

# PROBABILISTIC WIND/TORNADO/MISSILE ANALYSES FOR HAZARD AND FRAGILITY EVALUATIONS\*

by

Young J. Park and Morris Reich  
Brookhaven National Laboratory  
P.O. Box 5000  
Upton, NY 11973

## ABSTRACT

Detailed analysis procedures and examples are presented for the probabilistic evaluation of hazard and fragility against high wind, tornado, and tornado-generated missiles. In the tornado hazard analysis, existing risk models are modified to incorporate various uncertainties including modeling errors. A significant feature of this paper is the detailed description of the Monte-Carlo simulation analyses of tornado-generated missiles. A simulation procedure, which includes the wind field modeling, missile injection, solution of flight equations, and missile impact analysis, is described with application examples.

## INTRODUCTION

As part of the external event PRA of the High Flux Beam Reactor (HFBR), hazard and fragility analyses were performed against a combined tornado and high wind hazard (Ref. [1]). This paper summarizes the methodology development and application examples in the following areas:

- Tornado hazard analysis;
- Combined tornado/straight wind hazard evaluation;
- Fragility evaluation of structures against wind pressures;
- Analysis of tornado-borne missiles; and
- Fragility evaluation of equipment.

Although the presented hazard and fragility analyses generally follow the methodologies available in open literatures (e.g., Refs. [2], [3] and [4]), some new developments are also introduced in the areas related to the tornado hazard analysis and the tornado-borne missile analysis, including a comprehensive Monte-Carlo simulation scheme for the tornado-borne missile evaluation.

## TORNADO HAZARD ANALYSIS

The tornado hazard model described in Reinhold and Ellingwood (1982), that was originally proposed by Garson et al. (1975), is used after a few modifications.

The model is an outgrowth of the earlier models by Fujita (Ref. [9]) and McDonald (Ref. [10]), and is formulated by 6x6 matrices corresponding to Fujita's scale of F0 to F5 intensities. Because of this format, various sources of uncertainties can be systematically combined.

All the variables in the model are discretized according to the FPP scale as given in Table 1 and the associated median wind speed  $V_i$ , as

$$\{P_t(V_i)\} = \{P_{ps}(V_i)\} + \{P_{ll}(V_i)\}, \quad i = F0 \text{ to } F5 \quad (1)$$

in which,  $P_t(V_i)$  is the total tornado strike probability associated with the wind speed  $V_i$ ;  $P_{ps}(V_i)$  is the point-strike probability; and  $P_{ll}(V_i)$  is the strike probability due to life-line term, reflecting on the increase due to the size of a structure.  $P_{ps}$  and  $P_{ll}$  are expressed as,

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Table 1 Fujita-Pearson (FPP) Classifications

Fujita Scale	Maximum Wind Speed					
	F0	F1	F2	F3	F4	F5
Range, mph	40-72	73-112	113-157	158-206	207-260	261-318
Median, mph	56.0	92.5	135.0	182.0	233.5	289.5
Pearson Path Length Scale	Path Length					
	P0	P1	P2	P3	P4	P5
Range, mi	0.3-0.9	1.0-3.2	3.2-9.9	10.0-31.5	31.6-99	100-316
Median, mi	0.6	2.05	6.55	20.75	65.3	208
Pearson Path Width Scale	Path Width					
	P0	P1	P2	P3	P4	P5
Range, yds	6-17	18-55	56-175	176-556	557-1759	1760-4963
Median, yds	11.5	36.5	115.5	366	1158	3361.5

$$\{P_{ps}(V_i)\} = \lambda [V_w][V_l] \{A_{ps}(V_i)\}, \quad i = F0, F5 \quad (2)$$

$$A_{ll}(V_i) = W_s \cdot L'(V_i) \cdot P'(V_i), \quad i = F0, F5 \quad (5)$$

$$\{P_{ll}(V_i)\} = \lambda [V_l] \{A_{ll}(V_i)\}, \quad i = F0, F5 \quad (3)$$

where,

$A'(V_i)$  = the average area of the damage path for an  $F_i$  tornado obtained from the area-intensity relationship

$P'(V_i)$  = the conditional occurrence probability of an  $F_i$  tornado given an occurrence of a tornado

$W_s$  = the characteristic width of a structure

$L'(V_i)$  = the average path length of an  $F_i$  tornado.

where,

$\lambda$  = the average number of tornadoes per sq. mi. per year

$[V_w]$  = a matrix expressing the variation of wind speed across the width of the tornado path

$[V_l]$  = a matrix expressing the variation of wind speed along the length of the tornado path

$A_{ps}(V_i)$  = the average area of the damage path per tornado for an  $F_i$  tornado

$A_{ll}(V_i)$  = the average area of the life-line path per tornado for an  $F_i$  tornado.

The determination of the above six parameters,  $\lambda$ ,  $[V_w]$ ,  $[V_l]$ ,  $A'(V_i)$ ,  $P'(V_i)$  and  $L'(V_i)$  are summarized below.

#### Tornado Occurrence Rate, $\lambda$

The two average areas per occurrence of tornado are further expressed as:

$$A_{ps}(V_i) = A'(V_i) \cdot P'(V_i), \quad i = F0, F5 \quad (4)$$

The average occurrence rate of tornadoes in the U.S. is about  $10^{-4}$  per square mile per year. This value can be determined either by global regionalization schemes (e.g., Refs. [7] and [8]), or by using more regional data. According to Fujita (1981), the regional occurrence rate for the BNL site is estimated as,

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Table 2 Matrix,  $C_{ij}$ , Intensity-defined Damage Area vs Reported Damage Area

	F0	F1	F2	F3	F4	F5
F0	.176	.456	.249	.098	.021	0
F1	.048	.244	.370	.266	.070	.002
F2	.013	.108	.289	.400	.175	.015
F3	.003	.030	.149	.377	.375	.066
F4	0	.011	.059	.223	.549	.158
F5	0	0	0	.098	.439	.463

Table 3 Matrix,  $C_{ij}$ , for Path Length-Intensity Relationship (Intensity-Defined vs Reported Path Lengths)

	F0	F1	F2	F3	F4	F5
F0	.832	.099	.059	.010	0	0
F1	.563	.236	.147	.049	.005	0
F2	.379	.260	.241	.110	.010	0
F3	.156	.206	.307	.271	.060	0
F4	.039	.077	.263	.442	.140	.039
F5	0	.067	0	.666	.200	.067

$$\lambda = \frac{(40 \text{ tornadoes})}{(30.2 \text{ years}) \times (6.506 \text{ sq.mi.})} = 2.03 \times 10^{-4} (\text{tornadoes/year/sq.mi.})$$

McDonald (1982) also estimated a similar value of  $2.16 \times 10^{-4}$ . A median value of  $2.1 \times 10^{-4}$  was used for the hazard analysis of the BNL site.

#### Occurrence-Intensity Relationship, $P(V_i)$

This probability,  $P(V_i)$ , represents the conditioned probability of an  $F_i$  tornado, given an occurrence of a tornado. The regional data by McDonald (1982) are directly used as tabulated below.

i	F0	F1	F2	F3	F4	F5
no.of tornadoes, $N_i$	11	61	31	6	1	0
$P(V_i) = N_i / \sum N_i$	0.100	0.555	0.282	0.055	0.009	0

The above probabilities are modified later to account for the classification errors and random encounter errors.

#### Area-Intensity Relationship, $A(V_i)$

The area-intensity relationship may be expressed in a matrix format to account for the large uncertainty in assigning a specific damage area to each tornado intensity class.

$$\{A(V_i)\} = [C_a] \{A_o(V_i)\} \quad (6)$$

In which  $A_o(V_i)$  represents a best estimate of the area of the damage path for an  $F_i$  tornado, and the site-specific values obtained by McDonald are used (1982).

i	F0	F1	F2	F3	F4	F5
$A_o(V_i)$ , sq. mi.	0.0209	0.0722	0.184	0.385	0.713	1.21

The 6x6 matrix,  $[C_a]$ , represents the statistical relationship between the intensity-defined damage area and the reported damage area. Schaefer et al. (1980)

obtained this relationship from a statistical study of 10,204 tornadoes as given in Table 2.

### Path Length-Intensity Relationship, $L(V_i)$

A similar relationship to the above Eq. (6) may be considered for the path length corresponding to an  $F_i$  tornado as,

$$\{L(V_i)\} = [C_l] \{L_o(V_i)\} \quad (7)$$

Reinhold and Ellingwood (1982) suggested the following relationship between the damage path width, and the damage path length,  $L$ :

$$W = 0.01 L \quad (8)$$

Using this relationship, the best estimate values for the damage path length,  $L_o(V_i)$ , are determined as,

i	F0	F1	F2	F3	F4	F5
$L_o(V_i)$ , miles	1.4	2.7	4.3	6.2	8.4	11.0

Table.3 tabulates the matrix,  $C_l$ , which was originally obtained by McDonald (1983) by analyzing 7,953 tornadoes.

### Classification Errors

The foregoing three relationships, i.e., occurrence-intensity, area-intensity, and path length-intensity, are determined from observational data bases. The uncertainties associated with these determinations can be classified as direct classification errors and random encounter errors; each is represented by a 6x6 matrix,  $E_d$  and  $E_r$ , respectively.

$$\begin{aligned} \{P'(V_i)\} &= [E_r][E_d] \{P(V_i)\} \\ \{A'(V_i)\} &= [E_r][E_d] \{A(V_i)\} \\ \{L'(V_i)\} &= [E_r][E_d] \{L(V_i)\} \end{aligned} \quad (9)$$

in which,  $P'(V_i)$ ,  $A'(V_i)$  and  $L'(V_i)$  are the modified relationships. Two formulations were suggested by Twisdale et al. (1981) based on the engineering judgement for the above  $E_d$ . For the random encounter errors,  $E_r$ , one suggested by Twisdale et al. (1981), and

the other by Reinhold and Ellingwood (1982) are used in this study. In addition, the identify matrix is considered for both  $E_d$  and  $E_r$  to account for any possible "double counting" of the uncertainties. Overall, nine combination cases are considered for the classification errors (3 cases for  $E_d$  times 3 cases for  $E_r$ ).

### Variation of Wind Speed Within a Damage Path, $V_w$ and $V_i$

In this hazard model, the variation of wind speed within a tornado path is considered separately along and across the path, represented by matrices,  $V_i$  and  $V_w$  in Eqs. (2) and (3). Ref. [1] describes three formulations for both  $V_i$  and  $V_w$ , which were obtained by analyzing 149 tornadoes occurring during a super outbreak in Texas on April 3 and 4, 1974. Again, nine cases are considered for modeling the variation of wind speed within a tornado path (3 cases for  $V_i$  and 3 cases for  $V_w$ ).

### Generation of Tornado Hazard Curves

Fig. 1 shows the generated tornado hazard curves using the above Eqs. 2, 4, 6 and 7 for the point-strike probability, which is a collection of 81 curves (9 cases of classification error formulations times 9 cases of wind-speed variation formulations). Similarly, the collection of hazard curves for a 177 ft. wide structure are generated as shown in Fig. 2, which were obtained by adding the life-line term using Eqs. 1, 3, 5, 7 and 8.

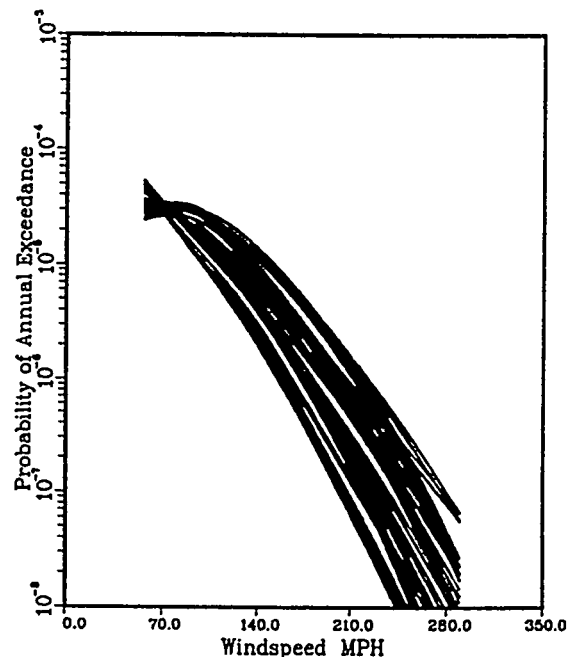


Figure 1. Generated Hazard Curves for Point Strike Risk

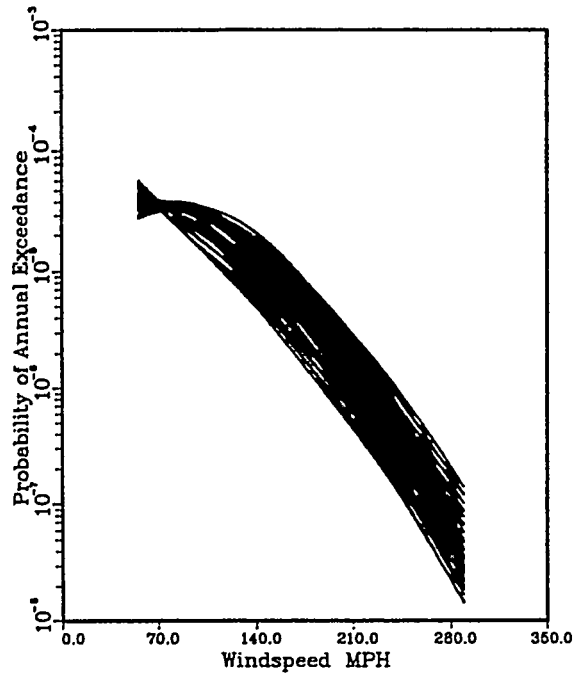


Figure 2. Generated Hazard Curves for Total Risk of 177 ft. Structure

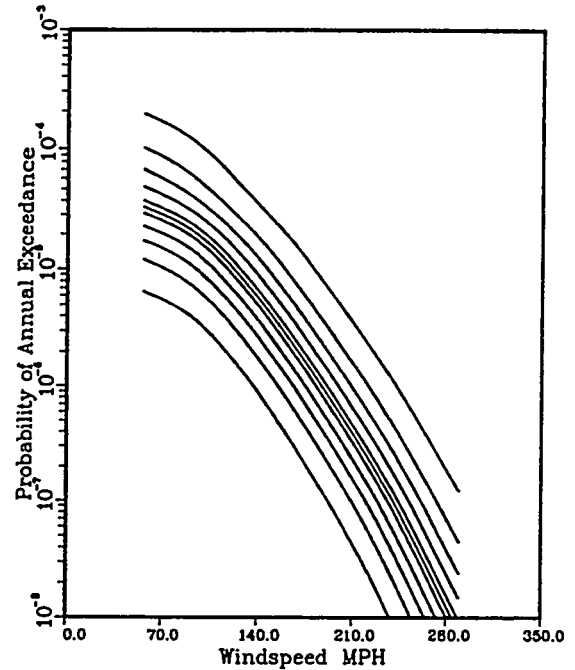


Figure 4. Tornado Hazard Curves for BNL Site (for "Point" Structures at 5, 15, 25, 35, 45, 50, 55, 65, 75, 85 and 95 Percentile)

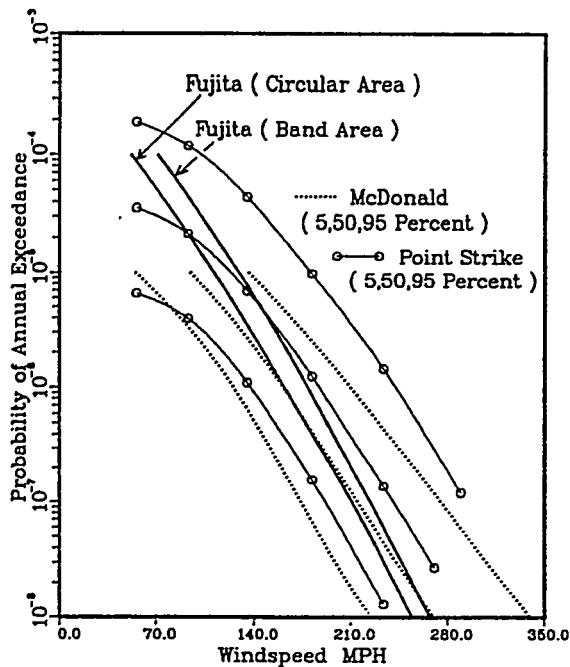


Figure 3. Comparison of Tornado Hazard Curves

The results, shown in Figs. 1 and 2, however do not account for the modeling error, i.e., the error associated with the selection and formulation of hazard model.

Moreover, even using the same model may not lead to similar results due to the subjective interpretation of the formulation and the tornado data.

Fig. 3 compares the hazard curves generated for the BNL site by Fujita (1981) and McDonald (1982). Fujita generated two curves for two different definitions of the regional area surrounding the site. McDonald, on the other hand, obtained the hazard curves at 5, 50 and 95 percentile confidence levels (broken lines in Fig. 3). The best estimate curve by Fujita gives higher risk values than McDonald's median curve, although both analyses used the same data base. Based on this comparison, the additional variability of the annual probability due to modeling error was estimated to be 1.0 in terms of the lognormal standard deviation,  $\beta$ . By assuming the lognormal distribution for the annual exceedance probability values,  $p$ , the wind speed,  $V$ , corresponding to a confidence level in terms of the  $q$ -percentile value, is expressed as,

$$V = \exp. \{ \beta \cdot \Phi^{-1} (q) + \ln V_m \} \quad (10)$$

where,  $\beta$  is the combined lognormal standard deviation;  $\Phi^{-1}(\cdot)$  is the inverse Gaussian distribution function; and  $V_m$  is the wind speed of the median hazard curve. Fig. 4 shows the calculated point-strike hazard curves for 11 percentile values.

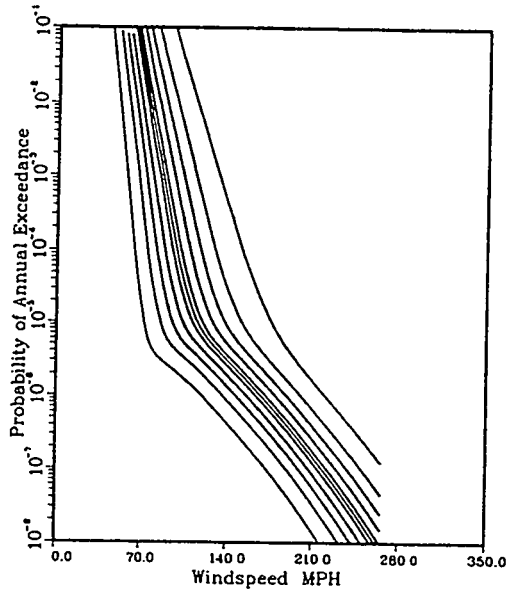


Figure 5. Combined Wind Hazard Curves at BNL Site for "Point" Structures (5, 15, 25, 35, 45, 50, 55, 65, 75, 85 and 95 Percentile)

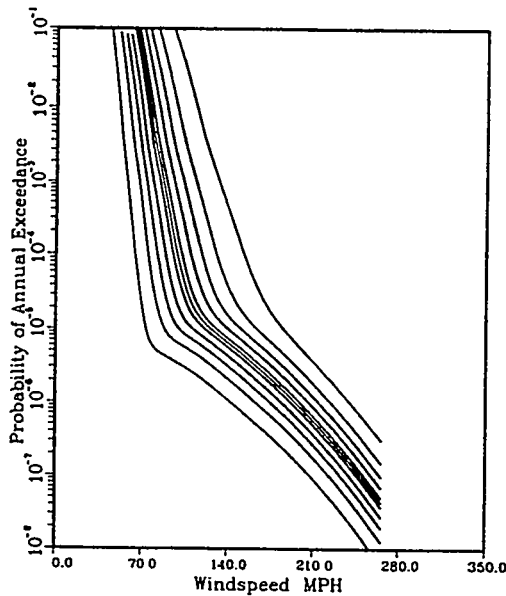


Figure 6. Combined Wind Hazard Curves at BNL Site for 177 ft. Structures (5, 15, 25, 35, 45, 50, 55, 65, 75, 85 and 95 Percentile)

### COMBINED WIND HAZARD EVALUATION

According to UCRL-15910 (Ref. [3]), the following equation is recommended to convert the tornado wind speed,  $V_p$  to the equivalent fastest-mile wind speed,  $V_{fm}$ :

$$V_{fm} = 0.958 V_p - 11.34 \quad (11)$$

The hazard curves for straight wind were evaluated separately (Ref. [1]). The combined hazard curves were obtained using Eq. 10, and shown in Figs. 5 and 6 for the BNL site.

### STRUCTURAL FRAGILITY AGAINST WIND PRESSURE

The following ASCE 7-88 formulation (Ref. [14]) is used to establish the relationship between the wind speed and the wind force,  $F$ :

$$F \text{ (lbs)} = q G_n C_f A_f \quad (12)$$

where:

- $q(\text{psf}) = 0.00256 K V^2$
- $G_n$  = gust factor, given in Table 8 of ASCE 7-88
- $C_f$  = force coefficients
- $A_f$  = projected area normal to wind
- $K$  = exposure coefficient, given in Table 5 of ASCE 7-88
- $V$  = wind speed in mph.

The wind capacity of structures is expressed as a function of the median pressure capacity,  $F_m$ , and the random variables,  $\epsilon_r$  and  $\epsilon_u$ , each representing the randomness of wind forces and the uncertainty in structural capacity.

$$\begin{aligned} F_p &= F_m \cdot \epsilon_r \cdot \epsilon_u \\ \text{and } \epsilon_r &= \epsilon_G \cdot \epsilon_c \cdot \epsilon_d, \\ \epsilon_u &= \epsilon_s \cdot \epsilon_m \end{aligned} \quad (13)$$

where:

- $\epsilon_G$  = Factor representing variability in determining the gust factor,  $G_n$ ;
- $\epsilon_c$  = Factor representing variability in pressure and force coefficients,  $C_f$ ;
- $\epsilon_d$  = Factor representing variability in wind direction;
- $\epsilon_s$  = Factor for structural capacity; and
- $\epsilon_m$  = Modeling error in structural analysis.

The above pressure capacity can be converted to the wind speed using the following relationship:

Table 4 Example of Fragility Evaluation of a Structure

Factor	Median Values	$\beta_R$	$\beta_U$
Basic strength	214 mph		
Structural capacity	1.0	-	0.20
Modeling	1.0	-	0.15
Gust factor	1.0	0.20	-
Pressure coefficient	1.3	0.21	-
Wind direction	1.1	0.10	-
Factors for pressure	1.43	0.31	0.25
Factors for wind speed	1.20	0.15	0.13
Total fragility	256.8 mph	0.15	0.13

$$F_v = \sqrt{F_p}, \quad \beta_v = 0.5 \beta_p \quad (14)$$

An example of the structural fragility evaluation is given in Table 4. More detailed description of the calculation procedure can be found in Ref. [1].

#### TORNADO MISSILE ANALYSIS

According to the USNRC Standard Review Plan [16], the total probability of missiles striking a vulnerable, critical area of the plant is estimated as a product of several probabilities. The total probability per year ( $P_t$ ) is described as:

$$P_t = P_E \cdot P_{MR} \cdot P_{SC} \cdot P_p \cdot N \quad (15)$$

where:

- $P_E$  = Probability per year of a tornado strike;
- $P_{MR}$  = Probability of missiles reaching the plant;
- $P_{SC}$  = Probability of missiles striking a vulnerable critical area of the plant;
- $P_p$  = probability of missiles exceeding the energies required to penetrate vital areas (e.g., based on wall thickness provided for tornado missiles), or producing secondary missiles which could damage vital equipment; and,
- $N$  = Number of missiles generated by the design basis event.

In this study, the 85-percentile point strike probability for F1 tornadoes,  $0.637 \times 10^{-4}$ , is used as a conservative estimate of the strike probability,  $P_E$ . The product of the two probabilities,  $P_{MR} P_{SC}$ , which represents an event in which a single tornado missile is picked up by a wind, and eventually hits the target structure, was estimated by a Monte-Carlo simulation. A computer program was developed for this purpose. The

program generates a large number of missiles with random initial locations and random flight parameters, predicts the 3-D flight path through a time-history analysis, and perform a statistical analysis on the number of missile hits and the strike velocity. The maximum wind speed and the direction of the tornado strike also are randomized.

The probability,  $P_p$ , is determined from the kinetic energy of the missiles and the structural capacity against impact and penetration. Structural capacity is estimated from nonlinear static analysis and penetration equations. Finally, the number of potential missiles,  $N$ , is estimated by surveying the BNL site for sources of missile. Relatively weak structures, e.g., wooden buildings, utility poles, stacks, and automobiles were considered typical missile sources.

To estimate the strike probability and the velocity of a missile at impact, a set of models and assumptions are necessary, including:

- Tornado wind-field model;
- Aerodynamic characteristics of tornado missile; and
- Initial position of missile relative to the target structure and the incoming tornado.

In this study, a simple Rankine wind-field model was used to characterize the 3-D wind speed.

The trajectory and velocity of a missile are described by the following flight equations:

$$\frac{d}{dt} \bar{V}_m = \frac{1}{2} \rho \frac{C_D A}{m} |\bar{V}_w - \bar{V}_m| (\bar{V}_w - \bar{V}_m) - g \bar{z} \quad (16)$$

in which,  $\bar{V}_m$  is the missile velocity vector;  $\bar{V}_w$  is the wind velocity vector;  $\rho$  is the air mass density;  $C_D$  is the drag coefficient;  $A$  is a suitably chosen area;  $\bar{z}$  is the unit



Table 5 Missile Parameters

Missiles	Weight (lbs)	$C_D A/m$ (ft <sup>2</sup> /lb)	
		(Best Estimate)*	(Range)
A. 6" Sch. 40 pipe (15')	285	0.0212	0.002 - 0.04
B. 12" Sch. 40 pipe (15')	740	0.016	0.002 - 0.03
C. 1" solid steel rod (3')	8	0.019	0.002 - 0.04
D. 13.5" Utility Pole (35')	1100	0.0254	0.002 - 0.05
E. Automobile	4000	0.0343	0.003 - 0.07

\* From Simin and Scanlan (1978)

vector along the vertical line;  $m$  is the mass of a missile; and  $g$  is the gravitational acceleration. According to this equation, the motion of the missile depends on the flight parameter,  $C_D A/m$  and its initial condition. The following approximate formulation was suggested to estimate the flight parameter (Ref. [17]):

$$C_D A = C(C_{D1}A_1 + C_{D2}A_2 + C_{D3}A_3) \quad (17)$$

where  $C_{Di}A_i$  ( $i=1, 2, 3$ ) are the values of  $C_D A$  corresponding to the principal axes of the body, and  $C$  is an empirical coefficient assumed to be 0.50 for rods, pipes and poles, and 0.33 for automobiles. Rotational movements, including a tumbling mode, are not considered in the analysis. The 4-th order Runge-Kutta method was used to numerically integrate the equations of motion.

Table 5 lists the missiles considered in the analysis. The uncertainties associated with estimating the flight parameter are considered by using random numbers, as indicated by the ranges in Table 5.

Figure 7 illustrates a simulation example in which 75 out of 1,000 tornado missiles generated from a collapsed weak wooden building hit the spherical target structure. In this simulation example, the following unknown parameters were randomized:

- Maximum tangential wind speed;
- Flight parameter of missiles; and
- Initial position of missiles ( $X$ ,  $Y$ ,  $Z$  coordinates) within the building site.

In addition, the direction of the tornado path relative to the target structure (and the source of missiles) should also be randomized. Figure 8 illustrates a scheme in which the source building is represented by a "ring" that has the width, height and distance from the target building. In this scheme, the direction of the tornado path

is randomized with an equal likelihood in all the directions.

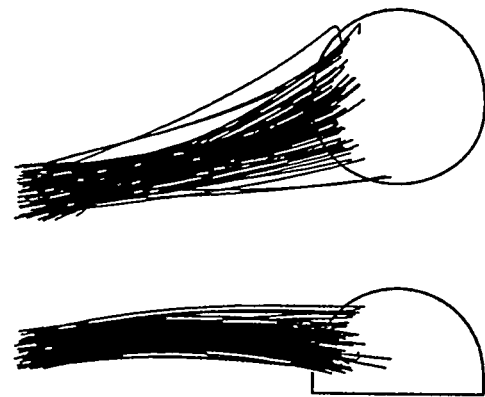


Figure 7. Trajectory of Missiles Generated due to Disintegration of a Wooden Building

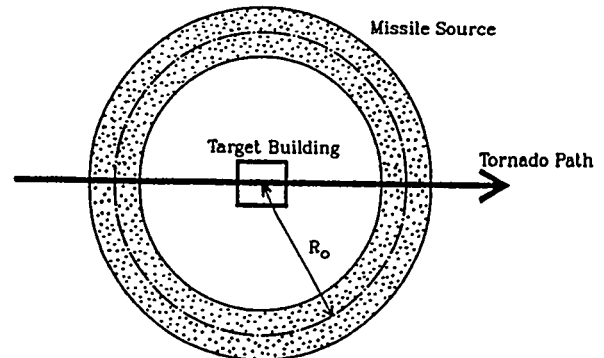


Figure 8. Circular Missile Source Surrounding the Target Building

Table 6 Number of Potential Missiles

Missile type	Distance from Target Building		
	320 ft	600 ft	1000 ft
A	400	1200	2000
B	200	600	1000
C	400	1200	2000
D	20	60	100
E	100		

Table 7 Results of Missile Simulation Analysis

Missile Type	Expected Number of Hits	Median Velocity (ft/sec.)	$\beta$ of Velocity
A	22.5	118	0.15
B	8.5	97	0.12
C	22.5	118	0.15
D	1.4	119	0.14
E	3.7	96	0.18

Based on a survey of wooden structures, utility poles and parked automobiles around the target building, the number of potential missiles was estimated as listed in Table 6. The calculated strike probability drops rapidly as the distance between the initial location of the missiles and the target structure increases. The simulation results for a target building are summarized in Table 7 in terms of the number of missile hits per a tornado strike and the impact velocity.

The probability,  $P_p$ , of Eq. 15 represents the probability of missiles exceeding the kinetic energies required to penetrate the structural barrier. By assuming a lognormal distribution both for the kinetic energy of a missile,  $E_k$ , and the energy capacity of a structure against impact loads,  $E_s$ , then the probability of a missile penetration,  $P_p$ , is expressed:

$$P_p = 1 - \Phi \left( \frac{\ln(\bar{E}_s / \bar{E}_k)}{\sqrt{\beta_k^2 + \beta_s^2}} \right) \quad (18)$$

in which,  $\bar{E}_s$  and  $\beta_s$  are the median and the lognormal standard deviation for the structural energy capacity; and  $\bar{E}_k$  and  $\beta_k$  are those for missile kinetic energy.

In evaluating the structural capacity against missile penetration, empirical penetration equations (e.g., Refs. [18], [19]) can be used for small objects. However, for larger objects such as automobiles, a nonlinear structural

analysis is necessary to estimate the energy absorbing capacity (Ref. [1]).

Table 8 summarizes the calculation results for a target building. To determine the effects on equipment inside the target structure, a further analysis is necessary as briefly described below.

#### EVALUATION OF EQUIPMENT FRAGILITY

The wind capacity of equipment housed inside a structure is determined from the following wind/tornado effects:

- Offsite power loss
- Collapse of building or part of building
- Tornado missile hit.

The fragility of equipment is expressed by the conditional probability of failure given a wind speed level expressed in terms of the fastest miles per hour (mph). However, when damage caused by tornado-borne missiles is considered, the fragility value is unconditionally defined since the variability in tornado wind speed has already been considered in the missile simulation analysis. In calculating the annual failure rate of an equipment item due to tornado missile hits, the following reduction factor should be multiplied to the probability values listed in Table 8.

Table 8 Tornado Missile Penetration Probability

Missile	Tornado Strike $P_E$ (year <sup>-1</sup> )	$N \times P_{MR} \times P_{SC}$	$P_p$	Penetrations per Year
A. 6 in. pipe	$0.637 \times 10^{-4}$	22.5	1.0	$1.43 \times 10^{-3}$
B. 12 in. pipe	$0.637 \times 10^{-4}$	8.5	0.95	$5.14 \times 10^{-4}$
C. 1 in. steel rod	$0.637 \times 10^{-4}$	22.5	0	0
D. Utility pole	$0.637 \times 10^{-4}$	1.4	0.95	$8.47 \times 10^{-5}$
E. Automobile	$0.637 \times 10^{-4}$	3.7	$0.3 \times 10^{-4}$	$7.07 \times 10^{-9}$

*Reduction Factor =*

$$\frac{(\text{Area of equipment possibly hit by object})}{(\text{Area of building possibly hit by object})}$$

An error factor (EF) of 10.0 is conservatively estimated for the annual failure rate of equipment due to tornado-borne missile hits, considering a large uncertainties involved in the tornado missile analysis as described above.

## CONCLUSIONS

The methodologies for the probabilistic evaluation of a combined tornado and straight-wind hazard, and the fragility analysis of structures and equipment were described in some details.

The presented simulation scheme for the tornado-borne missiles can be used for a more rational evaluation of the plant safety.

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