- Incremental governing equation in matrix form

1. Tangent matrix not being positive definite
2. may have severe oscillation problems

$$\begin{bmatrix} \bar{\mathbf{K}}_{ab} & \mathbf{0}_{a\beta} \\ \mathbf{0}_{b\alpha} & \widetilde{\mathbf{K}}_{\alpha\beta} \\ \hline \bar{\mathbf{C}}_{ba} & -\widetilde{\mathbf{C}}_{\beta a} \end{bmatrix} \begin{bmatrix} \Delta \bar{\varphi}_b \\ \Delta \widetilde{\varphi}_\beta \\ \Delta \phi_b \end{bmatrix} = \begin{bmatrix} \bar{\mathbf{F}}_a \\ \widetilde{\mathbf{F}}_\alpha \\ \mathbf{0}_a \end{bmatrix}$$

- Tangent for implicit integration

Weighted coarse scale tangential stiffness matrix

$$\bar{\mathbf{K}}_{ab} := \int_{\mathcal{B}} \text{Grad } N_a : \alpha \bar{\mathbb{C}} : \text{Grad } N_b \, dV$$

Weighted fine scale tangential stiffness matrix

$$\widetilde{\mathbf{K}}_{\alpha\beta} := \int_{\mathcal{B}} \text{Grad } N_\alpha : (1 - \alpha) \widetilde{\mathbb{C}} : \text{Grad } N_\beta \, dV$$

Coarse Coupling term

$$\bar{\mathbf{C}}_{ab} := \bar{\mathbf{C}}_{ba} = \int_{\mathcal{B}^c} N_a N_b \cdot + \kappa l^2 \text{Grad } N_a : \text{Grad } N_b \, dV$$

Fine Coupling term

$$\widetilde{\mathbf{C}}_{\alpha b} := \int_{\mathcal{B}^c} \lambda_\alpha N_b + \kappa l^2 \text{Grad } \lambda_\alpha : \text{Grad } N_b \, dV$$

\uparrow
L₂ coupling

\uparrow
H¹ coupling

Question:

Can we cut cost by only regularizing constitutive law in fine domain?

Yes, if the fine and coarse energy functionals are blended “correctly”.

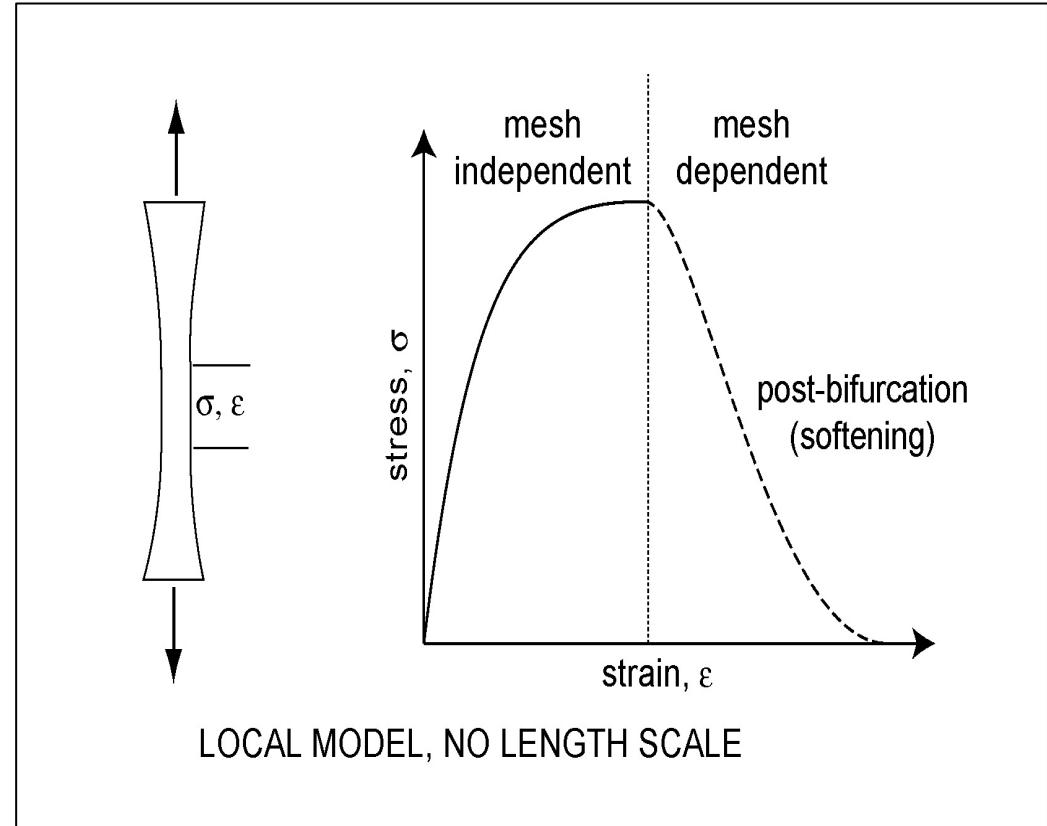


Figure from Bazant and Cedolin, 1991

Singular Bar Example

- Path dependent damage model

$$W(\mathbf{C}, \zeta) = (1 - \zeta)W_o(\mathbf{C})$$

- One dimensional simplification

$$W_o\left(\frac{d\varphi}{dX}\right) = \frac{1}{2}\bar{E}\left(\left(\frac{d\varphi}{dX}\right)^{-2} + \left(\frac{d\varphi}{dX}\right)^2 - 2\right)$$

$$\zeta = \zeta(q) = \zeta_\infty[1 - \exp(-q/\zeta)] \quad q(t) = \max_{s \in [0, t]} W_o(s)$$

- Nonlocal internal variable (damage) as localization limiter

$$\bar{q} = \frac{1}{\text{vol}(D)} \int_D q \, dV = \frac{1}{\text{vol}(D)} \int_{L_D} q A(X) \, dX$$

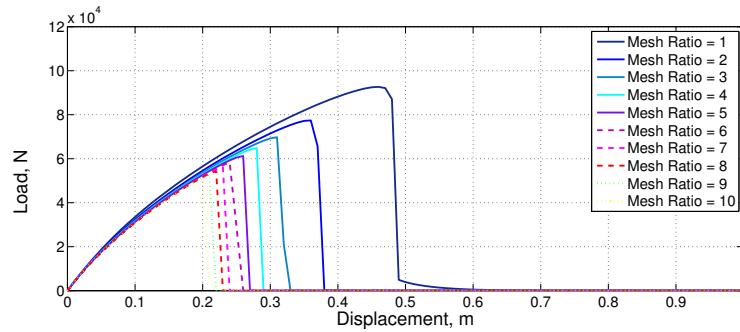
Singular tip  D=0

Prescribed displacement   D=D

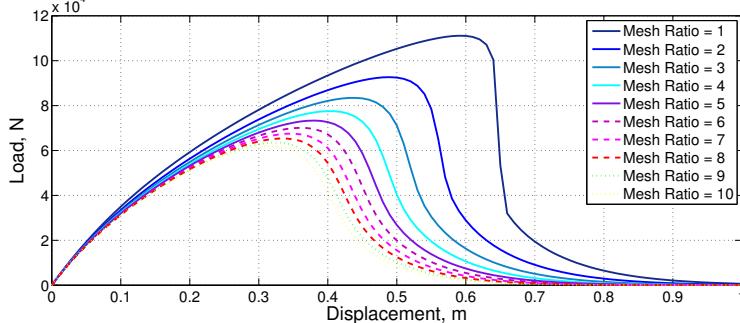
Responses from Nonlocal/local Continuum Coupled Model

- With DOC method, only a small nonlocal domain is required to regularize the PDE. A large portion of the domain is modeled by simpler, cheaper constitutive law with coarser mesh to cut down computational cost.

Without regularization

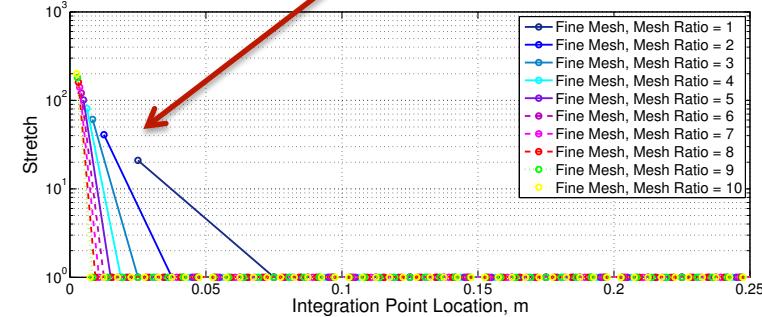


With regularization

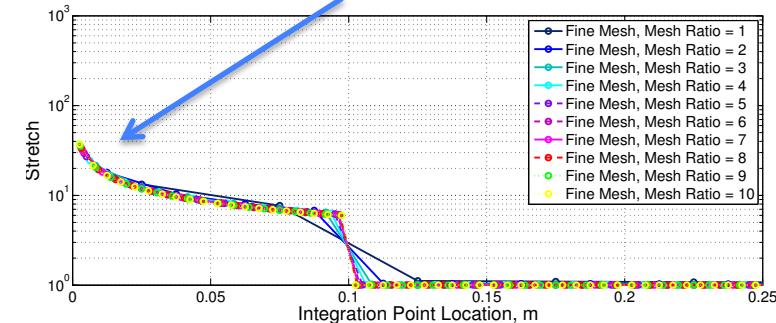


Load-displacement curve

Mesh dependent



Converged solution

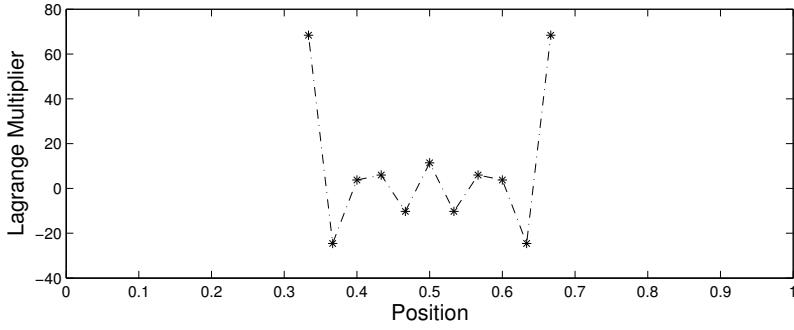
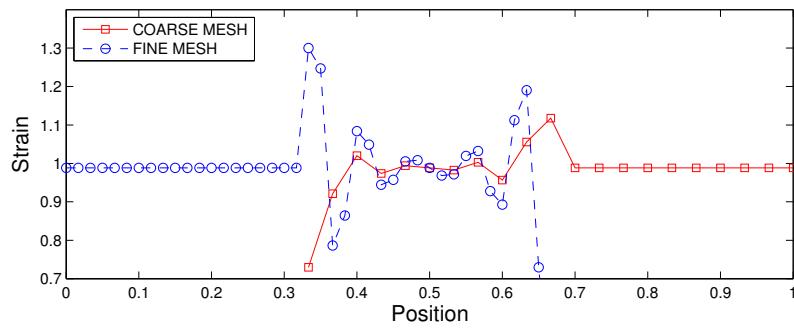
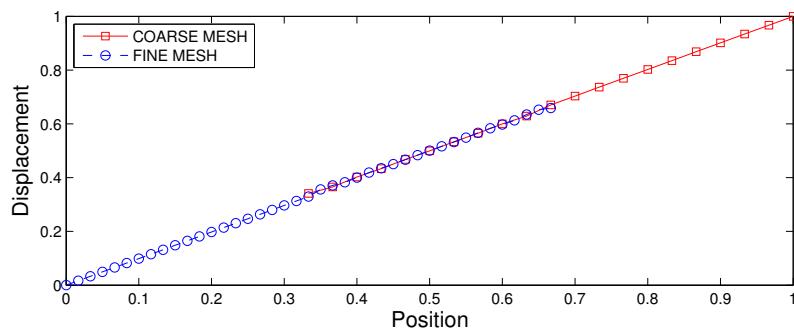


Stretch vs. reference location

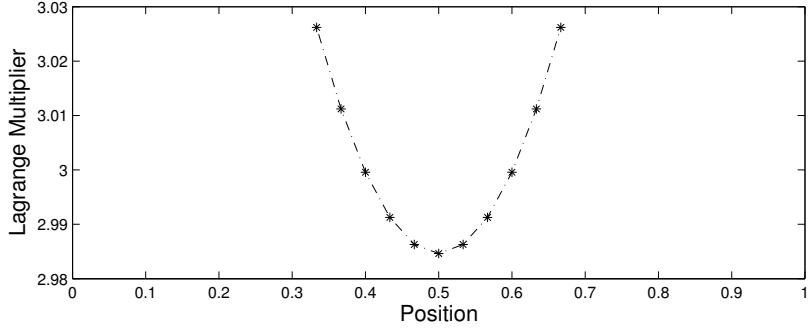
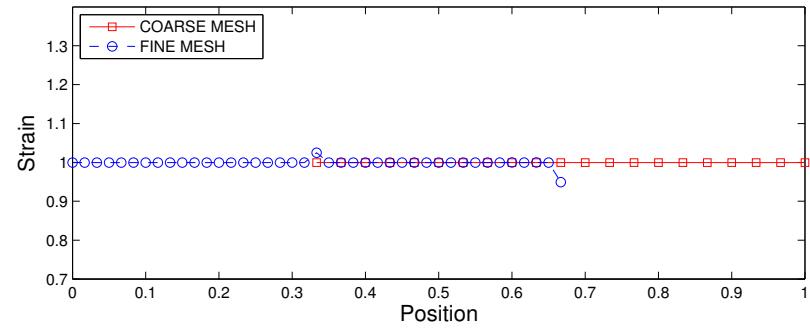
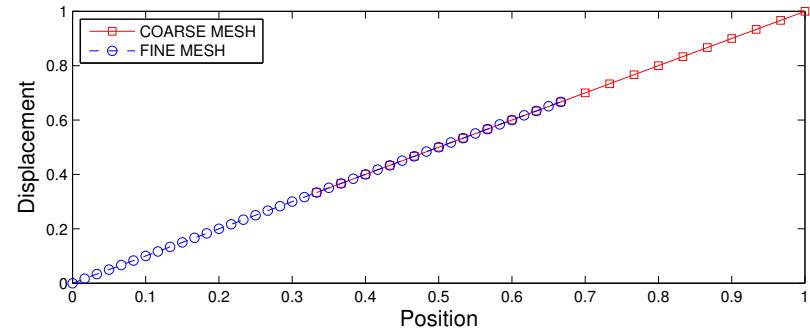
Question:

Under what circumstance will the coupling method fails?

1D Patch Test (constant weighting function)

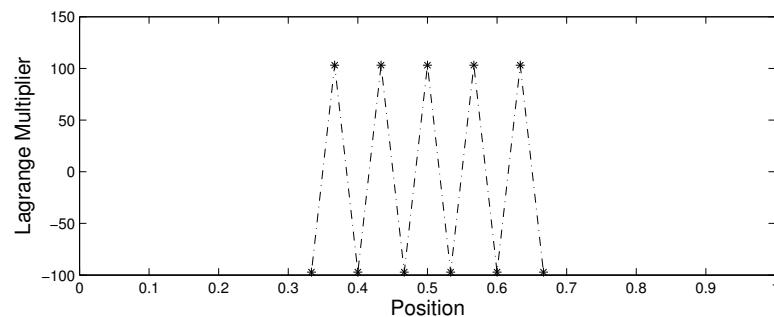
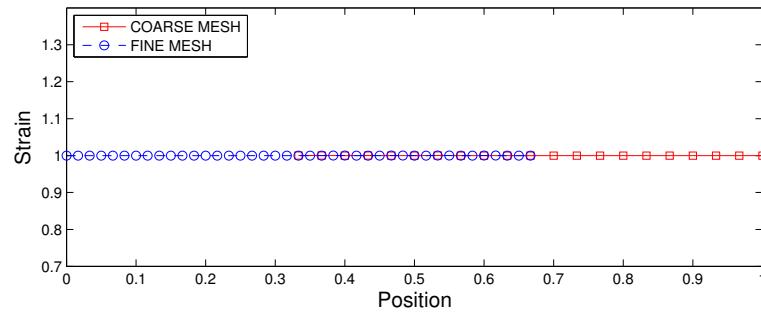
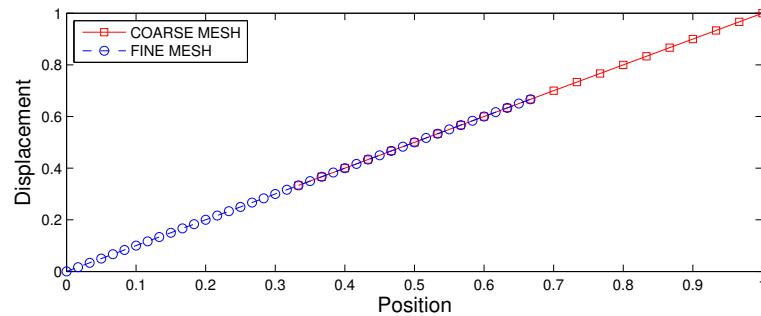


L_2 coupling

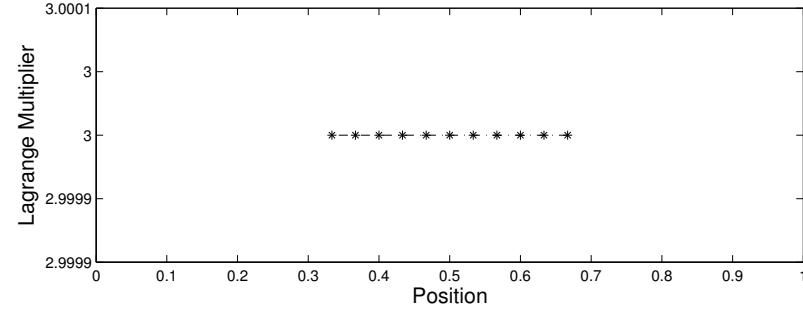
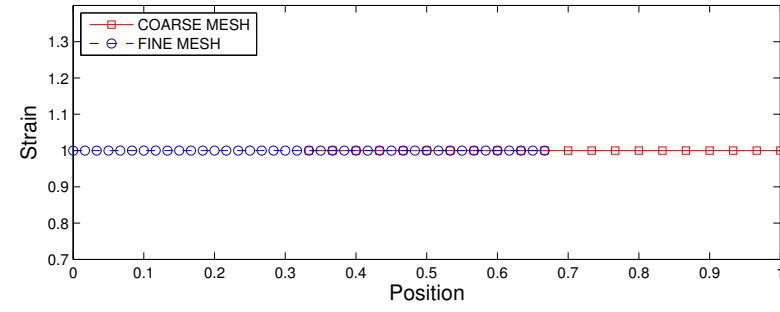
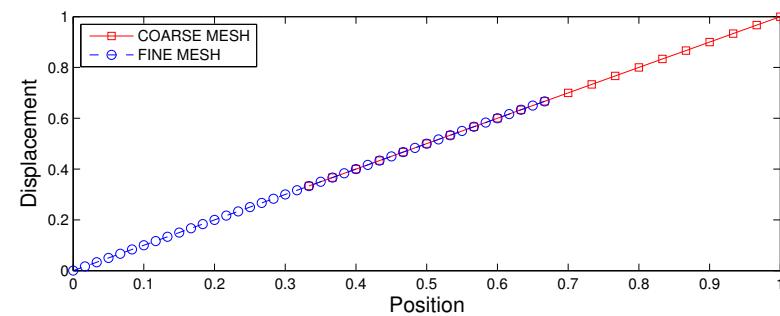


H^1 coupling

1D Patch Test (linear weighting function)



L_2 coupling



H^1 coupling

Question:

What are the major factor(s) that leads to stable coupling?

How can we analyze the numerical stability?

Inf-sup Condition

- Static condensation may reduce the three-field equations to two subsystems, each with two solution fields

$$\begin{bmatrix} \bar{\mathbf{K}}_{ab} & \mathbf{0}_{a\beta} & \bar{\mathbf{C}}_{ab} \\ \mathbf{0}_{b\alpha} & \bar{\mathbf{K}}_{\alpha\beta} & -\bar{\mathbf{C}}_{\alpha b} \\ \bar{\mathbf{C}}_{ba} & -\bar{\mathbf{C}}_{\beta a} & \mathbf{0}_{ab} \end{bmatrix} \begin{bmatrix} \Delta\bar{\boldsymbol{\varphi}}_b \\ \Delta\tilde{\boldsymbol{\varphi}}_\beta \\ \Delta\boldsymbol{\phi}_b \end{bmatrix} = \begin{bmatrix} \bar{\mathbf{F}}_a \\ \tilde{\mathbf{F}}_\alpha \\ \mathbf{0}_a \end{bmatrix}$$



$$\begin{bmatrix} \tilde{\mathbf{K}}_{\alpha\beta} & -\tilde{\mathbf{C}}_{a\beta} \\ -\tilde{\mathbf{C}}_{\beta a} & \bar{\mathbf{C}}_{am}\bar{\mathbf{K}}_{mn}^{-1}\bar{\mathbf{C}}_{nb} \end{bmatrix} \begin{bmatrix} \Delta\tilde{\boldsymbol{\varphi}}_\beta \\ \Delta\boldsymbol{\phi}_b \end{bmatrix} = \begin{bmatrix} \tilde{\mathbf{F}}_\alpha \\ -\bar{\mathbf{C}}_{am}\bar{\mathbf{K}}_{mb}^{-1}\bar{\mathbf{F}}_b \end{bmatrix}$$

$$\begin{bmatrix} \bar{\mathbf{K}}_{ab} & \bar{\mathbf{C}}_{ab} \\ \bar{\mathbf{C}}_{ba} & \tilde{\mathbf{C}}_{a\alpha}\tilde{\mathbf{K}}_{\alpha\beta}^{-1}\tilde{\mathbf{C}}_{\beta b} \end{bmatrix} \begin{bmatrix} \Delta\bar{\boldsymbol{\varphi}}_b \\ \Delta\boldsymbol{\phi}_b \end{bmatrix} = \begin{bmatrix} \bar{\mathbf{F}}_a \\ \tilde{\mathbf{C}}_{a\alpha}\tilde{\mathbf{K}}_{\alpha\beta}^{-1}\tilde{\mathbf{F}}_\beta \end{bmatrix}$$

- Inf-sup Condition can be used to test whether the spaces for the displacement increment and Lagrange multiplier are chosen correctly.

$$\inf_{\boldsymbol{\eta} \in V} \sup_{\bar{\boldsymbol{\xi}} \in \bar{U}} \frac{\int_{\mathcal{B}^c} \boldsymbol{\eta} \cdot \bar{\boldsymbol{\xi}} + \kappa l^2 \operatorname{Grad} \boldsymbol{\eta} : \operatorname{Grad} \bar{\boldsymbol{\xi}} \, dV}{\|\bar{\boldsymbol{\xi}}\|_{\bar{U}} \|\boldsymbol{\eta}\|_V} \geq \beta > 0$$

$$\inf_{\boldsymbol{\eta} \in V} \sup_{\tilde{\boldsymbol{\xi}} \in \tilde{U}} \frac{\int_{\mathcal{B}^c} \boldsymbol{\eta} \cdot \tilde{\boldsymbol{\xi}} + \kappa l^2 \operatorname{Grad} \boldsymbol{\eta} : \operatorname{Grad} \tilde{\boldsymbol{\xi}} \, dV}{\|\tilde{\boldsymbol{\xi}}\|_{\tilde{U}} \|\boldsymbol{\eta}\|_V} \geq \gamma > 0$$

Discrete Inf-sup Condition in Euclidean Space

- We can construct a one-to-one mapping from the function space to the Euclidean spaces such that the inf-sup condition (and the lack of) can be checked algebraically

$$\bar{\mathbf{v}} \equiv (\bar{v}_1, \bar{v}_2, \dots, \bar{v}_{N_{\bar{U}}}) \leftrightarrow \sum_{a=1}^{N_{\bar{U}}} \bar{v}_a N_a$$

$$\tilde{\mathbf{v}} \equiv (\tilde{v}_1, \tilde{v}_2, \dots, \tilde{v}_{N_{\tilde{U}}}) \leftrightarrow \sum_{\alpha=1}^{N_{\tilde{U}}} \tilde{v}_{\alpha} \lambda_{\alpha}$$

$$\boldsymbol{\theta} \equiv (\theta_1, \bar{\theta}_{12}, \dots, \bar{\theta}_{N_V}) \leftrightarrow \sum_{a=1}^{N_V} \theta_a N_a$$



$$\inf_{\boldsymbol{\theta} \in \mathbb{R}^{N_V}} \sup_{\bar{\mathbf{v}} \neq 0} \frac{\theta_a \bar{\mathbf{C}}_{ab} \bar{\mathbf{v}}_b}{(\boldsymbol{\theta}_m \bar{\mathbf{Q}}_{mn} \boldsymbol{\theta}_n)^{1/2} (\bar{\mathbf{v}}_r \bar{\mathbf{K}}_{rs} \bar{\mathbf{v}}_s)^{1/2}} \geq \beta > 0$$

$$\inf_{\boldsymbol{\theta} \in \mathbb{R}^{N_V}} \sup_{\tilde{\mathbf{v}} \neq 0} \frac{\theta_a \tilde{\mathbf{C}}_{ab} \tilde{\mathbf{v}}_b}{(\boldsymbol{\theta}_m \bar{\mathbf{Q}}_{mn} \boldsymbol{\theta}_n)^{1/2} (\tilde{\mathbf{v}}_r \widetilde{\mathbf{K}}_{rs} \tilde{\mathbf{v}}_s)^{1/2}} \geq \gamma > 0$$



$$(\bar{\mathbf{C}}_{am} \bar{\mathbf{K}}_{mn}^{-1} \bar{\mathbf{C}}_{nb} - \bar{\lambda} \bar{\mathbf{Q}}_{ab}) \boldsymbol{\theta}_b = \mathbf{0}$$

$$(\widetilde{\mathbf{C}}_{a\alpha} \widetilde{\mathbf{K}}_{\alpha\beta}^{-1} \widetilde{\mathbf{C}}_{\beta b} - \widetilde{\lambda} \widetilde{\mathbf{Q}}_{ab}) \boldsymbol{\theta}_b = \mathbf{0}$$

Now, inf-sup condition is guaranteed if the smallest eigenvalue is larger than 0!

Inf-sup Tests on Foulk's Singular Bar (2008)

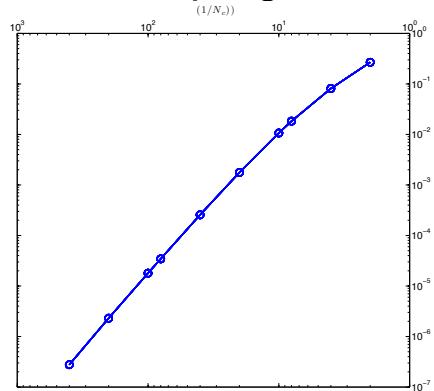
Inf-sup Test 1

$$(\bar{\mathbf{C}}_{am} \bar{\mathbf{K}}_{mn}^{-1} \bar{\mathbf{C}}_{nb} - \bar{\lambda} \bar{\mathbf{Q}}_{ab}) \boldsymbol{\theta}_b = \mathbf{0}$$

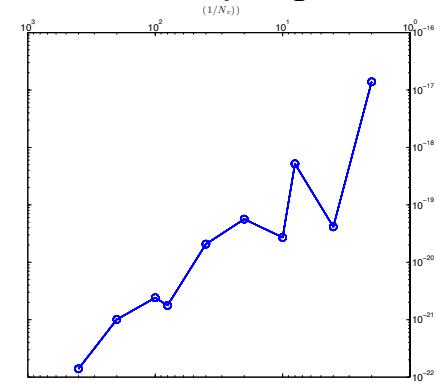
which is corresponding to

$$\begin{bmatrix} \bar{\mathbf{K}}_{ab} & \bar{\mathbf{C}}_{ab} \\ \bar{\mathbf{C}}_{ba} & \mathbf{C}'_{a\alpha} \bar{\mathbf{K}}'_{\alpha\beta}^{-1} \mathbf{C}'_{\beta b} \end{bmatrix} \begin{bmatrix} \Delta \bar{\varphi}_b \\ \Delta \phi_b \end{bmatrix} = \begin{bmatrix} \bar{\mathbf{F}}_a \\ \mathbf{C}'_{a\alpha} \bar{\mathbf{K}}'_{\alpha\beta}^{-1} \bar{\mathbf{F}}'_{\beta} \end{bmatrix}$$

H1 Coupling



L2 Coupling



Inf-sup value = $[10^{-7}, 1]$

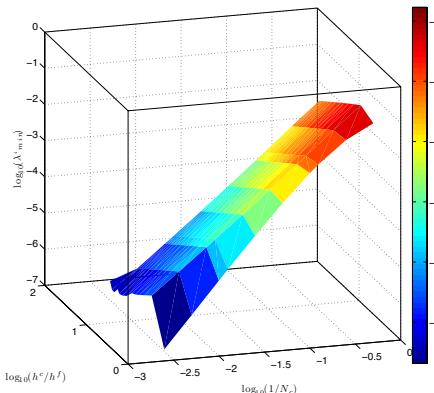
Inf-sup value = $[10^{-16}, 10^{-22}]$

Inf-sup Test 2

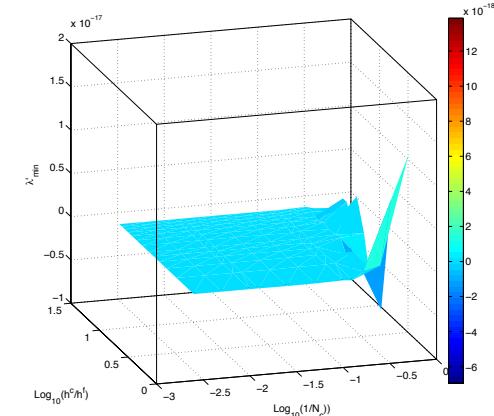
$$(\mathbf{C}'_{a\alpha} \bar{\mathbf{K}}'_{\alpha\beta}^{-1} \mathbf{C}'_{\beta b} - \lambda' \bar{\mathbf{Q}}_{ab}) \boldsymbol{\theta}_b = \mathbf{0}$$

which is corresponding to

$$\begin{bmatrix} \bar{\mathbf{K}}'_{\alpha\beta} & -\mathbf{C}'_{a\beta} \\ -\mathbf{C}'_{\beta a} & \bar{\mathbf{C}}_{am} \bar{\mathbf{K}}_{mn}^{-1} \bar{\mathbf{C}}_{nb} \end{bmatrix} \begin{bmatrix} \Delta \varphi'_{\beta} \\ \Delta \phi_b \end{bmatrix} = \begin{bmatrix} \bar{\mathbf{F}}'_{\alpha} \\ -\bar{\mathbf{C}}_{am} \bar{\mathbf{K}}_{mb}^{-1} \bar{\mathbf{F}}_b \end{bmatrix}$$



Inf-sup value = $[10^{-7}, 1]$



Inf-sup value = $[10^{-18}, 10^{-17}]$

Notice that the inf-sup values of both tests are very small for L2 coupling!

Conclusions

1. If coarse and fine meshes are conformal, then no search algorithm is needed and hence the implementation becomes much simpler.
2. Numerical study demonstrates that **H1 coupling eliminates spurious pattern** exhibiting in L2 coupling. This finding is consistent with the inf-sup condition.
3. **Localization limiter** can be used in a small domain via DOC coupling to enhance computational efficiency.
4. Numerical examples seem to suggest that **a more even partition** (e.g. half and half) or **higher order** weighted partition in energy leads to more stable and compatible solutions in overlapped domain.

On-going Challenges

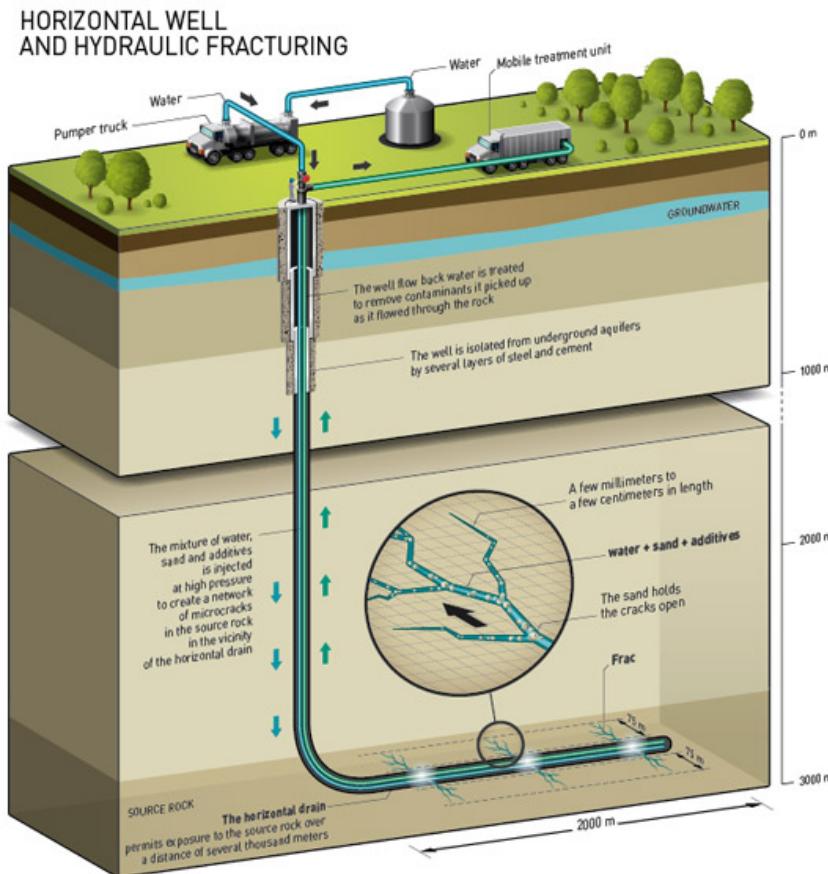
1. How thermal and pore-fluid diffusion affects mechanical stability, strain localization and fracture process at various **temporal** scales (e.g. undrained vs. drained, isothermal vs. adiabatic)?
2. How to homogenize or properly take account of the couplings of various physical processes at different **spatial** scales (e.g. anisotropy of diffusivity induced by deformation, yield surface size changes by heat) ?
3. How to model fully coupled thermo-hydro-mechanical–chemical (THMC) effect (biodegradation, Calcite formation, dissolving solid, **fate of fracking water**)?
4. Will fluid injection **mobilize existing fault systems**?
5. **Validations** of numerical modeling? Comparisons with other THM code, centrifuge models, field studies (In Salah Project)?



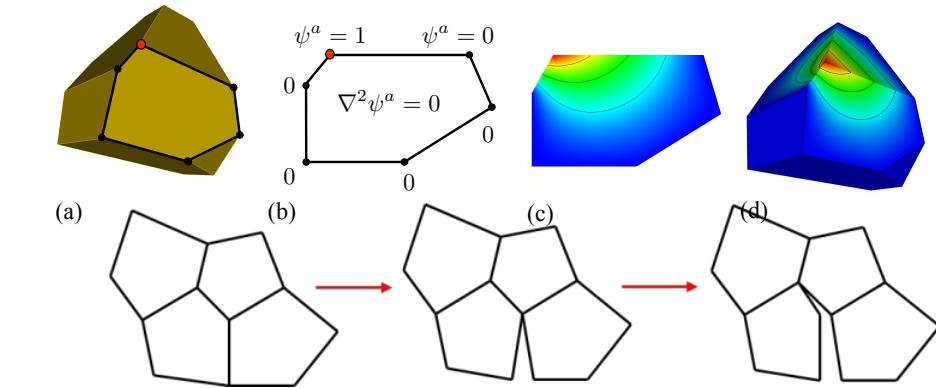
Figure from Columbia University



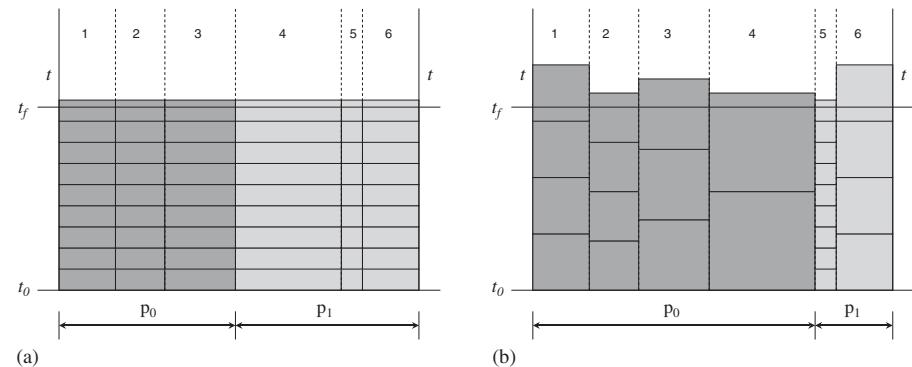
Future Works / Wish List



- Hydraulic Fracture
- injection induced seismic event
- Numerical modeling of rainfall instability



Usage of polyhedral finite element to eliminate mesh bias for fracture process (Bishop, IJNME, 2013)



Enabling asynchronous variational integrator for domain coupling methods (Lew, Marsden, Ortiz and West, 2003)

Thank you for your patience and time!

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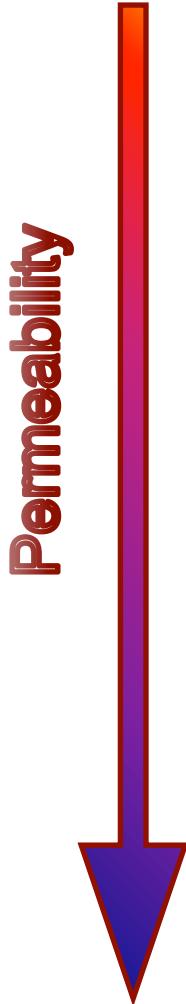
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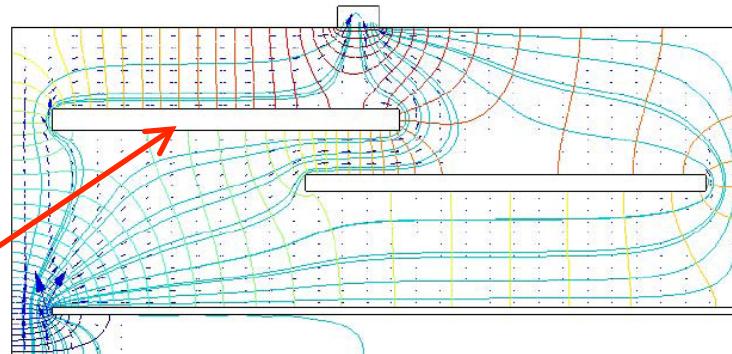
Extra Slides

Hydraulic property changes due to formation of deformation bands

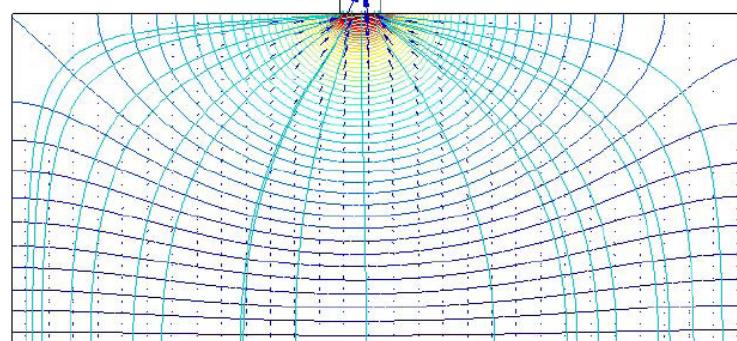


Compaction Band -flow barrier to fluid flow

Flow barriers trap fluid and improve efficiency of the storage

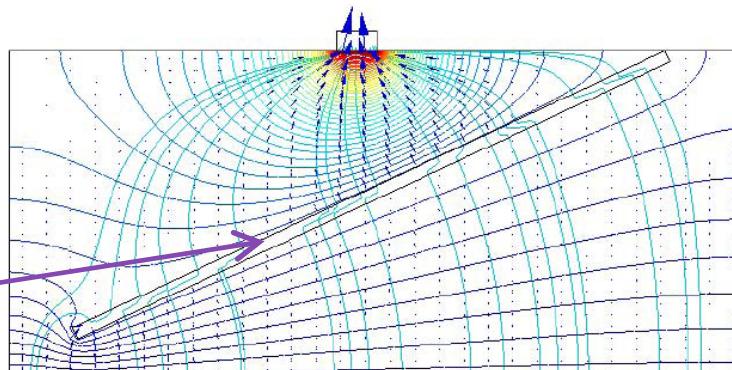


Uniform Compaction

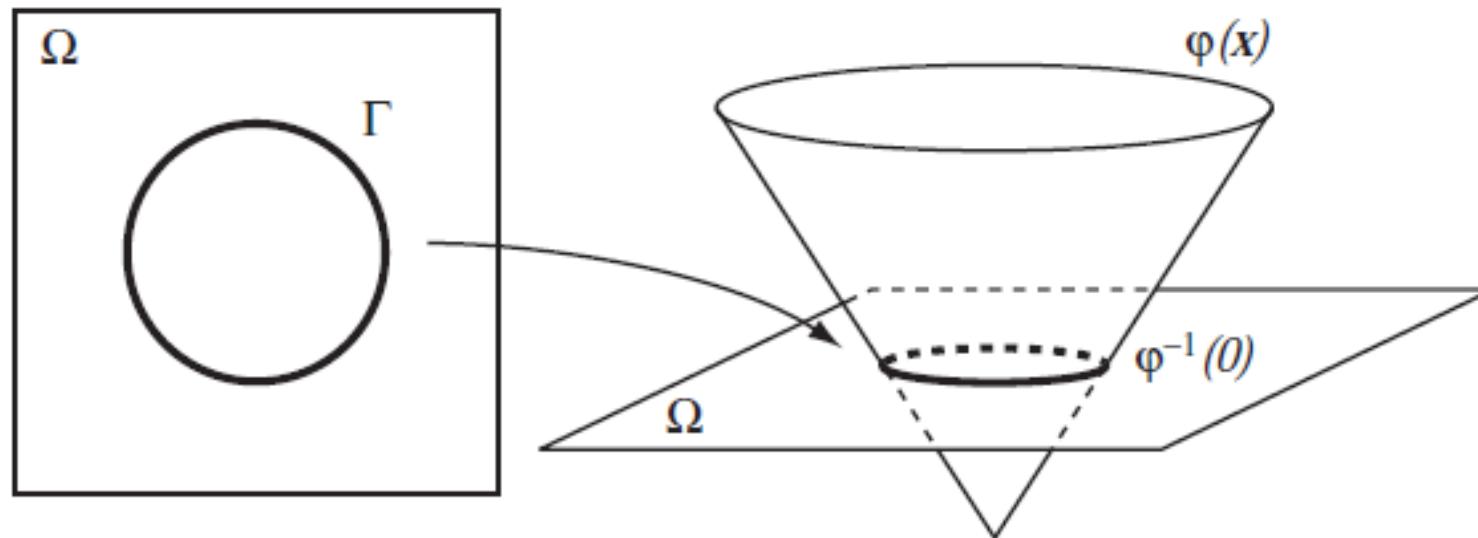


Dilatant Shear Band - flow conduits to fluid flow

Flow conduits may lead to leakage, which could be fatal.



Relation between Level Set Function and Medial Axis



- Local minimum of the signed distance function φ are located at medial axis.
- Use level set scheme to obtain signed distance function

Skeletonization with Level Set

- Action functional (Li et al 2005)

$$\mathcal{E}(\phi) = \mu \mathcal{P}(\phi) + \lambda \mathcal{L}_g(\phi) + \nu \mathcal{A}_g(\phi)$$

Diffusion term to convert level set into signed distance function

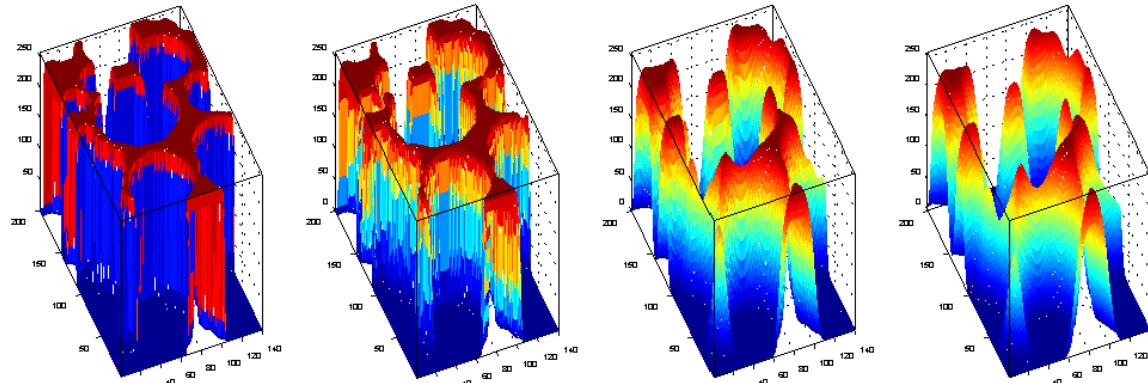
- Governing equation

$$\frac{\partial \phi}{\partial t} = \mu [\Delta^x \phi - \nabla^x \cdot \left(\frac{\nabla^x \phi}{|\nabla^x \phi|} \right)] + \lambda \delta(\phi) \nabla^x \cdot \left(g \frac{\nabla^x \phi}{|\nabla^x \phi|} \right) + \nu g \delta(\phi)$$

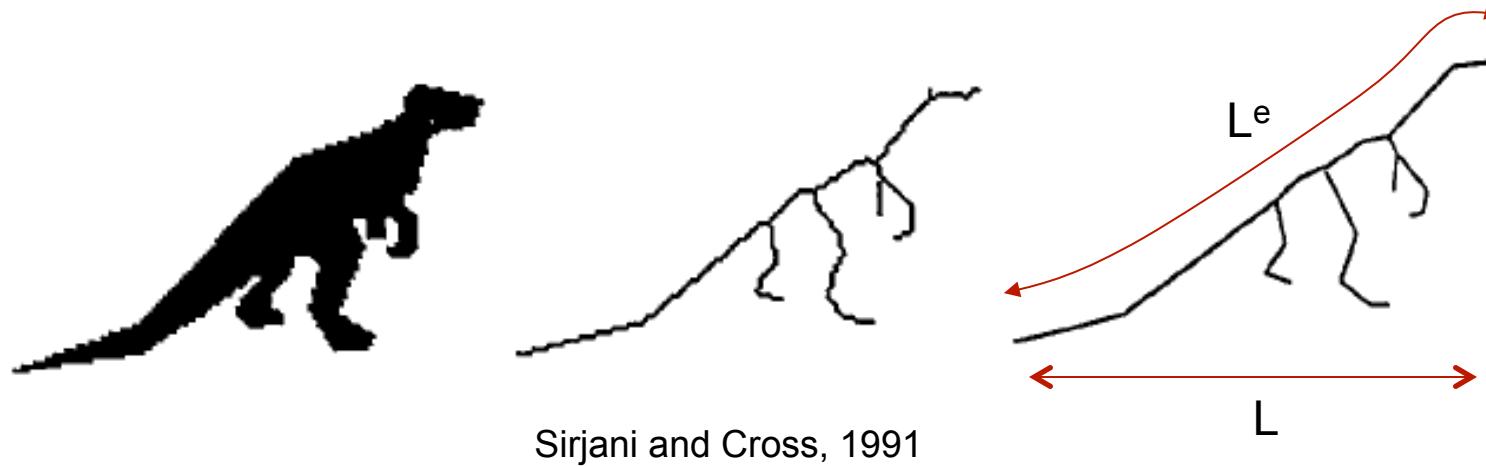
Fix level set value at the boundary

- Semi-Implicit Finite Difference Scheme

$$\frac{\phi^{n+1} - \phi^n}{t_{n+1} - t_n} = \mu \Delta^c \phi^{n+1} - \mu \nabla^c \cdot \frac{\nabla^c \phi}{\|\nabla^c \phi\|} + \lambda \delta(\phi^n) \nabla^c \cdot \left(g \frac{\nabla^c \phi}{\|\nabla^c \phi\|} \right) + \nu g \delta(\phi^n)$$

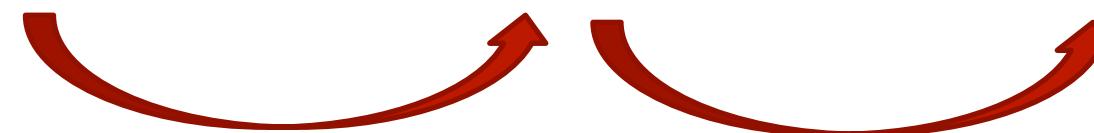
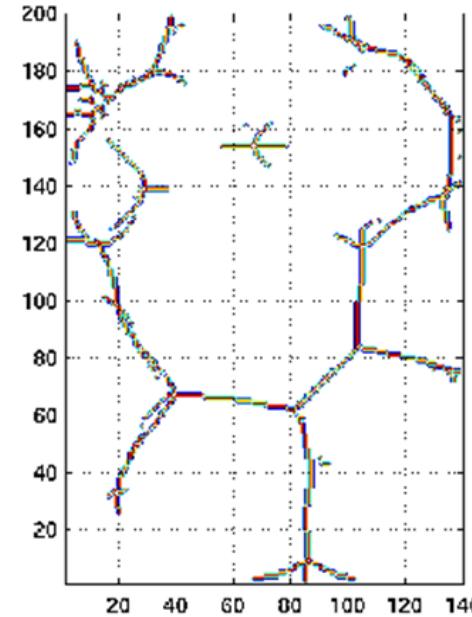
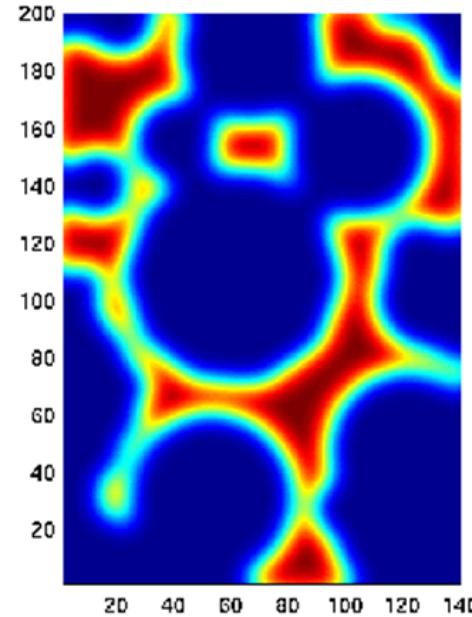
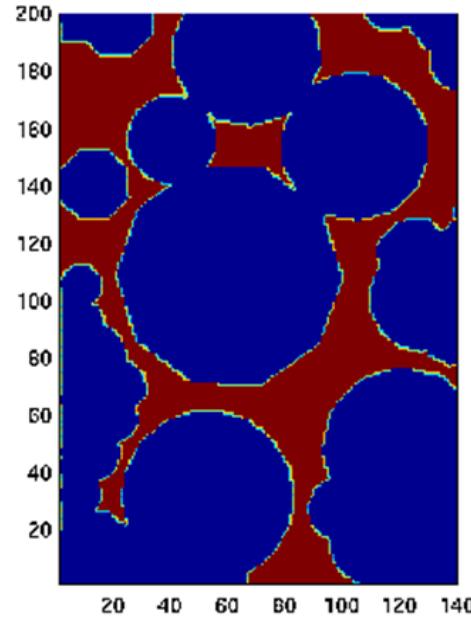


Medial Axis of Flow Paths



- Medial axis is the spine of a volume filling object
- Media axis is a union of curves that represent the topology and geometry of the volume

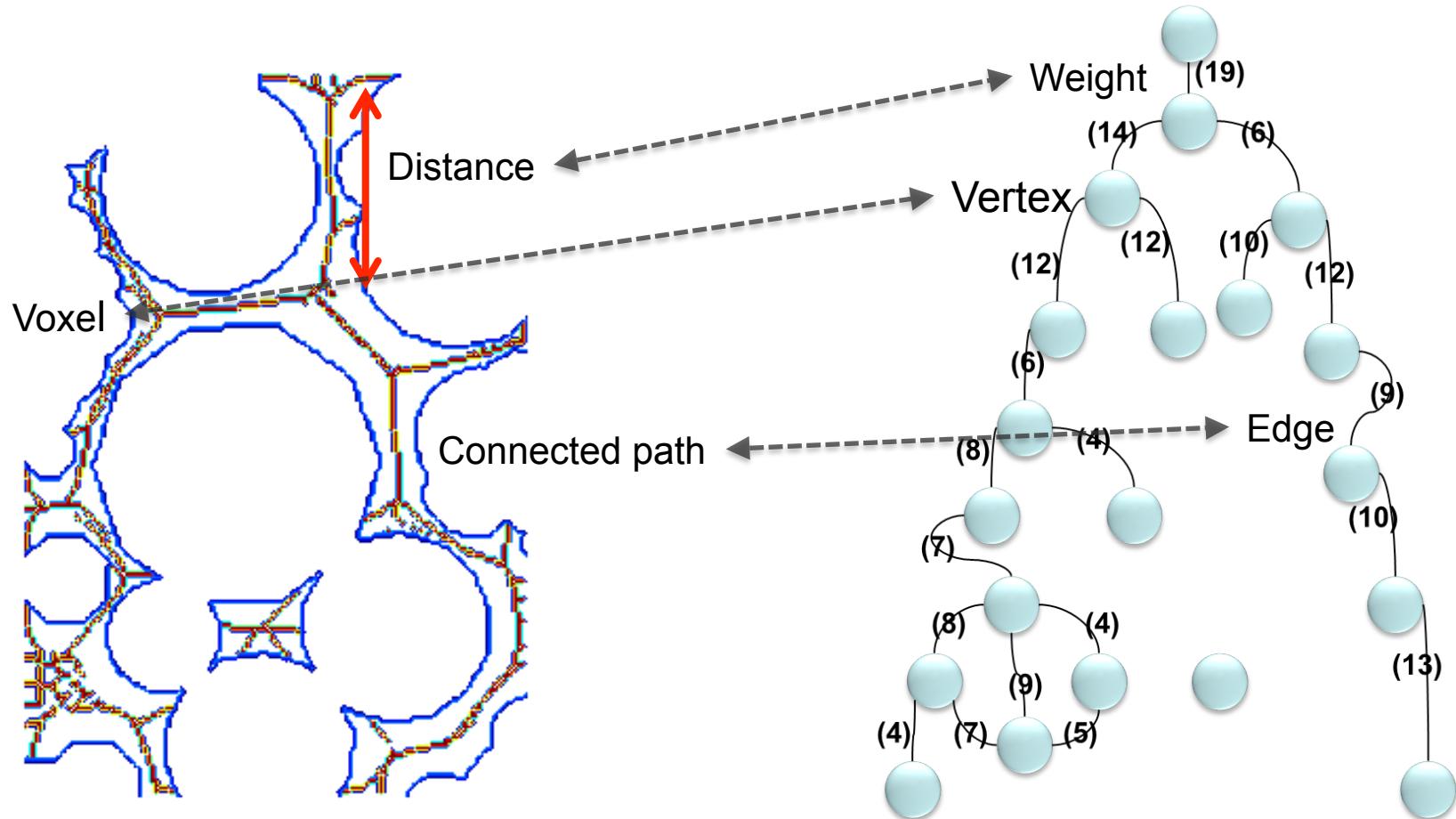
Locating Medial Axis of Flow Path via Level Set



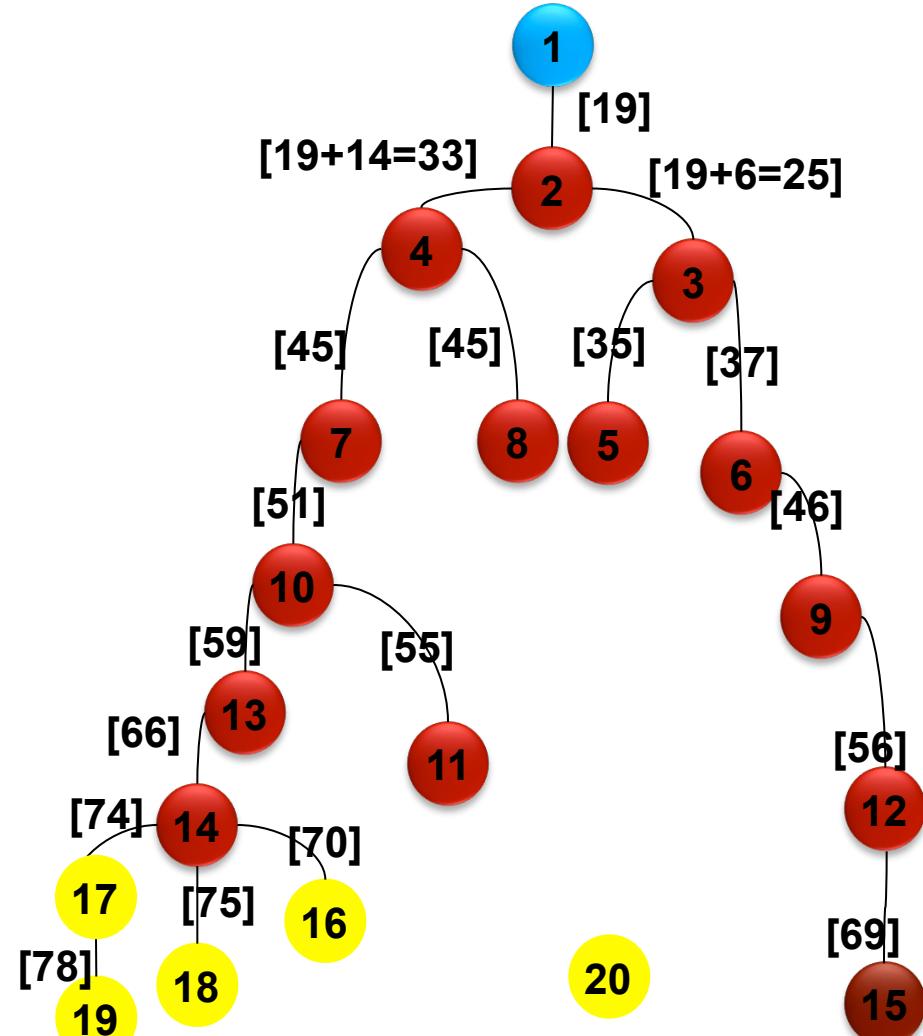
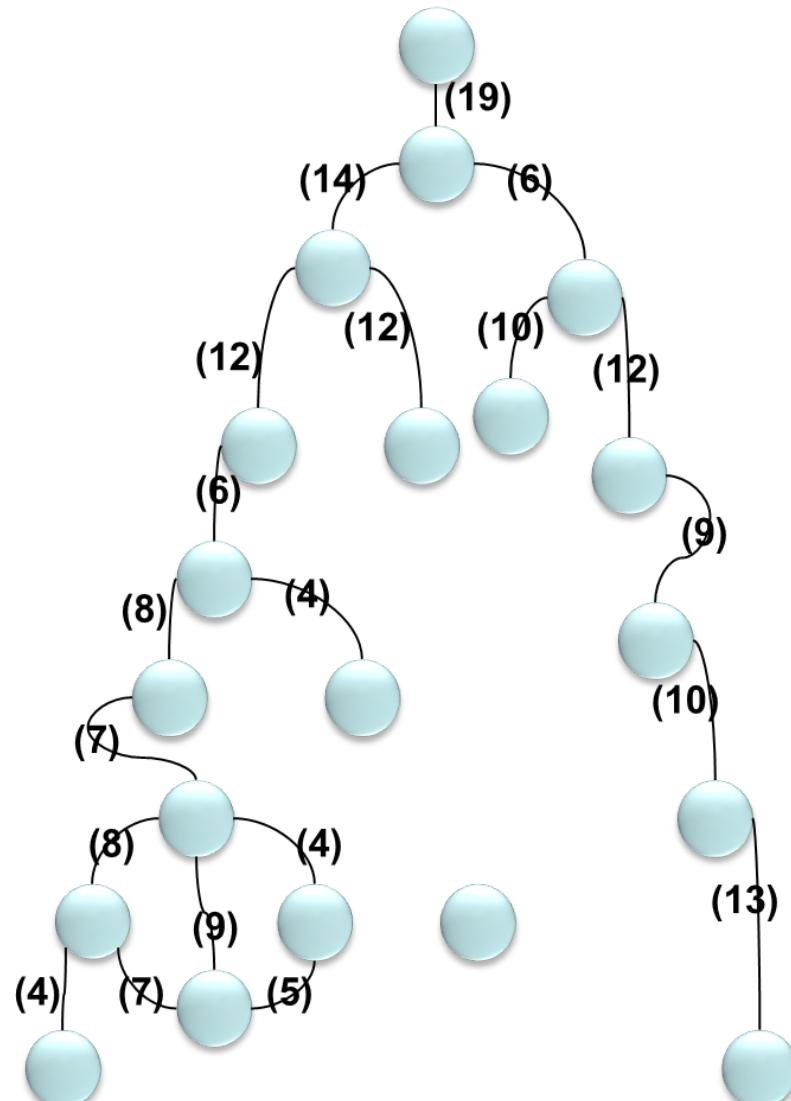
Convert binary
image into level
set via semi-
implicit scheme

Extract Local
minimum of level
set function

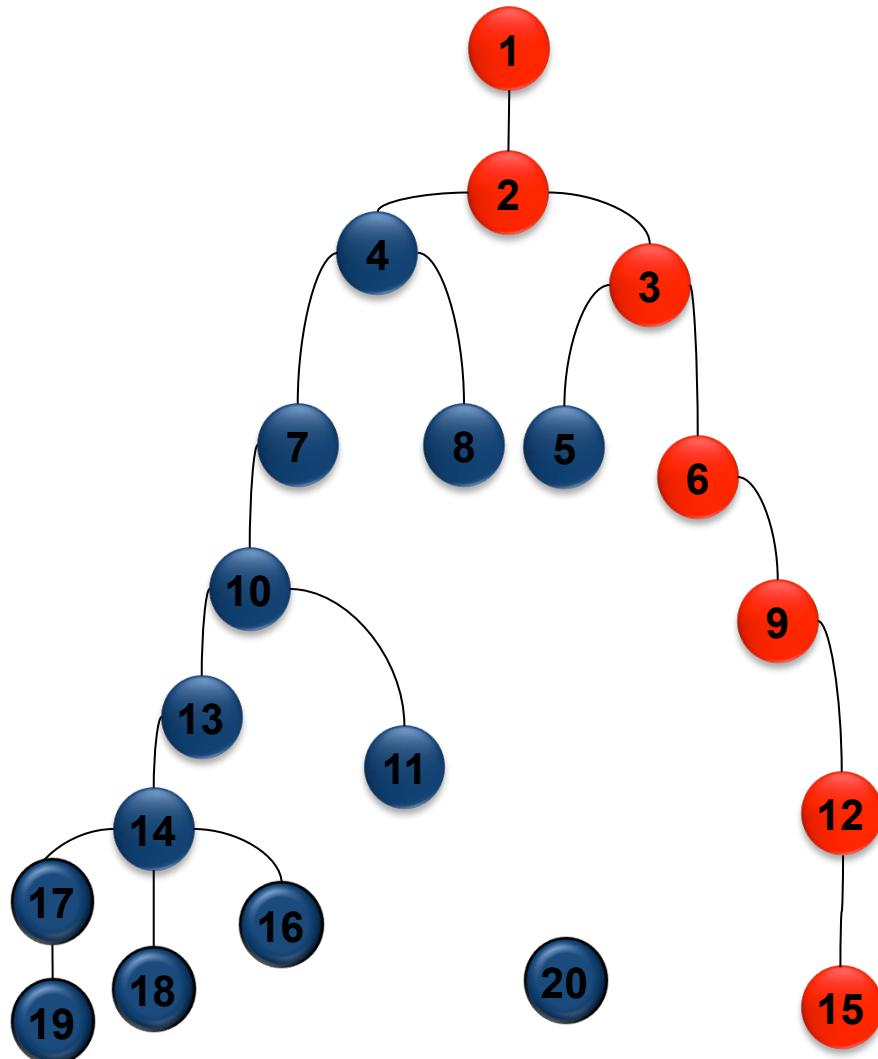
Pore Network and Weighed Graph



Shortest Path Searching Algorithm



Determining Connectivity with Recursive Functions



PROGRAM MAIN

1. Activate all vertices along the flow path as active nodes and mark them as visited vertices
2. While there exists at least one active node
3. call the recursive function MARKNEIGHBOR

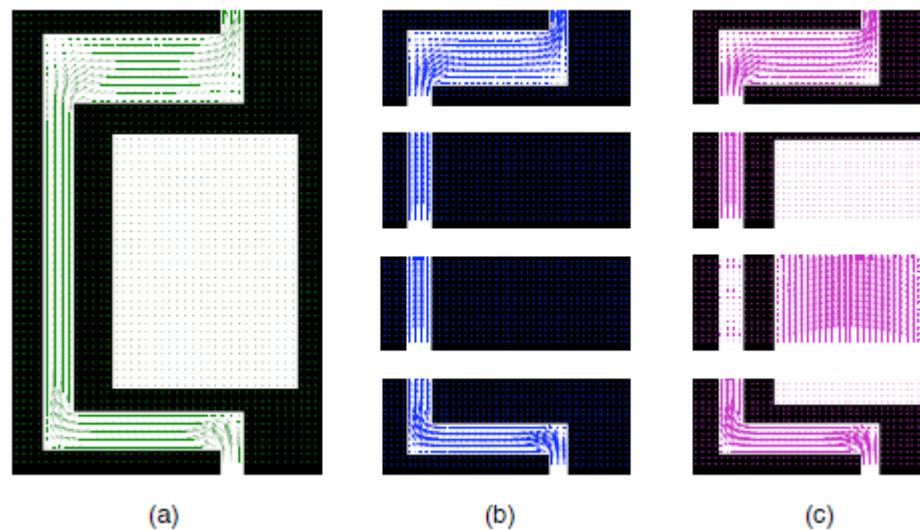
EXIT

FUNCTION MARKNEIGHBOR

1. IF at least one neighbors of the active nodes has not yet been visited
 1. Activate the unvisited neighbor vertices
 2. Mark them as visited vertices.
 3. Deactivate the old active nodes with unvisited neighbor(s).
 4. Call the recursive function MARKNEIGHBOR
2. ELSE
 1. Deactivate the active nodes with no unvisited neighbor.
3. EXIT



Partitions of Total and Connected Pores Space



Case	1	2	3
Number of Unit Cell(s)	1	4	4
Occluded Pore Identified?	No	Yes	No
Local Permeability, u^2 (top)	N/A	0.011	0.011
Local Permeability, u^2 (2nd top)	N/A	0.015	0.0029
Local Permeability, u^2 (2nd bottom)	N/A	0.015	0.45
Local Permeability, u^2 (bottom)	N/A	0.014	0.014
Global Permeability, u^2	0.015	0.013	0.0078
Relative Error	0	12 %	48 %

Size of Representative Elementary Volume

- Pore- to unit cell up-scaling criterion
(Energy Dissipation Rate)

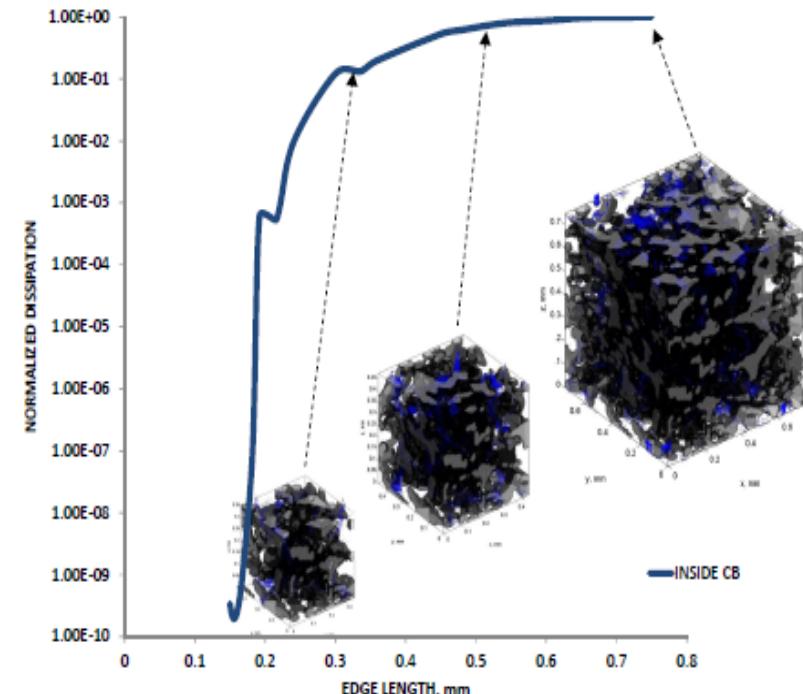
$$D^{micro} = 2\mu^v \epsilon : \epsilon; \quad \epsilon = \frac{1}{2}(\nabla^x v + (\nabla^x v)^T)$$

$$\frac{1}{\Omega^c} \int_{\Omega^c} D^{micro} d\Omega^c = \nabla^x p \cdot k^{meso} \cdot \nabla^x p$$

- Unit Cell- to specimen up-scaling criterion (Hill-Mandel Condition)

$$D^{macro} = \frac{1}{\Omega} \int_{\Omega} \nabla^x p \cdot v \, d\Omega = \frac{1}{\Omega} \int_{\Omega} \nabla^x p \, d\Omega \cdot \frac{1}{\Omega} \int_{\Omega} v \, d\Omega$$

$$D^{macro} = \frac{1}{\Omega} \int_{\Omega} \frac{1}{\mu^v} \nabla^x p \cdot k^{meso} \cdot \nabla^x p \, d\Omega = \frac{k_{zz}^{macro}}{\mu^v} \frac{(p_2 - p_1)^2}{(z_2 - z_1)^2}$$



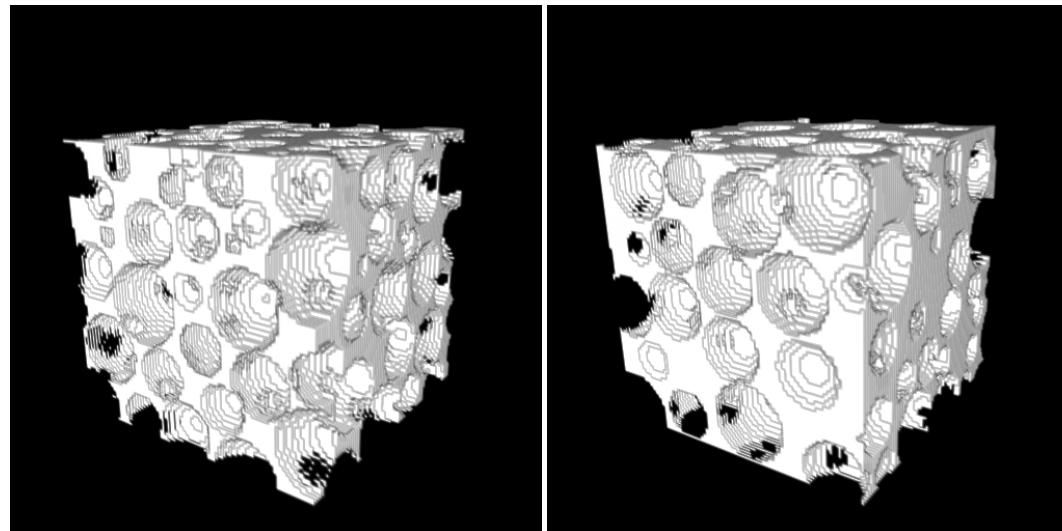
Specimen Type	Compaction Band	Outside Matrix
Volume Averaged Dissipation Rate, J/sec per $1m^3$	71.1	398.8
Dissipation Rate obtained from B.C. , J/sec per $1m^3$	69.8	383.3
Difference, J/sec	1.3	15.6
Homogenization Error e^{hom}	1.9 %	4.1 %

Obtain Pore Volume Binary Images from DEM Assemblies

Algorithm 1 Seed-fill (node, void-flag, solid-flag)

```

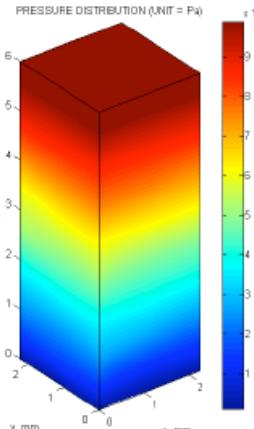
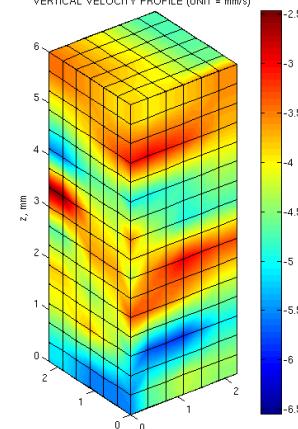
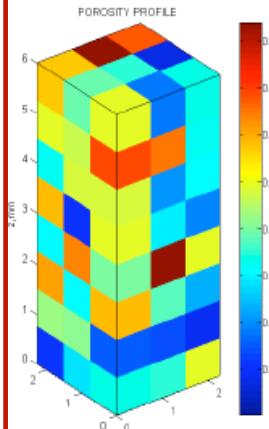
if the current node is not a void then
  return
else
  search the spherical particle closest to the current node
  Compute Euclidean distance between current node and the centroid of the closest spherical particle
  if Euclidean distance  $\neq$  radius of the closest spherical particle then
    Perform Flood-fill (the west neighbor of the current node, void-flag, solid-flag)
    Perform Flood-fill (the east neighbor of the current node, void-flag, solid-flag)
    Perform Flood-fill (the north neighbor of the current node, void-flag, solid-flag)
    Perform Flood-fill (the south neighbor of the current node, void-flag, solid-flag)
    Perform Flood-fill (the upper neighbor of the current node, void-flag, solid-flag)
    Perform Flood-fill (the lower neighbor of the current node, void-flag, solid-flag)
  end if
end if
  
```



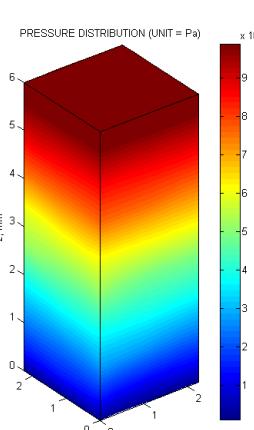
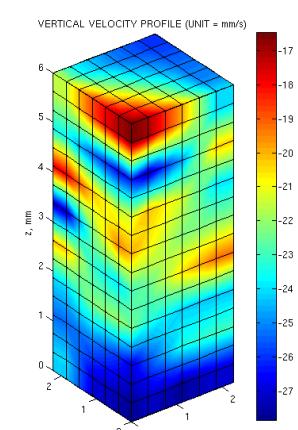
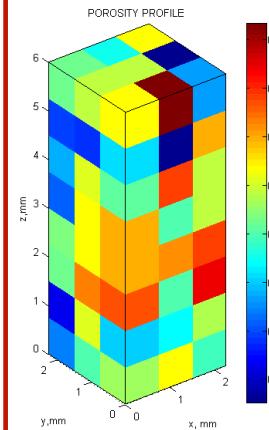
Binary images generated from DEM assemblies

Permeability of Aztec Sandstone Compaction Band Specimen

INSIDE CB
 $\phi = 0.14$



HOST MATRIX
 $\phi = 0.21$

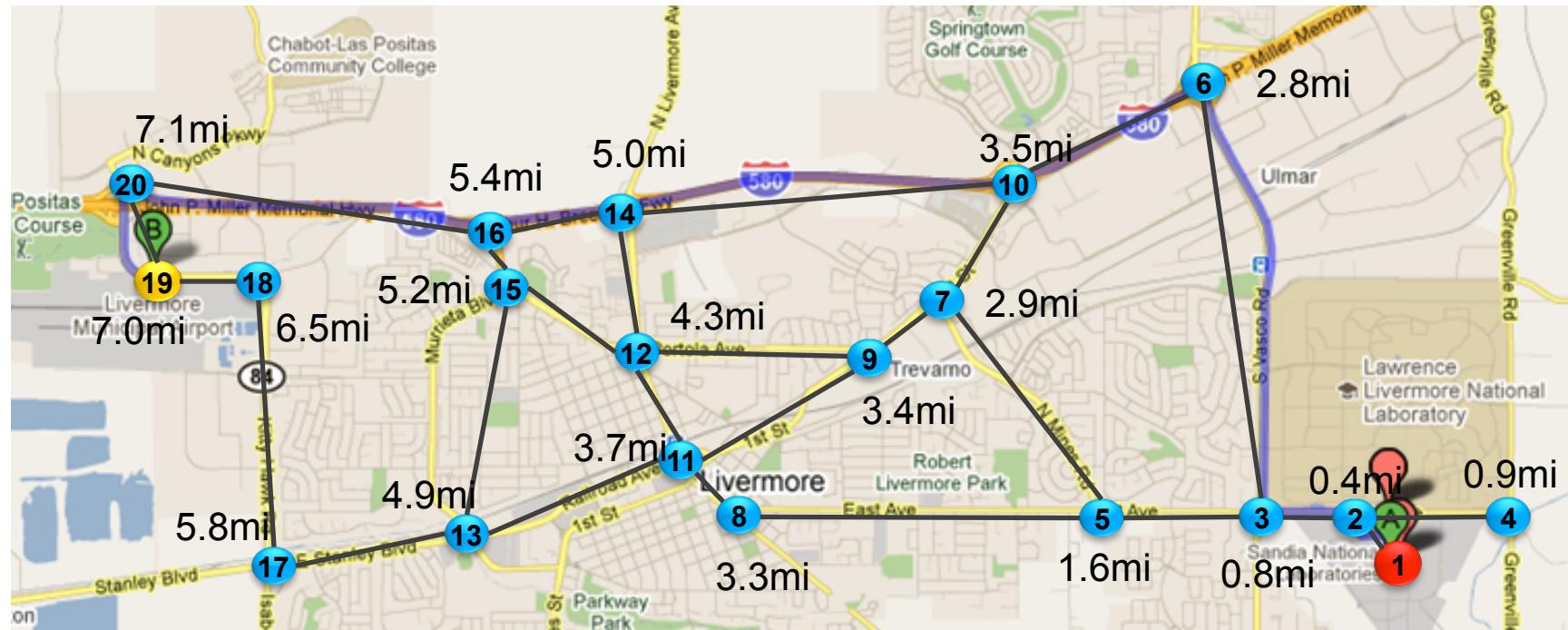


Effective Permeability

$2.5 \times 10^{-13} \text{ m}^2$

$1.4 \times 10^{-12} \text{ m}^2$

How to find the shortest path between the lab to Livermore airport?



Constitutive relations for solid deformation, flow and heat transport

Darcy's Law for deforming porous media

$$\frac{1}{\rho_f} \mathbf{W} = \mathbf{K} \cdot (-\nabla^{\mathbf{X}} p^f + \rho_f \mathbf{F}^T \cdot \mathbf{G})$$

$$\mathbf{Q} = (1/\rho_f) \mathbf{W}$$

$$\mathbf{K} = J \mathbf{F}^{-1} \cdot \mathbf{k} \cdot \mathbf{F}^{-T}$$

Fourier's law for deforming porous media

$$Q_{\theta} = -\mathbf{K}_{\theta} \cdot \nabla^{\mathbf{X}} \theta$$

$$\mathbf{K}_{\theta} = J \mathbf{F}^{-1} \cdot \mathbf{k}_{\theta} \cdot \mathbf{F}^{-T}$$

History dependent behavior of solid skeleton

$$\mathbf{P}(\mathbf{F}, \mathbf{z}, p^f, \theta) = \mathbf{P}'(\mathbf{F}, \mathbf{z}, \theta) - J B p^f \mathbf{F}^{-T}$$

$$\mathbf{P}' = \frac{\partial \psi(\mathbf{F}, \mathbf{z}, \theta)}{\partial \mathbf{F}}$$

$$\psi(\mathbf{F}, \mathbf{z}, \theta) = \hat{U}(J) + \hat{W}(\bar{\mathbf{b}}^e) + \hat{K}(\mathbf{z}) + \hat{M}(J, \theta)$$

Hyper-elastic

plastic
dissipation

Thermo-mechanical
coupling energy

Domain Overlapped Coupling Weak Form

Coarse
deformation
mapping

$$\begin{aligned}
 \bar{\mathbf{R}}_a[\bar{\boldsymbol{\varphi}}_h, \boldsymbol{\phi}_h] = & \int_{\mathcal{B}} \alpha \bar{\mathbf{P}} : \text{Grad } N_a \, dV - \beta \bar{\mathbf{B}} N_a \, dV \\
 & + \left(\int_{\mathcal{B}^c} N_a N_b \cdot + \kappa l^2 \text{Grad } N_a : \text{Grad } N_b \, dV \right) \boldsymbol{\phi}_b \\
 & - \int_{\partial_T \mathcal{B}} \beta \bar{\mathbf{T}} \cdot N_a \, dS
 \end{aligned}$$

Fine
deformation
mapping

$$\begin{aligned}
 \mathbf{R}'_\alpha[\boldsymbol{\varphi}'_h, \boldsymbol{\phi}_h] = & \int_{\mathcal{B}} (1 - \alpha) \mathbf{P}' : \text{Grad } \lambda_\alpha \, dV - (1 - \beta) \mathbf{B}' \lambda_\alpha \, dV \\
 & - \left(\int_{\mathcal{B}^c} \lambda_\alpha N_b + \kappa l^2 \text{Grad } \lambda_\alpha : \text{Grad } N_b \, dV \right) \boldsymbol{\phi}_b \\
 & - \int_{\partial_T \mathcal{B}} (1 - \beta) \mathbf{T}' \cdot \lambda_\alpha \, dS
 \end{aligned}$$

Lagrangian
Multiplier

$$\begin{aligned}
 \mathbf{R}_a^c[\bar{\boldsymbol{\varphi}}_h, \boldsymbol{\varphi}'_h] = & \left(\int_{\mathcal{B}^c} N_a N_b + \kappa l^2 \text{Grad } N_a : \text{Grad } N_b \, dV \right) \bar{\boldsymbol{\varphi}}_b \\
 & - \left(\int_{\mathcal{B}^c} N_a \lambda_\beta + \kappa l^2 \text{Grad } N_a : \text{Grad } \lambda_\beta \, dV \right) \boldsymbol{\varphi}'_\beta
 \end{aligned}$$

Optimal Stabilization Parameter Estimation

1D poromechanics governing equation

$$c \frac{\partial^2 \hat{p}}{\partial x^2} = \frac{\partial \hat{p}}{\partial t}, \quad c = \frac{k}{\mu} \frac{M' H}{H + \nu \beta M'}; M' = \frac{M(K + 4G/3)}{K + 4G/3 + B^2 M}$$

Three node stencil (standard Galerkin method)

$$\begin{aligned} & -\hat{p}_{A-1} + 2\hat{p}_A - \hat{p}_{A+1} \\ & + \frac{h^2}{6\vartheta c \Delta t} (\hat{p}_{A-1} + 4\hat{p}_A + \hat{p}_{A+1}) = 0 \end{aligned}$$

Three node stencil (Stabilized Galerkin method)

$$\begin{aligned} & -\hat{p}_{A-1} + 2\hat{p}_A - \hat{p}_{A+1} \\ & + \frac{h^2}{12\vartheta c \Delta t} [(2 - \gamma)\hat{p}_{A-1} \\ & + (8 + 2\gamma)\hat{p}_A + (2 - \gamma)\hat{p}_{A+1}] = 0 \end{aligned}$$

Growth/decay rate

$$\cosh \frac{h}{(\sqrt{\vartheta c \Delta t})^h} = \frac{(1 + h^2/\vartheta c \Delta t)(4 + \gamma)/6}{(1 - h^2/\vartheta c \Delta t)(2 - \gamma)/12}$$

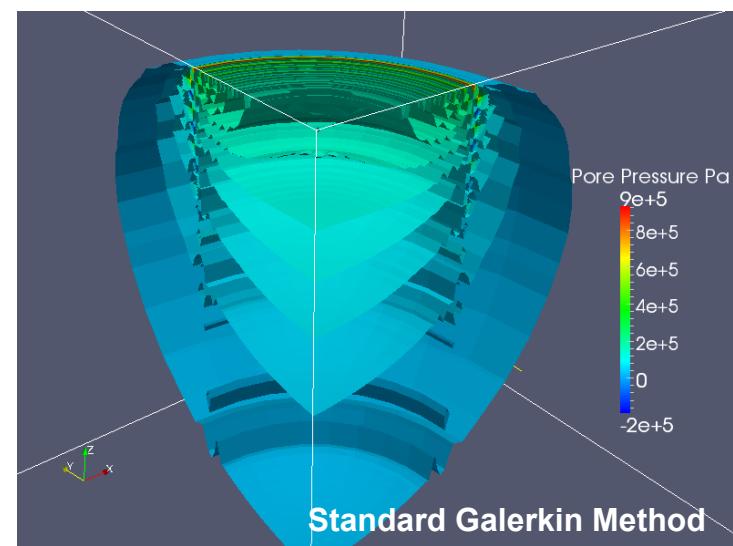
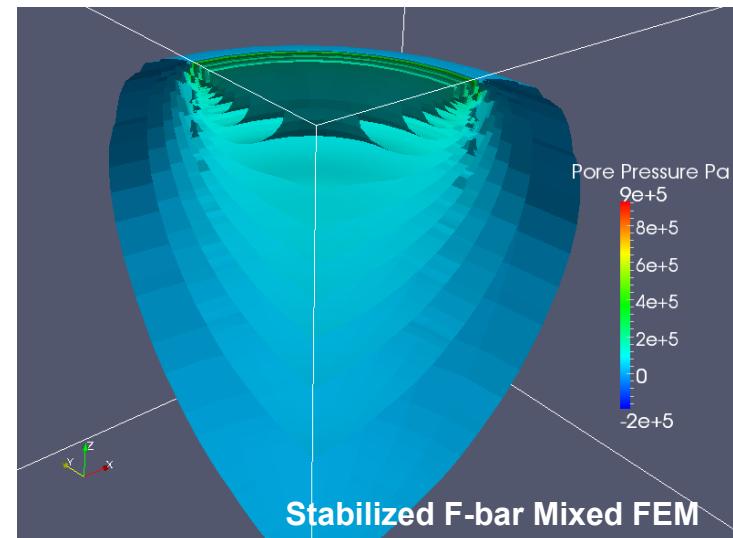
To have real growth/decay rate, we need

$$\gamma > 2 - 12 \frac{\vartheta c \Delta t}{h^2} > 0$$

$$\gamma = (2 - 6 \frac{\vartheta c \Delta t}{h^2}) \left(\frac{1}{2} + \frac{1}{2} \tanh(2 - 12 \frac{\vartheta c \Delta t}{h^2}) \right)$$

Safety factor

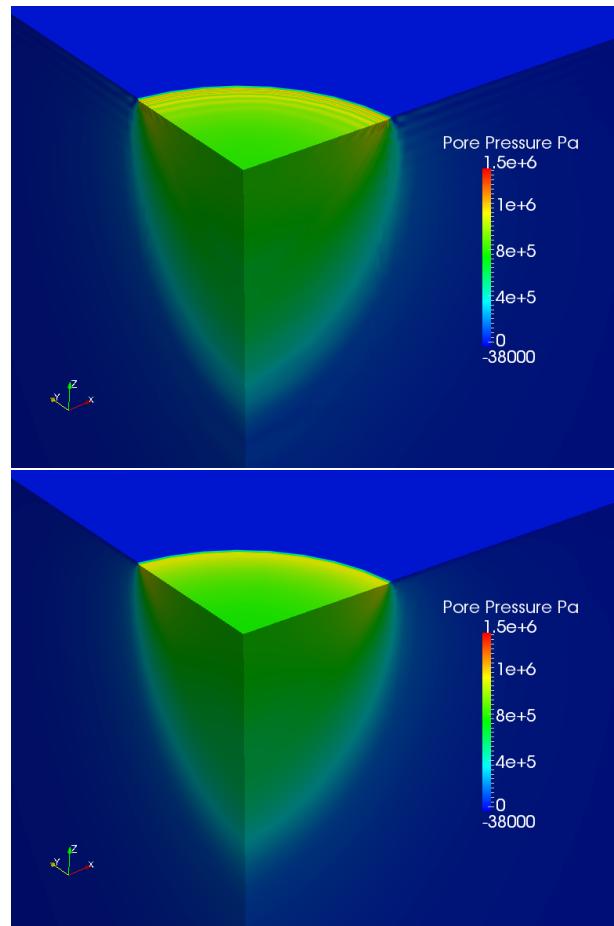
Turn off stabilization
without introducing switch



Footing placed on an elastoplastic porous medium

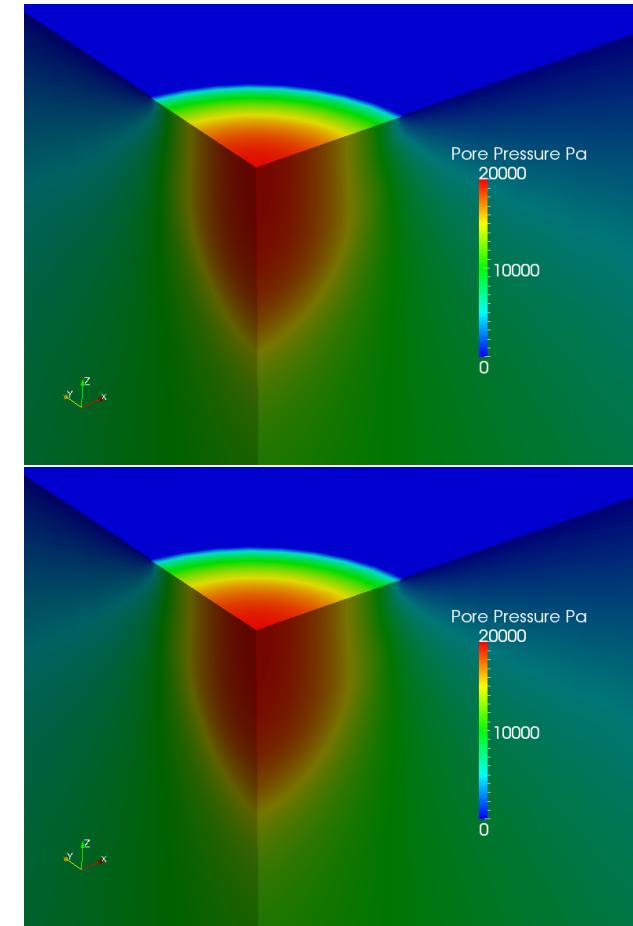
Without Stabilization

In low diffusivity case, the stabilization scheme is able to eliminate spurious oscillation.



With Stabilization

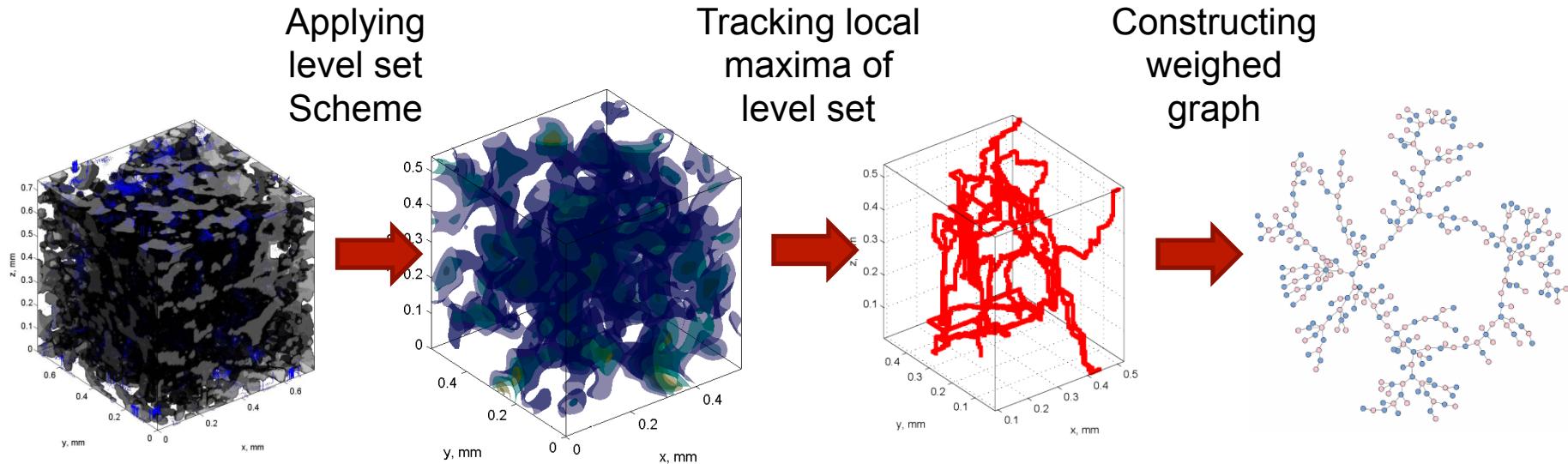
In high diffusivity case, the stabilization scheme does not introduce extra diffusivity that cause error.



Low diffusivity case

High diffusivity case

Application of Level Set Scheme and Graph Theory for geometrical analysis of tomographic images



See W.C Sun, J.E. Andrade, J.W. Rudnicki, A multiscale method for characterization of porous microstructures and their impact on macroscopic effective permeability, *International Journal of Numerical Methods in Engineering*, Vol. 88, No.12, 1260-1279, 2011.

Internal Variable Recovery via Lie Algebra

1. Application of Lie Algebra to interpolate internal variables within admissible space

Balance of Linear Momentum

$$D\Phi[\varphi, \bar{z}, \bar{y}](\boldsymbol{\eta}) = \int_B \mathbf{P} : \text{Grad } \boldsymbol{\eta} \, dV - \int_B \rho_0 \mathbf{B} \cdot \boldsymbol{\eta} \, dV - \int_{\partial TB} \mathbf{T} \cdot \boldsymbol{\eta} \, dS = 0$$

Variational Continuum Projection Operator

$$D\Phi[\varphi, \bar{z}, \bar{y}](\boldsymbol{\zeta}) = \int_B (\bar{y} - y) \cdot \boldsymbol{\zeta} \, dV = 0$$

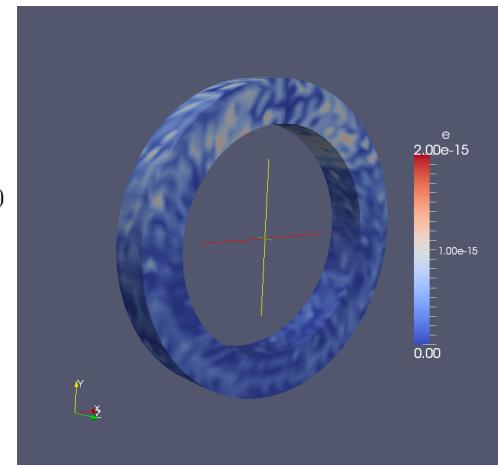
$$D\Phi[\varphi, \bar{z}, \bar{y}](\boldsymbol{\xi}) = \int_B (\bar{z} - z) \cdot \boldsymbol{\xi} \, dV = 0.$$

2. Application of Lie Algebra to interpolate internal variables within admissible space

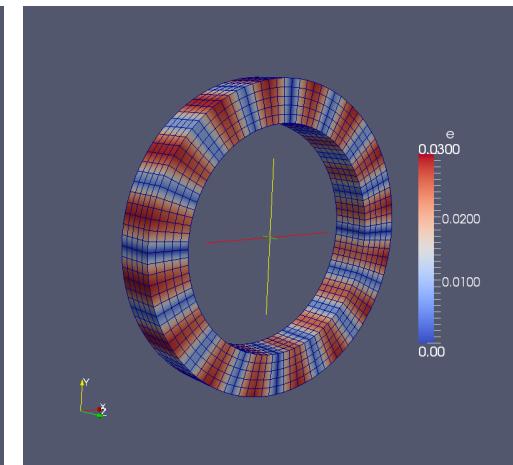
Logarithmic Mapping of Internal Variables

Polynomial Interpolation

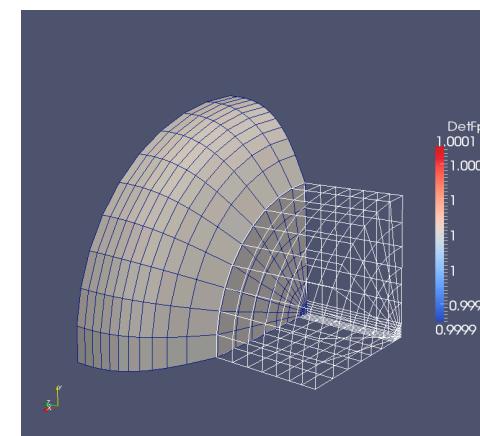
Exponential Mapping of Transformed Internal Variables



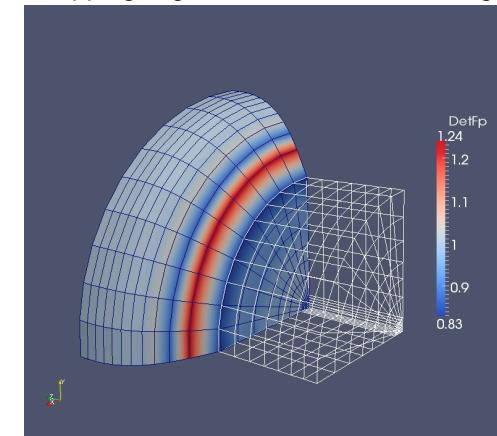
(a) Error of rotation is negligible if logarithmic mapping has been applied



(b) Error of rotation without logarithmic mapping is generated when remeshing

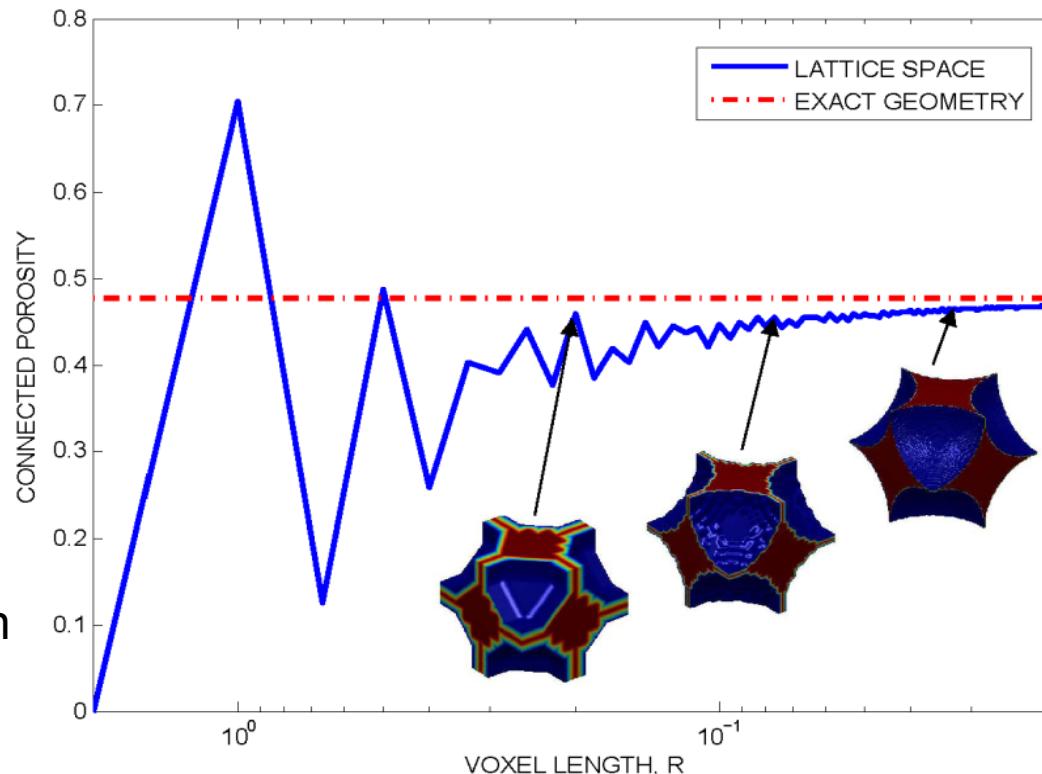


(a) Plastic split of deformation gradient remains isochoric when logarithmic mapping is applied for J2 plasticity model.



(b) Spurious plastic dilation/contraction generated due to improper linear mapping.

Resolution Issues for Conducting Lattice Boltzmann Simulation on Binary Images



- Low resolution
 - Faster and cheaper computation

- High resolution
 - Higher accuracy
 - Higher cost for computation



Size of Representative Elementary Volume

- Pore- to unit cell up-scaling criterion
(Energy Dissipation Rate)

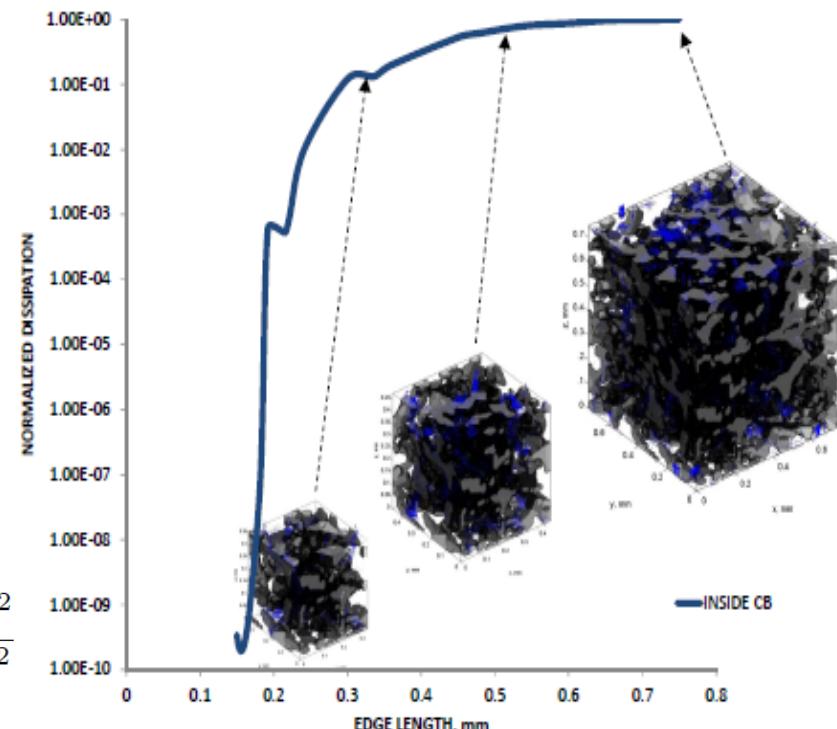
$$D^{micro} = 2\mu^v \epsilon : \epsilon; \quad \epsilon = \frac{1}{2}(\nabla^x v + (\nabla^x v)^T)$$

$$\frac{1}{\Omega^c} \int_{\Omega^c} D^{micro} d\Omega^c = \nabla^x p \cdot k^{meso} \cdot \nabla^x p$$

- Unit Cell- to specimen up-scaling criterion (Hill-Mandel Condition)

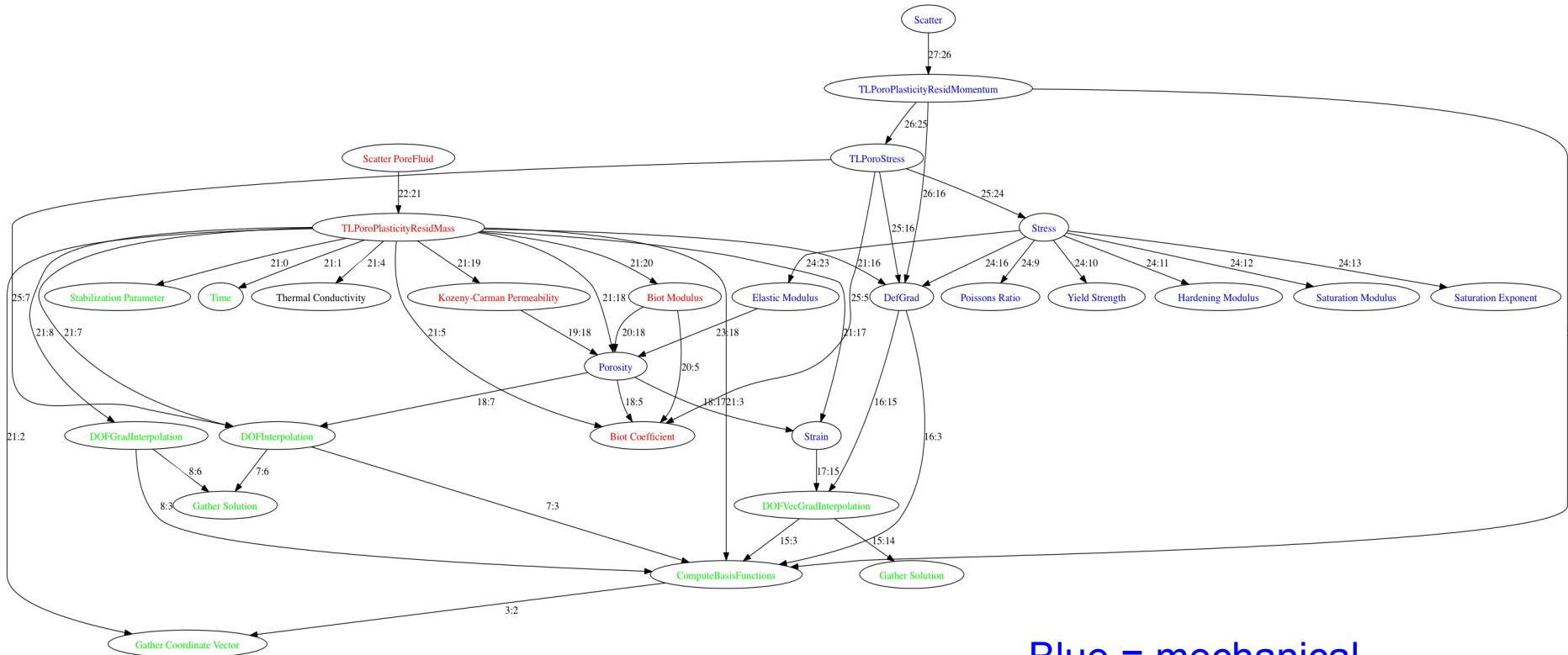
$$D^{macro} = \frac{1}{\Omega} \int_{\Omega} \nabla^x p \cdot v \, d\Omega = \frac{1}{\Omega} \int_{\Omega} \nabla^x p \, d\Omega \cdot \frac{1}{\Omega} \int_{\Omega} v \, d\Omega$$

$$D^{macro} = \frac{1}{\Omega} \int_{\Omega} \frac{1}{\mu^v} \nabla^x p \cdot k^{meso} \cdot \nabla^x p \, d\Omega = \frac{k^{macro}_{zz}}{\mu^v} \frac{(p_2 - p_1)^2}{(z_2 - z_1)^2}$$



Specimen Type	Compaction Band	Outside Matrix
Volume Averaged Dissipation Rate, J/sec per $1m^3$	71.1	398.8
Dissipation Rate obtained from B.C. , J/sec per $1m^3$	69.8	383.3
Difference, J/sec	1.3	15.6
Homogenization Error e^{hom}	1.9 %	4.1 %

Implementation of Total Lagrangian Poro-plasticity Problem



Blue = mechanical
 Red = pore-fluid diffusion
 Purple = coupled terms
 Green = General FEM

- Gather coordinates, displacement and pore-pressure fields
- Interpolate fields and gradients to integration points
- Chain together Evaluators to compute momentum and mass Residuals of the solid and pore-fluid constituents
- Apply LBB stabilizer
- Scatter back to the global system of equations

Hydrogen Transport

Stabilized hydrogen diffusion-deformation K-field problem for low diffusive materials

Balance of Linear Momentum

$$\nabla^X \cdot \mathbf{P}(\mathbf{F}, \bar{\mathbf{z}}, C_T) = \mathbf{0}$$

Concentration Sensitive Yield Function

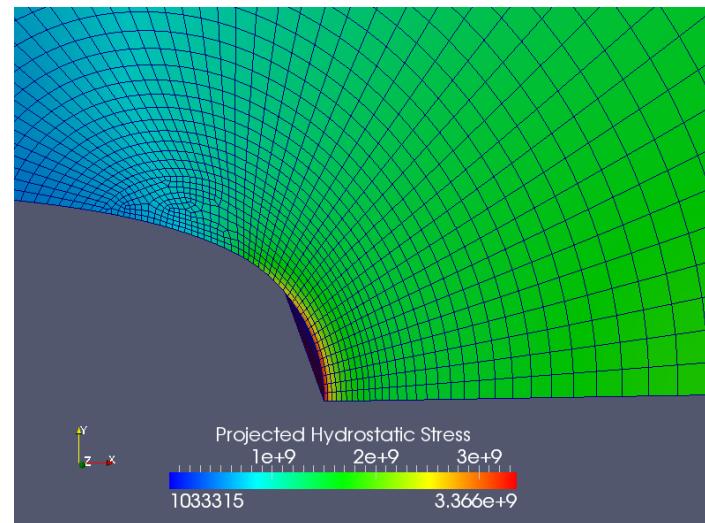
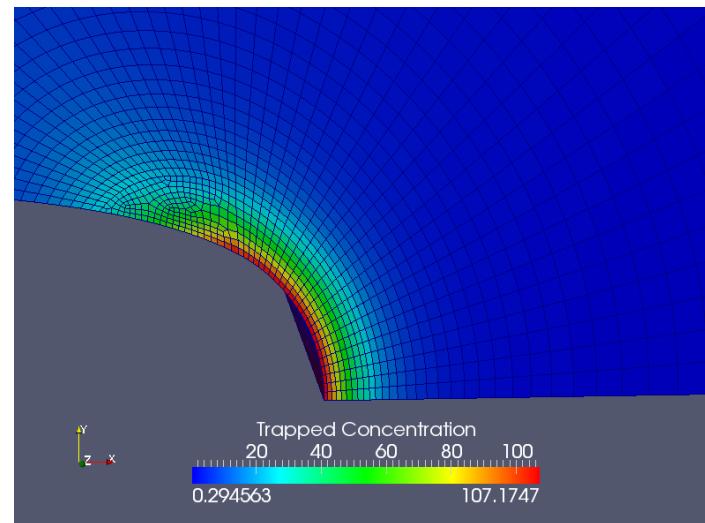
$$f(\boldsymbol{\tau}, \bar{\mathbf{z}}, C_T) = \|\text{dev}[\boldsymbol{\tau}]\| - \sqrt{\frac{2}{3}}[\sigma_Y(C_T) + K\alpha] \leq 0$$

Hydrogen Transport Theorem

$$D^* \dot{C}_L - \nabla^X \cdot \mathbf{D}_L \nabla^X C_L + \nabla^X \cdot \frac{V_H}{RT} C_L \mathbf{D}_L \nabla^X S_H + \theta_T \frac{dN_T}{d\epsilon_p} \dot{\epsilon}_p = 0$$

L2 Projection of Hydrostatic Stress

$$\int_{\mathcal{B}} N_a (\sigma_h - N_b \sigma_b) \, dV = 0$$

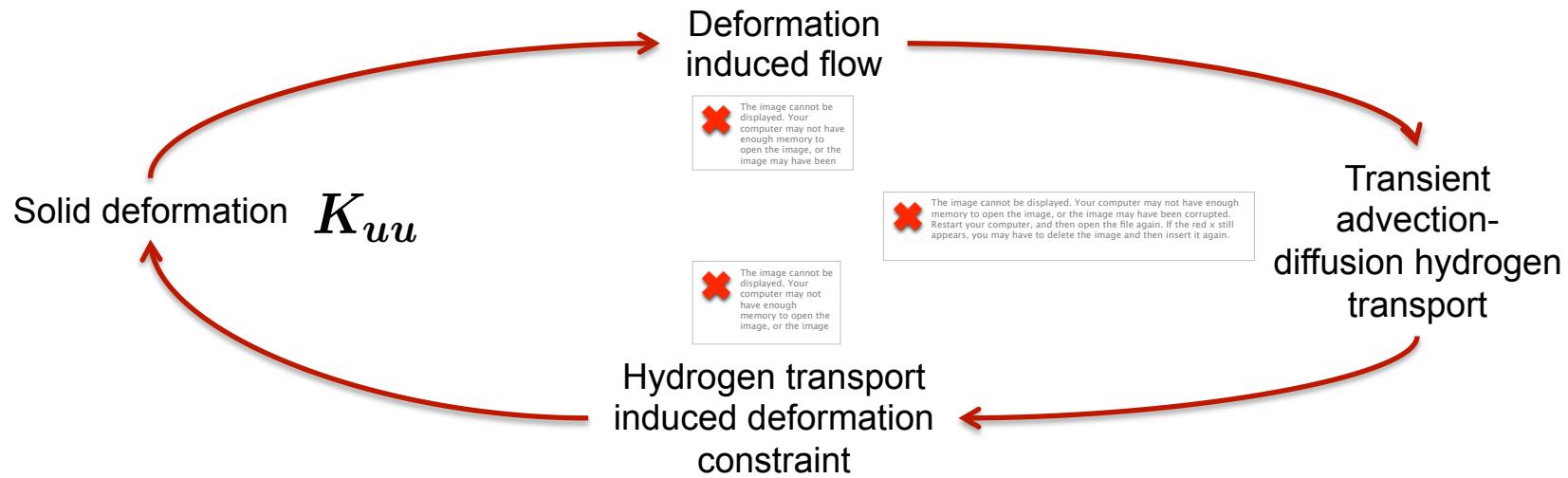


Implicit Time Integration of Fully Coupled Scheme

Backward Euler Scheme

$$\begin{bmatrix} 0 & 0 \\ M_{c_L u} & K_{c_L c_L}^{tran} \end{bmatrix} \begin{bmatrix} \dot{u} \\ \dot{c}_L \end{bmatrix} + \begin{bmatrix} K_{uu} & B_{uc_L} \\ 0 & K_{c_L c_L}^{st} \end{bmatrix} \begin{bmatrix} u \\ c_L \end{bmatrix} = \begin{bmatrix} F_u^{ext} \\ F_{c_L}^{ext} \end{bmatrix}$$

$$\begin{bmatrix} K_{uu} & B_{uc_L} \\ M_{c_L u} & K_{c_L c_L}^{st} \delta t + K_{c_L c_L}^{tran} \end{bmatrix} \begin{bmatrix} u_{n+1} \\ c_{L n+1} \end{bmatrix} = \begin{bmatrix} F_u^{ext} \\ F_{c_L}^{ext} + M_{c_L u} u_n + K_{c_L c_L}^{tran} c_{L n} \end{bmatrix}$$



Challenges on Implementation of Hydrogen Transport Problem

1. Numerical instability may occur if (1) time step is too small and/or (2) diffusivity is too small (i.e. stainless steel) (Harari, 2004) and (3) boundary layer due to the advection term is thinner than the side length of the finite element (Belytschko, Liu, Moran, 2012).
2. Hydrogen transport problem is highly nonlinear, thus require a consistent linearization to implicitly solve for solutions (i.e., **NUMEROUS** manual, mechanical derivations **EACH** time the problem is amended).
3. Volumetric Locking, which may occur under perfectly plastic response / isochoric deformation..etc.

Example

Say we want to find the linearized D^* (i.e., the first order term in the Taylor expansion). D^* depends on the trapped solvent, temperature, lattice concentration—but trapped solvent also depends on equivalent plastic strain and hence, displacement, yield stress...etc.

How do we derive consistent linearization with respect to all the dependent variables ?

Notice that we need to restart the entire process if we make any changes....

D^* – effective diffusion constant

$$D^* = \frac{N_T N_L}{K_T C_L^2} \left[\frac{1}{(1 + \frac{N_L}{K_T C_L})^2} \right]$$

$$C_T = \frac{N_T}{1 + \frac{N_L}{K_T C_L}}$$

$$\theta_T = \frac{1}{1 + \frac{N_L}{K_T C_L}}$$

$$C_T = \theta_T N_T$$

$$D_L = D_0 e^{-Q/RT}$$

$$N_L = N_A / V_M$$

$$K_T = e^{W_B/RT}$$

$$N_T = 10^{(A - B e^{-C \epsilon_p})}$$

$$\sigma_e = \sigma_0 + H \epsilon_p^m$$

$$\frac{\partial N_T}{\partial \epsilon_p} = 10^{(A - B e^{-C \epsilon_p})} \ln(10) B C e^{-C \epsilon_p} / N_A - \text{trapped solvent mol/m}^3$$

$$D_0 = 2.0 \times 10^{-6} \text{ m}^2/\text{s} - \text{pre-exponential factor}$$

$$Q = 6900 \text{ J/mol} - \text{diffusion activation enthalpy}$$

$$R = 8.314 \text{ J/(molK)} - \text{ideal gas constant}$$

$$T = 300 \text{ K} - \text{room temperature}$$

$$D_L = 1.27 \times 10^{-8} \text{ m}^2/\text{s} (300K) - \text{diffusion coefficient}$$

$$N_A = 6.0232 \times 10^{23} \text{ atoms/mol} - \text{Avogadro's number}$$

$$V_M = 7.166 \times 10^{-6} \text{ m}^3/\text{mol} - \text{molar volume of Fe}$$

$$N_L = 1.40 \times 10^5 \text{ solvent lattice mol/m}^3$$

$$W_B = 60 \times 10^3 \text{ J/mol} - \text{trap binding energy}$$

$$K_T = 2.801 \times 10^{10} - \text{equilibrium constant}$$

$$V_H = 2.0 \times 10^{-6} \text{ m}^3/\text{mol} (\sim 300K) - \text{partial molar volume}$$

$$\Delta H_s = 28.6 \times 10^3 \text{ J/mol} - \text{enthalpy of solution}$$

$$p_{H_2} = 1 \text{ atm} - \text{hydrogen pressure}$$

$$C_{L,0} = 2.0 \times 10^{26} e^{-\Delta H_s/RT} \sqrt{p_{H_2}} \text{ atoms/m}^3$$

$$C_{L,0} = 3.47 \times 10^{-3} \text{ mol/m}^3 - \text{initial concentration}$$

$$N_T = 10^{(23.3 - 2.33 e^{-4.0 \epsilon_p})} / N_A - \text{trapped solvent mol/m}^3$$

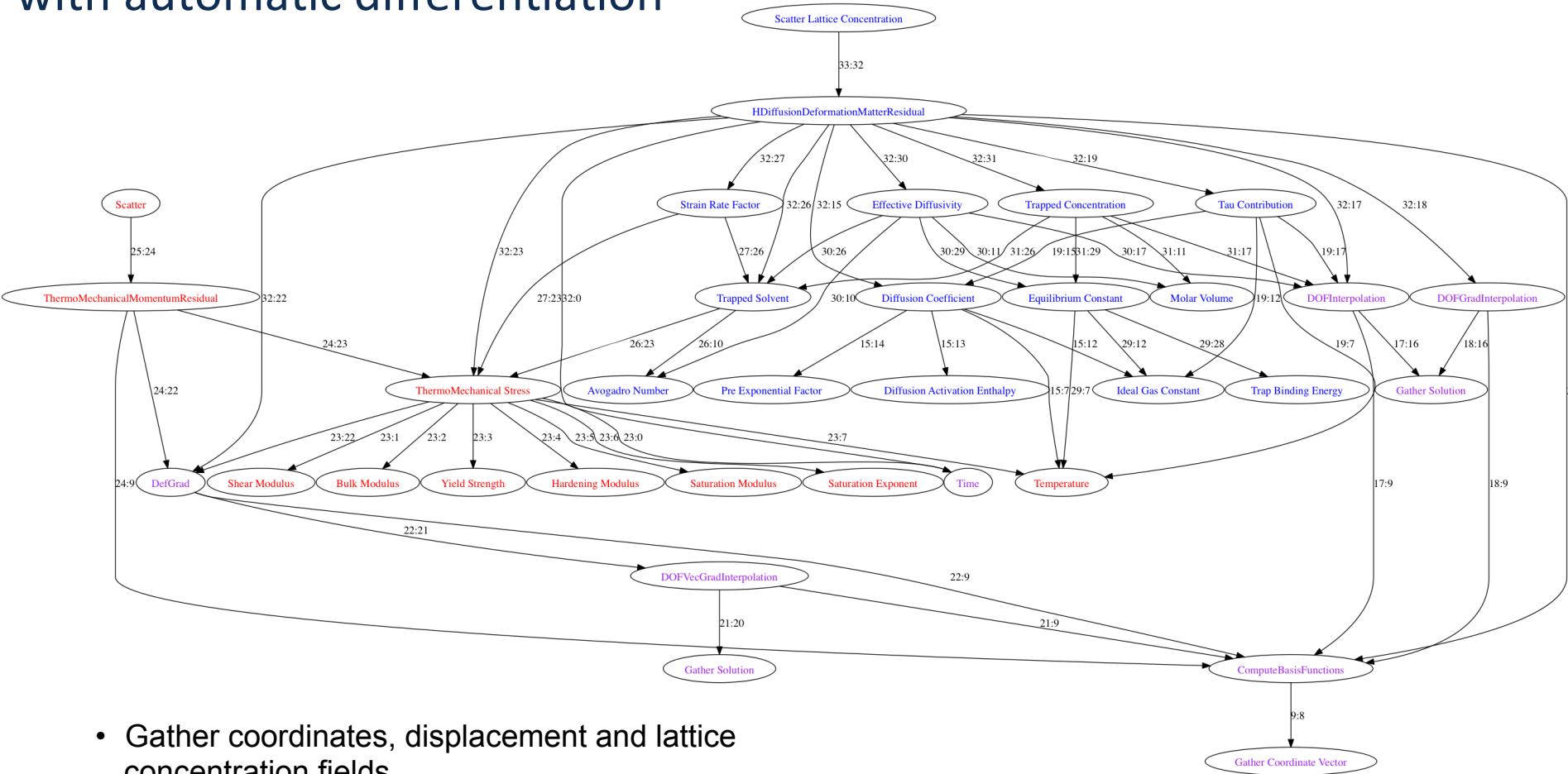
$$N_{T,0} = 1.549 \times 10^{-3} - \text{trapped solvent mol/m}^3$$

$$\theta_{T,0} = 0.9986$$

$$C_{T,0} = 1.547 \times 10^{-3} \text{ mol/m}^3 - \text{initial concentration}$$

$$C_{total,0} = C_{L,0} + C_{T,0} = 5.017 \times 10^{-3} \text{ mol/m}^3$$

Implementation of Hydrogen Diffusion-Mechanics Problem with automatic differentiation



- Gather coordinates, displacement and lattice concentration fields
- Interpolate fields and gradients to integration points
- Chain together Evaluators to compute Momentum and Conservation of Hydrogen Residuals
- Scatter back to the global system of equations

Blue = Hydrogen Transport
 Red = Solid Mechanics (J2 Plasticity)
 Purple = coupled terms

Combined F-bar formulation

Isochoric-volumetric split

$$\mathbf{F} = \mathbf{F}_{\text{vol}} \cdot \mathbf{F}_{\text{iso}}$$

Replacing volumetric split with assumed term

$$\overline{\mathbf{F}} = \bar{J}^{1/3} \mathbf{F}_{\text{iso}} = \bar{J}^{1/3} J^{-1/3} \mathbf{F}$$

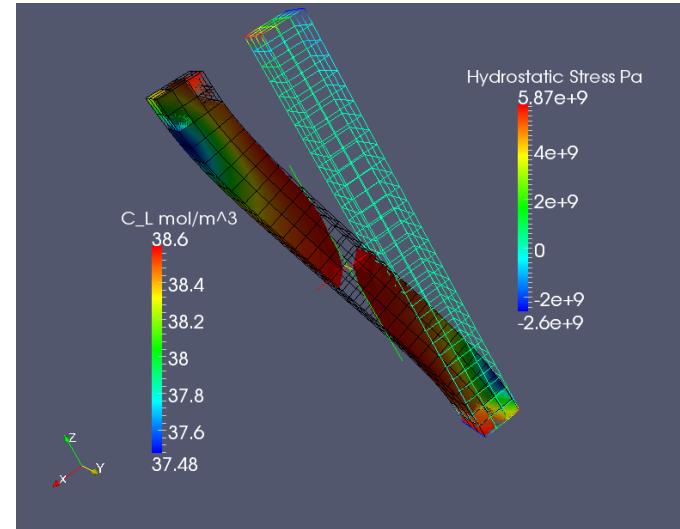
Classical Combined F-bar approach

$$\tilde{\mathbf{F}} = \alpha \mathbf{F} - (1 - \alpha) \overline{\mathbf{F}}.$$

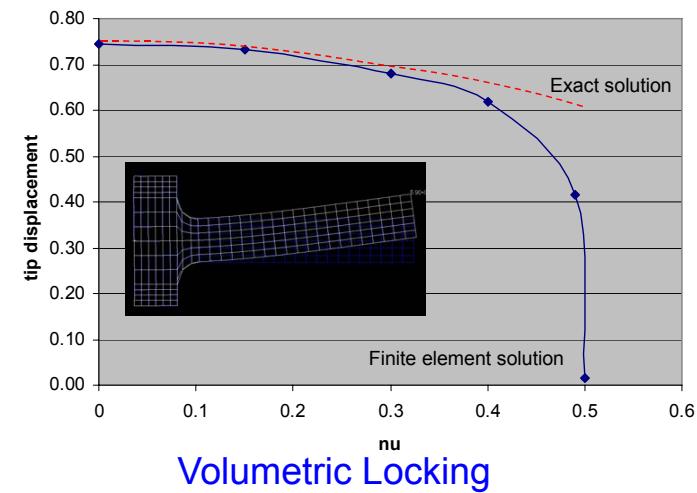
Current Approach via Lie algebra

$$J = J^e J^p J^C \quad ; \quad J^C = 1 + \lambda(C - C_o)$$

$$\tilde{J}(X) = \exp \left(\frac{1 - \alpha}{V_{\mathcal{B}^e}} \int_{\mathcal{B}^e} \log J(X) \, dV + \alpha \log J(X) \right)$$



Concentration prediction affected by locking



Stabilization at small time limit

Hydrogen Transport Theorem

$$D^* \dot{C}_L - \nabla^X \cdot D_L \nabla^X C_L + \nabla^X \cdot \frac{V_H}{RT} C_L D_L \nabla^X S_H + \theta_T \frac{dN_T}{d\epsilon_p} \dot{\epsilon}_p = 0$$



 transient term Hydrogen diffusion term Advection coupling term Plastic strain nonlinear coupling term

- Spurious oscillations may occur when
 - D^* is large, which means local rate of change dominates
 - The mesh size h is large (relative to the advection and diffusion length scale)
 - The time step is small (relative to the advection and diffusion time scale).
 - Notice that Peclet number measures whether advection of diffusion is more important, but does not tell much about the transient term!
- Examples of stabilization scheme
 - Petrov-Galerkin/streamline upwind method (Hughes, 1978, Johnson, 1984)
 - Space-time finite incremental calculus method (Onate and Manzan 2000)
 - SUPG with adaptive stabilization parameters (Tezduyar 2003)
 - Spurious oscillations at layers diminishing method (Volker and Schmeyer, 2008).
 - Artificial diffusivity (Onate and Manzan, 2000).

Stabilization for hydrogen convection-diffusion problem

- For transient problem, stability criteria must be satisfied for the pair of time step **and** element size h used in simulations. γ is the parameter for backward Euler time integrator (Harari, 2004).

$$\frac{h^2 D^*}{6\gamma D_L \Delta t} < 1$$

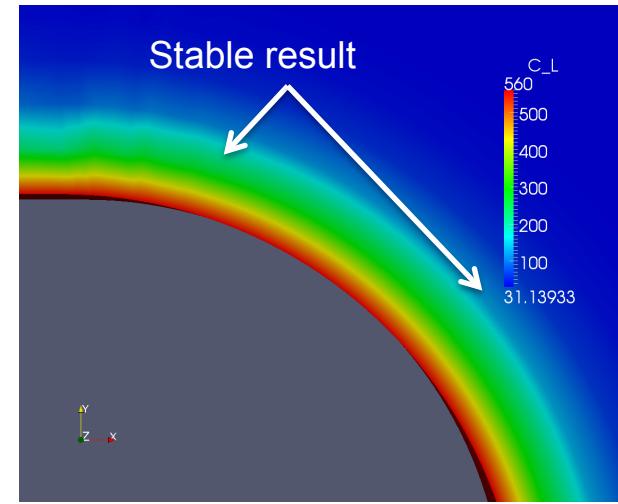
- Meanwhile, stability of the steady solution can be predicted by the Peclet number. If Pe is less than 1 and D^*/D_L is reasonably close to h^2/dt

$$Pe = \frac{V_H}{RT} |\nabla^X S_H| h < 1$$

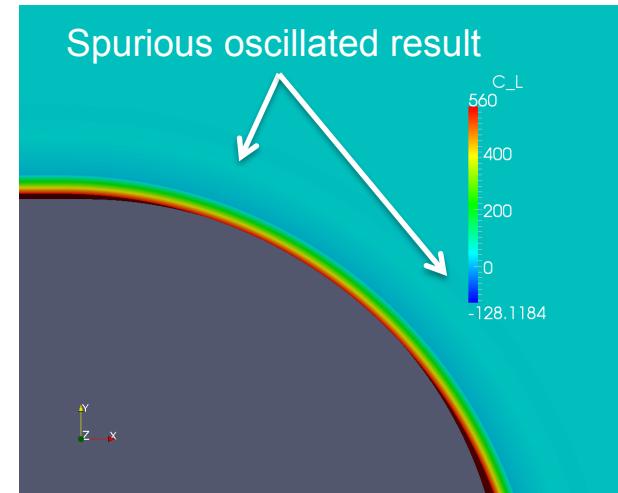
- add stabilization term to penalize the deficiency

$$R_{stab} = \int_{\Omega} [\tau(\eta^h - \frac{1}{V_{\Omega}} \int_{\Omega} \eta^h d\Omega)(\dot{C}_L^h - \frac{1}{V_{\Omega}} \int_{\Omega} \dot{C}_L^h d\Omega)] d\Omega$$

$$R_{stab} = \int_{\Omega} \tau \nabla^X \eta^h \nabla^X \dot{C}_L^h d\Omega$$



Stabilized Implicit Gakerlin Formulation



Standard Implicit Galerkin Formulation

Conclusion and Future Perspective

- We have implemented a fully implicit hydrogen transport model in the Laboratory of Computational Mechanics package with the following desirable features
 - Capacity to conduct fully coupled, fully implicit simulations with stabilization scheme.
 - **Automatic differentiation**, which makes it fast and easy to make amendment to existing model and eliminate chance of making error in derivation
 - **Stabilization scheme** available to handle material with extremely low diffusivity, thin boundary layer...etc.
 - **A L2 projection scheme** to obtain a C^0 -continuous stress gradient term that enables the advection term to be correctly modeled without introducing errors during the extrapolation process.
 - **Localization element** is currently developing for modeling boundary/ lattice diffusion and fracture.