



F19800051

POSIVA 98-01

Bentonite swelling pressure in strong NaCl solutions

**Correlation of model calculations to
experimentally determined data**

**Ola Karnland
Clay Technology
Lund, Sweden**

29 - 15

L

January 1998

ISBN 951-652-039-1
ISSN 1239-3096

The conclusions and viewpoints presented in the report are those of author(s) and do not necessarily coincide with those of Posiva.

NEXT PAGE(S)
left BLANK



Posiva-raportti – Posiva report

Posiva Oy
Mikonkatu 15 A, FIN-00100 HELSINKI, FINLAND
Puh. (09) 2280 30 – Int. Tel. +358 9 2280 30

Raportin tunnus – Report code

POSIVA 98-01

Julkaisuaika – Date

January 1998

Tekijä(t) – Author(s) Ola Kamland Clay Technology Lund, Sweden	Toimeksiantaja(t) – Commissioned by Posiva Oy
Nimeke – Title BENTONITE SWELLING PRESSURE IN STRONG NaCl SOLUTIONS — Correlation of model calculations to experimentally determined data	
Tiivistelmä – Abstract <p>A number of quite different quantitative models concerning swelling pressure in bentonite clay have been proposed. This report discusses a number of models which possibly can be used also for saline conditions. A discrepancy between calculated and measured values was noticed for all models at brine conditions. In general the models predicted a too low swelling pressure compared to what was experimentally found. An osmotic component in the clay/water system is proposed in order to improve the previous conservative use of the thermodynamic model. Calculations of this osmotic component is proposed to be made by use of the clay cation exchange capacity and Donnan equilibrium. Calculations made by this approach showed considerably better correlation to literature laboratory data, compared to calculations made by the previous conservative use of the thermodynamic model. A few verifying laboratory tests were made and are briefly described in the report.</p> <p>The improved model predicts a substantial bentonite swelling pressure also in a saturated sodium chloride solution if the density of the system is sufficiently high. This means in practice that the buffer in a KBS-3 repository will give rise to an acceptable swelling pressure, but that the positive effects of mixing bentonite into a backfill material will be lost if the system is exposed to brines.</p>	
Avainsanat – Keywords bentonite, brine, Donnan, model, montmorillonite, osmosis, repository, salt, swelling pressure	
ISBN ISBN 951-652-039-1	ISSN ISSN 1239-3096
Sivumäärä – Number of pages 35	Kieli – Language English



Posiva-raportti – Posiva report

Posiva Oy
Mikonkatu 15 A, FIN-00100 HELSINKI, FINLAND
Puh. (09) 2280 30 – Int. Tel. +358 9 2280 30

Raportin tunnus – Report code

POSIVA 98-01

Julkaisuaika – Date

Tammikuu 1998

Tekijä(t) – Author(s) Ola Karnland Clay Technology Lund, Sweden	Toimeksiantaja(t) – Commissioned by Posiva Oy
Nimeke – Title BENTONIITIN PAISUNTAPAINEN VÄKEVISSÄ NaCl-LIUOKSISSA — Mallilaskujen korrelointi kokeellisesti saatujen tulosten kanssa	
Tiivistelmä – Abstract <p>On olemassa useita erilaisia kvantitatiivisia malleja, joilla kuvataan bentoniitin paisuntapainetta. Tässä raportissa tarkastellaan joukkoa malleja, jotka saattavat soveltua myös suolaisiin pohjavesi-olosuhteisiin. Suolaisissa olosuhteissa esiintyy poikkeamia mitattujen arvojen ja kaikkien mallien antamien tulosten välillä. Yleensä mallit ennustivat liian pieniä paisuntapaineen arvoja verrattuna mitattuihin arvoihin. Malleissa on käytetty termodynamiikan periaatteita konservatiivisesti ja tuloksia voidaan parantaa lisäämällä osmoottinen malli savi/vesi-systeemiin. Osmoosin vaikutus otetaan huomioon saven kationinvaihtokapasiteetin ja Donnan tasapainoteorian avulla. Tällä mallilla saadut tulokset korreloivat aikaisempaa termodynaamisen mallin konservatiivista käyttöä paremmin kirjallisuudessa esiintyvien laboratoriotulosten kanssa. Muutama asiaa verifioiva laboratoriotesti on myös tehty ja lyhyesti kuvattu tässä raportissa.</p> <p>Parannettu malli ennustaa myös merkittävää bentoniitin paisuntapainetta saturoituneessa natriumkloridiliuoksessa, jos bentoniitin tiheys on riittävän suuri. Käytännössä tämä tarkoittaa, että KBS-3-loppusijoitustilan kompaktoitu bentoniitti sijoitusreiässä säilyttää riittävän paisuntapaineen. Toisaalta bentoniitin tunnelitäyteaineeseen sekoittamisen positiivinen vaikutus häviää korkeissa suolapitoisuuksissa.</p>	
Avainsanat – Keywords bentoniitti, suolavesi, Donnan, malli, montmorilloniitti, osmoosi, loppusijoitustila, suola, paisuntapaine	
ISBN ISBN 951-652-039-1	ISSN ISSN 1239-3096
Sivumäärä – Number of pages 35	Kieli – Language Englanti

TABLE OF CONTENTS

	page
Abstract	
1 BACKGROUND	8
1.1 General	8
1.2 Aim	9
1.3 Principles for bentonite swelling.....	9
2 SWELLING PRESSURE MODELS.....	11
2.1 General.....	11
2.2 Empirical models.....	11
2.2.1 Low's semiempirical model.....	11
2.2.2 Commentary on the models.....	12
2.3 Double layer models	12
2.3.1 General.....	12
2.3.2 Repulsive force between two parallel layers.....	13
2.3.3 Attractive force between two parallel layers	15
2.3.4 Osmotic DDL model.....	16
2.3.5 Commentary on diffuse double layer models	17
2.4 Thermodynamic models.....	17
2.4.1 General.....	17
2.4.2 Clay-water potential.....	18
2.4.3 Quantitative model.....	19
2.4.4 Commentary on the model	20
2.4.5 Donnan exclusion	24
2.4.6 Calculation of swelling pressure	25
2.4.7 Verification tests	29
3 CONCLUSIONS.....	33
REFERENCES	34

1 BACKGROUND

1.1 General

Bentonite clays generally have a high swelling capacity in low electrolyte water solutions, i.e. the bentonite swell in water to a volume several times the original dry clay volume. The swelling gives rise to a macroscopic swelling pressure acting uniformly on the surroundings if the swelling is restricted to a volume smaller than that of maximum swelling. The volume of a KBS-3 deposition hole in rock is in principle fixed and the bentonite mass may be balanced to give a desired swelling pressure, and the coupled low hydraulic conductivity leads to an efficient seal of the volume. This swelling ability is one of the basic reasons for using bentonite clay as buffer material in a repository for spent nuclear fuel.

It is a well known fact that salt solutions reduces the swelling capacity of bentonite, and no swelling pressure will obviously develop if the capacity is reduced to an extent where the actual volume is not filled by the maximum swollen clay. Several laboratory test series have consequently been made in order to determine the effects on bentonite swelling of typical ground-water at repository depth, and of water solutions with considerably higher salt content (Pusch 1980, Karnland 1992, Dickson 1996). The effects have also been considered by means of thermodynamics and the results correspond quite well with the laboratory data also for conservative ground-water conditions.

However, for very high salt content solutions (brines) a conservative use of the thermodynamic model predicts a total loss of swelling pressure which is not in accordance with laboratory test results.

The discrepancy between model and measured values may appear as an academic problem since the discrepancy is only significant at very high salt contents which have never been found at repository depth. An explanation is however desirable from at least the following two quite different aspects:

The discrepancy may throw suspicion upon the reliability of all measured data, also those at lower salt content. Normally, one would reject the model since laboratory results are looked upon as hard facts. This is not appropriate in this case, since the model is based on simple thermodynamic principles and has been commonly used and accepted, and because the laboratory data may represent conditions where equilibrium has not been reached.

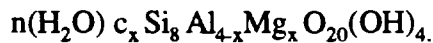
The salt content in the ground-water is observed to increase substantially with depth at several possible repository sites. Although there is no obvious scenario, it can not simply be excluded that there will be a draw-up of deep ground-water into a future repository. Consequently, in order not to reject such places on this ground, it is important to model bentonite swelling also for brine conditions.

1.2 Aim

Several models, both conceptual and quantifying, have been proposed to explain the nature of the swelling pressure and to calculate specific values for different physico-chemical conditions. The basic aims with this report are to correlate existing laboratory data and model data concerning swelling pressure in bentonite exposed to NaCl brines and to discuss discrepancies, if any. Special respect is paid to the thermodynamic model.

1.3 Principles for Bentonite swelling

The physical properties of bentonite are determined by the interaction between water and the smectite component, which usually is montmorillonite. The montmorillonite is derived from pyrophyllite through isomorphic replacement of aluminium by magnesium in the octahedral layer and an ideal formula can be written



The induced negative layer charge (x) is in the range of 0,4 to 1,2 unit charges per $\text{O}_{20}(\text{OH})_4$ -unit and is balanced by interlayer cations (c). The octahedral sheet is composed of magnesium and aluminium coordinated octahedrally with oxygen atoms or hydroxyl groups. The silica tetrahedra are interconnected in a silica sheet structure. Three of the four oxygen atoms in each tetrahedron are shared to form a hexagonal net. Three of the four oxygen atoms in each tetrahedron are shared to form a hexagonal net.

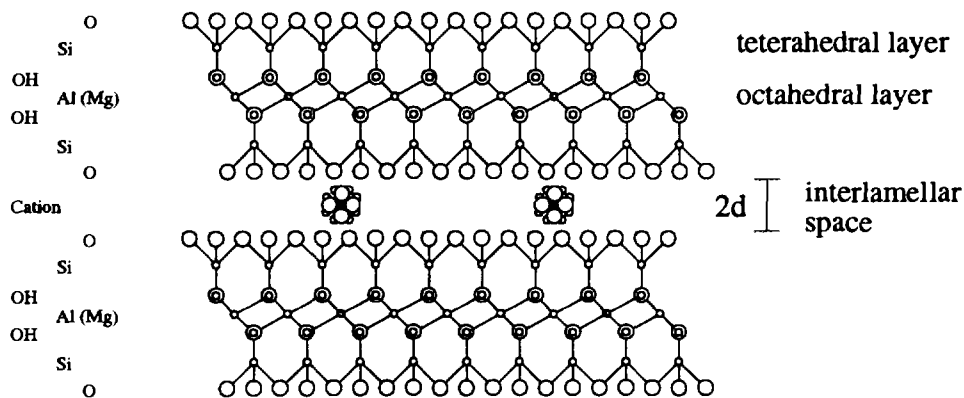


Figure 1-1. Generalised structure of montmorillonite perpendicular to the c-axis. The octahedral Mg (dark circles) substitution for Al results in a net negative charge, which is compensated by interlamellar cations.

The hydration state, or amount of interlayer water, is a function of the extent and location of the 2:1 layer charge (Foster 1953, Harward 1966,) interlayer cation species (Glaeser 1968, Schultz 1969), vapor pressure (Keren 1975, Ormerod 1983), temperature (Rowland 1956, Colten 1986) and salinity of saturating solution (Norrish 1954, Posner 1964, Shang 1995). Continuous variation in basal spacing with salinity and vapor pressure changes has been observed for Na-saturated smectites with basal spacings greater than 40 Å (Norrish 1954).

If expansion is prohibited, the hydration forces will instead give rise to a macroscopic swelling pressure. At equilibrium and full water saturation this pressure is closely related to the water ratio (m_w/m_s , mass of water and mass of solid, respectively) in the clay, which is related to the density of the clay-water system (ρ_m). The swelling pressure may be determined experimentally in different types of geotechnical equipment such as swelling pressure oedometer and triaxial cell. It may also be determined indirectly for an actual bentonite by known relations between swelling pressure and e.g. water vapor pressure, interlamellar spacing, water ratio or density.

2 SWELLING PRESSURE MODELS

2.1 General

Several models for predicting swelling pressure in bentonite clay have been proposed. In this study a few models, representing quite different groups of approaches, are looked at. The models are chosen because they have been presented in connection with repository construction and, more or less, are claimed to handle different physico-chemical conditions including electrolyte solutions in the system. The model groups they represent may be called:

empirical models,
diffuse double-layer models,
thermodynamic models.

The models are briefly presented and some results from calculations of swelling pressure are shown, both for non-saline and saline conditions in order to relate the measured results to the different models.

2.2 Empirical models

2.2.1 Low's semiempirical model

Philip Low early suggested that the interaction of water with clay surfaces produces a hydrostatic repulsive force that contributes to clay swelling (Low 1953). Numerous experiments by Low and co-workers have shown that the properties of water in montmorillonite-water systems obey the empirical equation (Low 1979):

$$J_i = J_i^0 \cdot e^{\frac{\beta_i m_m}{m_w}} \quad [1]$$

where J and J^0 are the values of a property, i , in the montmorillonite-water system and in pure water, respectively, and β is a constant that is characteristic of the property. The relation is alleged to be valid for thermodynamic, hydrodynamic and spectroscopic properties of the water.

Direct measurement of the swelling pressure on 35 different montmorillonites showed a relationship between P_s and the mass ratio of montmorillonite to water (m_m/m_w), (Low 1980), given by

$$\ln (P_s + 1) = \alpha (m_m/m_w) + \ln B, \quad [2]$$

where α and B are constants that are characteristic of the montmorillonite. Since the proportion of crystal edges in montmorillonite is small, Low argues that the interlamellar distance ($2d$ in Figure 1) is related to m_w/m_m , and that the relation in eq. [2] may be written

$$\ln (P_s+1) = k (1/2d) + \ln b \quad [3]$$

or

$$P_s + 1 = \exp (k (1/2d) - (1/\lambda_0))$$

where k is a constant, b is $\exp(-k/\lambda_0)$ and λ_0 is the modal interlamellar distance when the swelling pressure is zero ($P_s = 0$). Combination of equation [1] and [2] yields

$$P_s + 1 = B \cdot \left(\frac{J_i}{J_i^0} \right)^{\frac{\alpha}{\beta_i}}$$

which indicates that the swelling pressure in the system is related to any of the examined properties of the water in the system.

The validity of equation [3] has been examined in several laboratory studies by Low (e.g. Low 1983) by direct parallel measurement of the interlamellar distance and swelling pressure.

Consequently, according to Low it is possible to calculate the swelling pressure by use of equation [3] after the constants k and λ_0 have been determined in laboratory experiments.

2.2.2 Commentary on the models

The model is attractive because of its simplicity. However, two different values of k and λ_0 have to be used in order to describe the swelling of some montmorillonites. Further negative values of λ_0 have to be used in some cases which puts the physical significance in question (Grauer 1986). The effects of solutions with high salt contents are not thoroughly discussed and the model turns out to be of limited direct use for the actual application.

2.3 DOUBLE LAYER MODELS

2.3.1 General

There are several similar theories concerning the distribution of ions between charged surfaces which may generally be identified as double-layer models. The diffuse double layer model (DDL) was independently developed by Gouy and Chapman in 1910. The main purpose of the models, with respect to clay swelling, is to determine the mid point electrical potential (ϕ_d) between two parallel montmorillonite clay flakes. If the potential is properly determined for a certain physico-chemical condition it should in principle be

possible to calculate the repulsive force arising between the flakes, according to the Langmuir equation (below). The repelling force between the flakes will be counteracted by attraction forces, (van der Waals force) which can be calculated if the distances between the flakes can be determined.

The sections 2.3.2 and 2.3.3 describe the approach by Komine and Ogata in which they calculate the swelling pressure on the basis of Diffuse Double Layer (DDL) Theory. The reasons for using this as an example of DDL models are that the Komine and Ogata article is one of the most recently presented on the subject, and they have compared the results with laboratory data for different ion concentrations (Komine 1996).

The Komine and Ogata results have been reproduced and new calculations for the Posiva/SKB reference bentonite material MX-80 have been made with special respect to high NaCl concentrations in the saturating water solution. The original constants and MX-80 material specific data are shown in table 2-1, and the calculation results are shown in Figure 2-1.

Section 2.3.4 describes a model presented by Young and Warkentin where the swelling pressure is calculated as an osmotic pressure of the difference in concentration of ions between clay particles and in an external solution. Calculation of the concentration between the clay flakes is made by use of a DDL model.

2.3.2 Repulsive force between two parallel layers

A repulsive force is generated between two parallel layers if the two layers approach each other and their electrical potential distribution overlap. The repulsive force per unit area between two parallel layers is given by eqs. [1]-[3], which are derived from the electrical potential distribution around the two charged layers (Langmuir 1938, Olphen 1963, Mitchel 1993).

$$f_r = 2nkT(\cosh u - 1) \quad [4]$$

$$u = 2y_d \quad [5]$$

$$y_d = \frac{ve'\phi_d}{kT} \quad [6]$$

where f_r is the repulsive force per unit area between the two parallel layers (J/m^3), v is the ionic valence, e' is the electric charge ($=1.602 \times 10^{-19}$), k is the Boltzmann constant ($=1.38 \times 10^{-23} J/K$), T is the absolute temperature (K), n is the concentration of ions (number/ m^3) in pore water, u is the variable obtained by ϕ_d , which is the electrical potential at the center of the two parallel layers.

According to the Gouy-Chapman theory of diffuse double layer (Chapman 1913, Mitchel 1993) the distribution of the electrical potential in the clay water electrolyte system around the clay layer is given by:

$$\frac{d\phi}{dx} = -\sqrt{\frac{2nkT}{\epsilon}} 2 \sinh\left(\frac{ve'\phi}{2kT}\right) \quad [7]$$

$$\phi_0 = \frac{kTz}{ve'} \quad [8]$$

$$\text{for } z = 2 \sinh^{-1}\left(96.5 \times \frac{B}{S} \sqrt{\frac{1}{8\epsilon nkT}}\right)$$

where x is the position from the layer surface (m), ϕ is the electrical potential at position x , ϵ is the static permittivity of the water solution ($C^2 \cdot J^{-1} \cdot m^{-1}$), B is the cation exchange capacity (mequiv./g) and S is the specific surface (m^2/g) of the clay.

Eq. [4] has been solved for the conditions $\phi=\phi_0$ for $x=0$, and $\phi=\phi_d$ for $x=d$, which gave:

$$y_d = 4 \tanh^{-1} \left[\exp(-\kappa d) \tanh\left(\frac{z}{4}\right) \right] \quad [9]$$

$$\text{for } \kappa = \sqrt{\frac{2nv^2e'^2}{\epsilon kT}} \quad (m^{-1})$$

where d is half the distance between the two layers (m) (Figure 1-1), and κ is the double-layer parameter. The characteristic length ($1/\kappa$) is a measure of the thickness of the double layer (Iwata 1988; Mitchell 1993).

Therefore, the repulsive force per unit area, or simply the resulting pressure, between two parallel layers is given by

$$\begin{aligned} f_r &= 2nkT(\cosh u - 1) & (J/m^3) & [10] \\ &= 2nkT(\cosh u - 1) \times 10^{-6} & (MPa) \end{aligned}$$

$$\text{for } u = 2y_d = 8 \tanh^{-1} \left[\exp(-\kappa d) \tanh\left(\frac{z}{4}\right) \right]$$

$$\kappa = \sqrt{\frac{2nv^2e'^2}{\epsilon kT}}$$

$$z = 2 \sinh^{-1}\left(96.5 \times \frac{B}{S} \sqrt{\frac{1}{8\epsilon nkT}}\right)$$

2.3.3 Attractive force between two parallel layers

It is well known that attractive forces act between two layers if they approach extremely close to each other (Iwata 1988; Mitchell 1993). London's theory for attractive energy between a pair of molecules (London 1937) was extended to obtain the attractive energy between two parallel layers (Casimir 1948). The attractive force per unit area, which is commonly termed the van der Waals force, is given by

$$f_a = \frac{A_h}{24\pi} \left[\frac{1}{d^3} + \frac{1}{(d+t)^3} - \frac{2}{\left(d + \frac{t}{2}\right)^3} \right] \quad (\text{J/m}^3) \quad [11]$$

where f_a is the attractive force per unit area between the two parallel layers, A_h is the Hamaker constant, d is half the distance between the two layers (m), and t is the thickness of layer (m). The value of A_h calculated from theoretical analysis of coagulation measurements is 2.2×10^{-20} J for montmorillonite (Novich 1984).

Therefore, the force P per unit area (resulting pressure) between two parallel layers can be obtained by

$$P = f_r - f_a \quad (\text{repulsion: positive}) \quad [12]$$

Table 2-1. Parameters in the prediction method for swelling characteristics of compacted bentonite

Density of bentonite ρ_s	2.78 Mg/m ³
Specific surface of montmorillonite minerals S_m	750 m ² /g
Specific surface of nonswelling minerals S_{nm}	0 m ² /g
Montmorillonite content of bentonite C_m	75%
Cation exchange capacity B	0.8 eq./kg
Ionic valence ν	1
Absolute temperature T	295 K
Boltzmann constant k	1.38×10^{-23} J/K
Hamaker constant A_h	2.2×10^{-20} J
Electric charge e'	1.602×10^{-19} C
Static permittivity of pore water ϵ	$80 \times 8.8542 \times 10^{-12}$ C ² ·J ⁻¹ ·m ⁻¹
Thickness of montmorillonite layer t	9.60×10^{-10} m
Avogadro's number N_A	6.023×10^{23}
Concentration of ions in pore water n_0	20, 40, 86 and 600 mol/m ³

Results from calculation by use of eq. [12] and the constants in table 2-1 for MX-80 Posiva/SKB reference bentonite are shown in Figure 2-1. The model is sensitive to ion charge and the use of monovalent cations give calculated values of around one order of magnitude higher pressure compared to divalent ones. Komine and Ogata used the value

$\nu=1,5$ since the examined Kunigel material has approximately equal number of sodium and calcium ions in exchange position. The presented values are based on monovalent sodium. High content of salt in the solution lead to major changes in swelling pressure and saturated NaCl solution (6.1 M, 36% b.w.) gives total loss of swelling pressure for all clay densities.

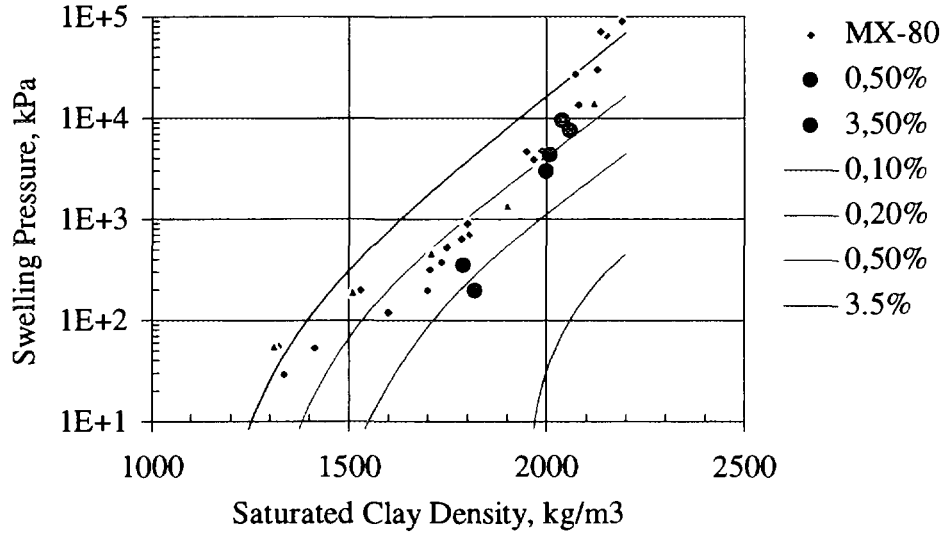


Figure 2-1 Swelling pressure as a function of clay density at full saturation as calculated by the Komine and Ogata approach of the diffuse double layer theory. Dots indicate measured values and lines results from calculations.

2.3.4 Osmotic DDL model

The swelling pressure can be calculated as an osmotic pressure by means of the Van't Hoff equation (Yong 1975).

$$P_s = RT(C_c - 2C_0) \quad [13]$$

where R is the gas constant, T is the absolute temperature, C_c (mole/liter) is the concentration of cations in the center between two clay flakes and C_0 (mole/liter) is the concentration of salt in the bulk water.

C_c may be calculated by

$$C_c = \frac{\pi^2}{z^2 B(d + x_0)^2 \cdot 10^{16}} \quad [14]$$

where z is the valence of the exchangeable cations, d is the half distance between the flakes in Å (10^{-10} m), x_0 is a correction factor of 1 to $4 \cdot 10^{-10}$ m depending on the ion valence and charge density of the flakes, and B is a constant with the value of $1 \cdot 10^{15}$ cm/mole and represent the combined effect of temperature and dielectric constant.

2.3.5 Commentary on diffuse double layer models

At very close particle separation distances there is an apparent problem with the basic principle of the diffuse double layers due to the spacial limitation. For densities of current interest in the buffer material the theoretical mean spacing is around 9 Å which has to be considered as very close since it only allows for a few water molecules. Further, the interlamellar spacing between the flakes will be even shorter if there is a significant distribution in spacing due to inhomogeneity in the system,

Komine and Ogata conclude in there study that the model well predicts the swelling pressure up to a pressure of 3 Mpa, which correspond to a clay density in the lower range of possible buffer densities. The attractive forces are only dependent on basal spacing and are not affected by any of the conditions which affect the repelling forces. The calculated attractive forces were relatively small compared to the calculated total pressures also at small basal distances. In the present calculation of pressure in MX-80 bentonite there is no major difference in accuracy for high pressure values compared to lower. The ability to predict the effect of different salt content seems, however, to be very limited. Only NaCl contents in the water solution of less that 0.5% are tolerable in the range of measured values.

According to Yong it is possible to compensate for the conditions at very close spacing by adding a hydration energy component to the expressions, by paying particular attention to the counter ions in the Stern layer. To calculate the energies of interaction in this layer, it is necessary to estimate the concentration of ions at the inner and outer Helmholtz plane, and to determine the energies involved in the hydration of the ions (Yong 1992). Calculations of swelling pressure for Na-montmorillonite, by use of an additional term, fits measured values quite well up to a swelling pressure of 30 MPa. However, at high salt concentrations, the measured pressures exceed calculated values. Yong proposes that this may be due to the use of concentrations instead of activities.

The models obviously suffer from the weaknesses of the double-layer theory; there are several double-layer models available, there are difficulties involved in their mathematical treatment, and in the selection of numerical values to be used. E.g. values between 2 and 50 are available for the relative dielectric constant of water on phyllosilicates (Sposito 1984). The problem is usually avoided in practice, as done above by Komine and Ogata, by using the value for pure water ($\epsilon_w = 80$).

2.4 Thermodynamic models

2.4.1 General

A thermodynamic model describes the properties of the macroscopic clay water system. Consequently, it is not possible to draw any direct unambiguous conclusions on the swelling mechanism and the structure of adsorbed water from this concept. Introduction of microscopic parameters may lead to unnecessary confusion (Sposito 1972).

Clay affinity for water leads to a low water vapor pressure in a dry clay. Water will thereby be transported into the clay volume, if the clay is contacted to a water solution with a higher vapor pressure. The uptake of water increases the vapor pressure in the clay-water system successively. A pressure will develop in this system if the vapor pressure is still lower, compared to that of the solution, at the time when the entire free space in the clay volume is filled with water. The magnitude of the pressure at equilibrium is governed by the difference in vapor pressure between the clay-water system and the solution, and may be calculated by means of thermodynamics (Sposito, 1972).

2.4.2 Clay-water potential

The following section is based on Yongs description of soil-water potential (1992).

Buckingham (1907) defined the capillary potential of a soil as the work required per unit weight of water to pull water away from the mass of soil. The potential in this approach is due to capillary forces at the air-water interfaces in the soil pores holding water in the soil.

In soil mechanics it is often more convenient to define the potential in terms of the water phase (soil-water potential, or free energy of the water in the soil), defined as the work required per unit weight of water to place water into a mass of soil. This leads to a change of sign and to increasing potential with increasing water content in the soil water system. The potential is consequently less negative in value as the water content increases, or in other words, the water vapor pressure is higher as the water content increases.

The soil-water potential of a soil-water system may be experimentally determined for various physico-chemical conditions. The values then represent the total soil-water potentials which may be divided in several water potential determining components.

- The matric potential, which is a matrix property and may be divided into one component due to air-water interface forces (matrix steric effect), and a second component due to swelling forces (matrix osmotic effect). Calculations of the matric potential for the two components will thereby be quite different. The water-air interface forces may be calculated for an ideal system by use of capillary equations for the appropriate geometry, and swelling forces may be calculated by DDL theory.

There is no simple way to separate these components experimentally in studies on partly saturated soils. In a fully saturated system, the air-water interface component is obviously zero, which give the possibility of determining the swelling component.

- The osmotic potential, which is related to the dissolved ions in the soil-water system and may be calculated from the concentration in the soil solution.

- The gravitational potential, which is the work required to transfer water from the reference elevation to the soil elevation.
- The pneumatic or pressure potential.

2.4.3 Quantitative model

The connection between swelling pressure P_s and relative partial free energy on the basis of equilibrium thermodynamics have been derived and discussed in several articles (Low 1958, Sposito 1972, Kahr 1990) and may be summarized in the following way (Grauer 1986).

Low and Anderson have shown that:

$$\bar{g}_w - g_w^\circ = -\bar{v}_w \cdot P_s \quad [15]$$

For changes in enthalpy, the following thermodynamic relation applies:

$$(\bar{g}_w - g_w^\circ) = (\bar{h}_w - h_w^\circ) - T \cdot (\bar{s}_w - s_w^\circ) \quad [16]$$

there results

$$P_s = -\frac{\bar{h}_w - h_w^\circ}{\bar{v}_w} + T \frac{\bar{s}_w - s_w^\circ}{\bar{v}_w} \quad [17]$$

Since

$$\bar{G}_w - G_w^\circ = M(\bar{g}_w - g_w^\circ) = RT \ln \frac{p}{p_0} \quad [18]$$

with equation (15) there results

$$P_s = -\frac{RT}{M \cdot \bar{v}_w} \ln \frac{p}{p_0} \quad [19]$$

where

$\bar{G}_w - G_w^\circ$: relative partial molar free energy of water in the clay/water system ($\text{J} \cdot \text{mol}^{-1}$).

$\bar{g}_w - g_w^\circ$: relative partial specific free energy of water in the clay/water system ($\text{J} \cdot \text{g}^{-1}$).
The corresponding relative partial parameters enthalpy (h) and entropy (s) are similarly defined.

\bar{v}_w : partial specific volume of the water. Empirical correction $\bar{v}_w = 1.002 \exp(0.036 \frac{m_w}{m_s})$ (Oliphant 1982).

p, p_0 : vapour pressure of adsorbed and pure water

M : molecular weight of the water

The vapor pressure in a certain clay-water system may be determined at equilibrium for various amounts of water adsorbed by the clay and the swelling pressure of the system may be calculated from eq. [19].

The equation can also be applied to salt-containing solutions (Low 1987). If the clay is equilibrated with an aqueous electrolyte solution instead of pure water, G_w^0 is replaced by the partial free energy of the water in the solution (Low 1958). This is equivalent to replacing the vapor pressure of pure water p_0 in eq. [19] by the the vapor pressure in the electrolyte solution (p_e) as shown in eq. [20]. This leads to a reduction of the swelling pressure P_s since the vapor pressure is lower in the salt solution.

$$P_s = -\frac{RT}{M \cdot \bar{v}_w} \ln \frac{p}{p_e} \quad p_e < p_0 \quad [20]$$

Since vapor pressure in electrolytes are tabulated for differnt concentrations it is possible to calculate the swelling pressure for a certain clay by use of equation [20] if the vapor pressure in the clay (p) is known.

2.4.4 Commentary on the model

Alternatively, equation [20] may be expressed

$$P_s = -\frac{RT}{M \cdot \bar{v}_w} \ln \left(\frac{p}{p_0} \right) - \left(-\frac{RT}{M \cdot \bar{v}_w} \ln \left(\frac{p_e}{p_0} \right) \right) \quad [21]$$

The first term represents swelling pressure of the clay according to eq. [19], and the second term represents the osmotic pressure of the surrounding salt solution (cf Figure 2-2). From eq. [21] it is obvious that the swelling pressure of the system will be zero if the vapor pressure in the clay-water system (p) is equal to the one in the surrounding solution (p_e).

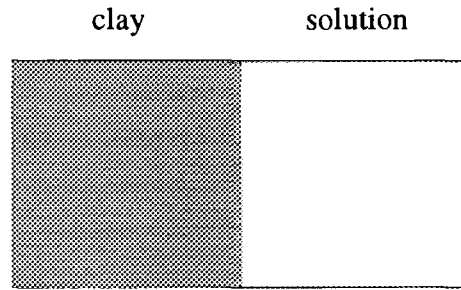


Figure 2-2 Principle clay-water/water-solution system. cf. to eq. [21]. The two physical volumes are separated by a semipermeable membrane permeable to dissolved ions and water, but not to the mineral flakes. The membrane may represent a filter in a laboratory test, the rock-buffer interface or internal membrane functions in the clay volume.

A conservative use of eq. [21], with respect to the preservation of swelling pressure, is to assume that the vapor pressure (p) in the clay-water system is not changed by the surrounding salt solution, i.e. the partial molar free energy of water (\bar{G}_w) in the clay-water system is not changed.

General calculations of swelling pressure as a function of density for different salt contents in the saturating solution, by use of this approach, are shown in Figure 2-3. The small dots represent measured pressure values of MX-80 bentonite saturated by pure water. The rightward curve show calculated pressures in a clay equilibrated with a saturated NaCl solution (6.1 M, 36% b.w.). Round open and filled larger dots represent measured values of samples saturated by 0.5 and 3.5% NaCl solutions, respectively.

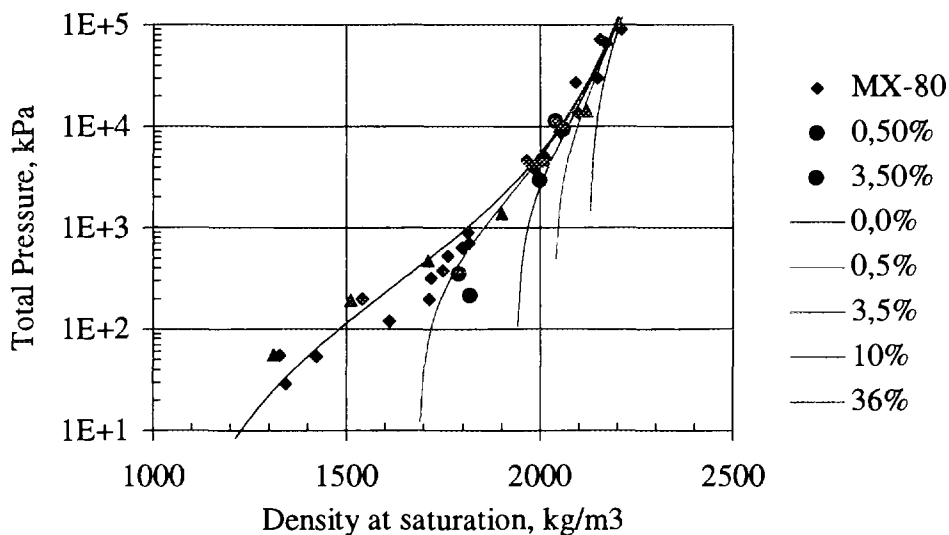


Figure 2-3. Results from measured swelling pressure (dots) and calculated pressures according to eq. [19] (lines). Figures show percent NaCl by weight in actual and imagined solutions.

In a KBS-3 type repository the proposed bentonite buffer density is 2000 kg/m^3 at full water saturation, corresponding to a water/clay ratio of 26%. The corresponding water vapor ratio (p/p_0) has been determined for MX-80 bentonite to be around 0.96, if saturated by pure water (Kahr, 1986). The corresponding water vapor ratio (p_e/p_0) in a NaCl solution is given by a concentration of approximately 1.25 M (70 g NaCl in 1 liter H_2O , between blue and red curve in Figure 2-3). The swelling pressure would consequently be zero for this combination of water vapor ratios in the clay-water system and in the surrounding water solution. At higher electrolyte concentrations the consequence of this use of equation [21] is, in addition to the lack of swelling pressure, also dehydration of the clay due to the dominant osmotic term.

However, literature data do not support the lack of swelling pressure, or dehydration, if bentonite clay at buffer conditions is saturated by brines (Dixon, 1996). In Figure 2-3 only the calculated data for 0.5% NaCl solution fits with experimental data. A first extreme verification test was made in this study, by use of a swelling pressure oedometer, in order to confirm the literature data. Airdry MX-80 bentonite was compacted to a density corresponding to 2000 kg/m^3 at full water saturation, and the clay was contacted to a saturated NaCl solution (6.1 M, 36% NaCl b.w.). The macroscopic swelling pressure in the clay was measured during water uptake and the course is shown in Figure 2-4.

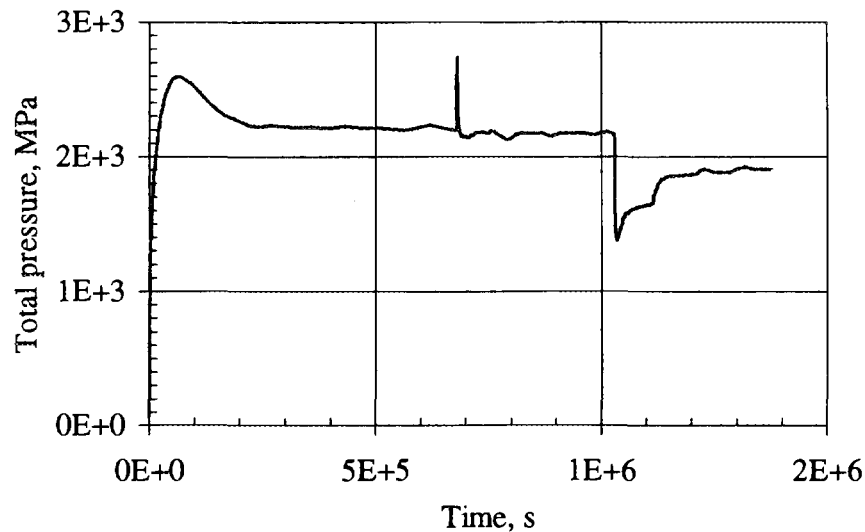


Figure 2-4. Swelling pressure built-up in MX-80 clay saturated by a 6.1 M NaCl solution. The clay density correspond to the proposed KBS-3 buffer (2000 kg/m^3 at full saturation).

In order to ensure that the recorded pressure did not originate from released compaction tension, the clay-water volume was increased after pressure had stabilized (after around $1 \text{E}6 \text{ s}$) and the pressure was thereby significantly reduced. The pressure was once again built up to a value of almost 2 MPa which strongly indicates that the pressure was related to true swelling. The expected pressure for a corresponding sample saturated with pure water is around 5 MPa.

Consequently, the experimental data strongly indicate that the use of eq. [21] is too conservative and not adequate for high salt concentrations. The expression does not take into account that ions may enter the clay-water system along with the saturating solution and thereby reduce the partial molar free energy of water (\bar{G}_w) in the clay-water system. This may be treated as an additional osmotic component acting in the clay-water volume according to section 2.4.1, and eq. [21] may then be completed by an osmotic term:

$$P_s = -\frac{RT}{M \cdot \bar{v}_w} \ln \frac{p}{p_0} + \left(-\frac{RT}{M \cdot \bar{v}_w} \ln \frac{p_{ie}}{p_0} \right) - \left(-\frac{RT}{M \cdot \bar{v}_w} \ln \frac{p_e}{p_0} \right) \quad [22]$$

where p is the vapor pressure in the clay-water solution as saturated by pure water, p_{ie} is the vapor pressure in a water solution with a concentration corresponding to the increase of ion concentration in the clay-water system as saturated by the electrolyte solution.

If the surrounding solution freely could enter the clay, without changes in ion content, this would lead to an internal osmotic component in the clay which fully would compensate for the external osmotic effect (i.e. $p_{ie} = p_e$). In other words, the swelling pressure would be unaffected by a salt solution, which nor is in agreement with the experimental data. Obviously, the laboratory data suggests that originally external ions are present in the clay pore water at equilibrium, and that the concentration is lower than in the surrounding water solution.

The validity of this approach may be examined by at least the three following techniques:

- Direct measurement of the vapor pressure in the clay at different physico-chemical conditions (e.g. densities and salt contents).
- Direct measurements of the ion content in the clay pore-water at different physico-chemical conditions.
- Calculation of the amount of ions in the clay pore solution for various external salt solution by use of the Donnan expression (section 2.4.5). The resulting calculated total pressures may then be compared with actual measured swelling pressures.

The latter technique, if the principle turns out to well describe the actual conditions, admit calculation of the swelling pressure in a clay-water system for any given salt solution if the swelling pressure, or vapor pressure, is known for the clay as saturated by pure water.

2.4.5 Donnan exclusion

The montmorillonite part in bentonite clay is composed of the negatively charged mineral flakes and charge compensating cations (Figure 1.1). The mineral flakes may consequently be seen as macro-ions and the system may be regarded as a polyelectrolyte. Donnan (1911) showed that a system similar to that in Figure 2-5 may lead to an unequal distribution of ions which are diffusible across a membrane separating the two volumes.

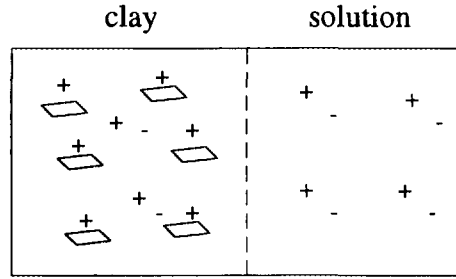


Figure 2-5. principle clay-water/water-solution system. Clay mineral flakes and charge compensating cations on the left side, and sodium and chloride ions on both sides of the filter. The number of chlorid ions is lower on the clay side of the separating membrane compared to the external solution.

The condition for equilibrium is that the chemical potentials be equal on both sides of the membrane for all components which can permeate. Since the macro-sized mineral flakes can not pass the membrane, the equilibrium will be established only for ions in the saturating solution and the original montmorillonite cations. Idealized, this will only be Na^+ and Cl^- . The conditions of electrical neutrality in the entire system is

$$zC_m = C_{\text{Cl}^-} - C_{\text{Na}^+} \quad [23]$$

where z is the valence of the macromolecule, C_m , C_{Na^+} and C_{Cl^-} is the concentration of the macromolecules, sodium ions and chloride ions, respectively.

In terms of activity the equilibrium condition for the diffusible ions will be

$$\{\text{Na}^+_c\}\{\text{Cl}^-_e\} = \{\text{Na}^+_e\}\{\text{Cl}^-_c\} \quad [24]$$

Where the indices c and e refer to the clay-water system and to the surrounding salt solution, respectively. Approximation of the involved activity factors to a uniform value is reasonable since the concentrations of interest are all generally high. This gives

$$[\text{Na}^+_c][\text{Cl}^-_e] = [\text{Na}^+_e][\text{Cl}^-_c] \quad [25]$$

The $[\text{Na}^+_c]$ factor is the sum of the concentration of original charge compensating ions $[\text{Na}^+_{cc}]$ and introduced electrolyte ions $[\text{Na}^+_{ce}]$. Lokal electrical neutrality has to be established why $[\text{Na}^+_e] = [\text{Cl}^-_e]$ and $[\text{Na}^+_{cc}] = [\text{Cl}^-_{cc}]$ and eq. [25] may be written

$$([\text{Na}^+_{cc}] + [\text{Na}^+_{ce}]) [\text{Na}^+_{ce}] = [\text{Na}^+_e] [\text{Na}^+_e] \quad [26]$$

The total concentration of the original charge compensating ions $[\text{Na}^+_{cc}]$ are calculable for any water content from the clay CEC value, and the concentration of the surrounding solution may be determined. Consequently, it is possible to calculate the concentration of sodium ion introduced to the clay-water system according to

$$[Na_{ce}^+] = \frac{-[Na_{cc}^+] \pm \sqrt{[Na_{cc}^+]^2 + 4 \cdot [Na_e^+]^2}}{2} \quad [27]$$

and thereby also the total number of ions in the clay-water system.

2.4.6 Calculation of swelling pressure

Since the concentrations of the ions introduced into the clay are possible to calculate, it is convenient to calculate the effect of the osmotic pressure directly from concentrations. This may be done by use of van't Hoff equation:

$$P_o = i \cdot C \cdot R \cdot T \quad [28]$$

where P_o is the osmotic pressure, i is the van't Hoff factor, C is the concentration of solute in moles per liter, R is the gas constant and T is the absolute temperature.

Eq. [21] may now be rewritten as

$$P_s = -\frac{RT}{M \cdot \bar{v}_w} \ln \frac{p}{p_0} - i \cdot R \cdot T (C_e - C_{ce}) \quad [29]$$

where C_e is the electrolyte concentration in the surrounding water solution and C_{ce} is the concentration of the introduced solution.

The first term in eq. [29], representing the clay component of swelling pressure, may be replaced by experimental data of bentonite saturated by pure water. The advantage by doing this is that possible experimental artifacts will be similar in calculated and measured values and the effects of the salt solutions may be better compared.

In order to facilitate the calculations, the experimental swelling pressure values are adapted to a function of the water/clay ratio, by use of an expression based on the Langmuir equation [1]. The swelling pressure can then be calculated as a function of the same water/clay ratio as used in order to calculate the concentration of charge compensating ions in the original clay. The accuracy of the adaption is indicated by the actual measured values in the various diagrams.

A specific issue in eq. [29] is the van't Hoff factor which by definition may vary between 1 and 2 for a 1:1 salt. The value is expected to be close to 2 in dilute solutions of strong electrolytes such as NaCl. Empirically it is found to decrease as the concentration increases. The most conservative use of eq. [29] is to assume the van't Hoff factor to be 2 (Figure 2-6). This gives a far better correlation between measured and calculated values than in the previous use of eq. [21] (Figure 2-3). The best fit, however, is arrived at if the van't Hoff factor is set to 1. (Figure 2-7). The physical meaning of this is that no

free ions exist in the solutions, or that the sodium and chloride ions act as pairs, which makes this value less probable.

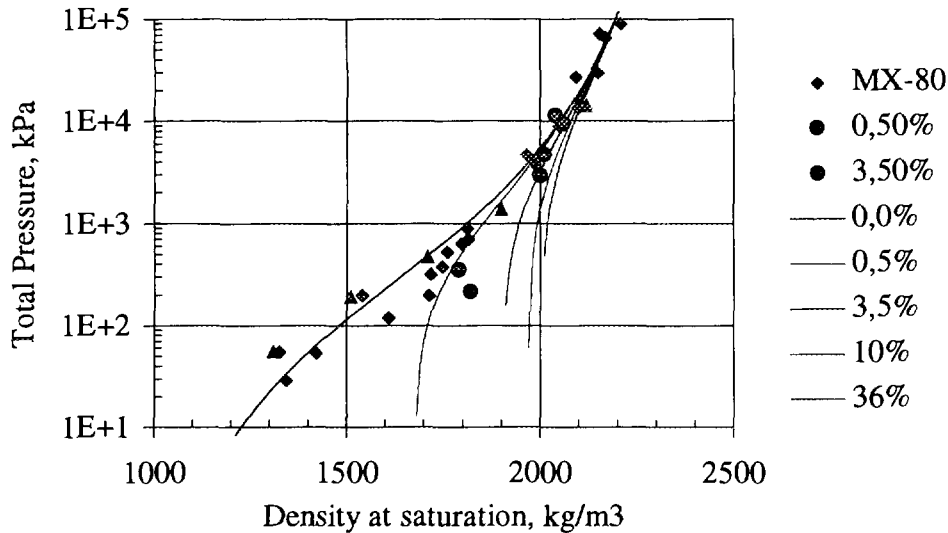


Figure 2-6. Results from measured swelling pressure (dots) and calculated pressures according to eq. [19] (lines). Figures show percent NaCl by weight in actual and imagined solutions. Calculations are made by use of eq [29] and $i = 2$.

Figure 2-3 and 2-6, respectively, show the total pressure without and with the internal osmotic term as calculated by use of the Donnan equation. Although the latter represents the most possible conservative i -values ($i=2$) it still gives a much better fit to experimental data. An alternative presentation of the calculated total pressures for specific buffer densities is shown in Figure 2-7 and 2-8.

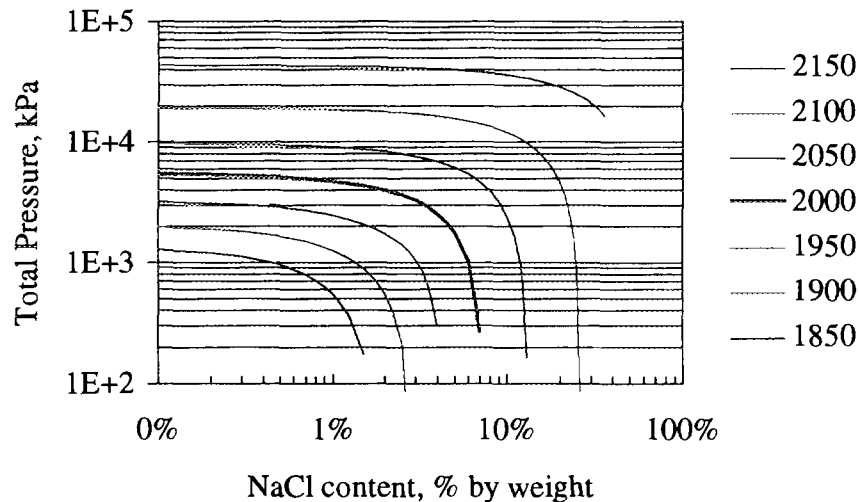


Figure 2-7. Calculated total pressure in the clay-water system without an internal osmotic term. Figures indicate clay density at saturation in kg/m^3 . Zero pressure is postulated for the proposed buffer density at a NaCl content of approximately 7%.

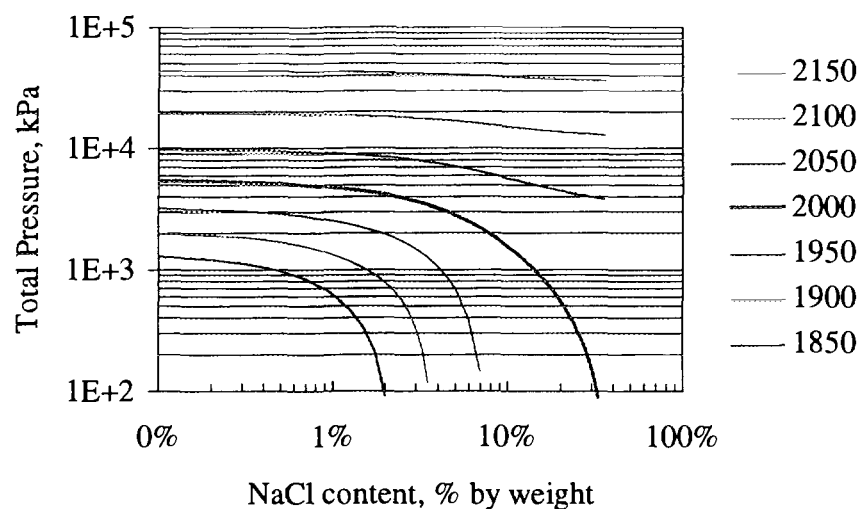


Figure 2-8. Calculated total pressure in the clay-water system including an internal osmotic term calculated by use of the Donnan expression. Figures indicate clay density (kg/m^3) at saturation. A substantial swelling pressure is postulated for the proposed buffer density also for brine conditions.

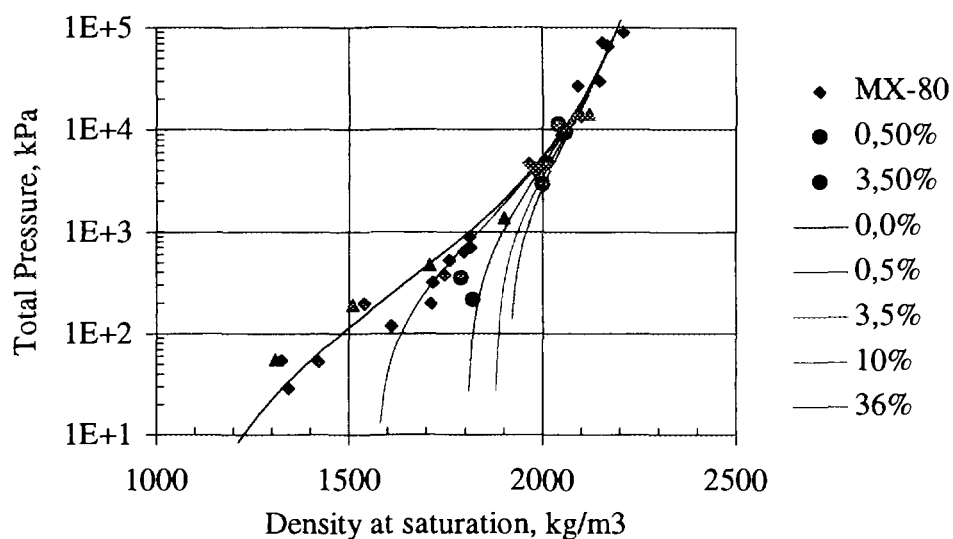


Figure 2-9. Calculations made by use of eq. [29] and $i = 1$.

Literature values of osmotic pressure for relatively concentrated NaCl solutions may be well reproduced by use of eq. [28] and a constant i -value of 1.9 as shown in Table 2-2. The use of this probable i -value gives the calculated pressures shown in Figure 2-10.

Table 2-2 Calculated osmotic pressure by use of eq. [28] and $i=1.9$, and literature values (Lang, 1967).

[NaCl]	NaCl	p/p ₀ , eq. [21]	P ₀ , eq. [28]	P ₀ lit. values
M	% b. w.	-	kPa	kPa
0.5	2.93	0.984	2354	2281
1.0	5.85	0.968	4708	4640
1.5	8.78	0.952	7061	7134
2.0	11.70	0.937	9415	9780

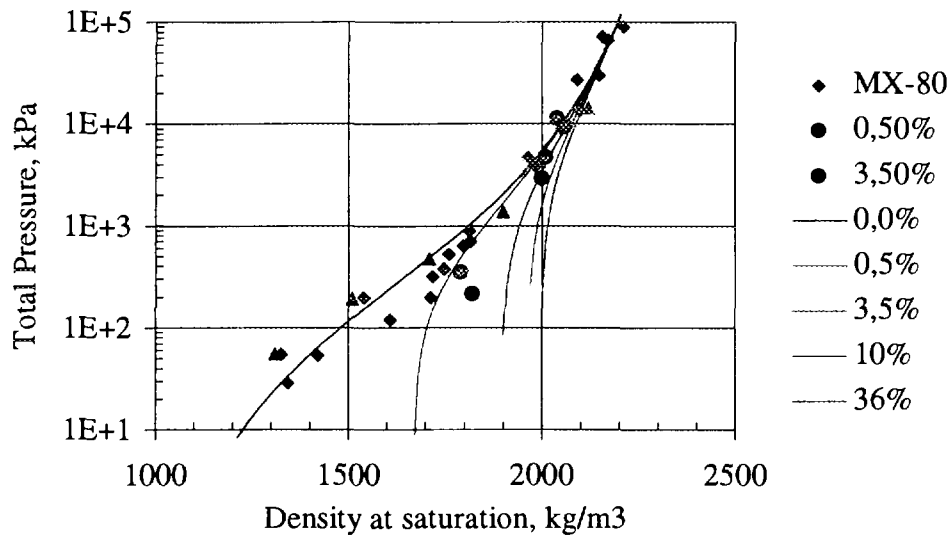


Figure 2-10. Calculations made by use of eq. [29] and $i = 1.9$.

In order to even better hit the experimental data the following approach was tested.

The calculations presented so far have considered the charge balancing cations in the montmorillonite as free ions. The van't Hoff factor reflects that all possible ions are not acting as individual particles even in strong electrolytes as NaCl. In the montmorillonite polyelectrolyte the conditions may be similar, i.e. not all charge balancing cations may act as free ions for all physico-chemical conditions. If so, the Donnan exclusion will be less significant than previously calculated. A "van't Hoff factor" may therefore be proposed to be used also for the montmorillonite. The effect of the negatively charged montmorillonite flakes is zero since the calculations only concerns the diffusible ions. Also with respect to osmosis the macro-sized flakes may be considered as insignificant since this is a colligative property. Thus, the "clay van't Hoff factor" (i_{clay}) has a theoretical value between 0 and 1 as calculated from sodium concentration.

A "clay van't Hoff factor" may seem as a "fudge factor", but the physical meaning is clear and it is possible to indirectly determine the correctness of a certain value by chemical analysis of the clay at equilibrium. Further, a constant value, for all conditions with respect to NaCl concentration and clay density, seems to be sufficient in order to

correlate calculated and measured pressure values. Speculatively, this indicate that an i_{clay} -value may be used as a material constant depending on the factors described in section 1.3 (cf. the constants proposed by Yong and Warkentin eq. [11]). The result from calculations with $i_{\text{clay}} = 0.3$ for MX-80 bentonite is shown in Figure 2-11.

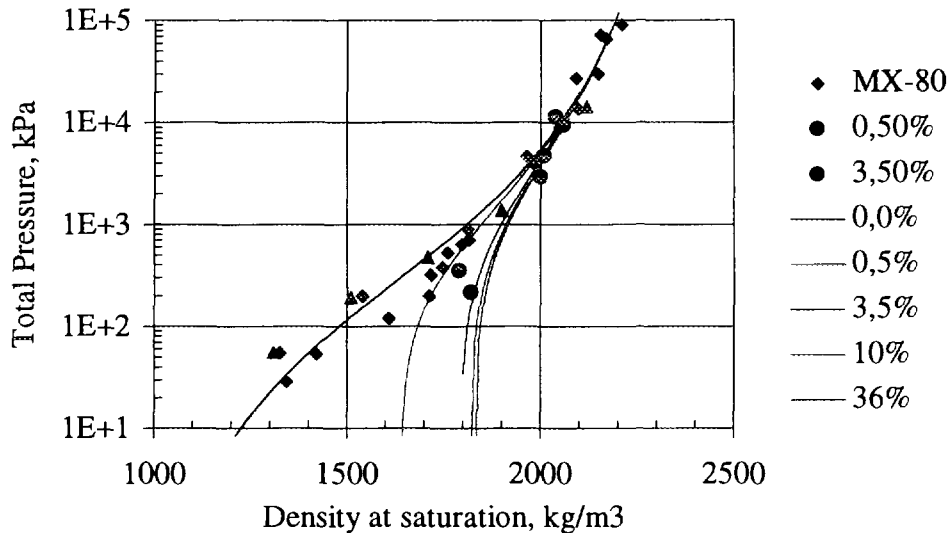


Figure 2-11. Results from measured swelling pressure (dots) and calculated pressures according to eq. [19] (lines). Figures show percent NaCl by weight in actual and imagined solutions. Calculations are made by use of eq. [29] and $i_{\text{clay}} = 0.3$ and $i_{\text{NaCl}} = 1.9$.

2.4.7 Verification tests

The thermodynamic model indicates a number of implications for testing bentonite material and for buffer application. The following examples are of special interest.

- Original salt in the bentonite material may not unambiguous be regarded as conservative test conditions with respect to swelling pressure preservation, since the salt may add an osmotic component.
- Presaturation of a bentonite buffer by use of pure water may under certain condition increase the negative effects of saline ground-water on swelling pressure instead of reducing them.
- No swelling pressure will be present at bentonite densities lower than 1.8 g/cm^3 in a saturated NaCl solution
- CaCl_2 solutions will have approximately the same effect on swelling pressure as NaCl solutions for bentonite at densities of current interest.

The latter consequence is an effect of the colligative nature of osmosis. If the solution concentration is expressed in moles per liter, the same concentration value for NaCl and CaCl₂ accordingly results in a slightly higher effect of the CaCl₂ solution (theoretically 3/2 times). If the values are given in percent by weight or as chloride contents the effect is reversed. This has been substantiated in previous laboratory tests for densities between 1800 and 2100 kg/m³ (Karlund 1992).

Three additional verification tests were made to further check these consequences predicted by the model. The conditions and results were the following:

Test V2

MX-80 bentonite was prepared to a clay-water density corresponding to 2000 kg/m³ after saturation with pure water. The sample was thoroughly mixed with finely ground sodium chloride powder before saturation. The amount of salt corresponded to a fully saturated solution in the pore-water at the actual density. The sample was thereafter saturated by a 6.1 M sodium chloride solution. The development of swelling pressure is shown in Figure 2-12.

The final swelling pressure was approximately 5 MPa, which is twice the measured value for conditions without internal salt, and what is expected if the osmotic internal and external effects of sodium chloride are the same. The recorded high pressure agrees with the model predictions, both in principle and in amount. However, an artifact may be the volume of non-dissolved sodium chloride which may act as bulk material, and thereby increase the bentonite density.

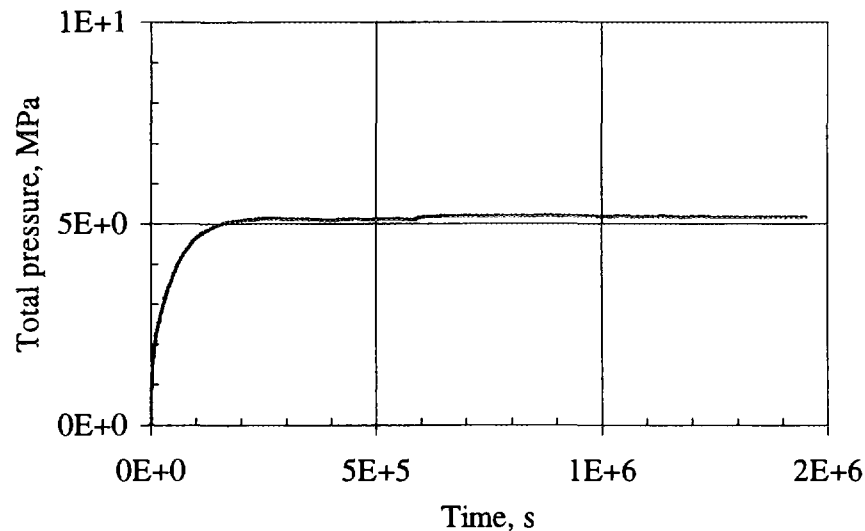


Figure 2-12. Measured swelling pressure in oedometer test with MX-80 clay mixed with powdered NaCl at a clay-water density of 2000 kg/m³ and saturated by 6.1 M NaCl solution.

Test V3

MX-80 clay-water density corresponding to 2000 kg/m³ after saturation with pure water. The sample was presaturated by deionized water and thereafter contacted to a saturated NaCl solution (6.1 M). The swelling pressure development is shown in Figure 2-13.

The marked drop in swelling pressure due to the contact to the salt solution is in agreement with model predictions. The residual final swelling pressure was in principle the same as what was measured when the clay was initially saturated by a 6.1 M NaCl solution and may be explained by a relatively fast diffusion of ions into the clay from the salt solution.

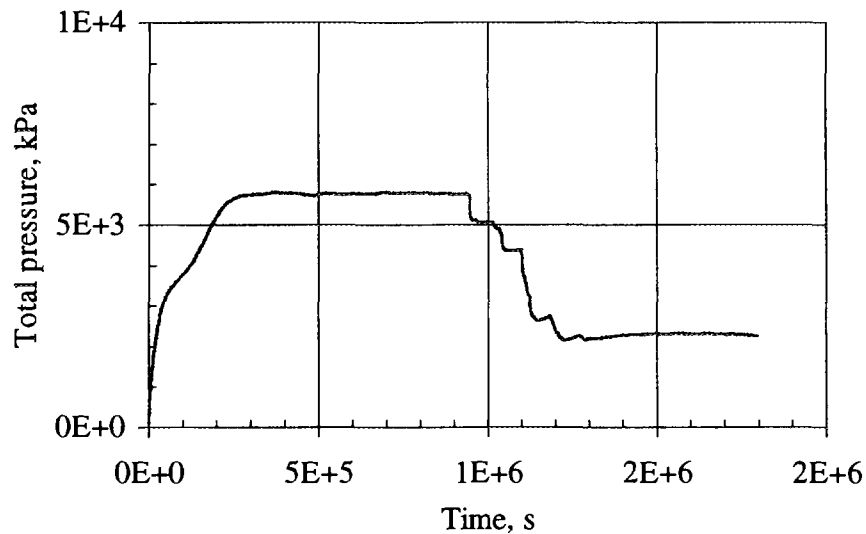


Figure 2-13. Measured swelling pressure in oedometer test with MX-80 clay at a clay-water density of 2000 kg/m³. The clay was saturated by pure water and, at stable pressure condition, contacted to 6.1 M NaCl solution. The plateaus in the pressure line after contact to the salt solution are due to intermittent replacement of solution in the filters.

Test V4

MX-80 clay-water density corresponding to 1800 kg/m³ after saturation with pure water. The sample was saturated by 6.1 M NaCl solution. The swelling pressure development is shown in Figure 2-14. The final swelling pressure was 130 kPa, which is a possible value according to the prediction shown in Figure 2-11.

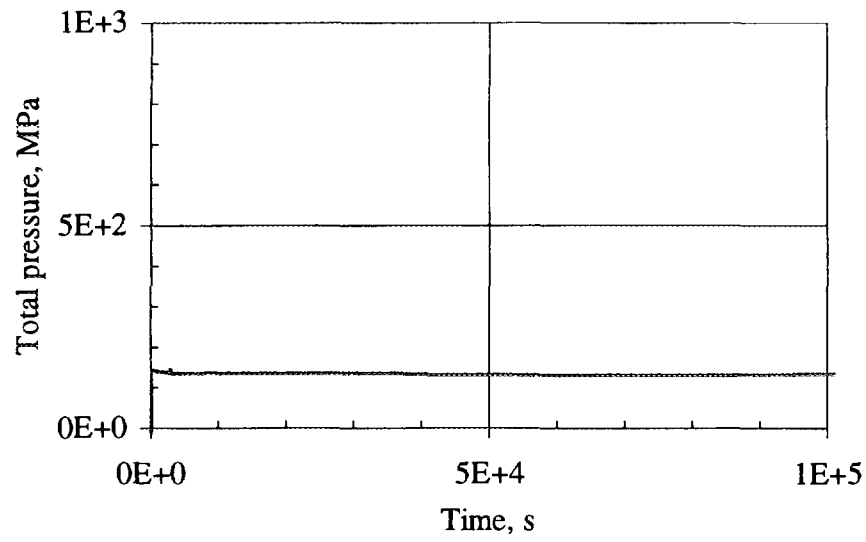


Figure 2-14. Measured swelling pressure in oedometer test with MX-80 clay at a clay-water density of 1800 kg/m^3 and saturated by 6.1 M NaCl solution.

3 CONCLUSIONS

The performed literature study showed that the swelling capacity and swelling pressure of different types of bentonites and bentonite mixtures have been determined at several independent laboratories. A large number of investigations have also given detailed information concerning the physico-chemical conditions in the clay-water system. This have led to a consensus concerning the physical effects of bentonite swelling both on the macroscopic and microscopic levels. However, there is no general consensus concerning the detailed mechanisms leading to these effects, i.e. concerning the forces involved and how they may be determined or calculated. Several quite different approaches and theories have been proposed and there is an ongoing debate on the subject. In this study, only the thermodynamic model show satisfactory concordance between calculated and the measured values of swelling pressure in bentonite exposed to concentrated salt solutions. It should, however, be noticed that improved models are presently being developed.

The most important conclusion from this study is that the thermodynamic model may well predict the measured swelling pressures by use of eq. [20]. The only prerequisite for this is that the partial free energy of water in the clay-water system changes if the system is contacted to a salt solution compared to if the system is contacted to pure water. This means that there is no principle incongruity between the model and measured swelling pressures, and the main aim of this report are thereby fulfilled. The change may be treated as an osmotic component in the clay-water system according to the performed calculations.

The proposed quantification technique for swelling pressure at different clay densities and salt contents, as based on Donnan ion equilibrium and van't Hoff equation, gives significantly better correlation between calculated and measured values compared to previous models, also if the theoretically most conservative conditions are assumed. If a "clay van't Hoff factor" is introduced the correlation is further improved. The physical existence of such a factor and its value may be experimentally determined.

The experimental data and the performed calculations indicates that swelling pressures may be expected also under repository conditons with a high content of NaCl in the ground water. The theoretical difference in effect from NaCl and CaCl₂ solutions is small according to this approach. However, the reduced swelling capacity in a brine type of ground-water significantly raises the lowest possible buffer density, and in practice, eliminate the positive effects of mixing bentonite into a backfill material.

ACKNOWLEDGEMENT

This work was equally financed by SKB (Swedish nuclear fuel and waste management Co.) and POSIVA (Finland). The author wishes to thank Dr. Fred Karlsson at SKB for vital suggestions and helpful discussions.

REFERENCES

- Buckingham E., 1907. Studies on the Movement of Soil moisture. U.S. Dep. Agr., Bur. Soils, Bull., 38:61 pp.
- Casimir H.B.C., Polder D., 1948. The influence of retardation of the London-van der Waals forces. *Physical Review*, 73:360.
- Chapman D. L. 1913. A contribution to the theory of electro-capillarity. *Philosophical magazine*, 25 (6): 475-481.
- Colten V.A., 1986. Hydration States of Smectite in NaCl Brines at Elevated Pressures and Temperatures. *Clays and Clay Minerals*, Vol. 34:4, p. 385-389
- Dixon D.A., Gray M.N., Graham J., 1996. Swelling and hydraulic properties of bentonites from Japan, Canada and the USA. *Environmental Geotechnics*, Kamon (ed), Balkema, Rotterdam.
- Foster M.D., 1953. Geochemical studies of clay minerals, II; relation between ionic substitution and swelling in montmorillonites. *American Mineralogist* 38, 994-1006.
- Glaeser R., Méring J., 1968. Homogeneous hydration domains of the smectites. *C.r. hebdomadaire Séances Acad. Sci., Paris* 267, 436-466.
- Grauer R., 1986. Bentonite as a backfill material in the high-level waste repository: chemical aspects. NAGRA Technocal report 86-12, 1986.
- Harward M.E., Brindley G.W., 1966. Swelling of synthetic smectites in relation to lattice substitutions. 13th National Conference of Clays and Clay Minerals Preceedings, p 209-222.
- Iwata S, Tabuchi T. 1988. Soil-water interaction. MarcelDekker, inc., pp. 131-193.
- Kahr G., Kraehenbuehl F., Muller-Vonmoos M. & Stoeckli H. F., 1986. Wasseraufnahme und Wasserbewegung in hochverdichtetem Bentonit. NAGRA Technicher Bericht 86-14.
- Kahr G, Kraehenbuehl F, Stoeckli H F & Muller-Vonmoos M, 1990. Study of the water-bentonite system by vapour adsorption, immersion calorimetry and X-ray techniques: II. Heats of immersion, swelling pressures and thermodynamic properties. *Clay Minerals*, Vol. 25:4, p. 499-506
- Karnland O., Pusch R., Sandén T., 1992. Electrolytens betydelse för de fysikaliska egenskaperna hos MX-80 bentonit. SKB AR 92-35.

Kehren R., Schainberg I., 1975. Water vapor isotherms and heat of immersion of Na/Ca-montmorillonite systems. *Clays and Clay minerals* 23, 193-200.

POSIVA REPORTS 1998, situation 1/98

POSIVA 98-01 Bentonite swelling pressure in strong NaCl solutions
- Correlation of model calculations to experimentally determined data
Ola Karnland
Clay Technology, Lund, Sweden
January 1998
ISBN 951-652-039-1